Purchasing Division

Invitation for Bid

IFB-4785-20-DH
Fire Training Facility – Shed Foundation & Asphalt Pad

Responses Due:

April 21, 2020 prior to 3:30pm

Accepting Electronic Responses Only

Responses Only Submitted Through the Rocky Mountain E-Purchasing System (RMEPS)

https://www.rockymountainbidsystem.com/default.asp

(Purchasing Representative does not have access or control of the vendor side of RMEPS. If website or other problems arise during response submission, vendor MUST contact RMEPS to resolve issue prior to the response deadline. 800-835-4603)

IMPORTANT NOTICE:

Due to the recent developments with COVID-19, public bid openings are temporarily being suspended. Bid openings will still take place on their designated date and time, and bid tabulations will still be posted for public view/access. However, the bid openings itself will be closed to the public. Once the crisis has passed, public bid openings will resume as normal.

Purchasing Representative:
Duane Hoff Jr., Senior Buyer
duaneh@gjcity.org
970-244-1545

This document has been developed specifically to solicit competitive responses for this solicitation, and may not be the same as previous City of Grand Junction solicitations. All vendors are urged to thoroughly review this solicitation prior to responding. Submittal by **FAX, EMAIL or HARD COPY IS NOT ACCEPTABLE** for this solicitation.

Invitation for Bids

Table of Contents

1. Instructions to Bidders

1.1. **Purpose:** The City of Grand Junction is soliciting competitive bids from qualified and interested companies for all labor, equipment, and materials required to construct a reinforced concrete monoslab foundation for a storage shed (storage shed by others), as well as an asphalt driving pad surrounding the existing burn building and slab at the Grand Junction Fire Department Fire Training Center on Whitewater Hill. All dimensions and scope of work should be verified by Contractors prior to submission of bids.

IFB Questions:
Duane Hoff Jr., Senior Buyer
duaneh@gjcity.org

The City would like to remind all Contractors, Sub-Contractors, Vendors, Suppliers, Manufacturers, Service Providers, etc. that (with the exception of Pre-Bid or Site Visit Meetings) all questions, inquiries, comments, or communication pertaining to any formal solicitation (whether process, specifications, scope, etc.) must be directed (in writing) to the Purchasing Agent assigned to the project, or Purchasing Division. Direct communication with the City assigned Project Managers/Engineers is not appropriate for public procurement, and may result in disqualification.

1.2. **Recommended Site Visit Meeting:** **Prospective bidders are encouraged to attend a recommended site visit meeting on April 10, 2020 at 10:00am**. Meeting location shall be at the Fire Training Facility on Whitewater Hill, located at 3340 Whitewater Hill Road, Grand Junction, CO. The purpose of this visit will be to inspect and to clarify the contents of this Invitation for Bids (IFB).

1.3. **The Owner:** The Owner is the City of Grand Junction, Colorado and is referred to throughout this Solicitation. The term Owner means the Owner or his authorized representative.

1.4. **Procurement Process:** Procurement processes shall be governed by the most current version of the City of Grand Junction Purchasing Policy and Procedure Manual.

1.5. **Submission:** ***Each bid shall be submitted in electronic format only, and only through the Rocky Mountain E-Purchasing website (https://www.rockymountainbidsystem.com/default.asp).*** *This site offers both "free" and "paying" registration options that allow for full access of the Owner's documents and for electronic submission of proposals. (Note: "free" registration may take up to 24 hours to process. Please Plan accordingly.)* Please view our "**Electronic Vendor Registration Guide"** at http://www.gjcity.org/business-and-economic-development/bids/ for details. (Purchasing Representative does not have access or control of the vendor side of RMEPS. If website or other problems arise during response submission, vendor **MUST** contact RMEPS to resolve issue prior to the response deadline. **800-835-4603**)

1.6. **Modification and Withdrawal of Bids Before Opening.** Bids may be modified or withdrawn by an appropriate document stating such, duly executed and submitted to the place where Bids are to be submitted at any time prior to Bid Opening.

1.7. **Printed Form for Price Bid:** All Price Bids must be made upon the Price Bid Schedule attached, and should give the amounts both in words and in figures, and must be signed and acknowledged by the bidder.

The Offeror shall specify a unit price in figures for each pay item for which a quantity is given and shall provide the products (in numbers) of the respective unit prices and quantities in the Extended Amount column. The total Bid price shall be equal to the sum of all extended amount prices. When an item in the Price Bid Schedule provides a choice to be made by the Offeror, Offeror's choice shall be indicated in accordance with the specifications for that particular item and thereafter no further choice shall be permitted.

Where the unit of a pay item is lump sum, the lump sum amount shall be shown in the "extended amount" column and included in the summation of the total Bid.

All blank spaces in the Price Bid Schedule must be properly filled out.

Bids by corporations must be executed in the corporate name by the president or vice president or other corporate office accompanied by evidence of authority to sign. The corporate address and state of incorporation shall be shown below the signature.

Bids by partnerships must be executed in the partnership name and signed by a partner whose title must appear under the signature and the official address of the partnership must be shown below the signature.

All names must be typed or printed below the signature.

The Offeror's Bid shall contain an acknowledgement of receipt of all Addenda, the numbers of which shall be filled in on the Contractor's Bid Form.

The contact information to which communications regarding the Bid are to be directed must be shown.

1.8. **Exclusions:** No oral, telephonic, emailed, or facsimile bid will be considered

1.9. **Contract Documents:** The complete IFB and bidder's response compose the Contract Documents. Copies of bid documents can be obtained from the City Purchasing website, http://www.gjcity.org/business-and-economic-development/bids/ .

1.10. **Additional Documents:** The July 2010 edition of the "City Standard Contract Documents for Capital Improvements Construction", Plans, Specifications and other Bid Documents are available for review or download on the Public Works & Planning/Engineering page at www.gjcity.org. Electronic copies may be obtained on a CD format at the Department of Public Works and Planning at City Hall.

1.11. **Definitions and Terms:** See Article I, Section 3 of the General Contract Conditions in the *Standard Contract Documents for Capital Improvements Construction*.

1.12. Examination of Specifications: Bidders shall thoroughly examine and be familiar with the project Statement of Work. The failure or omission of any Offeror to receive or examine any form, addendum, or other document shall in no way relieve any Offeror from any obligation with respect to his bid. The submission of a bid shall be taken as evidence of compliance with this section. Prior to submitting a bid, each Offeror shall, at a minimum:

a. Examine the *Contract Documents* thoroughly;

b. Visit the site to familiarize themselves with local conditions that may in any manner affect cost, progress, or performance of the Work;

c. Become familiar with federal, state, and local laws, ordinances, rules, and regulations that may in any manner affect cost, progress or performance of the Work;

d. Study and carefully correlate Bidder's observations with the *Contract Documents*, and;

e. Notify the Purchasing Agent of all conflicts, errors, ambiguities or discrepancies in or among the *Contract Documents* within the designated inquiry period.

On request, the Owner will provide each Offeror access to the site to conduct such investigations and tests as each Bidder deems necessary for submission of a Bid. It shall be the Offeror's responsibility to make or obtain any additional examinations, investigations, explorations, tests and studies and obtain any additional information and data which pertain to the physical conditions (including without limitation, surface, subsurface and underground utilities) at or contiguous to the site or otherwise which may affect cost, progress or performance of the work and which the Offeror deems necessary to determine its Bid for performing the work in accordance with the time, price and other terms and conditions of the Contract Documents. Location of any excavation or boring made by Offeror shall be subject to prior approval of Owner and applicable agencies. Offeror shall fill all holes, restore all pavements to match the existing structural section and shall clean up and restore the site to its former condition upon completion of such exploration. The Owner reserves the right to require the Offeror to execute an access agreement with the Owner prior to accessing the site.

The lands upon which the Work is to be performed, rights of way, and access thereto, and other lands designated for use by Contractor in performing the Work, are identified on the Drawings.

Information and data reflected in the *Contract Documents* with respect to underground utilities at or contiguous to the site are based upon information and data furnished to the Owner and the Engineer by the owners of such underground utilities or others, and the Owner does not assume responsibility for the accuracy or completeness thereof, unless it is expressly provided otherwise in the *Contract Documents*.

By submission of a Bid, the Offeror shall be conclusively presumed to represent that the Offeror has complied with every requirement of these Instructions to Bidders, that

the *Contract Documents* are not ambiguous and are sufficient in scope and detail to indicate and convey understanding of all terms and conditions for performance of the Work.

1.13. Questions Regarding Statement of Work: Any information relative to interpretation of Scope of Work or specifications shall be requested of the Purchasing Representative, in writing, in ample time, prior to the inquiry deadline.

1.14. Addenda & Interpretations: If it becomes necessary to revise any part of this solicitation, a written addendum will be posted electronically on the City's website at http://www.gjcity.org/business-and-economic-development/bids/ . The Owner is not bound by any oral representations, clarifications, or changes made in the written specifications by Owner, unless such clarification or change is provided in written addendum form from the City Purchasing Representative.

1.15. Taxes: The Owner is exempt from State retail and Federal tax. The bid price must be net, exclusive of taxes.

1.16. Sales and Use Taxes: The Contractor and all Subcontractors are required to obtain exemption certificates from the Colorado Department of Revenue for sales and use taxes in accordance with the provisions of the General Contract Conditions. Bids shall reflect this method of accounting for sales and use taxes on materials, fixtures and equipment.

1.17. Offers Binding 60 Days: Unless additional time is required by the Owner, or otherwise specified, all formal offers submitted shall be binding for sixty (60) calendar days following opening date, unless the Bidder, upon request of the Purchasing Representative, agrees to an extension.

1.18. Exceptions and Substitutions: Bidders taking exception to the specifications and/or scope of work shall do so at their own risk. The Owner reserves the right to accept or reject any or all substitutions or alternatives. When offering substitutions and/or alternatives, Bidder must state these exceptions in the section pertaining to that area. Exception/substitution, if accepted, must meet or exceed the stated intent and/or specifications and/or scope of work. The absence of such a list shall indicate that the Bidder has not taken exceptions, and if awarded a contract, shall hold the Bidder responsible to perform in strict accordance with the specifications and/or scope of work contained herein.

1.19. Collusion Clause: Each bidder by submitting a bid certifies that it is not party to any collusive action or any action that may be in violation of the Sherman Antitrust Act. Any and all bids shall be rejected if there is evidence or reason for believing that collusion exists among bidders. The Owner may, or may not, accept future bids for the same services or commodities from participants in such collusion.

1.20. Disqualification of Bidders: A Bid will not be accepted from, nor shall a Contract be awarded to, any person, firm, or corporation that is in arrears to the Owner, upon debt or contract, or that has defaulted, as surety or otherwise, upon any obligation to the Owner, or that is deemed irresponsible or unreliable.

Bidders may be required to submit satisfactory evidence that they are responsible, have a practical knowledge of the project bid upon and that they have the necessary financial and other resources to complete the proposed Work.

Either of the following reasons, without limitation, shall be considered sufficient to disqualify a Bidder and Bid:

a. More than one Bid is submitted for the same Work from an individual, firm, or corporation under the same or different name; and

b. Evidence of collusion among Bidders. Any participant in such collusion shall not receive recognition as a Bidder for any future work of the Owner until such participant has been reinstated as a qualified bidder.

1.21. Public Disclosure Record: If the bidder has knowledge of their employee(s) or sub-contractors having an immediate family relationship with a City employee or elected official, the bidder must provide the Purchasing Representative with the name(s) of these individuals. These individuals are required to file an acceptable "Public Disclosure Record", a statement of financial interest, before conducting business with the City.

2. General Contract Conditions for Construction Projects

2.1. The Contract: This Invitation for Bid, submitted documents, and any negotiations, when properly accepted by the City, shall constitute a contract equally binding between the City and Contractor. The contract represents the entire and integrated agreement between the parties hereto and supersedes all prior negotiations, representations, or agreements, either written or oral. The contract may be amended or modified with Change Orders, Field Orders, or Addendums.

2.2. The Work: The term Work includes all labor necessary to produce the construction required by the Contract Documents, and all materials and equipment incorporated or to be incorporated in such construction.

2.3. Execution, Correlation, Intent, and Interpretations: The Contract Documents shall be signed by the Owner (City) and Contractor. City will provide the contract. By executing the contract, the Contractor represents that he/she has visited the site, familiarized himself with the local conditions under which the Work is to be performed, and correlated his observations with the requirements of the Contract Documents. The Contract Documents are complementary, and what is required by any one, shall be as binding as if required by all. The intention of the documents is to include all labor, materials, equipment and other items necessary for the proper execution and completion of the scope of work as defined in the technical specifications and drawings contained herein. All drawings, specifications and copies furnished by the City are, and shall remain, City property. They are not to be used on any other project, and with the exception of one contract set for each party to the contract, are to be returned to the owner on request at the completion of the work.

2.4. The Owner: The Owner is the City of Grand Junction, Colorado and is referred to throughout the Contract Documents. The term Owner means the Owner or his authorized representative. The Owner shall, at all times, have access to the work wherever it is in preparation and progress. The Contractor shall provide facilities for such access. The Owner will make periodic visits to the site to familiarize himself generally with the progress and quality of work and to determine, in general, if the work is proceeding in accordance with the contract documents. Based on such observations and the Contractor's Application for Payment, the Owner will determine the amounts owing to the Contractor and will issue Certificates for Payment in such amounts, as provided in the contract. The Owner will have authority to reject work which does not conform to the Contract documents. Whenever, in his reasonable opinion, he considers it necessary or advisable to insure the proper implementation of the intent of the Contract Documents, he will have authority to require the Contractor to stop the work or any portion, or to require special inspection or testing of the work, whether or not such work can be then be fabricated, installed, or completed. The Owner will not be responsible for the acts or omissions of the Contractor, and sub-Contractor, or any of their agents or employees, or any other persons performing any of the work.

2.5. Contractor: The Contractor is the person or organization identified as such in the Agreement and is referred to throughout the Contract Documents. The term Contractor means the Contractor or his authorized representative. The Contractor shall carefully study and compare the General Contract Conditions of the Contract, Specification and Drawings, Scope of Work, Addenda and Modifications and shall at once report to the Owner any error, inconsistency or omission he may discover. Contractor shall not be liable to the Owner for any damage resulting from such errors, inconsistencies or omissions. The Contractor shall not commence work without clarifying Drawings, Specifications, or Interpretations.

2.6. Sub-Contractors: A sub-contractor is a person or organization who has a direct contract with the Contractor to perform any of the work at the site. The term sub-contractor is referred to throughout the contract documents and means a sub-contractor or his authorized representative.

2.7. Award of Sub-Contractors & Other Contracts for Portions of the Work: Contractor shall submit with their bid response to the Owner, in writing for acceptance, a list of the names of the sub-contractors or other persons or organizations proposed for such portions of the work as may be designated in the proposal requirements, or, if none is so designated, the names of the sub-contractors proposed for the principal portions of the work. Prior to the award of the contract, the Owner shall notify the successful Contractor in writing if, after due investigation, has reasonable objection to any person or organization on such list. If, prior to the award of the contract, the Owner has a reasonable and substantial objection to any person or organization on such list, and refuses in writing to accept such person or organization, the successful Contractor may, prior to the award, withdraw their proposal without forfeiture of proposal security. If the successful Contractor submits an acceptable substitute with an increase in the proposed price to cover the difference in cost occasioned by the substitution, the Owner may, at their discretion, accept the increased proposal or may disqualify the Contractor. If, after the award, the Owner refuses to accept any person or organization on such list, the Contractor shall submit an acceptable substitute and the contract sum shall be increased

or decreased by the difference in cost occasioned by such substitution and an appropriate Change Order shall be issued. However, no increase in the contract sum shall be allowed for any such substitution unless the Contractor has acted promptly and responsively in submitting a name with respect thereto prior to the award.

2.8. Quantities of Work and Unit Price: Materials or quantities stated as unit price items in the Bid are supplied only to give an indication of the general scope of the Work, and are as such, estimates only. The Owner does not expressly or by implication agree that the actual amount of Work or material will correspond therewith, and reserves the right after award to increase or decrease the quantity of any unit item of the Work without a change in the unit price except as set forth in Article VIII, Section 70 of the *General Contract Conditions*. The City also reserves the right to make changes in the Work (including the right to delete any bid item in its entirety or add additional bid items) as set forth in Article VIII, Sections 69 through 71 of the *General Contract Conditions*.

2.9. Substitutions: The materials, products and equipment described in the *Solicitation Documents* shall be regarded as establishing a standard of required performance, function, dimension, appearance, or quality to be met by any proposed substitution. No substitution will be considered prior to receipt of Bids unless the Offeror submits a written request for approval to the City Purchasing Division at least ten (10) days prior to the date for receipt of Bids. Such requests for approval shall include the name of the material or equipment for which substitution is sought and a complete description of the proposed substitution including drawings, performance and test data, and other information necessary for evaluation, including samples if requested. The Offeror shall set forth changes in other materials, equipment, or other portions of the Work including changes of the work of other contracts, which incorporation of the proposed substitution would require to be included. The Owner's decision of approval or disapproval of a proposed substitution shall be final. If the Owner approves a proposed substitution before receipt of Bids, such approval will be set forth in an Addendum. Offeors shall not rely upon approvals made in any other manner.

2.10. Supervision and Construction Procedures: The Contractor shall supervise and direct the work, using his best skill and attention. He shall be solely responsible for all construction means, methods, techniques, sequences and procedures and for coordinating all portions of the work under the contract.

2.11. Warranty: The Contractor warrants to the Owner that all materials and equipment furnished under this contract will be new unless otherwise specified, and that all work will be of good quality, free from faults and defects and in conformance with the Contract Documents. All work not so conforming to these standards may be considered defective. If required by Owner, the Contractor shall furnish satisfactory evidence as to the kind and quality of materials and equipment. If within ten (10) days after written notice to the Contractor requesting such repairs or replacement, the Contractor should neglect to make or undertake with due diligence to the same, the City may make such repairs or replacements. All indirect and direct costs of such correction or removal or replacement shall be at the Contractor's expense. The Contractor will also bear the expenses of making good all work of others destroyed or damaged by the correction, removal or replacement of his defective work.

2.12. Permits, Fees, & Notices: The Contractor shall secure and pay for all permits, governmental fees and licenses necessary for the proper execution and completion of the work. The Contractor shall give all notices and comply with all laws, ordinances, rules, regulations and orders of any public authority bearing on the performance of the work. If the Contractor observes that any of the Contract Documents are at variance in any respect, he shall promptly notify the Owner in writing, and any necessary changes shall be adjusted by approximate modification. If the Contractor performs any work knowing it to be contrary to such laws, ordinances, rules and regulations, and without such notice to the Owner, he shall assume full responsibility and shall bear all costs attributable.

2.13. Responsibility for Those Performing the Work: The Contractor shall be responsible to the Owner for the acts and omissions of all his employees and all sub-contractors, their agents and employees, and all other persons performing any of the work under a contract with the Contractor.

2.14. Use of the Site: The Contractor shall confine operations at the site to areas permitted by law, ordinances, permits and the Contract Documents, and shall not unreasonably encumber the site with any materials or equipment.

2.15. Cleanup: The Contractor at all times shall keep the premises free from accumulation of waste materials or rubbish caused by his operations. At the completion of work he shall remove all his waste materials and rubbish from and about the project, as well as all his tools, construction equipment, machinery and surplus materials.

2.16. Insurance: The Contractor shall secure and maintain such insurance policies as will provide the coverage and contain other provisions specified in the General Contract Conditions, or as modified in the Special Contract Conditions.

The Contractor shall file a copy of the policies or Certificates of Insurance acceptable to the City with the Engineer within ten (10) Calendar Days after issuance of the Notice of Award. These Certificates of Insurance shall contain a provision that coverage afforded under the policies shall not be canceled unless at least thirty (30) Calendar Days prior written notice has been given to the City.

2.17. Indemnification: The Contractor shall defend, indemnify and save harmless the Owner, and all its officers, employees, insurers, and self-insurance pool, from and against all liability, suits, actions, or other claims of any character, name and description brought for or on account of any injuries or damages received or sustained by any person, persons, or property on account of any negligent act or fault of the Contractor, or of any Contractor's agent, employee, sub-contractor or supplier in the execution of, or performance under, any contract which may result from proposal award. Contractor shall pay any judgment with cost which may be obtained against the Owner growing out of such injury or damages.

2.18. Miscellaneous Conditions: Material Availability: Contractors must accept responsibility for verification of material availability, production schedules, and other pertinent data prior to submission of bid. It is the responsibility of the bidder to notify the Owner immediately if materials specified are discontinued, replaced, or not available for

an extended period of time. **OSHA Standards:** All bidders agree and warrant that services performed in response to this invitation shall conform to the standards declared by the US Department of Labor under the Occupational Safety and Health Act of 1970 (OSHA). In the event the services do not conform to OSHA standards, the Owner may require the services to be redone at no additional expense to the Owner.

2.19. Time: Time is of the essence with respect to the time of completion of the Project and any other milestones or deadline which are part of the Contract. It will be necessary for each Bidder to satisfy the City of its ability to complete the Work within the Contract Time set forth in the Contract Documents. The Contract Time is the period of time allotted in the Contract Documents for completion of the work. The date of commencement of the work is the date established in a Notice to Proceed. If there is no Notice to Proceed, it shall be the date of the Contract or such other date as may be established therein, or as established as entered on the Bid Form. The Date of Final Completion of the work is the date certified by the Owner when all construction, and all other work associated to include, but not be limited to: testing, QA/QC, receipt of required reports and/or forms, grant requirements (if applicable), punch list items, clean-up, receipt of drawings and/or as-builts, etc., is fully complete, and in accordance with the Contract Documents.

2.20. Progress & Completion: The Contractor shall begin work on the date of commencement as defined in the Contract, and shall carry the work forward expeditiously with adequate forces and shall complete it within the contract time.

2.21. Payment & Completion: The Contract Sum is stated in the Contract and is the total amount payable by the Owner to the Contractor for the performance of the work under the Contract Documents. Upon receipt of written notice that the work is ready for final inspection and acceptance and upon receipt of application for payment, the Owner's Project Manager will promptly make such inspection and, when he finds the work acceptable under the Contract Documents and the Contract fully performed, the Owner shall make payment in the manner provided in the Contract Documents.

2.22. Bid Bond: Each Bid shall as a guaranty of good faith on the part of the Bidder be accompanied by a Bid Guaranty consisting of: a certified or cashier's check drawn on an approved national bank or trust company in the state of Colorado, and made payable without condition to the City; or a **Bid Bond** written by an approved corporate surety in favor of the City. The amount of the Bid Guaranty shall not be less than 5% of the total Bid amount. Once a Bid is accepted and a Contact is awarded, the apparent successful bidder has ten calendar days to enter into a contractor in the form prescribed and to furnish the bonds with a legally responsible and approved surety. Failure to do so will result I forfeiture of the Bid Guaranty to the City as Liquidated Damages.

Each bidder shall guaranty its total bid price for a period of sixty (60) Calendar Days from the date of the bid opening.

2.23. Performance & Payment Bonds: Contractor shall furnish a Performance and a Payment Bond, each in an amount at least equal to that specified for the contract amount as security for the faithful performance and payment of all Contractor's obligations under the Contract Documents. These bonds shall remain in effect for the duration of the Warranty Period (as specified in the Special Conditions). Contractor shall also furnish

other bonds that may be required by the Special Conditions. All bonds shall be in the forms prescribed by the Contract Documents and be executed by such sureties as (1) are licensed to conduct business in the State of Colorado and (2) are named in the current list of "Companies Holding Certificates of Authority as Acceptable Sureties on Federal Bonds and as Acceptable Reinsuring Companies" as published in Circular 570 (amended) by the Audit Staff, Bureau of Accounts, U.S. Treasury Department. All bonds singed by an agent must be accompanied by a certified copy of the Authority Act. If the surety on any bond furnished by the Contractor is declared bankrupt, or becomes insolvent, or its rights to do business in Colorado are terminated, or it ceases to meet the requirements of clauses (1) and (2) of this section, Contractor shall within five (5) days thereafter substitute another bond and surety, both of which shall be acceptable to the City.

2.24. Retention: The Owner will deduct money from the partial payments in amounts considered necessary to protect the interest of the Owner and will retain this money until after completion of the entire contract. The amount to be retained from partial payments will be five (5) percent of the value of the completed work, and not greater than five (5) percent of the amount of the Contract. When the retainage has reached five (5) percent of the amount of the Contract no further retainage will be made and this amount will be retained until such time as final payment is made.

2.25. Liquidated Damages for Failure to Enter Into Contract: Should the Successful Bidder fail or refuse to enter into the Contract within ten Calendar Days from the issuance of the Notice of Award, the City shall be entitled to collect the amount of such Bidder's Bid Guaranty as Liquidated Damages, not as a penalty but in consideration of the mutual release by the City and the Successful Bidder of all claims arising from the City's issuance of the Notice of Award and the Successful Bidder's failure to enter into the Contract and the costs to award the Contract to any other Bidder, to readvertise, or otherwise dispose of the Work as the City may determine best serves its interest.

2.26. Liquidated Damages for Failure to Meet Project Completion Schedule: If the Contractor does not achieve Final Completion by the required date, whether by neglect, refusal or any other reason, the parties agree and stipulate that the Contractor shall pay liquidated damages to the City for each such day that final completion is late. As provided elsewhere, this provision does not apply for delays caused by the City. The date for Final Completion may be extended in writing by the Owner.

The Contractor agrees that as a part of the consideration for the City's awarding of this Contract liquidated damages in the daily amount of **$500.00** is reasonable and necessary to pay for the actual damages resulting from such delay. The parties agree that the real costs and injury to the City for such delay include hard to quantify items such as: additional engineering, inspection and oversight by the City and its agents; additional contract administration; inability to apply the efforts of those employees to the other work of the City; perceived inefficiency of the City; citizens having to deal with the construction and the Work, rather than having the benefit of a completed Work, on time; inconvenience to the public; loss of reputation and community standing for the City during times when such things are very important and very difficult to maintain.

The Contractor must complete the Work and achieve final completion included under the Bid Schedule in the number of consecutive calendar days after the City gives is written Notice to Proceed. When the Contractor considers the entire Work ready for its intended use, Contractor shall certify in writing that the Work is fully complete. Final Completion date is the date by which the Contractor shall have fully completed all clean-up, and all items that were identified by the City in the inspection for final completion. Unless otherwise stated in the Special Conditions, for purposes of this liquidated damages clause, the Work shall not be finished and the Contract time shall continue to accrue until the City gives its written Final Acceptance.

If the Contractor shall fail to pay said liquidated damages promptly upon demand thereof after having failed to achieve Final Completion on time, the City shall first look to any retainage or other funds from which to pay said liquidated damages; if retainage or other liquid funds are not available to pay said liquidated damages amounts, the Surety on the Contractor's Performance Bond and Payment Bond shall pay such liquidated damages. In addition, the City may withhold all, or any part of, such liquidated damages from any payment otherwise due the Contractor.
Liquidated damages as provided do not include any sums to reimburse the City for extra costs which the City may become obligated to pay on other contracts which were delayed or extended because of the Contractor's failure to complete the Work within the Contract Time. Should the City incur additional costs because of delays or extensions to other contracts resulting from the Contractor's failure of timely performance, the Contractor agrees to pay these costs that the City incurs because of the Contractor's delay, and these payments are separate from and in addition to any liquidated damages.

The Contractor agrees that the City may use its own forces or hire other parties to obtain Final Completion of the work if the time of completion has elapsed and the Contractor is not diligently pursuing completion. In addition to the Liquidated Damages provided for, the Contractor agrees to reimburse the City for all expenses thus incurred.

2.27. Contingency/Force Account: Contingency/Force Account work will be authorized by the Owner's Project Manager and is defined as minor expenses to cover miscellaneous or unforeseen expenses related to the project. The expenses are not included in the Drawings, Specifications, or Scope of Work and are necessary to accomplish the scope of this contract. Contingency/Force Account Authorization will be directed by the Owner through an approved form. Contingency/Force Account funds are the property of the Owner and any Contingency/Force Account funds, not required for project completion, shall remain the property of the Owner. Contractor is not entitled to any Contingency/Force Account funds, that are not authorized by Owner or Owner's Project Manager.

2.28. Protection of Persons & Property: The Contractor shall comply with all applicable laws, ordinances, rules, regulations and orders of any public authority having jurisdiction for the safety of persons or property or to protect them from damage, injury or loss. Contractor shall erect and maintain, as required by existing safeguards for safety and protection, and all reasonable precautions, including posting danger signs or other warnings against hazards promulgating safety regulations and notifying owners and users of adjacent utilities. When or where any direct or indirect damage or injury is done to public or private property by or on account of any act, omission, neglect, or misconduct

by the Contractor in the execution of the work, or in consequence of the non-execution thereof by the Contractor, he shall restore, at his own expense, such property to a condition similar or equal to that existing before such damage or injury was done, by repairing, rebuilding, or otherwise restoring as may be directed, or it shall make good such damage or injury in an acceptable manner.

2.29. Changes in the Work: The Owner, without invalidating the contract, may order changes in the work within the general scope of the contract consisting of additions, deletions or other revisions, the contract sum and the contract time being adjusted accordingly. All such changes in the work shall be authorized by Change Order and shall be executed under the applicable conditions of the contract documents. A Change Order is a written order to the Contractor signed by the Owner issued after the execution of the contract, authorizing a change in the work or an adjustment in the contract sum or the contract time. The contract sum and the contract time may be changed only by Change Order.

2.30. Claims for Additional Cost or Time: If the Contractor wishes to make a claim for an increase in the contract sum or an extension in the contract time, he shall give the Owner written notice thereof within a reasonable time after the occurrence of the event giving rise to such claim. This notice shall be given by the Contractor before proceeding to execute the work, except in an emergency endangering life or property in which case the Contractor shall precede in accordance with the regulations on safety. No such claim shall be valid unless so made. Any change in the contract sum or contract time resulting from such claim shall be authorized by Change Order.

2.31. Minor Changes in the Work: The Owner shall have authority to order minor changes in the work not involving an adjustment in the contract sum or an extension of the contract time and not inconsistent with the intent of the contract documents.

2.32. Field Orders: The Owner may issue written Field Orders which interpret the Contract Documents in accordance with the specifications, or which order minor changes in the work in accordance with the agreement, without change in the contract sum or time. The Contractor shall carry out such Field Orders promptly.

2.33. Uncovering & Correction of Work: The Contractor shall promptly correct all work rejected by the Owner as defective or as failing to conform to the contract documents whether observed before or after substantial completion and whether or not fabricated installed or competed. The Contractor shall bear all costs of correcting such rejected work, including the cost of the Owner's additional services thereby made necessary. If within one (1) year after the date of completion or within such longer period of time as may be prescribed by law or by the terms of any applicable special guarantee required by the contract documents, any of the work found to be defective or not in accordance with the contract documents, the Contractor shall correct it promptly after receipt of a written notice from the Owner to do so unless the Owner has previously given the Contractor a written acceptance of such condition. The Owner shall give such notice promptly after discover of condition. All such defective or non-conforming work under the above paragraphs shall be removed from the site where necessary and the work shall be corrected to comply with the contract documents without cost to the Owner. The Contractor shall bear the cost of making good all work of separate Contractors destroyed or damaged by such removal or correction. If the Owner prefers to accept defective or

non-conforming work, he may do so instead of requiring its removal and correction, in which case a Change Order will be issued to reflect an appropriate reduction in the payment or contract sum, or, if the amount is determined after final payment, it shall be paid by the Contractor.

2.30. Amendment: No oral statement of any person shall modify or otherwise change, or affect the terms, conditions or specifications stated in the resulting contract. All amendments to the contract shall be made in writing by the Owner.

2.31. Assignment: The Contractor shall not sell, assign, transfer or convey any contract resulting from this IFB, in whole or in part, without the prior written approval from the Owner.

2.32. Compliance with Laws: Bids must comply with all Federal, State, County and local laws governing or covering this type of service and the fulfillment of all ADA (Americans with Disabilities Act) requirements.

2.33. Confidentiality: All information disclosed by the Owner to the Contractor for the purpose of the work to be done or information that comes to the attention of the Contractor during the course of performing such work is to be kept strictly confidential.

2.34. Conflict of Interest: No public official and/or City/County employee shall have interest in any contract resulting from this IFB.

2.35. Contract Termination: This contract shall remain in effect until any of the following occurs: (1) contract expires; (2) completion of services; (3) acceptance of services or, (4) for convenience terminated by either party with a written *Notice of Cancellation* stating therein the reasons for such cancellation and the effective date of cancellation.

2.36. Employment Discrimination: During the performance of any services per agreement with the Owner, the Contractor, by submitting a Bid, agrees to the following conditions:

2.36.1. The Contractor shall not discriminate against any employee or applicant for employment because of race, religion, color, sex, age, handicap, or national origin except when such condition is a legitimate occupational qualification reasonably necessary for the normal operations of the Contractor. The Contractor agrees to post in conspicuous places, visible to employees and applicants for employment, notices setting forth the provisions of this nondiscrimination clause.

2.36.2. The Contractor, in all solicitations or advertisements for employees placed by or on behalf of the Contractor, shall state that such Contractor is an Equal Opportunity Employer.

2.36.3. Notices, advertisements, and solicitations placed in accordance with federal law, rule, or regulation shall be deemed sufficient for the purpose of meeting the requirements of this section.

2.37. Affirmative Action: In executing a Contract with the City, the Contractor agrees to comply with Affirmative Action and Equal Employment Opportunity regulations presented in the General Contract Conditions.

2.38. Immigration Reform and Control Act of 1986 and Immigration Compliance: The Offeror certifies that it does not and will not during the performance of the contract employ illegal alien workers or otherwise violate the provisions of the Federal Immigration Reform and Control Act of 1986 and/or the immigration compliance requirements of State of Colorado C.R.S. § 8-17.5-101, *et.seq.* (House Bill 06-1343).

2.39. Ethics: The Contractor shall not accept or offer gifts or anything of value nor enter into any business arrangement with any employee, official, or agent of the Owner.

2.40. Failure to Deliver: In the event of failure of the Contractor to deliver services in accordance with the contract terms and conditions, the Owner, after due oral or written notice, may procure the services from other sources and hold the Contractor responsible for any costs resulting in additional purchase and administrative services. This remedy shall be in addition to any other remedies that the Owner may have.

2.41. Failure to Enforce: Failure by the Owner at any time to enforce the provisions of the contract shall not be construed as a waiver of any such provisions. Such failure to enforce shall not affect the validity of the contract or any part thereof or the right of the Owner to enforce any provision at any time in accordance with its terms.

2.42. Force Majeure: The Contractor shall not be held responsible for failure to perform the duties and responsibilities imposed by the contract due to legal strikes, fires, riots, rebellions, and acts of God beyond the control of the Contractor, unless otherwise specified in the contract.

2.43. Independent Contractor: The Contractor shall be legally considered an Independent Contractor and neither the Contractor nor its employees shall, under any circumstances, be considered servants or agents of the Owner. The Owner shall be at no time legally responsible for any negligence or other wrongdoing by the Contractor, its servants, or agents. The Owner shall not withhold from the contract payments to the Contractor any federal or state unemployment taxes, federal or state income taxes, Social Security Tax or any other amounts for benefits to the Contractor. Further, the Owner shall not provide to the Contractor any insurance coverage or other benefits, including Workers' Compensation, normally provided by the Owner for its employees.

2.44. Nonconforming Terms and Conditions: A bid that includes terms and conditions that do not conform to the terms and conditions of this Invitation for Bid is subject to rejection as non-responsive. The Owner reserves the right to permit the Contractor to withdraw nonconforming terms and conditions from its bid prior to a determination by the Owner of non-responsiveness based on the submission of nonconforming terms and conditions.

Items for non-responsiveness may include, but not be limited to:

a. Submission of the Bid on forms other than those supplied by the City;

b. Alteration, interlineation, erasure, or partial detachment of any part of the forms which are supplied herein;

c. Inclusion of unauthorized additions conditional or alternate Bids or irregularities of any kind which may tend to make the Bid incomplete, indefinite, or ambiguous as to its meaning;

d. Failure to acknowledge receipt of any or all issued Addenda;

e. Failure to provide a unit price or a lump sum price, as appropriate, for each pay item listed except in the case of authorized alternative pay items;

f. Failure to list the names of Subcontractors used in the Bid preparation as may be required in the Solicitation Documents;

g. Submission of a Bid that, in the opinion of the Owner, is unbalanced so that each item does not reasonably carry its own proportion of cost or which contains inadequate or unreasonable prices for any item;

h. Tying of the Bid with any other bid or contract; and

i. Failure to calculate Bid prices as described herein.

2.45. Evaluation of Bids and Offerors: The Owner reserves the right to:

- reject any and all Bids,
- waive any and all informalities,
- take into account any prompt payment discounts offered by Bidder,
- negotiate final terms with the Successful Bidder,
- take into consideration past performance of previous awards/contracts with the Owner of any Contractor, Vendor, Firm, Supplier, or Service Provider in determining final award. and
- disregard any and all nonconforming, nonresponsive or conditional Bids.

Discrepancies between words and figures will be resolved in favor of words. Discrepancies between Unit Prices and Extended Prices will be resolved in favor of the Unit Prices. Discrepancies between the indicated sum of any column of figures and the correct sum thereof will be resolved in favor of the correct sum. The corrected extensions and totals will be shown in the tabulation of Bids.

The Owner may consider the qualifications and experience of Subcontractors and other persons and organizations (including those who are to furnish the principal items of material or equipment) proposed for those portions of the work as to which the identity of Subcontractors and other persons and organizations must be submitted. Operating costs, maintenance considerations performance data, and guarantees of materials and equipment may also be considered by the Owner.

The Owner will conduct such investigations as deemed necessary to assist in the evaluation of any Bid and to establish the responsibility, qualifications and financial ability of the Offeror, proposed Subcontractors and other persons and organizations to do the Work in accordance with the *Contract Documents* to the City's satisfaction within the Contract Time.

The Offeror shall furnish the Owner all information and data requested by the Owner to determine the ability of the Offeror to perform the Work. The Owner reserves the right to reject the Bid if the evidence submitted by, or investigation of such Offeror fails to satisfy the Owner that such Offeror is properly qualified to carry out the obligations of the Contract and to complete the Work contemplated therein.

By submitting a Bid, each Offeror authorizes the Owner to perform such investigation of the Offeror as the Owner deems necessary to establish the responsibility, qualifications and financial ability of the Offeror and, by its signature thereon, authorizes the Owner to obtain reference information concerning the Offeror and releases the party providing such information and the Owner from any and all liability to the Offeror as a result of such reference information so provided.

The Owner reserves the right to reject the Bid of any Offeror who does not pass any evaluation to the Owner’s satisfaction.

If the Contract is to be awarded, it will be awarded to the Offeror who, by evaluation, the Owner determines will best meet the Owner's interests.

The Owner reserves the right to accept or reject the Work contained in any of the Price Bid Schedules or alternates, either in whole or in part.

2.46. Award of Contract: Unless otherwise indicated, a single award will be made for all the bid items in an individual bid schedule. In the event that the Work is contained in more than one Bid Schedule, the City may award Schedules individually or in combination. In the case of two Bid Schedules which are alternative to each other, only one of such alternative Schedules will be awarded. Within forty-five (45) Calendar Days of Bid Opening, the City will issue a Notice of Award to the Successful Bidder which will be accompanied by four (4) unsigned copies of the Contract and the Performance and Payment Bond forms. Within ten (10) Calendar Days thereafter, the Successful Bidder shall sign and deliver four (4) copies of the Contract, Performance Bond, Payment Bond and Certificates of Insurance to the City. Within ten (10) Calendar Days thereafter, the City will deliver two (2) fully executed counterparts of the Contract to the Contractor. No contract shall exist between the Successful Bidder and the City and the Successful Bidder shall have no rights at law or in equity until the Contract has been duly executed by the City.

The Successful Bidder’s failure to sign and submit a Contract and other documents set forth in this Paragraph within the prescribed time shall be just cause of annulment of the award, and forfeiture of the Bid Guaranty. The award of Contract may then be made to the next qualified Bidder in the same manner as previously prescribed.

2.47. Ownership: All plans, prints, designs, concepts, etc., shall become the property of the Owner.

2.48. Oral Statements: No oral statement of any person shall modify or otherwise affect the terms, conditions, or specifications stated in this document and/or resulting agreement. All modifications to this request and any agreement must be made in writing by the Owner.

2.49. Patents/Copyrights: The Contractor agrees to protect the Owner from any claims involving infringements of patents and/or copyrights. In no event shall the Owner be liable to the Contractor for any/all suits arising on the grounds of patent(s)/copyright(s) infringement. Patent/copyright infringement shall null and void any agreement resulting from response to this IFB.

2.50. Remedies: The Contractor and Owner agree that both parties have all rights, duties, and remedies available as stated in the Uniform Commercial Code.

2.51. Venue: Any agreement as a result of responding to this IFB shall be deemed to have been made in, and shall be construed and interpreted in accordance with, the laws of the City of Grand Junction, Mesa County, Colorado.

2.52. Expenses: Expenses incurred in preparation, submission and presentation of this IFB are the responsibility of the company and cannot be charged to the Owner.

2.53. Sovereign Immunity: The Owner specifically reserves its right to sovereign immunity pursuant to Colorado State Law as a defense to any action arising in conjunction to this agreement.

2.54. Non-Appropriation of Funds: The contractual obligation of the Owner under this contract is contingent upon the availability of appropriated funds from this fiscal year budget as approved by the City Council or Board of County Commissioners from this fiscal year only. State of Colorado Statutes prohibit obligation of public funds beyond the fiscal year for which the budget was approved. Anticipated expenditures/obligations beyond the end of the current Owner's fiscal year budget shall be subject to budget approval. Any contract will be subject to and must contain a governmental non-appropriation of funds clause.

2.55. Cooperative Purchasing: Purchases as a result of this solicitation are primarily for the City/County. Other governmental entities may be extended the opportunity to utilize the resultant contract award with the agreement of the successful provider and the participating agencies. All participating entities will be required to abide by the specifications, terms, conditions and pricings established in this Bid. The quantities furnished in this bid document are for only the City/County. It does not include quantities for any other jurisdiction. The City or County will be responsible only for the award for its jurisdiction. Other participating entities will place their own awards on their respective Purchase Orders through their purchasing office or use their purchasing card for purchase/payment as authorized or agreed upon between the provider and the individual entity. The City/County accepts no liability for payment of orders placed by other participating jurisdictions that choose to piggy-back on our solicitation. Orders placed by

participating jurisdictions under the terms of this solicitation will indicate their specific delivery and invoicing instructions.

2.56. Keep Jobs in Colorado Act: Contractor shall be responsible for ensuring compliance with Article 17 of Title 8, Colorado Revised Statutes requiring 80% Colorado labor to be employed on public works. Contractor shall, upon reasonable notice provided by the Owner, permit the Owner to inspect documentation of identification and residency required by C.R.S. §8-17-101(2)(a). If Contractor claims it is entitled to a waiver pursuant to C.R.S. §8-17-101(1), Contractor shall state that there is insufficient Colorado labor to perform the work such that compliance with Article 17 would create an undue burden that would substantially prevent a project from proceeding to completion, and shall include evidence demonstrating the insufficiency and undue burden in its response.

Unless expressly granted a waiver by the Owner pursuant to C.R.S. §8-17-101(1), Contractor shall be responsible for ensuring compliance with Article 17 of Title 8, Colorado Revised Statutes requiring 80% Colorado labor to be employed on public works. Contractor shall, upon reasonable notice provided by the Owner, permit the Owner to inspect documentation of identification and residency required by C.R.S. §8-17-101(2)(a).

2.56.1. "Public project" is defined as:

(a) any construction, alteration, repair, demolition, or improvement of any land, building, structure, facility, road, highway, bridge, or other public improvement suitable for and intended for use in the promotion of the public health, welfare, or safety and any maintenance programs for the upkeep of such projects

(b) for which appropriate or expenditure of moneys may be reasonably expected to be $500,000.00 or more in the aggregate for any fiscal year

(c) except any project that receives federal moneys.

3. Statement of Work

3.1. GENERAL: The work request is for improvements to the Fire Training Facility on Whitewater Hill for the City of Grand Junction and the State of Colorado. The improvements will include Geotextile Fabric, Aggregate Base Course, Reinforced Concrete slab, and Asphalt placement.

3.2. PROJECT DESCRIPTION: The project includes approximately 71 cubic yards of 6 inch thick reinforced concrete monoslab and 8,300 pounds of reinforcement underlain by 87 CY of 12 inch thick Aggregate Base Course (Class 6) and 258 SY of Geotextile (Class 1). The 41 feet by 57 feet reinforced concrete monoslab will support the storage shed building (by others) on top of Whitewater Hill.

The asphalt driving pad includes approximately 556 tons of Hot Mix Asphalt underlain by 979 CY of Aggregate Base Course (Class 6). The asphalt driving pad will surround the newly built Burn Building and Slab on Whitewater Hill. All dimensions and scope of work should be verified by Contractors prior to submission of bids.

3.3. SPECIAL CONDITIONS & PROVISIONS:

3.3.1 Recommended Site Visit Meeting: **Prospective bidders are encouraged to attend a recommended site visit meeting on April 10, 2020 at 10:00am**. Meeting location shall be at the Fire Training Facility on Whitewater Hill, located at 3340 Whitewater Hill Road, Grand Junction, CO. The purpose of this visit will be to inspect and to clarify the contents of this Invitation for Bids (IFB).

3.3.2 QUESTIONS REGUARDING SOLICIATION PROCESS/SCOPE OF WORK:

Duane Hoff Jr., Senior Buyer
City of Grand Junction
duaneh@gjcity.org

3.3.2 Project Manager: The Project Manager for the Project is Kirsten Armbruster, Project Engineer, who can be reached at (970) 244-1421. During Construction, all notices, letters, submittals, and other communications directed to the City shall be addressed and mailed or delivered to:

City of Grand Junction
Department of Public Works - Engineering
Attn: Kirsten Armbruster, Project Manager
250 North Fifth Street
Grand Junction, CO 81501

3.3.3 Affirmative Action: The Contractor is not required to submit a written Affirmative Action Program for the Project.

3.3.4 Pricing: Pricing shall be all inclusive to include but not be limited to: all labor, equipment, supplies, materials, freight (F.O.B. Destination – Freight Pre-paid and Allowed to each site), travel, mobilization costs, fuel, set-up and take down costs, and full-time inspection costs, and all other costs related to the successful completion of the project.

The Owner shall not pay nor be liable for any other additional costs including but not limited to: taxes, shipping charges, insurance, interest, penalties, termination payments, attorney fees, liquidated damages, etc.

3.3.5 Freight/Shipping: All freight/shipping shall be F.O.B. Destination – Freight Pre-Paid and Allowed to the project site(s), Grand Junction, CO.

Contractor must meet all federal, state, and local rules, regulations, and requirements for providing such services.

3.3.6 Contract: A binding contract shall consist of: (1) the IFB and any amendments thereto, (2) the bidder's response (bid) to the IFB, (3) clarification of the bid, if any, and (4) the City's Purchasing Department's acceptance of the bid by "Notice of Award" or by "Purchase Order". All Exhibits and Attachments included In the IFB shall be incorporated into the contract by reference.

A. The contract expresses the complete agreement of the parties and, performance shall be governed solely by the specifications and requirements contained therein.

B. Any change to the contract, whether by modification and/or supplementation, must be accomplished by a formal contract amendment signed and approved by and between the duly authorized representative of the bidder and the City Purchasing Division or by a modified Purchase Order prior to the effective date of such modification. The bidder expressly and explicitly understands and agrees that no other method and/or no other document, including acts and oral communications by or from any person, shall be used or construed as an amendment or modification to the contract.

3.3.7 Time of Completion: The scheduled time of Completion for the Project is 56 Calendar Days from the starting date specified in the Notice to Proceed.

Completion is achieved when site cleanup and all punch list items (resulting from the final inspection) have been completed. Completion shall have the meaning set forth in Article I, Section 3 (Definitions and Terms) of the General Contract Conditions.

3.3.8 Working Days and Hours: The working days and hours shall be as stated in the General Contract Conditions or as mutually agreed upon in the preconstruction meeting with the following exception:

All work shall be performed between the hours of 7:00 AM to 5:00 PM.

3.3.9 Licenses and Permits: Contractor is responsible for obtaining all necessary licenses and permits required for Construction, at Contractors expense. See Section 2.12. Contractor shall supply to Owner all copies of finalized permits.

3.3.10 Permits: The following permits are required for the Project and will be obtained by the City at no cost to the Contractor:

None

The following permits are required for the Project and shall be obtained and paid for by the Contractor, with the costs included in the total bid price for the Project:

Building permit (State of Colorado)

3.3.11 City Furnished Materials: The City will furnish the following materials for the Project:

None

3.3.12 Project Newsletters: A newsletter for the Project will be prepared and distributed by the City. It will include general information about the Project including interruptions in utility services, street closures, parking restrictions, project schedule, and the names and telephone numbers of the contacts for the City and Contractor. The newsletter will be mailed approximately one week before the Contractor commences work.

The Contractor will be responsible for notifying all businesses and / or residents located adjacent to the work. Door hanger notifications shall be distributed at least two (2) working days prior to the day the work is scheduled to begin.

3.3.13 Project Sign: Project signs, if any, will be furnished and installed by the City.

3.3.14 Authorized Representatives of the City: Those authorized to represent the City shall include Purchasing Agent, Engineers, and Inspectors employed by the City, only.

3.3.15 Stockpiling Materials and Equipment: All stockpiling/storage shall be in accordance with General Contract Condition Section 51.

3.3.16 Traffic Control: The Contractor shall provide and maintain traffic control in accordance with the approved Traffic Control Plan and the Manual on Uniform Traffic Control Devices. A Traffic Control Plan shall be prepared by the Contractor and reviewed by the City two days prior to the pre-construction meeting.

3.3.17 Clean-Up: The Contractor is responsible for cleaning up all loose materials that have been deposited or swept into gutters, and onto sidewalks and driveways as a result of sidewalk operations. The costs for all clean-up work shall be considered incidental and will not be paid for separately.

3.3.18 Quality Control Testing: Supplier shall perform quality control testing on concrete. The City will perform all other necessary QA/QC.

3.3.19 Schedule of Submittals: Contractor shall deliver these submittals at least two days prior to the pre-construction meeting:

- Traffic Control Plans
- Project Schedule

3.3.20 Uranium Mill Tailings: It is anticipated that radioactive mill tailings will not be encountered on this Project.

3.3.21 Fugitive Petroleum or Other Contamination: It is anticipated that soil contamination from fugitive petroleum or other contaminants will not be encountered with the Project.

3.3.22 Excess Material: All excess materials shall be disposed in accordance with General Contract Condition Section 50.

3.3.23 Existing Utilities and Structures: Utilities were **not** potholed during design of this project. The location of existing utilities and structures shown on the Plans is approximate with the information gathered during design. It is the responsibility of the Contractor to pothole/locate and protect all structures and utilities in accordance with General Contract Condition Section 37.

3.3.24 Incidental Items: Any item of work not specifically identified or paid for directly, but which is necessary for the satisfactory completion of any paid items of work, will be considered as incidental to those items, and will be included in the cost of those items.

3.3.25 Survey: The Contractor shall give the City survey crew a minimum of 72 hours' notice for all requested survey.

3.3.26 Work to be Performed by the City (Prior to Construction):

- None

3.3.27 Existing Concrete Sidewalks, Pans, Fillets, Curbs and Gutters: The existing sidewalks, pans, fillets, curb and gutter are in good serviceable condition. In most instances the installation of new sidewalk and pavement will be adjacent to existing concrete. The Contractor will need to protect all concrete adjacent to construction. If the concrete is damaged during construction the Contractor will be responsible for its replacement at no cost to the City. The Contractor, the City Project Inspector, and/or the City Project Manager will walk and record any concrete that is deemed to be damaged before construction has started.

3.3.28 ACI Concrete and Flatwork Finisher and Technician: Hand finishing concrete will be permitted only when performed under the direct supervision of a craftsman holding the following certificate: ACI Concrete Flatwork Finisher and Technician (ACICFFT) or other Flatwork Finisher certification program approved by the City Engineering Manager.

3.4. SCOPE OF WORK:

STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION:

The ***City of Grand Junction Standard Specifications for Road and Bridge Construction*** are hereby modified or supplemented for this Project by the following modifications to ***The Standard Specifications for Road and Bridge Construction***, State Department of Highways, Division of Highways, State of Colorado:

SP-1 SECTION 601 – STRUCTURAL CONCRETE

Section 601 of the Standard Specifications is hereby revised for this project as follows:

Subsection 601.02, Classification:

CONCRETE SHALL MEET THE FOLLOWING REQUIREMENTS:

- 4,500 PSI Compressive at 28 Days
- 6% air ±1.5%
- Slump 4", Loads exceeding 4 ½" shall be rejected
- Maximum Water Cement Ratio no greater than 0.45.

Subsection 601.06, Batching:

This CDOT Specification has been added to this Project:

The Contractor shall furnish a batch ticket (delivery ticket) with each load for all concrete. Concrete delivered without a batch ticket containing complete information as specified shall be rejected. The Contractor shall collect and complete the batch ticket at the placement site and deliver all batch tickets to the Engineer or his representative at the end of each day. The Engineer or his representative shall have access to the batch tickets at any time during the placement. The following information shall be provided on each ticket:

1. Suppliers name and date
2. Truck number
3. Project name and location
4. Concrete class and designation number
5. Cubic yards batched
6. Type brand and amount of each admixture
7. Type, brand, and amount of cement and fly ash
8. Weights of fine and coarse aggregates
9. Moisture of fine and coarse aggregates
10. Gallons of batch water

The contractor shall add the following information to the batch ticket at time of placement:

1. Gallons of water added by the truck operator.
2. Number of revolutions of the drum for mixing
3. Discharge time

SP-2 SECTION 304 – AGGREGATE BASE COURSE

Section 304 of the Standard Specifications is hereby revised for this project as follows:

Subsection 304.01 shall include the following:

In areas of asphalt overlay where there is no curb and gutter and/or a drainage pan, the Contractor shall backfill the shoulder with Class-6 Aggregate Base Course immediately following the overlay. The shoulder shall have a slope of 12:1 or flatter and shall extend a maximum 4' from the edge of asphalt. The aggregate base course shall be brought level to the new pavement surface and compacted. An exception to this will be made in areas where shoulder material would extend into existing yards or landscaping adjacent to the roadway. In these locations, it will be necessary to provide materials that match the existing landscape.

SP-3 SECTION 401 – PLANT MIX PAVEMENTS - GENERAL

REVISION OF SECTION 401
PLANT MIX PAVEMENTS

Section 401 if the Standard Specifications are hereby revised for this project as follows:

401.01 Description.

Add the following:

This work shall consist of providing a Hot Mix Asphalt (HMA) to be placed as shown on the plans, or as directed by the Owner. The Contractor shall be responsible for Process Control (PC) of the HMA; including the design, and control of the quality of the material incorporated into the project.

401.02 Composition of Mixtures.

Delete subparagraph (a) *Mix Design* and replace with the following:

A Job Mix Formula (JMF) design shall be submitted for each mixture required, at least 10 calendar days prior to placing any mix on the project, for acceptance by the Owner. JMF's previously approved by CDOT within the past six months may be utilized. The JMF design shall be determined using AASHTO T-312 or CP-L 5115 for the Method of Mixture Design. Grading ST, SX, and S shall be designed using 100mm molds. The job mix gradation shall be wholly within the Master Range Table in subsection 703.04 before the tolerances shown in Section 401 are applied.

Designs shall be developed and performed in a materials laboratory that meets the requirements set forth by AASHTO Materials Reference Laboratory (AMRL) for all testing procedures. The design shall be stamped and signed by a Professional Engineer licensed in the State of Colorado. In addition, the Contractor shall submit, as part of the mixture design, laboratory data documents to verify the following:

- Gradation, specific gravity, source and description of individual aggregate and properties, and the final blend.
- Aggregate physical properties.
- Source and Grade of the Performance Graded Binder.
- Proposed Design Job Mix: aggregate and additive blending, final gradation, optimum binder content.
- Mixing and compaction temperatures used.
- Mixture properties shall be determined with a minimum of four binder contents.

The JMF for each mixture shall establish a single percentage of aggregate passing each required sieve size, a single percentage of asphalt cement to be added to the aggregate, and a single temperature for the mixture at the discharge point of the plant.

The Owner reserves the right to verify the asphalt supplier's mix design for each JMF design utilizing materials produced and stockpiled. The asphalt supplier shall provide, at no cost, a sufficient quantity of each aggregate, mineral filler, Recycled Asphalt Pavement (RAP), and additive for the required laboratory tests, as well as all Certificates of Conformance/ Compliance at any time on any material used. The Asphalt Supplier shall provide copies of quality control testing results during the production of HMA used within one business day from the sampling date.

Mixture design of HMA shall meet the requirements of Table 403-1 and Table 403-2 in the Revision to Section 403. For mixes requiring a design gyration of 100 (ESALs greater than 3 million) the Project Special Conditions should be used. This gyration is not recommended for the majority of roads within Mesa County.

Delete subparagraph *(b) Mixtures Furnished to the Project* and replace with the following:

Production verification shall occur prior to, or during, the start of the project. Volumetric properties of the mix shall be verified by LabCAT Level C Certified Technicians. If the mix was produced for another project within the last 90 days, data from that project can be submitted for verification. All mixtures furnished for the project shall conform within the ranges of tolerance listed in Table 401.02A. The mix verification test reports shall be submitted to the Owner prior to mix placement.

TABLE
401.02A
Production Mix
Tolerances

Property	Tolerance
Asphalt Cement Content	± 0.3%
VMA	± 1.2%
Air Voids	± 1.2%

Verification testing for binder content, gradation and physical properties shall be performed at the frequencies listed in Table 401.23-1.

There shall be no substitutions of materials allowed during production, unless approved in advance by the Owner. All substitutions will require checkpoint verification. If the checkpoint differs from the Job Mix Formula (JMF), a new mix design will be required. Upon request of the Owner, the binder grade may be changed by one available binder grade level without requiring a new mix design.

Should a change in the source of any material used in the production of HMA (aggregate, mineral filler, lime, or performance graded asphalt binder) occur, a one point verification test (at optimum binder content) of the mix must be performed to verify that the applicable criteria shown on Table 403-1 (HMA) and Table 403-2 (VMA) of Revision to Section 403 are still met. If this testing shows noncompliance, the Contractor shall establish a new job mix design and obtain approval by the Owner before the new HMA is used.

Add the following new subparagraphs:

(c) *Reclaimed Asphalt Pavement (RAP).* RAP shall be allowed in HMA up to a maximum binder replacement of 23 percent, unless otherwise specified in the contract, and provided that all the specifications for the HMA are met. Fine Aggregate Angularity requirements shall apply only to the virgin fraction of the fine aggregate. RAP shall be of uniform quality and gradation with a maximum size no greater than the nominal aggregate size of the mix. RAP shall not contain clay balls, vegetable matter, or other deleterious substances.

The Contractor shall have an approved mix design for the amount of RAP to be used. The AC content of the RAP utilized in the Contractor RAP mix design shall be the average AC content determined in accordance with 1B or 1C, below, or alternatively, a minimum of five samples of the Contractors RAP stockpile may be sampled and the average AC content of the RAP be determined

using AASHTO T-164, Method A or B, or in accordance with 1C below. The Contractor shall determine the total binder replaced by the binder in the RAP pursuant to the following equation:

Total Binder Replaced = (A x B) x 100/E
Where:

A = RAP % Binder Content *
B = RAP % in Mix *
E = Total Effective Binder Content *
* in decimal format (i.e. 2% is 0.02)

The Total Binder Replaced by the binder in the RAP shall not exceed 23 percent of the effective binder content of either the mix design or the produced mix.
The Contractor shall have an approved Quality Control (QC) Plan that details how the RAP will be processed and controlled. The QC plan shall address the following:

1. RAP Processing Techniques. This requires a schematic diagram and narrative that explains the processing (crushing, screening, and rejecting) and stockpile operation for this specific project.

2. Control of RAP Asphalt Binder Content (AASHTO T-164, Method A or B). RAP Asphalt Binder Content may also be determined in accordance with CP-L 5120, provided an RAP AC content correction factor is determined through correlation testing with AASHTO T-164, Method A or B. The correction factor shall be determined by performing correlation testing on the first five samples of the RAP AC content, then at a frequency of one for every five AC content tests thereafter. The correction factor shall be determined by calculating the average difference in AC content between CP-L 5120 and AASHTO T-164, Method A or B, and applying the correction to the AC content determined in accordance with CP-L 5120 :
   Frequency: 1 per 1000 tons of processed RAP material (minimum five tests)

3. (Alternate) The Contractor may propose a RAP asphalt content correction factor to be used in conjunction with CP-L 5120. The proposed CP-L 5120 RAP asphalt content correction factor shall be used with all RAP asphalt contents tested for the mixture design and quality control sampling and testing. The methodology of the proposed CP-L 5120 RAP asphalt content correction factor shall be outlined in detail in the approved RAP QC Plan. At a minimum, the proposed CP-L 5120 correction factor shall identify the principal source locations of the RAP aggregate, gradation of the material tested, and specific ignition oven serial number used in all the RAP asphalt content testing. The RAP source locations, material gradation, and specific equipment used shall substantiate the CP-L 5120 asphalt content correction factor used for the testing. The substantiation must be from data gathered from historical information or specific asphalt content correction data obtained from tests performed on similar virgin aggregate sources, virgin material gradations, and the specific equipment used.

4. Control of RAP Gradation (CP31 or AASHTO T-30):
   Frequency: 1 per1000 tons of processed RAP material (minimum three tests, sampling from belt feed and not stockpile).

5. Process Control Charts shall be maintained for binder content and each screen listed in Table 401.2C, during addition of any RAP material to the stockpile. The Contractor shall maintain separate control charts for each RAP stockpile. The control charts shall be

displayed and shall be made available, along with RAP AC extraction testing laboratory reports to the Engineer upon request.

The processed RAP must be 100 percent passing the 31.5 mm (1¼ inch) sieve. The aggregate obtained from the processed RAP shall be 100 percent passing the 25.0 mm (1 inch) sieve. The aggregate and binder obtained from the processed RAP shall be uniform in all the measured parameters in accordance with the following:

Table 401.2C
RAP Binder & Aggregate Uniformity Tolerances

Element	StandardDeviation
Binder Content	0.5
% Passing ¾"	4.0
% Passing ½"	4.0
% Passing 3/8"	4.0
%Passing #4	4.0
% Passing #8	4.0
% Passing #30	3.0
% Passing #200	1.5

(d) *Warm Mix Asphalt (WMA) Technology.* The Contractor may choose to use a WMA Technology that is included on the CDOT approved products list (https://www.codot.gov/business/apl/asphalt-warm-mix.html).

WMA technologies (additive or foaming) used shall be identified on the mix design, indicating usage as a workability additive and/or anti-strip additive. WMA shall be submitted and approved by the Owner for use on a project.

The addition of WMA additives during production, including foaming, shall be controlled by a calibrated metering system interlocked with the plant's controls per the manufacturers' recommendation. Additives may be added at the asphalt terminal at the dosage rate recommended by the WMA technology provider. The foaming process mixes water and binder to create microscopic steam bubbles. Typical water injection rate is ≤ 2% of binder flow rate or per manufacturers' recommendation.

(e) *Anti-Strip Additives.* Anti-Strip shall be added into the HMA. Anti-Strip agents may be liquids (added to the binder), lime (added to the aggregates) or other products, and shall be submitted for approval by the Owner.

The minimum value for Tensile Strength Ratio (TSR) tested in accordance with Table 401.21-1 shall be 80% for the mix design and 70% during production.

There are various types of liquid Anti-Strips. Amine and Organo-silane type liquid Anti-Strip additives are physically mixed with the asphalt binder. Liquid Anti-Strip agents shall be added per the manufacture's recommendations. Typical product dosages are provided in Table 401.2E-1.

TABLE 401.2E-1
Liquid Anti-Strip Dosage Rates

Type	Typical Dosage Rate
Amine	0.4% to 0.8%
Organo-silane	0.05% to 0.15%

WMA chemical products which display Anti-Stripping characteristics will be classified, and identified on the mix design, as a liquid Anti-Strip additive.

When a liquid Anti-Strip additive is used, the Contractor shall include the following information with the mix design submission:

- Information on the type of liquid Anti-Strip additive to be supplied, including product name, product manufacturer/supplier
- Additive rate
- TSR values for the treated mixes
- The proposed method for incorporating the additive into the plant produced mix

401.03 Aggregates.

Add the following:

The percentage of fractured faces shall be as shown in Table 403.1 of the Revision to Section 403.

Grading ST (3/8" nominal) mixes may be used for leveling, maintenance, bike paths, sidewalks and thin lift overlays. Grading SX (1/2" nominal) mixes shall be used on top and bottom lifts and for patching. Grading S (3/4" nominal) mixes may be used for bottom lifts.

401.05 Hydrated Lime.

Add the following:

When used in the HMA, hydrated lime shall be added at the rate of 1% by dry weight of the aggregate and shall be included in the amount of material passing the No. 200 sieve.

401.06 Asphalt Cement.

Revise the second paragraph to read as follows:

The asphalt cement shall meet the applicable requirements of subsection 702.01.

Add the following:

The Contractor shall provide to the Owner acceptable 'Certifications of Compliance' of each applicable asphalt binder grade from the supplier. Should testing or certificate show nonconformance with the specifications, the asphalt binder may be rejected. When production begins, the Contractor

shall, upon request, provide to the Owner a one quart can of each specified asphalt binder for analysis. Additionally, the Contractor shall provide the refinery test results that pertain to the asphalt binders used during production.

Based on climatic conditions and reliability, binder grades approved for use in Mesa County are as follows in Table 401.06A-1:

TABLE
401.06A-1
Recommended Performance Graded Binders

Condition	Non-modified Binder	Modified Binder
Free flowing traffic loads and 300,000 to 1 million 18K ESAL	PG 64-22	
Free flowing traffic loads and 300,000 to 1 million 18K ESAL, plus above 6000 elevation	PG 58-28	
Slow moving or standing trucks, major street intersections and/or 10,000,000 18K ESAL		PG 76-28 (top lift only)

Binder grades other than those shown above shall not be used unless the proposed binder and the mix design are approved in writing by the OWNER. The asphalt cement shall meet the requirements of subsection 702.01

401.07 Weather Limitations and Placement Temperatures.

Revise as follows:

Surface temperatures shall be used to determine placement of APM. APM produced with documented WMA will be allowed a reduction in minimum surface temperatures for placement as provided in Table 401.07A-1. Ambient temperatures and other weather conditions shall be considered prior to placement.

TABLE 401.07A-1
Minimum Surface Temperatures for placement of APM

Compacted Layer Thickness (in.)	Minimum Surface Temperature (ºF)			
	Top Layer		Layers Below the Top Layer	
Product	APM	with WMA	APM	with WMA
<1½	60	50	50	40
1½ - <3	50	45	40	35
3 or more	45	40	35	35

If the Contractor modifies the placement and compaction processes when ambient temperatures are below minimum surface temperatures in Table 401.07A-1, they shall demonstrate to the Owner the

required in-place density has been achieved. APM cooling software such as PaveCool, or MultiCool can be used to determine placement and compaction times available.

401.08 Asphalt Mixing Plant.

Delete the last paragraph of the subsection.

401.09 Hauling Equipment.

Add the following:

The Owner may reject any HMA which demonstrates it has been contaminated from a petroleum distillate release agent. The Owner may reject any uncovered HMA which demonstrates it has been impacted by contamination and/or weather.

401.10 Asphalt Pavers.

Delete the twelve paragraph and replace with the following:

Contractor shall submit for and receive approval of the screed control devices to be utilized on the paver prior to use for placing HMA on the project.

Add the following:

A Material Transfer Vehicle (MTV) or Material Transfer Device (MTD) may be required for placement of the HMA when specified in the contract documents. The MTV shall be a self-propelled unit with on board storage of material. An MTD is a non-self-propelled unit. Both MTV and MTD are capable of receiving material from trucks or from the ground, transferring the material from the unit to a paver hopper insert via a conveyor system.

401.11 Tack Coat.

Delete and replace with the following:

A tack coat shall be applied between pavement course and to all existing concrete and asphalt surfaces per Section 407. Tack coat is considered incidental to the cost of the HMA.

401.15 Mixing.

Add the following:

If a WMA technology (additive or foaming) is used, the discharge temperatures may be lowered during production at the discretion of the Contractor provided all specifications are achieved. Mix design is to indicate revised allowable discharge temperatures with WMA usage.

401.16 Spreading and Finishing.

Revise as follows:

Joints in the top layer of new pavement shall be located on lane lines unless otherwise shown on the plans. Longitudinal joints shall be minimized with wide paving pulls. Transverse joints shall be formed by cutting back on the previous run to expose the full depth of the course. Tack coat material shall be applied to contact surfaces of all joints before additional mixture is placed against the previously compacted material.

401.17 Compaction.

Revise as follows:

Equipment used for compaction of the HMA will be at the discretion of the Contractor. The number, weight, and type of rollers furnished shall be sufficient to obtain the required density and surface texture.

All joints shall be compacted to 92% of maximum theoretical specific gravity (Rice), taken six inches offset from the joint. The allowable variance shall be ±2%. Joint density will be determined using nuclear density equipment.

Delete paragraphs six through eight, and paragraphs eleven to the end of the subsection and replace with the following:

Cores may be used to verify compaction results. The Contractor shall core the pavement, as required by the Owner**;** in accordance with AASHTO T 230, Method B, or for field calibration of nuclear density equipment in accordance with the ASTM D 2950. At a minimum, cores for nuclear density equipment correlation shall be taken at the beginning of placement of each project or change of mixture materials or gradation, unless otherwise approved by the Engineer. If the correlation cores were produced for another project within the last 90 days, data from that project can be submitted for verification, if no change in materials or gradation has occurred. When cores are used, the Contractor shall provide all labor and equipment for the coring and repair of the holes.

Along forms, curbs, headers, walls, and all other places not accessible to the rollers, the mixture shall meet all project compaction specifications. Any mixture that is defective, shall be corrected to meet the project specifications at the expense of the Contractor.

401.20 Surface Smoothness.

Delete and replace with the following:

The finish transverse and longitudinal surface elevation of the pavement shall be measured using a 10-foot straightedge. Surface smoothness shall be verified following the finish roller pass. Surface variation shall not exceed 3/16 inch in 10 feet for full lane width paving. For patching, the variation shall not exceed 3/8 inch in 10 feet. The final pavement surface shall not vary from the specified cross section by more than one inch at any point. Transverse measurements for variations shall exclude breaks in the crown sections. If the surface tolerance exceeds 3/16” across transverse joints, measured in at least three locations, the Contractor shall make corrections to the joint before proceeding. All corrections shall be made at the Contractor’s expense.

The final surface pavement adjacent to curb and gutter shall be finished from 1/8-inch to 3/8- inches above the lip for catch curb and shall not extend above the lip for spill curb.

The Contractor shall adjust all manholes, valve boxes, and survey range boxes 1/8 to 1/4- inch below final grade and adjusted to match the slope of the roadway. Valve boxes and manholes are to be maintained fully accessible at all times for emergency and maintenance operations. The cost of adjusting valve boxes, manholes, and survey range boxes shall be included in the work, unless otherwise specified. The Contractor shall be responsible for any cost incurred by the Owner to provide access to the covered manholes or valve boxes. Final adjustment of all utility access points shall be completed within seven days of from the time the HMA was placed.

Add the following new subsections:

401.23 Testing and Inspection

The Contractor shall assume full responsibility for controlling all operations and processes to meet the Specifications. The Contractor shall perform all tests necessary for process control purposes on all elements at the frequency listed in Table 401.23-1. The Contractor shall maintain a log of all process control testing. Test results that have sampling or testing errors shall not be used. Process control testing shall be performed at the expense of the Contractor.

Laboratories shall be accredited by AASHTO Materials Reference Laboratory (AMRL) for the tests being performed. Technicians obtaining samples and conducting compaction tests must have a LabCAT Level A certification. Technicians conducting tests of asphalt content and gradation must have a LabCAT Level B certification. Technicians performing volumetric testing must have a LabCAT Level C certification. Equivalent NICET certification for all technicians is acceptable.

When requested by the Owner, the Contractor shall submit a quality control plan that addresses production, sampling, testing, and qualifications of testing personnel, timing, and methods for making adjustments to meet the specifications. The Contractor will provide a process or schedule for making corrections for material that was placed but does not meet specifications as well as obtain a follow up sample immediately after corrective actions are taken to assess the adequacy of the corrections. In the event the follow-up process control sample also fails to meet Specification requirements; the Contractor shall cease production of the asphalt mixture until the problem is adequately resolved to the satisfaction of the Owner.

TABLE
401.23-1
Minimum Materials Sampling and Testing for Process Control and Owners Acceptance

Test	Standard	Minimum Frequency
Sampling	AASHTO T168, ASTM D 979 and ASTM D3665, CP 41	1/1000 tons or fraction thereof (not less than one test per day)
In-Place Density	AASHTO T 166, T 238, T 230, CP 81 (nuclear), CP 44 (coring)	One test for each 250 lineal feet per lane and one test per 1,000 lineal feet of joint per lift
Thickness (Core) (when called for in Project specs.)	ASTM D3549	One test for each 1000 lineal feet per lane

Air Voids & VMA	CP-L 5115 A.I. SP-2	1/1000 tons or fraction thereof (not less than one test per day)
Gradation	AASHTO T 27/T 11, CP 31	1/1000 tons or fraction thereof (not less than one test per day)
Hveem/Marshall Stability As Applicable	CP-L 5016	One per project per mix used
Binder Content	CP-L 5120, AASHTO T 164 or other methods agreed upon between Owner and Contractor	1/1000 tons or fraction thereof (not less than one test per day)
Maximum Theoretical Specific Gravity (Rice)	AASHTO T 209 (Rice), CP-L 51	1/1000 tons or fraction thereof (not less than one test per day)
Lottman Stripping, TSR & Dry Density	CP-L 5109	One per project per mix used.

Field control testing of dense graded asphalt mixes for the above tests shall meet the requirements of Table 403-1 and Table 403-2 in the Revision to Section 403.

401.24 Acceptance

If any materials furnished, or work performed, fails to meet the specification requirements, such deficiencies shall be documented and reported to the Owner. Copies of all process control tests shall be delivered to the Owner within one business day. Test results that cannot be completed within one day shall be provided to the Owner no later than three days after the sample was obtained.

Owners Acceptance (OA) test results, if any, and/or Process Control (PC) test results will be evaluated to determine acceptability. If the Contractor does not meet the project specifications, but acceptable work has been produced, the Owner shall determine the extent of the work to be accepted. If the Owner determines the work is not acceptable, the Contractor shall correct the work, as approved by the Owner, at the expense of the Contractor.

SP-4 SECTION 403 – HOT MIX ASPHALT

REVISION OF SECTION 403
HOT MIX ASPHALT

Section 403 of the Standard Specifications is hereby revised for this project as follows:

403.02 Materials

Delete and replace with the following:

The materials shall conform to the requirements of subsections 401.2 of the Revised Section 401 above.

The design mix for hot mix asphalt (HMA) shall conform to the following Table 403-1 and Table 403-2:

Table 403-1
Mixture Properties for Hot Mix Asphalt

Property	Test Method	Value
Air Voids, percent at: N (design)	AASHTO T-132, CPL 5115	3.0 – 4.0
Lab Compaction (Revolutions): N (design)	CPL 5115	75
Hveem Stability, (Grading ST, SX & S only)	CPL 5106	28 min.
Aggregate Retained on the 4.75 mm (No. 4) Sieve for S, SX and SG, and on the 2.36mm (No. 8) Sieve for ST and SF with at least 2 Mechanically Induced fractured faces	CP 45	60% min.
Accelerated Moisture Susceptibility Tensile Strength Ratio (Lottman)(for S & SX mixes)	AASHTO T-283 Method B, CPL 5109 Method B	80 min.
Minimum Dry Split Tensile Strength, kPa (psi)	CPL 5109 Method B	205 (30) min.
Voids in the Mineral Aggregate (VMA) % minimum	CP 48, AI-SP2	See Table 403-2
Voids Filled with Asphalt (VFA)	AI MS-2	65-80%
Dust to Asphalt Ratio: Fine Gradation Coarse Gradation	CP 50	 0.6 – 1.2 0.8 – 1.6

Note: AI MS-2 = Asphalt Institute Manual Series 2

Note: Mixes with gradations having less than 40% passing the 4.75 mm (No. 4) sieve shall be approached with caution because of constructability problems.

Note: Gradations for mixes with a nominal maximum aggregate size of one-inch or larger are considered a coarse gradation if they pass below the maximum density line at the #4 screen.
Gradations for mixes with a nominal maximum aggregate size of 3/4" to 3/8" are considered a coarse gradation if they pass below the maximum density line at the #8 screen.
Gradations for mixes with a nominal maximum aggregate size of #4 or smaller are considered a coarse gradation if they pass below the maximum density line at the #16 screen.

Table 403-2
Minimum Voids in Mineral Aggregate (VMA)

Nominal Maximum Size*, mm (inches)	3.5%	***Design Air Voids **	
		4.0%	4.5%
37.5 (1½)	11.6	11.7	11.8
25.0 (1)	12.6	12.7	12.8
19.0 (¾)	13.6	13.7	13.8
12.5 (½)	14.6	14.7	14.8
9.5 (⅜)	15.6	15.7	15.8
4.75 (No. 4)	16.6	16.7	16.8

* The Nominal Maximum Size is defined as one sieve larger than the first sieve to retain more than 10%.
** Interpolate specified VMA values for design air voids between those listed.
*** Extrapolate specified VMA values for production air voids beyond those listed.

403.03 Construction Requirements

Delete the first paragraph and replace with the following:

The construction requirements shall be as prescribed in subsections 401.3 through 401.14 of the Revised Section 401 above.

403.04 Method of Measurement

Delete and replace with the following:

Hot Mix Asphalt will be measured by the ton or the square yard. Batch weights will not be permitted as a method of measurement when measured by the ton. The tonnage shall be the weight used in the accepted pavement.

403.05 Basis of Payment

Delete and replace with the following:

The accepted quantities of hot mix asphalt will be paid for in accordance with subsection 401.22, at the contract unit price per ton or square yard for the asphalt mixture.

Payment will be made under:

Pay Item	**Pay Unit**
Hot Mix Asphalt (Grading __)(PG___)	Ton
Hot Mix Asphalt (Grading __)(PG ___)	Square Yard
Hot Mix Asphalt (Patching)	Square Yard

Aggregate, asphalt cement, asphalt recycling agent, additives, hydrated lime, tack coat, and all other work necessary to complete each hot mix asphalt items will not be paid for separately but shall be included in the unit price bid.

Excavation, preparation, and tack coat of areas to be patched will not be measured and paid for separately, but shall be included in the work.

SP-5 SECTION 407 – PRIME COAT, TACK COAT, AND REJUVINATING AGENT

REVISIONS OF SECTION 407
PRIME COAT, TACK COAT, AND REJUVENATING AGENT

Section 407 of the Standard Specifications is hereby revised for this project as follows:

407.01 Description

Add the following:

Prior to placement of APM, a tack coat shall be applied to all existing concrete and asphalt surfaces.

407.02 Asphalt Material.

Add the following:

The tack coat shall meet the specification for emulsified asphalt, consisting of CSS-1h or SS-1h, and conform to AASHTO M208 or M140.

407.07 Application of Asphalt Material.

Add the following:

The tack coat shall be applied at the rates specified in Table 407-1. The surface receiving the tack coat shall be dry and clean, and dust, debris, and foreign matter shall be removed. Tack coat shall be applied uniformly. The Contractor shall allow the tack coat to cure (dehydrate) prior to the placement of APM. If the tack becomes contaminated during construction, it shall be cleaned, and if necessary, additional tack coat shall be reapplied and allowed to cure before paving resumes.

TABLE 407-1
Tack Coat Application Rates

Pavement Condition	Application Rate (gal/yd^2)		
	Residual	Undiluted	Diluted (1:1)
New asphalt	0.03 - 0.04	0.05 – 0.07	0.10 – 0.13
Oxidized asphalt	0.04 – 0.06	0.07 – 0.10	0.13 – 0.20
Milled Surface (asphalt)	0.06 – 0.08	0.10 – 0.13	0.20 – 0.30
Milled Surface (PCC)	0.06 – 0.08	0.10 – 0.13	0.20 – 0.30
Portland Cement Concrete	0.04 – 0.06	0.07 – 0.10	0.13 – 0.20

407.09 Method of Measurement and Basis of Payment.

Delete and replace the following:

Tack Coat will not be measured and paid separately but shall be considered included in the work for Section 401 – Asphalt Pavement Materials.

SP-6 UTILITIES

The contractor shall coordinate the concrete foundation work with the installation of the electrical conduit (by others). Prior to the concrete pour, the contractor shall contact Marc McAtlin at (970) 257-7414 to verify that the electrical conduit has been installed and is secured for the concrete placement.

3.5. Attachments:

Appendix A: Project Submittal Form
Appendix B: Geotechnical Reports
Appendix C: Construction Drawings

3.6. Contractor Bid Documents: For Contractor's convenience, the following is a list of forms/items to be submitted with the Contractor's bid response. However, should a form/item not be listed in this section, but required in the solicitation documents, it is the Contractor's responsibility to ensure all forms/items are submitted.

- **Contractor's Bid Form**
- **Price Bid Schedule**
- **References**

3.7. IFB TENTATIVE TIME SCHEDULE:

Invitation For Bids available	April 2, 2020
Recommended Site Visit Meeting	April 10, 2020
Inquiry deadline, no questions after this date	April 15, 2020
Addendum Posted	April 16, 2020
Submittal deadline for proposals	April 21, 2020
City Council Approval	May 6, 2020
Notice of Award & Contract execution	May 7, 2020
Bonding & Insurance Cert due	May 15, 2020
Preconstruction meeting	May 15, 2020
Work begins no later than	Upon Receipt of Notice to Proceed
Final Completion	56 Calendar Days from Notice to Proceed
Holidays:	May 25, 2020 July 3-4, 2020

4. Contractor's Bid Form

Bid Date: _____

Project: IFB-4785-20-DH "Fire Training Facility – Shed Foundation & Asphalt Pad"

Bidding Company: _____

Name of Authorized Agent: _____

Email _____

Telephone_____ Address_____

City_____State_____Zip_____

The undersigned Bidder, in compliance with the Invitation for Bids, having examined the Instruction to Bidders, General Contract Conditions, Statement of Work, Specifications, and any and all Addenda thereto, having investigated the location of, and conditions affecting the proposed work, hereby proposes to furnish all labor, materials and supplies, and to perform all work for the Project in accordance with Contract Documents, within the time set forth and at the prices stated below. These prices are to cover all expenses incurred in performing the work required under the Contract Documents, of which this Contractor's Bid Form is a part.

The undersigned Contractor does hereby declare and stipulate that this offer is made in good faith without collusion or connection to any person(s) providing an offer for the same work, and that it is made in pursuance of, and subject to, all terms and conditions of the Instructions to Bidders, the Specifications, and all other Solicitation Documents, all of which have been examined by the undersigned.

The Contractor also agrees that if awarded the Contract, to provide insurance certificates within ten (10) working days of the date of Notification of Award. Submittal of this offer will be taken by the Owner as a binding covenant that the Contractor will be prepared to complete the project in its entirety.

The Owner reserves the right to make the award on the basis of the offer deemed most favorable, to waive any formalities or technicalities and to reject any or all offers. It is further agreed that this offer may not be withdrawn for a period of sixty (60) calendar days after closing time. Submission of clarifications and revised offers automatically establish a new thirty day (30) period.

Prices in the bid proposal have not knowingly been disclosed with another provider and will not be prior to award.

- Prices in this bid proposal have been arrived at independently, without consultation, communication or agreement for the purpose of restricting competition.
- No attempt has been made nor will be to induce any other person or firm to submit a bid proposal for the purpose of restricting competition.
- The individual signing this bid proposal certifies they are a legal agent of the offeror, authorized to represent the offeror and is legally responsible for the offer with regard to supporting documentation and prices provided.
- Direct purchases by the City of Grand Junction are tax exempt from Colorado Sales or Use Tax. Tax exempt No. 98-903544. The undersigned certifies that no Federal, State, County or Municipal tax will be added to the above quoted prices.
- City of Grand Junction payment terms shall be Net 30 days.
- Prompt payment discount of _____ percent of the net dollar will be offered to the Owner if the invoice is paid within _____ days after the receipt of the invoice. The Owner reserves the right to take into account any such discounts when determining the bid award.

RECEIPT OF ADDENDA: the undersigned Contractor acknowledges receipt of Addenda to the Solicitation, Specifications, and other Contract Documents.

State number of Addenda received: _____.

It is the responsibility of the Bidder to ensure all Addenda have been received and acknowledged.

By signing below, the Undersigned agree to comply with all terms and conditions contained herein.

Company: _____

Authorized Signature: _____
Title: _____

Schedule: Fire Training Facility - Shed Foundation & Asphalt

Item No.	CDOT, City Ref.	Description	Quantity	Units	Unit Price	Total Price
1	201-00000	Clearing and Grubbing	0.6	ACRE	$ _____	$ _____
2	202-04060	Dust Abatement	Lump	Sum	- - -	$ _____
3	203-00000	Unclassified Excavation	74.	CY	$ _____	$ _____
4	206-00100	Structural Backfill (Class 1)	74.	CY	$ _____	$ _____
5	208-00045	Concrete Washout Structure	1.	EA	$ _____	$ _____
6	208-00070	Vehicle Tracking Pad	1.	EA	$ _____	$ _____
7	208	Erosion Control	Lump	Sum	- - -	$ _____
8	304	Aggregate Base Course (Class 6)	1,016.	CY	$ _____	$ _____
9	403	Hot Mix Asphalt (Grading SX, PG 76 - 28)	278.	TON	$ _____	$ _____
10	403	Hot Mix Asphalt (Grading SX, PG 64 - 22)	278.	TON	$ _____	$ _____
11	420	Geotextile (Class 1)	258.	SY	$ _____	$ _____
12	601-03020	Concrete Class GV-B	71.	CY	$ _____	$ _____
13	602-00010	Reinforcing Steel (Black)	8,300.	LB	$ _____	$ _____
14	603-01125	12 Inch Reinforced Concrete Pipe (Complete In Place)	65.	LF	$ _____	$ _____
15	608	Bituminous Curb (6" Wide, 6" High)	520.	LF	$ _____	$ _____
16	620-00020	Sanitary Facility	1.	EA	$ _____	$ _____
17	625-00000	Construction Surveying	Lump	Sum	- - -	$ _____
18	626-00000	Mobilization	Lump	Sum	- - -	$ _____
MCR		Minor Contract Revisions	- - -	- - -	- - -	$ 25,000.00

Bid Amount: **$** _____

Bid Amount:

_____ **dollars**

The undersigned Bidder proposes to subcontract the following portion of Work:

Name & address of Sub-Contractor	Description of work to be performed	% of Contract

The undersigned Bidder acknowledges the right of the City to reject any and all Bids submitted and to waive informalities and irregularities therein in the City's sole discretion.

By submission of the Bid, each Bidder certifies, and in the case of a joint Bid each party thereto certifies as to his own organization, that this Bid has been arrived at independently, without collusion, consultation, communication, or agreement as to any matter relating to this Bid with any other Bidder or with any competitor.

APPENDIX A
Project Submittal Form

PROJECT SUBMITTAL FORM

PROJECT: **2020 Fire Training Facility – Storage Shed Foundation & Asphalt Pad**

CONTRACTOR:

PROJECT ENGINEER: Kirsten Armbruster

Description	Date Received	Resubmittal Requested	Resubmittal Received	Date Accepted
STORAGE SHED FOUNDATION CONSTRUCTION				
Base course gradation, Proctor curves (Str Bkfl Cl 1)				
Geotextile cut sheet				
Concrete Mix Design				
Rebar Shop Drawings				
Anchor Bolt cut sheet				
ASPHALT PAD CONSTRUCTION				
Base course gradation, Proctor curves (ABC Cl 6)				
Asphalt Mix Design				
12-inch RCP cut sheet				
EROSION CONTROL / STORMWATER MANAGEMENT				
Vehicle Tracking Pad				
Concrete Washout				
PERMITS, PLANS, OTHER				
Construction Schedule				
Erosion Control Plan				

APPENDIX B
Geotechnical Investigations

White Water Hill Training Facility Geotechnical Investigation

March 20, 2015

Test hole #1 Station 0+50 location north side of HWY 141 at the toe of the road prism

0 to 2 ½ Feet the material was generally consistent with that of a silty sand (SM) material. 2 ½ - 5 feet the material was a pit run cobble material (SM). 5 – 7 feet the material turned to a decomposed shale material.

Test hole #2 approximate station 2+50 location south side of HWY 141 at the toe of the road prism

The top 1 foot was generally consistent with that of a sandy clay (CL) material. 1 - 4 feet the material was a pit run, cobble material (SM). 4 – 7 feet the material turned to a decomposed shale material.

Test hole #3 approximate station 10+50, location top of the hill

The full depth of the test hole to 5 feet consisted of pit run cobble material poorly graded with a sand-silt mixtures (SM).

Test hole #4 approximate station 63+38, Center location of the proposed water tank

The top 1 ½ foot was generally consistent with that of a sandy clay (CL) material. 1 ½ - 4 feet the material was a very fine sand /rock flour material (ML).

Huddleston-Berry
Engineering & Testing, LLC

GEOTECHNICAL AND GEOLOGIC HAZARDS INVESTIGATION REGIONAL LAW ENFORCEMENT TRAINING FACILITY MESA COUNTY, COLORADO PROJECT#00456-0009

RIVER CITY CONSULTANTS, INC.
744 HORIZON COURT, SUITE 110
GRAND JUNCTION, COLORADO 81506

MARCH 1, 2012

Huddleston-Berry Engineering and Testing, LLC
640 White Avenue, Unit B
Grand Junction, Colorado 81501

SUMMARY OF CONCLUSIONS AND RECOMMENDATIONS

A geologic hazards and geotechnical investigation was conducted for the proposed Regional Law Enforcement Training Facility in Mesa County, Colorado. The project location is shown on Figure 1 – Site Location Map. The purpose of the investigation was to evaluate the surface and subsurface conditions at the site with respect to geologic hazards, foundation design, pavement design, and earthwork for the proposed construction. This summary has been prepared to include the information required by civil engineers, structural engineers, and contractors involved in the project.

Subsurface Conditions (p. 3)

The subsurface investigation consisted of eight borings, drilled on February 1st, 2012. The locations of the borings are shown on Figure 2 – Site Plan. The borings generally encountered lean clay soils above dense gravel soils. Groundwater was not encountered in the borings at the time of the investigation. The native clay soils are moderately plastic and range from slightly collapsible at their existing density to slightly expansive upon compaction.

Geologic Hazards (p. 4)

The primary geologic hazard at the site is the presence of moisture sensitive soils. However, steep slopes are also present near the southeastern property boundary.

Summary of Foundation Recommendations

- *Foundation Type* – Spread Footings or Monolithic Structural Slabs (p. 5)
- *Structural Fill* – Minimum of 24-inches below foundations or to the native gravel soils, whichever is less. The native clay soils are not suitable for reuse as structural fill. The native gravel soils are suitable for reuse as structural fill. Imported structural fill should consist of pit-run, CDOT Class 6 base course, or other granular material approved by the engineer. (p. 5)
- *Maximum Allowable Bearing Capacity* – 1,500 psf where native clay soils present in subgrade. 2,000 psf where native gravel soils present in subgrade. (p. 5)
- *Subgrade Modulus* –250 pci for native gravels, pit-run, crusher fines, or base course. (p. 6)
- *Lateral Earth Pressure* – 60 pcf (p. 6)
- *Seismic Site Classification* – Site Class D (p. 6)
- *Setback from Steep Slopes* – Minimum of 50 feet from crest of slopes (p. 7)

Summary of Pavement Recommendations (p. 7)

Automobile Parking Areas
EDLA = 5, Structural Number = 2.75

ALTERNATIVE	PAVEMENT SECTION (Inches)				
	Hot-Mix Asphalt Pavement	CDOT Class 6 Base Course	CDOT Class 3 Subbase Course	Rigid Pavement	TOTAL
Full Depth HMA	7.0				7.0
A	3.0	10.0			13.0
B	4.0	7.0			11.0
C	3.0	6.0	6.0		15.0
Full Depth RP		6.0		6.0	12.0

Site Access Roads/Driving Tracks
EDLA = 20, Structural Number = 3.50

ALTERNATIVE	PAVEMENT SECTION (Inches)				
	Hot-Mix Asphalt Pavement	CDOT Class 6 Base Course	CDOT Class 3 Subbase Course	Rigid Pavement	TOTAL
Full Depth HMA	8.0				8.0
A	3.0	16.0			19.0
B	4.0	13.0			17.0
C	5.0	10.0			15.0
D	3.0	6.0	14.0		23.0
Full Depth RP		6.0		7.0	13.0

Where gravel pavements are proposed in automobile parking areas, a minimum gravel pavement thickness of 12-inches is recommended. Where gravel pavements are proposed along the site access roads and/or driving tracks, the recommended section is 12-inches of gravel pavement above geogrid consisting of Tensar TX140, or equivalent.

TABLE OF CONTENTS

1.0 INTRODUCTION

As part of extensive infrastructure improvements in Western Colorado, Mesa County proposes to construct a new Regional Law Enforcement Training Facility. As part of the development process, Huddleston-Berry Engineering and Testing, LLC (HBET) was retained by River City Consultants, Inc. to conduct a geologic hazards and geotechnical investigation at the site.

1.1 Scope

As discussed above, a geologic hazards and geotechnical investigation was conducted for the proposed Regional Law Enforcement Training Facility in Mesa County, Colorado. The scope of the investigation included the following components:

- Conducting a subsurface investigation to evaluate the subsurface conditions at the site.
- Collecting soil samples and conducting laboratory testing to determine the engineering properties of the soils at the site.
- Providing recommendations for foundation type and subgrade preparation.
- Providing recommendations for bearing capacity.
- Providing recommendations for lateral earth pressure.
- Providing recommendations for drainage, grading, and general earthwork.
- Providing recommendations for pavements.
- Evaluating potential geologic hazards at the site.

The investigation and report were completed by a Colorado registered professional engineer in accordance with generally accepted geotechnical and geological engineering practices. This report has been prepared for the exclusive use of River City Consultants, Inc. and Mesa County.

1.2 Site Location and Description

The site is located on approximately 80 acres located east of the Grand Junction drag racing track in Mesa County, Colorado. The project location is shown on Figure 1 – Site Location Map.

At the time of the investigation, the site was generally open. The site lies on top of a ridge with undulating terrain. Several small drainage channels cross the site. Steep slopes are present along the southeastern edge of the site. Vegetation at the site consisted primarily of weeds and brush. The site was bordered to the north, south, and east by open land, and to the west by a drag racing track and model airplane flying facility.

1.3 Proposed Construction

The proposed construction is anticipated to include the following:

- Access roads and parking areas.
- High speed serpentine driver training track.
- Skid pads.
- Simulated city block training area.
- ATV training track.
- Cross country running track and obstacle course.
- Shooting range.
- Mobile shoot house.
- Classroom/shelter area.
- Security fencing around the facility.
- Utilities.

2.0 GEOLOGIC SETTING

2.1 Soils

Soils data was obtained from the USDA Natural Resource Conservation Service Web Soil Survey. The data indicates that the soils at the site consist of Badlands-Deaver-Chipeta complex, 25 to 99 percent slopes, extremely stony and Leebench, warm-Avalon-Blackston complex, 1 to 12 percent slopes. Soil survey data, including descriptions of the soil units, is included in Appendix A.

Structure construction in the site soils is described as ranging from somewhat limited to very limited due to slope, shrink-swell, and/or depth to soft bedrock. Road construction is described as ranging from not limited to very limited due to slope, shrink-swell, depth to soft bedrock, and/or low strength. Excavation in the site soils is described as being very limited due to slope, depth to soft bedrock, clay content, and cutbank caving. The soils are indicated to have a low potential for frost action, moderate to high potential for corrosion of uncoated steel, and low to high risk of corrosion of concrete.

2.2 Geology

According to the *Geologic Map of Colorado* by Ogden Tweto (1979), the site is underlain by Mancos Shale bedrock. The Mancos Shale unit is thick in Western Colorado and has a low to moderate potential for swelling.

2.3 Groundwater

Groundwater was not encountered in the borings at the time of the investigation.

3.0 SUBSURFACE INVESTIGATION

The subsurface investigation was conducted on February 1st, 2012 and consisted of eight borings, drilled to depths of between 4.0 and 13.0 feet below the existing ground surface. The locations of the borings are shown on Figure 2 – Site Plan. The borings were located in the field relative to existing site features. Typed boring logs are included in Appendix B. Samples of the native soils were collected during Standard Penetration Testing (SPT) and using bulk sampling methods at the locations shown on the logs.

As indicated on the logs, the subsurface conditions at the site were slightly variable. However, Borings B-1, B-2, B-3, B-7, and B-8 generally encountered 1.0 foot of lean clay with organics topsoil at the surface. In all but B-7, the topsoil was underlain by tan to white, dry, stiff to very stiff lean clay to depths of between 2.0 and 7.0 feet. Below the topsoil in B-7 and below the lean clay in the other borings, tan to white to brown, dry to moist, medium dense to very dense silty gravel with sand extended to depths of between 3.5 and 12.0 feet. The silty gravel was underlain by tan to gray, dry to moist, dense to very dense sandy gravel and cobbles to the bottoms of the borings. Groundwater was not encountered in any of these borings at the time of the investigation.

Borings B-4, B-5, and B-6 generally encountered 1.0 foot of lean clay with organics topsoil above tan to white to brown to red, dry to moist, stiff to hard lean clay to depths of between 2.0 and 9.0 feet. Below the clay, tan to gray, dry to moist, dense to very dense sandy gravel and cobbles extended to the bottoms of the borings. Groundwater was not encountered in any of these borings at the time of the investigation.

4.0 LABORATORY TESTING

Selected native soil samples collected from the borings were tested in the Huddleston-Berry Engineering and Testing LLC geotechnical laboratory for natural moisture and density, grain size analysis, maximum dry density and optimum moisture (Proctor), Atterberg limits, swell/consolidation, soluble sulfates content, and California Bearing Ratio (CBR). The laboratory testing results are included in Appendix C.

The laboratory testing results indicate that the native clay soils are moderately plastic. In addition, the native clay soils were shown to be slightly collapsible at their existing density with up to approximately 1.4% collapse measured in the laboratory. However, the CBR results indicate that the native clay soils have a slight potential for expansion if compacted and introduced to excess moisture with up to 1.4% expansion measured in the laboratory. The native silty gravel with sand soils were indicated to be slightly plastic. Water soluble sulfates were detected in the site soils in a concentration of 0.4%.

5.0 GEOLOGIC INTERPRETATION

5.1 Geologic Hazards

The primary geologic hazard at the site is the presence of moisture sensitive soils. However, as discussed previously, steep slopes were present along the southeastern property boundary.

5.2 Geologic Constraints

The primary geologic constraint to construction at the site is the presence of moisture sensitive soils. However, the steep slopes discussed above may impact the location of facilities at the site. In addition, in some of the borings, the sandy gravel and cobble soils encountered were strongly cemented and these materials may be difficult to excavate. This may impact foundation construction and/or utility installation at the site.

5.3 Water Resources

No water supply wells were observed on the property. In addition, as discussed previously, shallow groundwater was not encountered during the subsurface investigation. Although no surface water bodies were observed at the site, several ephemeral drainages were observed crossing the site. However, in general, with proper grading and stormwater management, the proposed construction at the site is not anticipated to adversely affect surface water or groundwater.

5.4 Mineral Resources

No significant mineral resources were identified on the property. Potential mineral resources in Western Colorado generally include gravel, uranium ore, and commercial rock products such as flagstone. The site is mapped in the Mesa County GIS database as containing possible gravel resources. As indicated on the boring logs, gravels were encountered in the subsurface. However, the shallow gravels were limited in thickness. In addition, the deeper gravels were strongly cemented in some locations. In general, although evaluation of the gravels as a resource was beyond the scope of the current investigation, HBET does not believe that the gravels at the site represent an economic resource.

6.0 CONCLUSIONS

Based upon the available data sources, field investigation, and nature of the proposed construction, HBET does not believe that there are any geologic conditions which should preclude construction at the site.

7.0 RECOMMENDATIONS

7.1 Foundations

Based upon the results of the subsurface investigation and nature of the proposed construction, shallow foundations are recommended. Spread footings and monolithic (turndown) structural slabs are both appropriate alternatives. However, as discussed previously, the native clay soils are anticipated to range from slightly collapsible at their existing density to slightly expansive when compacted. Therefore, to limit the potential for excessive differential movements, it is recommended that the foundations be constructed above a minimum of 24-inches of structural fill or structural fill extending to the native gravel soils, whichever is less.

In addition, as discussed previously, the native clay soils were indicated to be moderately plastic and are anticipated to be slightly expansive upon compaction. Therefore, it is recommended that the native clay soils not be reused as structural fill. However, the native gravel soils are suitable for reuse as structural fill. Imported structural fill should consist of a granular, non-expansive, non-free draining material such as pit run, crusher fines, or CDOT Class 6 base course. However, if pit-run or the native gravel and cobble soils are used as structural fill, a minimum of six inches of Class 6 base course or crusher fines should be placed on top of the pit-run/gravels to prevent large point stresses on the bottoms of the foundations due to large particles in the pit-run/gravels.

Prior to placement of structural fill, it is recommended that the bottoms of the foundation excavations in the native clays be scarified to a depth of 6 to 8-inches, moisture conditioned, and re-compacted to a minimum of 95% of the standard Proctor maximum dry density, within 0 to -2% of the optimum moisture content as determined in accordance with ASTM D698. It is recommended that the bottoms of the foundation excavations in the gravels be proofrolled to identify any soft or weak materials. Soft or weak materials should be removed and replaced with structural fill.

Structural fill should extend laterally beyond the edges of the foundations a distance equal to the thickness of structural fill. Structural fill should be moisture conditioned, placed in maximum 8-inch loose lifts, and compacted to a minimum of 95% of the standard Proctor maximum dry density for fine grained soils or modified Proctor maximum dry density for coarse grained soils, within 0 to -2% of the optimum moisture content as determined in accordance with ASTM D698 or D1557C, respectively. Pit-run and/or native gravel and cobble materials should be proofrolled to the Engineer's satisfaction.

For foundation building pads prepared as recommended with structural fill consisting of the native gravels or imported granular materials, a maximum allowable bearing capacity of 1,500 psf may be used where the native clay soils are present below the structural fill. A maximum allowable bearing capacity of 2,000 psf may be used where the native gravel soils are present below the structural fill.

In addition, a modulus of subgrade reaction of 250 pci may be used for structural fill consisting of the native gravels, pit-run, crusher fines, or base course. It is recommended that the bottoms of exterior foundations be at least 24-inches below the final grade for frost protection.

As discussed previously, water soluble sulfates were detected in the site soils in a concentration of 0.4%. This concentration represents a severe degree of potential sulfate attack on concrete exposed to these materials. Therefore, Type V sulfate resistant cement is recommended for construction at this site in accordance with the International Building Code (IBC). However, Type V cement can be difficult to obtain in Western Colorado. Where Type V cement is unavailable, a minimum of Type I-II cement is recommended.

7.2 Non-Structural Floor Slabs and Exterior Flatwork

In general, slabs-on-grade cannot develop sufficient bearing pressures to resist heave. Therefore, some movement of non-structural slabs-on-grade should be anticipated over time where the native clays are present in the subgrade. However, in order to reduce the potential for and/or magnitude of movement of slabs-on-grade, it is recommended that non-structural floor slabs be constructed above a minimum of 12-inches of structural fill or structural fill extending to the native gravel soils, whichever is less. Subgrade preparation and fill placement should be in accordance with the *Foundations* section of this report.

It is recommended that exterior flatwork be constructed above native soils, below the topsoil, that have been scarified to a depth of 12-inches, moisture conditioned, and compacted to a minimum of 95% of the standard Proctor maximum dry density, within 0 to -2% of the optimum moisture content as determined in accordance with ASTM D698. Slabs-on-grade should not be connected to the foundations in any manner.

7.3 Lateral Earth Pressures

Stemwalls and/or retaining walls should be designed to resist lateral earth pressures. For backfill consisting of the native soils or imported granular, non-free draining, non-expansive material, we recommend that the walls be designed for an equivalent fluid unit weight of 60 pcf in areas where no surcharge loads are present. Lateral earth pressures should be increased as necessary to reflect any surcharge loading behind the walls.

7.4 Seismic Site Classification

Based upon the results of the subsurface investigation, the site generally classifies as Seismic Site Class D for a stiff soil profile.

7.5 Slope Stability

As discussed previously, steep slopes were observed along the southeastern property boundary. In general, the existing slopes appeared to be stable. However, it is recommended that any new construction at the site (structures, pavements, etc.) be set back a minimum of 50 feet from the crest of the slopes.

7.6 Drainage

In order to improve the long-term performance of the foundations and slabs-on-grade, grading around the structure should be designed to carry precipitation and runoff away from the structures. It is recommended that the finished ground surface drop at least twelve inches within the first ten feet away from the structures. However, where impermeable surfaces (i.e. sidewalks, pavements, etc.) are adjacent to the structures, the grade can be reduced to three inches within the first ten feet away from the structures. Downspouts should empty beyond the backfill zone. It is recommended that landscaping within five feet of the structures include primarily desert plants with low water requirements. In addition, it is recommended that automatic irrigation within ten feet of foundations be minimized or controlled with automatic shut off valves.

7.7 Excavations

Excavations in the soils at the site may stand for short periods of time but should not be considered to be stable. Trenching and excavations should be sloped back, shored, or shielded for worker protection in accordance with applicable OSHA standards. The soils generally classify as Type C soil with regard to OSHA's *Construction Standards for Excavations*. For Type C soils, the maximum allowable slope in temporary cuts is 1.5H:1V.

7.8 Pavements

The proposed construction is anticipated to include access roadways, driving tracks, and parking areas. As discussed previously, the pavement subgrade materials consist primarily of lean clay soils. The design CBR of the native clay soils was determined in the laboratory to be less than 2.0. Therefore, the minimum recommended Resilient Modulus of 3,000 psi was used for the pavement design.

Based upon the subgrade conditions and anticipated traffic loading, pavement section alternatives were developed in accordance with the *Guideline for the Design and Use of Asphalt Pavements for Colorado Roadways* by the Colorado Asphalt Pavement Association and CDOT *Pavement Design Manual*. The following minimum pavement section alternatives are recommended:

Automobile Parking Areas
EDLA = 5, Structural Number = 2.75

ALTERNATIVE	PAVEMENT SECTION (Inches)				
	Hot-Mix Asphalt Pavement	CDOT Class 6 Base Course	CDOT Class 3 Subbase Course	Rigid Pavement	TOTAL
Full Depth HMA	7.0				7.0
A	3.0	10.0			13.0
B	4.0	7.0			11.0
C	3.0	6.0	6.0		15.0
Full Depth RP		6.0		6.0	12.0

Site Access Roads/Driving Tracks
EDLA = 20, Structural Number = 3.50

ALTERNATIVE	PAVEMENT SECTION (Inches)				
	Hot-Mix Asphalt Pavement	CDOT Class 6 Base Course	CDOT Class 3 Subbase Course	Rigid Pavement	TOTAL
Full Depth HMA	8.0				8.0
A	3.0	16.0			19.0
B	4.0	13.0			17.0
C	5.0	10.0			15.0
D	3.0	6.0	14.0		23.0
Full Depth RP		6.0		8.0	14.0

Where gravel pavements are proposed in automobile parking areas, a minimum gravel pavement thickness of 12-inches is recommended. Where gravel pavements are proposed along the site access roads and/or driving tracks, the recommended pavement section is 12-inches of gravel above geogrid consisting of Tensar TX140, or equivalent.

Prior to pavement placement, areas to be paved should be stripped of all topsoil, fill, or other unsuitable materials. It is recommended that the subgrade soils be scarified to a depth of 12-inches; moisture conditioned, and recompacted to a minimum of 95% of the standard Proctor maximum dry density, within 0 to -2% of optimum moisture content as determined by AASHTO T-99.

Aggregate base course and subbase course should be placed in maximum 9-inch loose lifts, moisture conditioned, and compacted to a minimum of 95% and 93% of the maximum dry density, respectively, at -2% to +3% of optimum moisture content as determined by AASHTO T-180. In addition to density testing, base course should be proofrolled to verify subgrade stability.

It is recommended that Hot-Mix Asphaltic (HMA) pavement conform to CDOT grading SX or S specifications and consist of an approved 75 gyration Superpave method mix design. HMA pavement should be compacted to between 92% and 96% of the maximum theoretical density. An end point stress of 50 psi should be used. It is recommended that rigid pavements consist of CDOT Class P concrete or alternative approved by the Engineer. In addition, pavements should conform to local specifications.

Due to the presence of moisture sensitive soils at the site, the long-term performance of the pavements is dependent on positive drainage away from the pavements. In addition, ditches, culverts, and inlet structures in the vicinity of paved areas must be maintained to prevent ponding of water on the pavement.

8.0 GENERAL

The recommendations included above are based upon the results of the subsurface investigation and on our local experience. These conclusions and recommendations are valid only for the proposed construction.

It is important to note that the recommendations provided in this report are intended to reduce, but not eliminate, the potential for structural movement as a result of swelling and/or collapse of the native clay soils. While the recommendations are consistent with generally accepted engineering practices in areas of moisture sensitive subgrade materials, HBET cannot predict long-term changes in subsurface moisture conditions and/or the precise magnitude or extent of volume change. Although the potential for movement still exists, HBET believes that with proper application of the recommendations in this report, any structural movements will be within acceptable levels.

As discussed previously, the subsurface conditions at the site were slightly variable. However, the precise nature and extent of subsurface variability may not become evident until construction. Therefore, it is recommended that a representative of HBET be retained to provide engineering oversight and construction materials testing services during the construction. This is to verify compliance with the recommendations included in this report or permit identification of significant variations in the subsurface conditions which may require modification of the recommendations.

Huddleston-Berry Engineering and Testing, LLC is pleased to be of service to your project. Please contact us if you have any questions or comments regarding the contents of this report.

Respectfully Submitted:
Huddleston-Berry Engineering and Testing, LLC

Michael A. Berry, P.E.
Vice President of Engineering

FIGURES

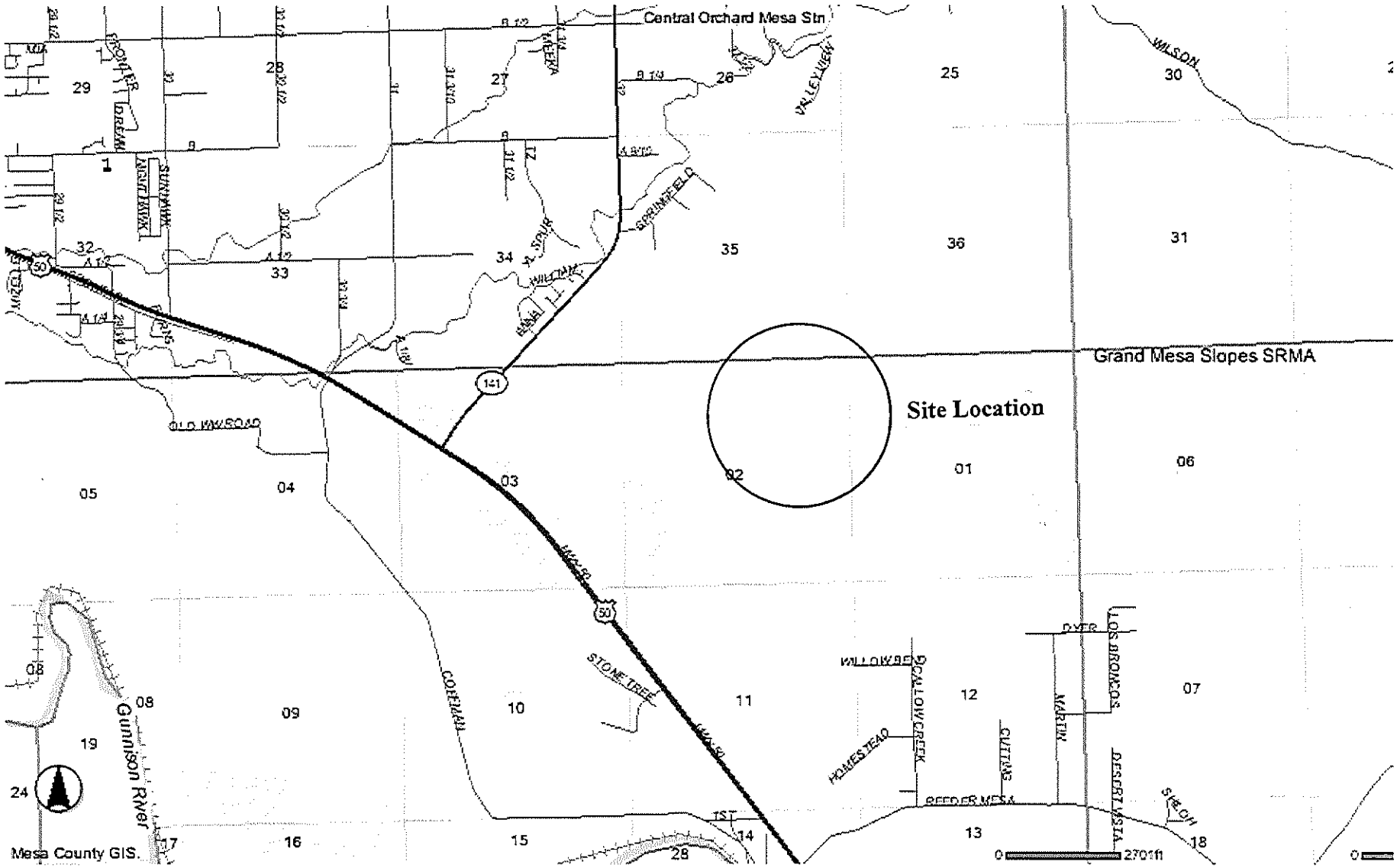


FIGURE 1
Site Location Map

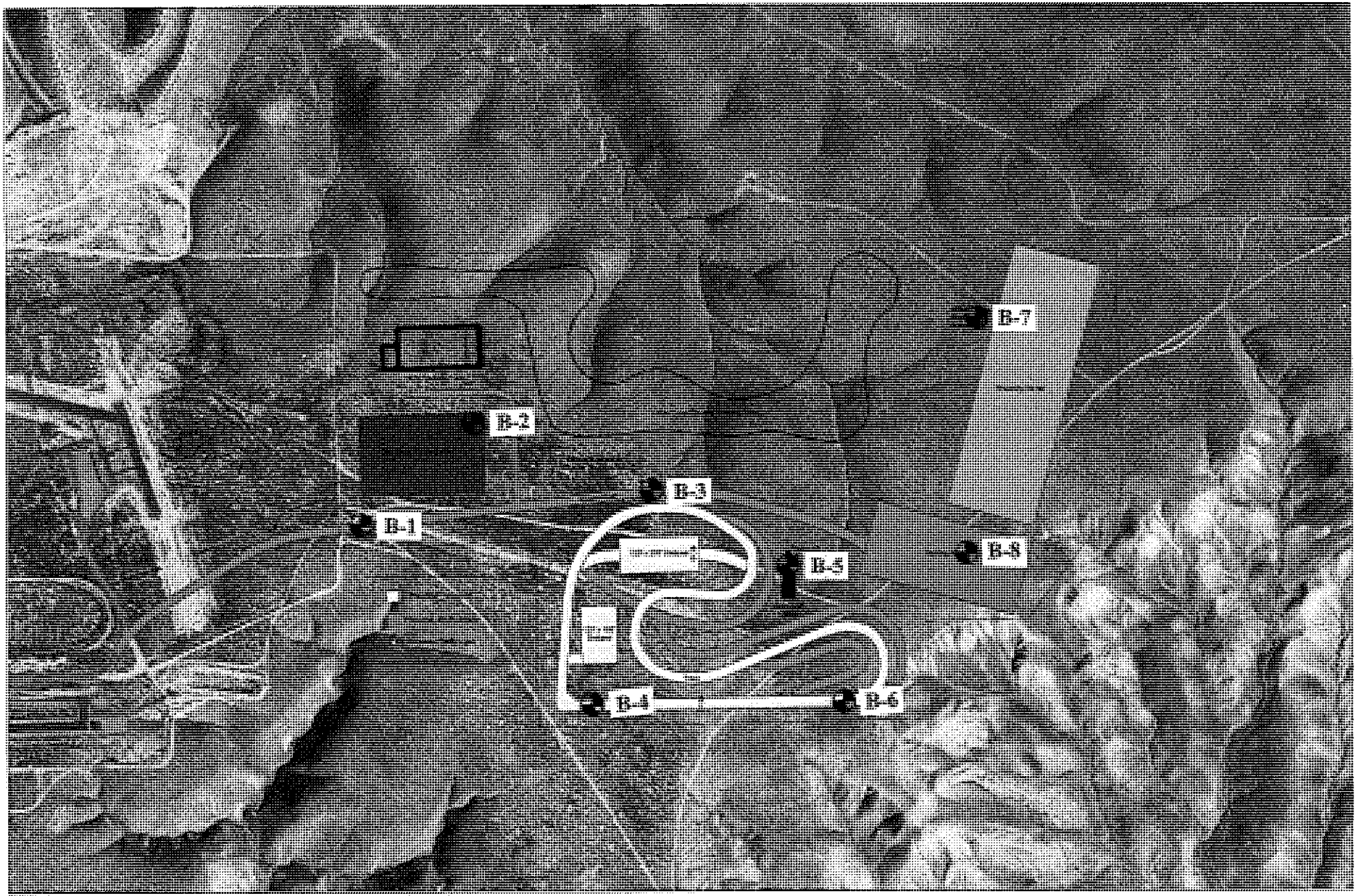
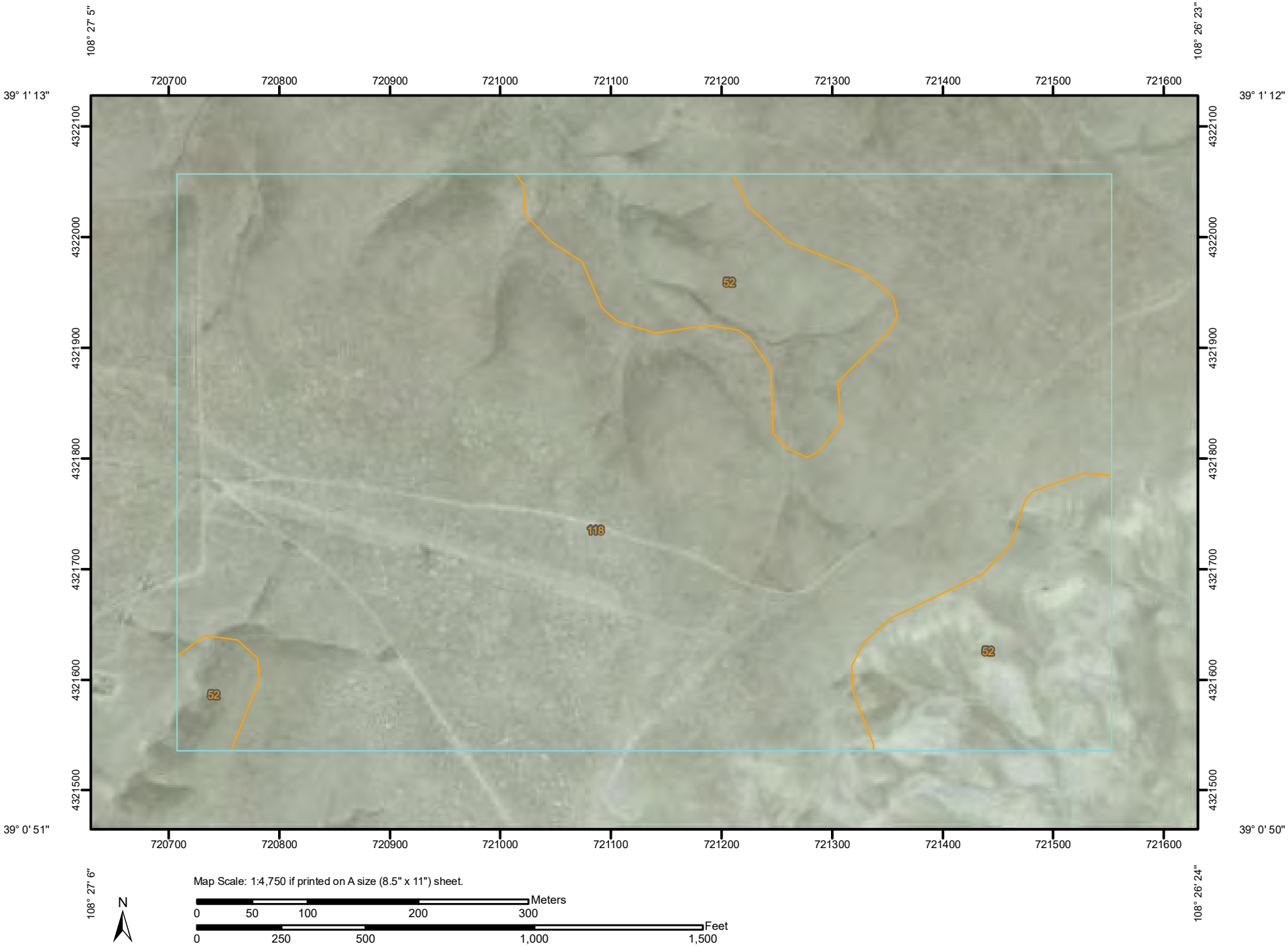


FIGURE 2
Site Plan

APPENDIX A
Soil Survey Data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108° 26' 23"
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721000
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721600
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39° 1' 12"
4322100
4322000
4321900
4321800
4321700
4321600
4321500
52
118
52
52
39° 0' 51"
39° 0' 50"
108° 27' 6"
108° 26' 24"
Map Scale: 1:4,750 if printed on A size (8.5" x 11") sheet.
N
Meters
0 50 100 200 300
Feet
0 250 500 1,000 1,500

MAP LEGEND

Area of Interest (AOI)
- Area of Interest (AOI)

Soils
- Soil Map Units

Special Point Features
- Blowout
- Borrow Pit
- Clay Spot
- Closed Depression
- Gravel Pit
- Gravelly Spot
- Landfill
- Lava Flow
- Marsh or swamp
- Mine or Quarry
- Miscellaneous Water
- Perennial Water
- Rock Outcrop
- Saline Spot
- Sandy Spot
- Severely Eroded Spot
- Sinkhole
- Slide or Slip
- Sodic Spot
- Spoil Area
- Stony Spot
- Very Stony Spot
- Wet Spot
- Other

Special Line Features
- Gully
- Short Steep Slope
- Other

Political Features
- Cities

Water Features
- Streams and Canals

Transportation
- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

MAP INFORMATION

Map Scale: 1:4,750 if printed on A size (8.5" × 11") sheet.

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for accurate map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov
Coordinate System: UTM Zone 12N NAD83

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Mesa County Area, Colorado
Survey Area Data: Version 3, Sep 25, 2007

Date(s) aerial images were photographed: 8/24/2005

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Mesa County Area, Colorado (CO680)			
Map Unit Symbol	**Map Unit Name**	**Acres in AOI**	**Percent of AOI**
52	Badlands-Deaver-Chipeta complex, 25 to 99 percent slopes, extremely stony	21.5	19.8%
118	Leebench, warm-Avalon-Blackston complex, 1 to 12 percent slopes, stony	87.3	80.2%
Totals for Area of Interest		**108.8**	**100.0%**

Map Unit Description

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions in this report, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. All the soils of a series have major horizons that are similar in composition, thickness, and arrangement. Soils of a given series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Additional information about the map units described in this report is available in other soil reports, which give properties of the soils and the limitations, capabilities, and potentials for many uses. Also, the narratives that accompany the soil reports define some of the properties included in the map unit descriptions.

Report—Map Unit Description

Mesa County Area, Colorado

52—Badlands-Deaver-Chipeta complex, 25 to 99 percent slopes, extremely stony

Map Unit Setting

Elevation: 4,800 to 7,000 feet
Mean annual precipitation: 7 to 10 inches
Mean annual air temperature: 47 to 50 degrees F
Frost-free period: 120 to 150 days

Map Unit Composition

Badland: 35 percent

Deaver and similar soils: 30 percent
Chipeta and similar soils: 25 percent

Description of Badland

Setting

Landform: Hills, erosion remnants
Down-slope shape: Convex
Across-slope shape: Linear
Parent material: Residuum weathered from sandstone and shale

Properties and qualities

Slope: 25 to 99 percent
Depth to restrictive feature: 0 to 4 inches to paralithic bedrock
Drainage class: Somewhat excessively drained
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately high (0.00 to 0.20 in/hr)
Calcium carbonate, maximum content: 10 percent
Gypsum, maximum content: 4 percent
Maximum salinity: Very slightly saline to moderately saline (4.0 to 16.0 mmhos/cm)
Sodium adsorption ratio, maximum: 4.0
Available water capacity: Very low (about 0.0 inches)

Interpretive groups

Land capability (nonirrigated): 8

Typical profile

0 to 2 inches: Silty clay loam
2 to 12 inches: Weathered bedrock

Description of Deaver

Setting

Landform: Mesas
Landform position (two-dimensional): Backslope
Down-slope shape: Convex
Across-slope shape: Linear
Parent material: Colluvium derived from basalt over residuum weathered from clayey shale

Properties and qualities

Slope: 25 to 65 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: 20 to 40 inches to paralithic bedrock
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 10 percent
Gypsum, maximum content: 5 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 4.0 mmhos/cm)

Sodium adsorption ratio, maximum: 10.0
Available water capacity: Low (about 4.1 inches)

Interpretive groups
Land capability (nonirrigated): 7e
Ecological site: Stony Saltdesert (R034XY404CO)

Typical profile
0 to 3 inches: Very cobbly silty clay loam
3 to 30 inches: Clay
30 to 34 inches: Weathered bedrock

Description of Chipeta

Setting
Landform: Mesas
Landform position (two-dimensional): Backslope
Down-slope shape: Concave
Across-slope shape: Linear
Parent material: Residuum weathered from clayey shale

Properties and qualities
Slope: 25 to 65 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: 5 to 20 inches to paralithic bedrock
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Very low (0.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 10 percent
Gypsum, maximum content: 2 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 4.0 mmhos/cm)
Available water capacity: Very low (about 2.4 inches)

Interpretive groups
Land capability (nonirrigated): 7s
Ecological site: Stony Saltdesert (R034XY404CO)

Typical profile
0 to 2 inches: Gravelly silty clay
2 to 16 inches: Silty clay
16 to 20 inches: Weathered bedrock

118—Leebench, warm-Avalon-Blackston complex, 1 to 12 percent slopes, stony

Map Unit Setting
Elevation: 5,000 to 6,000 feet
Mean annual precipitation: 6 to 10 inches
Mean annual air temperature: 50 to 54 degrees F
Frost-free period: 150 to 180 days

Map Unit Composition

Leebench, warm, and similar soils: 45 percent
Avalon and similar soils: 35 percent
Blackston and similar soils: 15 percent

Description of Leebench, Warm

Setting

Landform: Mesas
Landform position (two-dimensional): Summit
Down-slope shape: Convex
Across-slope shape: Linear
Parent material: Alluvium derived from sandstone and shale

Properties and qualities

Slope: 1 to 12 percent
Surface area covered with cobbles, stones or boulders: 0.1 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.20 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 35 percent
Available water capacity: Moderate (about 7.5 inches)

Interpretive groups

Land capability (nonirrigated): 7s
Ecological site: Loamy Saltdesert (R034XY401CO)

Typical profile

0 to 4 inches: Fine sandy loam
4 to 10 inches: Sandy clay loam
10 to 35 inches: Sandy clay loam
35 to 45 inches: Very cobbly sandy clay loam
45 to 60 inches: Very gravelly sandy clay loam

Description of Avalon

Setting

Landform: Mesas
Down-slope shape: Concave
Across-slope shape: Linear
Parent material: Alluvium and/or slope alluvium derived from sandstone and shale

Properties and qualities

Slope: 1 to 12 percent
Surface area covered with cobbles, stones or boulders: 0.6 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)

Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 40 percent
Maximum salinity: Nonsaline (0.0 to 2.0 mmhos/cm)
Available water capacity: High (about 9.6 inches)

Interpretive groups

Land capability (nonirrigated): 7c
Ecological site: Loamy Saltdesert (R034XY401CO)

Typical profile

0 to 2 inches: Sandy loam
2 to 8 inches: Sandy loam
8 to 50 inches: Sandy clay loam
50 to 60 inches: Clay loam

Description of Blackston

Setting

Landform: Mesas
Landform position (two-dimensional): Summit
Down-slope shape: Convex
Across-slope shape: Convex
Parent material: Alluvium derived from sandstone and shale and/or alluvium derived from basalt

Properties and qualities

Slope: 3 to 12 percent
Surface area covered with cobbles, stones or boulders: 0.1 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 35 percent
Maximum salinity: Nonsaline (0.0 to 2.0 mmhos/cm)
Available water capacity: Very low (about 2.8 inches)

Interpretive groups

Land capability (nonirrigated): 7s
Ecological site: Loamy Saltdesert (R034XY401CO)

Typical profile

0 to 3 inches: Very gravelly sandy clay loam
3 to 7 inches: Gravelly clay loam
7 to 15 inches: Very gravelly sandy clay loam
15 to 35 inches: Extremely gravelly sandy loam

35 to 60 inches: Extremely gravelly sand

Data Source Information

Soil Survey Area: Mesa County Area, Colorado
Survey Area Data: Version 3, Sep 25, 2007

Dwellings and Small Commercial Buildings

Soil properties influence the development of building sites, including the selection of the site, the design of the structure, construction, performance after construction, and maintenance. This table shows the degree and kind of soil limitations that affect dwellings and small commercial buildings.

The ratings in the table are both verbal and numerical. Rating class terms indicate the extent to which the soils are limited by all of the soil features that affect building site development. *Not limited* indicates that the soil has features that are very favorable for the specified use. Good performance and very low maintenance can be expected. *Somewhat limited* indicates that the soil has features that are moderately favorable for the specified use. The limitations can be overcome or minimized by special planning, design, or installation. Fair performance and moderate maintenance can be expected. *Very limited* indicates that the soil has one or more features that are unfavorable for the specified use. The limitations generally cannot be overcome without major soil reclamation, special design, or expensive installation procedures. Poor performance and high maintenance can be expected.

Numerical ratings in the table indicate the severity of individual limitations. The ratings are shown as decimal fractions ranging from 0.01 to 1.00. They indicate gradations between the point at which a soil feature has the greatest negative impact on the use (1.00) and the point at which the soil feature is not a limitation (0.00).

Dwellings are single-family houses of three stories or less. For dwellings without basements, the foundation is assumed to consist of spread footings of reinforced concrete built on undisturbed soil at a depth of 2 feet or at the depth of maximum frost penetration, whichever is deeper. For dwellings with basements, the foundation is assumed to consist of spread footings of reinforced concrete built on undisturbed soil at a depth of about 7 feet. The ratings for dwellings are based on the soil properties that affect the capacity of the soil to support a load without movement and on the properties that affect excavation and construction costs. The properties that affect the load-supporting capacity include depth to a water table, ponding, flooding, subsidence, linear extensibility (shrink-swell potential), and compressibility. Compressibility is inferred from the Unified classification. The properties that affect the ease and amount of excavation include depth to a water table, ponding, flooding, slope, depth to bedrock or a cemented pan, hardness of bedrock or a cemented pan, and the amount and size of rock fragments.

Small commercial buildings are structures that are less than three stories high and do not have basements. The foundation is assumed to consist of spread footings of reinforced concrete built on undisturbed soil at a depth of 2 feet or at the depth of maximum frost penetration, whichever is deeper. The ratings are based on the soil properties that affect the capacity of the soil to support a load without movement and on the properties that affect excavation and construction costs. The properties that affect the load-supporting capacity include depth to a water table, ponding, flooding, subsidence, linear extensibility (shrink-swell potential), and compressibility (which is inferred from the Unified classification). The properties that affect the ease and amount of excavation include flooding, depth to a water table, ponding, slope, depth to bedrock or a cemented pan, hardness of bedrock or a cemented pan, and the amount and size of rock fragments.

Information in this table is intended for land use planning, for evaluating land use alternatives, and for planning site investigations prior to design and construction. The information, however, has limitations. For example, estimates and other data generally apply only to that part of the soil between the surface and a depth of 5 to 7 feet. Because of the map scale, small areas of different soils may be included within the mapped areas of a specific soil.

The information is not site specific and does not eliminate the need for onsite investigation of the soils or for testing and analysis by personnel experienced in the design and construction of engineering works.

Government ordinances and regulations that restrict certain land uses or impose specific design criteria were not considered in preparing the information in this table. Local ordinances and regulations should be considered in planning, in site selection, and in design.

Report—Dwellings and Small Commercial Buildings

[Onsite investigation may be needed to validate the interpretations in this table and to confirm the identity of the soil on a given site. The numbers in the value columns range from 0.01 to 1.00. The larger the value, the greater the potential limitation. The table shows only the top five limitations for any given soil. The soil may have additional limitations]

Dwellings and Small Commercial Buildings– Mesa County Area, Colorado							
Map symbol and soil name	Pct. of map unit	Dwellings without basements		Dwellings with basements		Small commercial buildings	
		Rating class and limiting features	Value	Rating class and limiting features	Value	Rating class and limiting features	Value
52—Badlands-Deaver-Chipeta complex, 25 to 99 percent slopes, extremely stony							
Badland	35	Not rated		Not rated		Not rated	
Deaver	30	Very limited		Very limited		Very limited	
		Slope	1.00	Slope	1.00	Slope	1.00
		Shrink-swell	1.00	Shrink-swell	1.00	Shrink-swell	1.00
				Depth to soft bedrock	0.46		
Chipeta	25	Very limited		Very limited		Very limited	
		Slope	1.00	Slope	1.00	Slope	1.00
		Shrink-swell	1.00	Shrink-swell	1.00	Depth to soft bedrock	1.00
		Depth to soft bedrock	0.50	Depth to soft bedrock	1.00	Shrink-swell	1.00

Dwellings and Small Commercial Buildings– Mesa County Area, Colorado							
Map symbol and soil name	Pct. of map unit	Dwellings without basements		Dwellings with basements		Small commercial buildings	
		Rating class and limiting features	Value	Rating class and limiting features	Value	Rating class and limiting features	Value
118—Leebench, warm-Avalon-Blackston complex, 1 to 12 percent slopes, stony							
Leebench, warm	45	Not limited		Not limited		Somewhat limited	
						Slope	0.88
Avalon	35	Not limited		Not limited		Somewhat limited	
						Slope	0.88
Blackston	15	Somewhat limited		Somewhat limited		Very limited	
		Slope	0.01	Slope	0.01	Slope	1.00

Data Source Information

Soil Survey Area: Mesa County Area, Colorado
Survey Area Data: Version 3, Sep 25, 2007

Roads and Streets, Shallow Excavations, and Lawns and Landscaping

Soil properties influence the development of building sites, including the selection of the site, the design of the structure, construction, performance after construction, and maintenance. This table shows the degree and kind of soil limitations that affect local roads and streets, shallow excavations, and lawns and landscaping.

The ratings in the table are both verbal and numerical. Rating class terms indicate the extent to which the soils are limited by all of the soil features that affect building site development. *Not limited* indicates that the soil has features that are very favorable for the specified use. Good performance and very low maintenance can be expected. *Somewhat limited* indicates that the soil has features that are moderately favorable for the specified use. The limitations can be overcome or minimized by special planning, design, or installation. Fair performance and moderate maintenance can be expected. *Very limited* indicates that the soil has one or more features that are unfavorable for the specified use. The limitations generally cannot be overcome without major soil reclamation, special design, or expensive installation procedures. Poor performance and high maintenance can be expected.

Numerical ratings in the table indicate the severity of individual limitations. The ratings are shown as decimal fractions ranging from 0.01 to 1.00. They indicate gradations between the point at which a soil feature has the greatest negative impact on the use (1.00) and the point at which the soil feature is not a limitation (0.00).

Local roads and streets have an all-weather surface and carry automobile and light truck traffic all year. They have a subgrade of cut or fill soil material; a base of gravel, crushed rock, or soil material stabilized by lime or cement; and a surface of flexible material (asphalt), rigid material (concrete), or gravel with a binder. The ratings are based on the soil properties that affect the ease of excavation and grading and the traffic-supporting capacity. The properties that affect the ease of excavation and grading are depth to bedrock or a cemented pan, hardness of bedrock or a cemented pan, depth to a water table, ponding, flooding, the amount of large stones, and slope. The properties that affect the traffic-supporting capacity are soil strength (as inferred from the AASHTO group index number), subsidence, linear extensibility (shrink-swell potential), the potential for frost action, depth to a water table, and ponding.

Shallow excavations are trenches or holes dug to a maximum depth of 5 or 6 feet for graves, utility lines, open ditches, or other purposes. The ratings are based on the soil properties that influence the ease of digging and the resistance to sloughing. Depth to bedrock or a cemented pan, hardness of bedrock or a cemented pan, the amount of large stones, and dense layers influence the ease of digging, filling, and compacting. Depth to the seasonal high water table, flooding, and ponding may restrict the period when excavations can be made. Slope influences the ease of using machinery. Soil texture, depth to the water table, and linear extensibility (shrink-swell potential) influence the resistance to sloughing.

Lawns and landscaping require soils on which turf and ornamental trees and shrubs can be established and maintained. Irrigation is not considered in the ratings. The ratings are based on the soil properties that affect plant growth and trafficability after vegetation is established. The properties that affect plant growth are reaction; depth to a water table; ponding; depth to bedrock or a cemented pan; the available water capacity in the upper 40 inches; the content of salts, sodium, or calcium carbonate; and sulfidic materials. The properties that affect trafficability are flooding, depth to a water table, ponding, slope, stoniness, and the amount of sand, clay, or organic matter in the surface layer.

Information in this table is intended for land use planning, for evaluating land use alternatives, and for planning site investigations prior to design and construction. The information, however, has limitations. For example, estimates and other data generally apply only to that part of the soil between the surface and a depth of 5 to 7 feet. Because of the map scale, small areas of different soils may be included within the mapped areas of a specific soil.

The information is not site specific and does not eliminate the need for onsite investigation of the soils or for testing and analysis by personnel experienced in the design and construction of engineering works.

Government ordinances and regulations that restrict certain land uses or impose specific design criteria were not considered in preparing the information in this table. Local ordinances and regulations should be considered in planning, in site selection, and in design.

Report—Roads and Streets, Shallow Excavations, and Lawns and Landscaping

[Onsite investigation may be needed to validate the interpretations in this table and to confirm the identity of the soil on a given site. The numbers in the value columns range from 0.01 to 1.00. The larger the value, the greater the potential limitation. The table shows only the top five limitations for any given soil. The soil may have additional limitations]

Roads and Streets, Shallow Excavations, and Lawns and Landscaping– Mesa County Area, Colorado							
Map symbol and soil name	Pct. of map unit	Local roads and streets		Shallow excavations		Lawns and landscaping	
		Rating class and limiting features	Value	Rating class and limiting features	Value	Rating class and limiting features	Value
52—Badlands-Deaver-Chipeta complex, 25 to 99 percent slopes, extremely stony							
Badland	35	Not rated		Not rated		Very limited	
						Depth to bedrock	1.00
						Slope	1.00
						Droughty	1.00
						Salinity	0.50
Deaver	30	Very limited		Very limited		Very limited	
		Slope	1.00	Slope	1.00	Slope	1.00
		Shrink-swell	1.00	Depth to soft bedrock	0.46	Large stones content	1.00
				Too clayey	0.13	Depth to bedrock	0.46
				Cutbanks cave	0.10		
Chipeta	25	Very limited		Very limited		Very limited	
		Slope	1.00	Depth to soft bedrock	1.00	Slope	1.00
		Depth to soft bedrock	1.00	Slope	1.00	Depth to bedrock	1.00
		Shrink-swell	1.00	Cutbanks cave	0.10	Too clayey	1.00
		Low strength	1.00			Droughty	0.92
						Gravel content	0.33
118—Leebench, warm-Avalon-Blackston complex, 1 to 12 percent slopes, stony							
Leebench, warm	45	Not limited		Very limited		Not limited	
				Cutbanks cave	1.00		
Avalon	35	Not limited		Somewhat limited		Not limited	
				Cutbanks cave	0.10		
Blackston	15	Somewhat limited		Very limited		Very limited	
		Slope	0.01	Cutbanks cave	1.00	Gravel content	1.00
				Slope	0.01	Droughty	0.92
						Slope	0.01

Data Source Information

Soil Survey Area: Mesa County Area, Colorado
Survey Area Data: Version 3, Sep 25, 2007

Soil Features

This table gives estimates of various soil features. The estimates are used in land use planning that involves engineering considerations.

A *restrictive layer* is a nearly continuous layer that has one or more physical, chemical, or thermal properties that significantly impede the movement of water and air through the soil or that restrict roots or otherwise provide an unfavorable root environment. Examples are bedrock, cemented layers, dense layers, and frozen layers. The table indicates the hardness and thickness of the restrictive layer, both of which significantly affect the ease of excavation. *Depth to top* is the vertical distance from the soil surface to the upper boundary of the restrictive layer.

Subsidence is the settlement of organic soils or of saturated mineral soils of very low density. Subsidence generally results from either desiccation and shrinkage, or oxidation of organic material, or both, following drainage. Subsidence takes place gradually, usually over a period of several years. The table shows the expected initial subsidence, which usually is a result of drainage, and total subsidence, which results from a combination of factors.

Potential for frost action is the likelihood of upward or lateral expansion of the soil caused by the formation of segregated ice lenses (frost heave) and the subsequent collapse of the soil and loss of strength on thawing. Frost action occurs when moisture moves into the freezing zone of the soil. Temperature, texture, density, saturated hydraulic conductivity (Ksat), content of organic matter, and depth to the water table are the most important factors considered in evaluating the potential for frost action. It is assumed that the soil is not insulated by vegetation or snow and is not artificially drained. Silty and highly structured, clayey soils that have a high water table in winter are the most susceptible to frost action. Well drained, very gravelly, or very sandy soils are the least susceptible. Frost heave and low soil strength during thawing cause damage to pavements and other rigid structures.

Risk of corrosion pertains to potential soil-induced electrochemical or chemical action that corrodes or weakens uncoated steel or concrete. The rate of corrosion of uncoated steel is related to such factors as soil moisture, particle-size distribution, acidity, and electrical conductivity of the soil. The rate of corrosion of concrete is based mainly on the sulfate and sodium content, texture, moisture content, and acidity of the soil. Special site examination and design may be needed if the combination of factors results in a severe hazard of corrosion. The steel or concrete in installations that intersect soil boundaries or soil layers is more susceptible to corrosion than the steel or concrete in installations that are entirely within one kind of soil or within one soil layer.

For uncoated steel, the risk of corrosion, expressed as *low*, *moderate*, or *high*, is based on soil drainage class, total acidity, electrical resistivity near field capacity, and electrical conductivity of the saturation extract.

For concrete, the risk of corrosion also is expressed as *low*, *moderate*, or *high.* It is based on soil texture, acidity, and amount of sulfates in the saturation extract.

Report—Soil Features

Soil Features– Mesa County Area, Colorado									
Map symbol and soil name	Restrictive Layer				Subsidence		Potential for frost action	Risk of corrosion	
	Kind	Depth to top	Thickness	Hardness	Initial	Total		Uncoated steel	Concrete
		In	*In*		*In*	*In*			
52—Badlands-Deaver-Chipeta complex, 25 to 99 percent slopes, extremely stony									
Badland	Paralithic bedrock	0-4	—	Moderately cemented	—	—	Low	High	Low
Deaver	Paralithic bedrock	20-40	—	Weakly cemented	0	—	Low	High	High
Chipeta	Paralithic bedrock	5-20	—	Weakly cemented	0	—	Low	High	High
118—Leebench, warm-Avalon-Blackston complex, 1 to 12 percent slopes, stony									
Leebench, warm		—	—		0	—	Low	High	Moderate
Avalon		—	—		0	—	Low	Moderate	Low
Blackston		—	—		0	—	Low	High	High

Data Source Information

Soil Survey Area: Mesa County Area, Colorado
Survey Area Data: Version 3, Sep 25, 2007

APPENDIX B
Typed Boring Logs

Huddleston-Berry Engineering & Testing, LLC
640 White Avenue, Unit B
Grand Junction, CO 81501
970-255-8005
970-255-6818

BORING NUMBER B-1

CLIENT River City Consultants
PROJECT NAME Regional Law Enforcement Training Facility
PROJECT NUMBER 00456-0009
PROJECT LOCATION Mesa County, CO
DATE STARTED 2/1/12 COMPLETED 2/1/12
GROUND ELEVATION ____ HOLE SIZE 4"
DRILLING CONTRACTOR S. McKracken
GROUND WATER LEVELS:
DRILLING METHOD Simco 2000 Truck Rig
AT TIME OF DRILLING dry
LOGGED BY AS CHECKED BY MAB
AT END OF DRILLING dry
NOTES
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX	FINES CONTENT (%)
0.0		Lean CLAY with Organics (TOPSOIL), tan, dry										
		Lean CLAY (cl), tan to white, dry, very stiff, abundant sulfates										
2.5			MC 1	89	10-10-11 (21)		78	18				
5.0												
7.5		Silty GRAVEL with SAND (gm), tan, dry to moist, very dense	SS 1	33	11-22-31 (53)							
10.0												
12.5		Sandy GRAVEL and COBBLES (gw), tan, dry, very dense										
		Bottom of hole at 13.0 feet.										

Huddleston-Berry Engineering & Testing, LLC
640 White Avenue, Unit B
Grand Junction, CO 81501
970-255-8005
970-255-6818

BORING NUMBER B-2

CLIENT River City Consultants
PROJECT NAME Regional Law Enforcement Training Facility
PROJECT NUMBER 00456-0009
PROJECT LOCATION Mesa County, CO
DATE STARTED 2/1/12 COMPLETED 2/1/12
GROUND ELEVATION ___ HOLE SIZE 4"
DRILLING CONTRACTOR S. McKracken
GROUND WATER LEVELS:
DRILLING METHOD Simco 2000 Truck Rig
AT TIME OF DRILLING dry
LOGGED BY AS CHECKED BY MAB
AT END OF DRILLING dry
NOTES
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX	FINES CONTENT (%)
0.0		Lean CLAY with Organics (TOPSOIL), tan, dry										
		Lean CLAY (cl), tan, dry, stiff, abundant sulfates										
2.5												
5.0		Silty GRAVEL with Sand (gm), tan, dry to moist, very dense	SS 1	72	9-25-32 (57)							
7.5												
10.0		Sandy GRAVEL and COBBLES (gw), tan, moist, very dense	SS 2	80	50/5"							
		Bottom of hole at 10.4 feet.										

Huddleston-Berry Engineering & Testing, LLC
640 White Avenue, Unit B
Grand Junction, CO 81501
970-255-8005
970-255-6818

BORING NUMBER B-3

CLIENT River City Consultants
PROJECT NAME Regional Law Enforcement Training Facility
PROJECT NUMBER 00456-0009
PROJECT LOCATION Mesa County, CO
DATE STARTED 2/1/12 COMPLETED 2/1/12
GROUND ELEVATION ___ HOLE SIZE 4"
DRILLING CONTRACTOR S. McKracken
GROUND WATER LEVELS:
DRILLING METHOD Simco 2000 Truck Rig
AT TIME OF DRILLING dry
LOGGED BY AS CHECKED BY MAB
AT END OF DRILLING dry
NOTES
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX	FINES CONTENT (%)
0.0		Lean CLAY with Organics (TOPSOIL), tan, dry										
		Lean CLAY (cl), trace gravel, tan, dry, stiff, abundant sulfates										
2.5		Silty GRAVEL with Sand (GM), tan, dry to moist, dense										
5.0		SS1: Lab Classified	SS 1	75	11-14			6	23	22	1	23
7.5		Sandy GRAVEL and COBBLES (gw), tan, dry, very dense										
10.0		Bottom of hole at 10.0 feet.										

Huddleston-Berry Engineering & Testing, LLC
640 White Avenue, Unit B
Grand Junction, CO 81501
970-255-8005
970-255-6818

BORING NUMBER B-4

CLIENT River City Consultants
PROJECT NAME Regional Law Enforcement Training Facility
PROJECT NUMBER 00456-0009
PROJECT LOCATION Mesa County, CO
DATE STARTED 2/1/12 COMPLETED 2/1/12
GROUND ELEVATION ____ HOLE SIZE 4"
DRILLING CONTRACTOR S. McKracken
GROUND WATER LEVELS:
DRILLING METHOD Simco 2000 Truck Rig
AT TIME OF DRILLING dry
LOGGED BY AS CHECKED BY MAB
AT END OF DRILLING dry
NOTES
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX	FINES CONTENT (%)
0.0		Lean CLAY with Organics (TOPSOIL), tan, dry										
		Lean CLAY (CL), tan, white, reddish brown, dry to moist, stiff to very stiff, abundant sulfates										
2.5			MC 1	94	9-11-13 (24)							
		GB1: Lab Classified	GB 1					9	38	17	21	89
5.0												
7.5												
		Sandy GRAVEL and COBBLES (gw), tan, moist, very dense										
10.0			SS 1	50	25-50							
		Bottom of hole at 11.0 feet.										

GEOTECH BH COLUMNS 00456-0009 REGIONAL LAW ENFORCEMENT TRAINING FAC.GPJ GINT US LAB.GDT 2/29/12

Huddleston-Berry Engineering & Testing, LLC
640 White Avenue, Unit B
Grand Junction, CO 81501
970-255-8005
970-255-6818

BORING NUMBER B-5

CLIENT River City Consultants
PROJECT NAME Regional Law Enforcement Training Facility
PROJECT NUMBER 00456-0009
PROJECT LOCATION Mesa County, CO
DATE STARTED 2/1/12 COMPLETED 2/1/12
GROUND ELEVATION ____ HOLE SIZE 4"
DRILLING CONTRACTOR S. McKracken
GROUND WATER LEVELS:
DRILLING METHOD Simco 2000 Truck Rig
AT TIME OF DRILLING dry
LOGGED BY AS CHECKED BY MAB
AT END OF DRILLING dry
NOTES
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX	FINES CONTENT (%)
0.0		Lean CLAY with Organics (TOPSOIL), tan, dry										
		Lean CLAY (cl), tan, dry, stiff, abundant sulfates										
2.5		Sandy GRAVEL and COBBLES (gw), tan, dry to moist, dense to very dense										
5.0			SS 1	50	18-21							
7.5												
10.0												
12.5		***Auger Refusal*** Bottom of hole at 12.5 feet.										

GEOTECH BH COLUMNS 00456-0009 REGIONAL LAW ENFORCEMENT TRAINING FAC.GPJ GINT US LAB.GDT 2/29/12

Huddleston-Berry Engineering & Testing, LLC
640 White Avenue, Unit B
Grand Junction, CO 81501
970-255-8005
970-255-6818

BORING NUMBER B-6

CLIENT River City Consultants
PROJECT NAME Regional Law Enforcement Training Facility
PROJECT NUMBER 00456-0009
PROJECT LOCATION Mesa County, CO
DATE STARTED 2/1/12 COMPLETED 2/1/12
GROUND ELEVATION ____ HOLE SIZE 4"
DRILLING CONTRACTOR S. McKracken
GROUND WATER LEVELS:
DRILLING METHOD Simco 2000 Truck Rig
AT TIME OF DRILLING dry
LOGGED BY AS CHECKED BY MAB
AT END OF DRILLING dry
NOTES
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX	FINES CONTENT (%)
0.0		Lean CLAY with Organics (TOPSOIL), tan, dry										
		Lean CLAY (cl), trace gravel, tan, light red, reddish brown and brown, dry to moist, stiff to hard, abundant sulfates										
2.5			SS 1	83	9-12-14 (26)							
5.0												
7.5			SS 2	78	14-15-16 (31)							
		Sandy GRAVEL and COBBLES (gw), gray, dry, very dense, strongly cemented										
10.0		***Auger Refusal*** Bottom of hole at 11.0 feet.										

GEOTECH BH COLUMNS 00456-0009 REGIONAL LAW ENFORCEMENT TRAINING FAC.GPJ GINT US LAB.GDT 2/29/12

Huddleston-Berry Engineering & Testing, LLC
640 White Avenue, Unit B
Grand Junction, CO 81501
970-255-8005
970-255-6818

BORING NUMBER B-7

CLIENT River City Consultants
PROJECT NAME Regional Law Enforcement Training Facility
PROJECT NUMBER 00456-0009
PROJECT LOCATION Mesa County, CO
DATE STARTED 2/1/12 COMPLETED 2/1/12
GROUND ELEVATION ____ HOLE SIZE 4"
DRILLING CONTRACTOR S. McKracken
GROUND WATER LEVELS:
DRILLING METHOD Simco 2000 Truck Rig
AT TIME OF DRILLING dry
LOGGED BY AS CHECKED BY MAB
AT END OF DRILLING dry
NOTES
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX	FINES CONTENT (%)
0		Lean CLAY with Organics (TOPSOIL), tan, dry										
1		Silty GRAVEL with Sand (gm), tan to white, dry to moist, dense, abundant sulfates										
2			SS 1	67	13-14-20 (34)							
3												
4		Sandy GRAVEL and COBBLES (gw), gray, dry, very dense, strongly cemented ***Auger Refusal***										
		Bottom of hole at 4.0 feet.										

Huddleston-Berry Engineering & Testing, LLC
640 White Avenue, Unit B
Grand Junction, CO 81501
970-255-8005
970-255-6818

BORING NUMBER B-8

CLIENT River City Consultants
PROJECT NAME Regional Law Enforcement Training Facility
PROJECT NUMBER 00456-0009
PROJECT LOCATION Mesa County, CO
DATE STARTED 2/1/12 COMPLETED 2/1/12
GROUND ELEVATION ___ HOLE SIZE 4"
DRILLING CONTRACTOR S. McKracken
GROUND WATER LEVELS:
DRILLING METHOD Simco 2000 Truck Rig
AT TIME OF DRILLING dry
LOGGED BY AS CHECKED BY MAB
AT END OF DRILLING dry
NOTES
AFTER DRILLING ---

DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (tsf)	DRY UNIT WT. (pcf)	MOISTURE CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX	FINES CONTENT (%)
0.0		Lean CLAY with Organics (TOPSOIL), tan, dry										
		Lean CLAY (cl), tan, dry, stiff, abundant sulfates										
2.5		Silty GRAVEL with Sand (gm), tan to reddish brown, dry to moist, medium dense, abundant sulfates	MC 1	89	6-5-5 (10)							
			GB 1									
5.0												
7.5		Sandy GRAVEL and COBBLES (gw), tan, dry, dense to very dense	SS 1	89	20-18-22 (40)							
		Auger Refusal Bottom of hole at 9.5 feet.										

APPENDIX C
Laboratory Testing Results

Huddleston-Berry Engineering & Testing, LLC
640 White Avenue, Unit B
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970-255-8005
970-255-6818

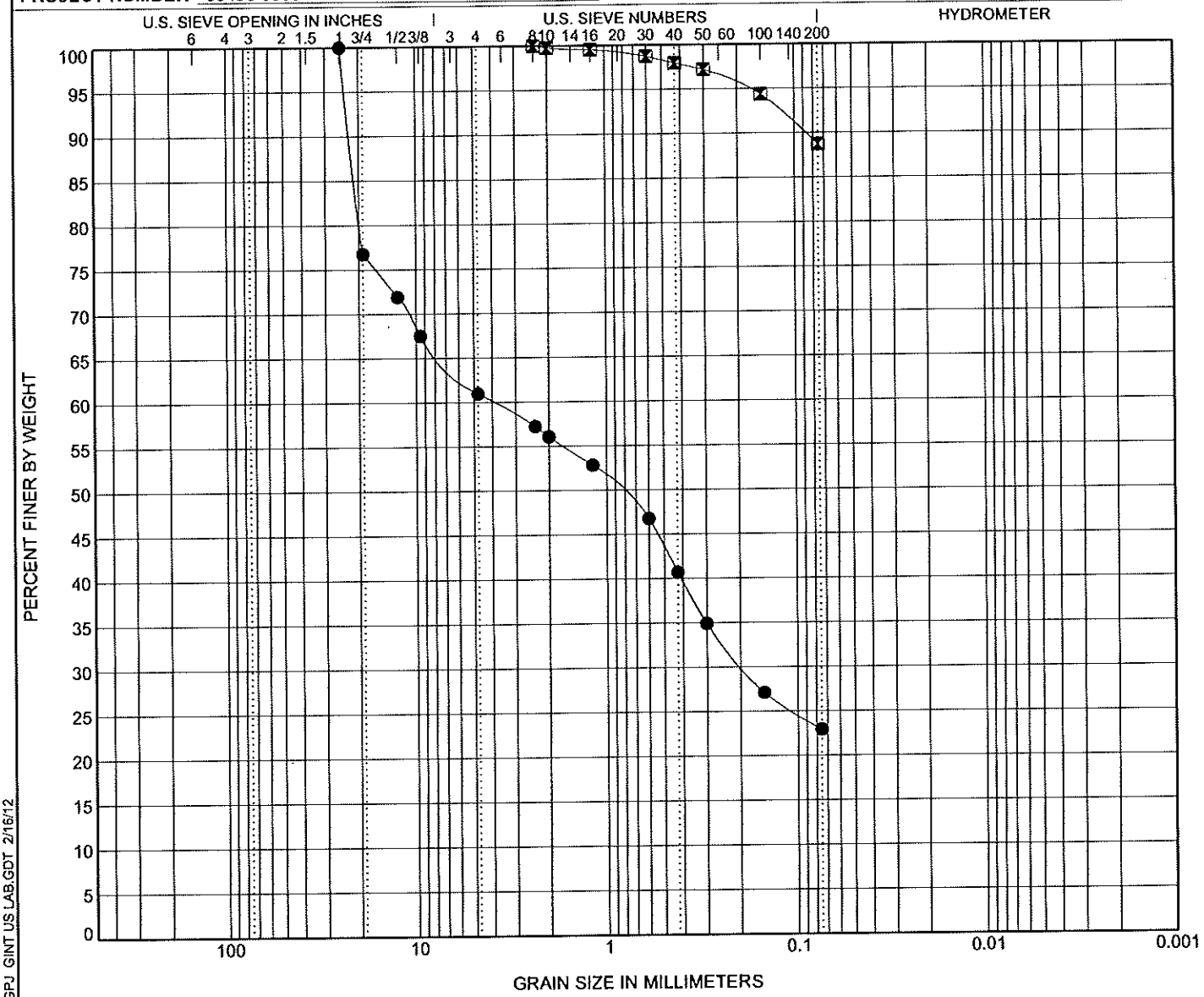
GRAIN SIZE DISTRIBUTION

CLIENT River City Consultants

PROJECT NAME Regional Law Enforcement Training Facility

PROJECT NUMBER 00456-0009

PROJECT LOCATION Mesa County, CO



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

Specimen Identification	Classification	LL	PL	PI	Cc	Cu
● B-3 SS-1 2/2012	SILTY GRAVEL with SAND(GM)	23	22	1		
⊠ B-4 GB-1 2/2012	LEAN CLAY(CL)	38	17	21		

Specimen Identification	D100	D60	D30	D10	%Gravel	%Sand	%Silt	%Clay
● B-3 SS-1 2/2012	25	3.953	0.194		39.0	38.0	22.9	
⊠ B-4 GB-1 2/2012	2.36				0.0	11.3	88.7	

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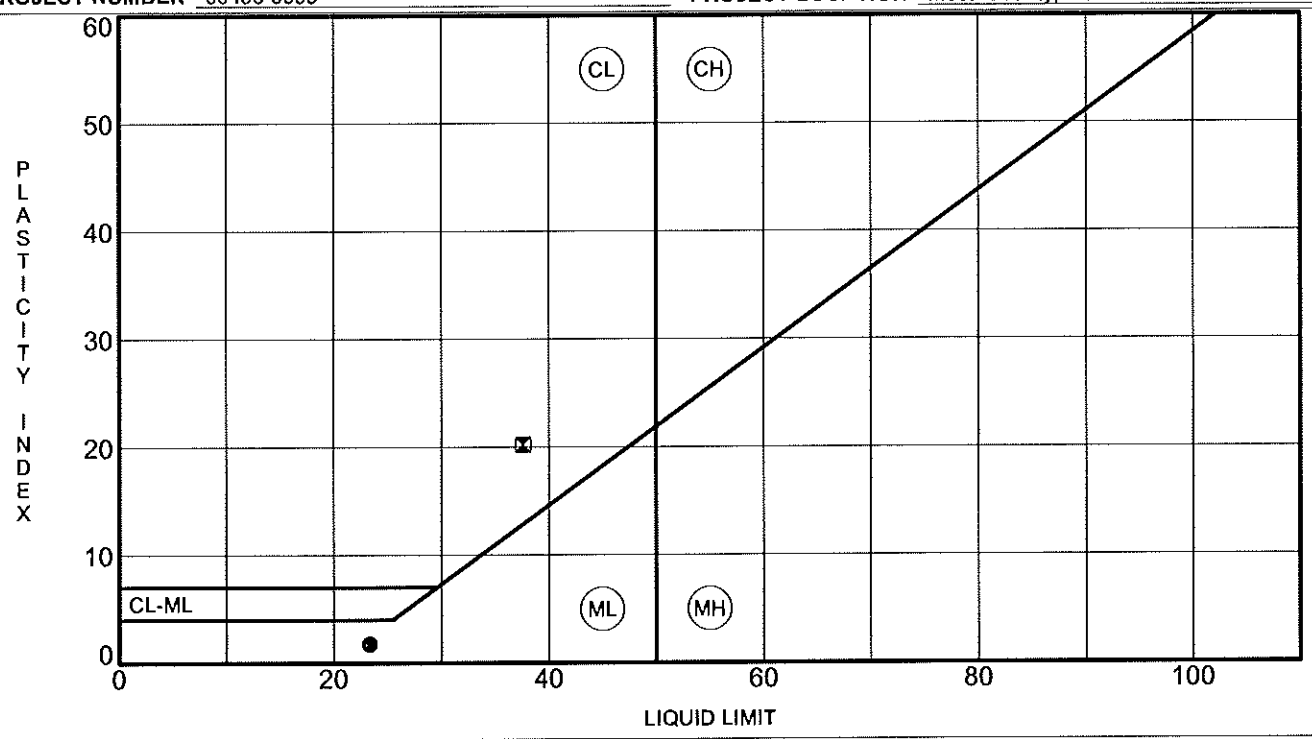
970-255-6818

ATTERBERG LIMITS' RESULTS

CLIENT River City Consultants **PROJECT NAME** Regional Law Enforcement Training Facility

PROJECT NUMBER 00456-0009 **PROJECT LOCATION** Mesa County, CO



	Specimen Identification		LL	PL	PI	#200	Classification
●	B-3 SS-1	2/2012	23	22	1	23	SILTY GRAVEL with SAND(GM)
⊠	B-4 GB-1	2/2012	38	17	21	89	LEAN CLAY(CL)

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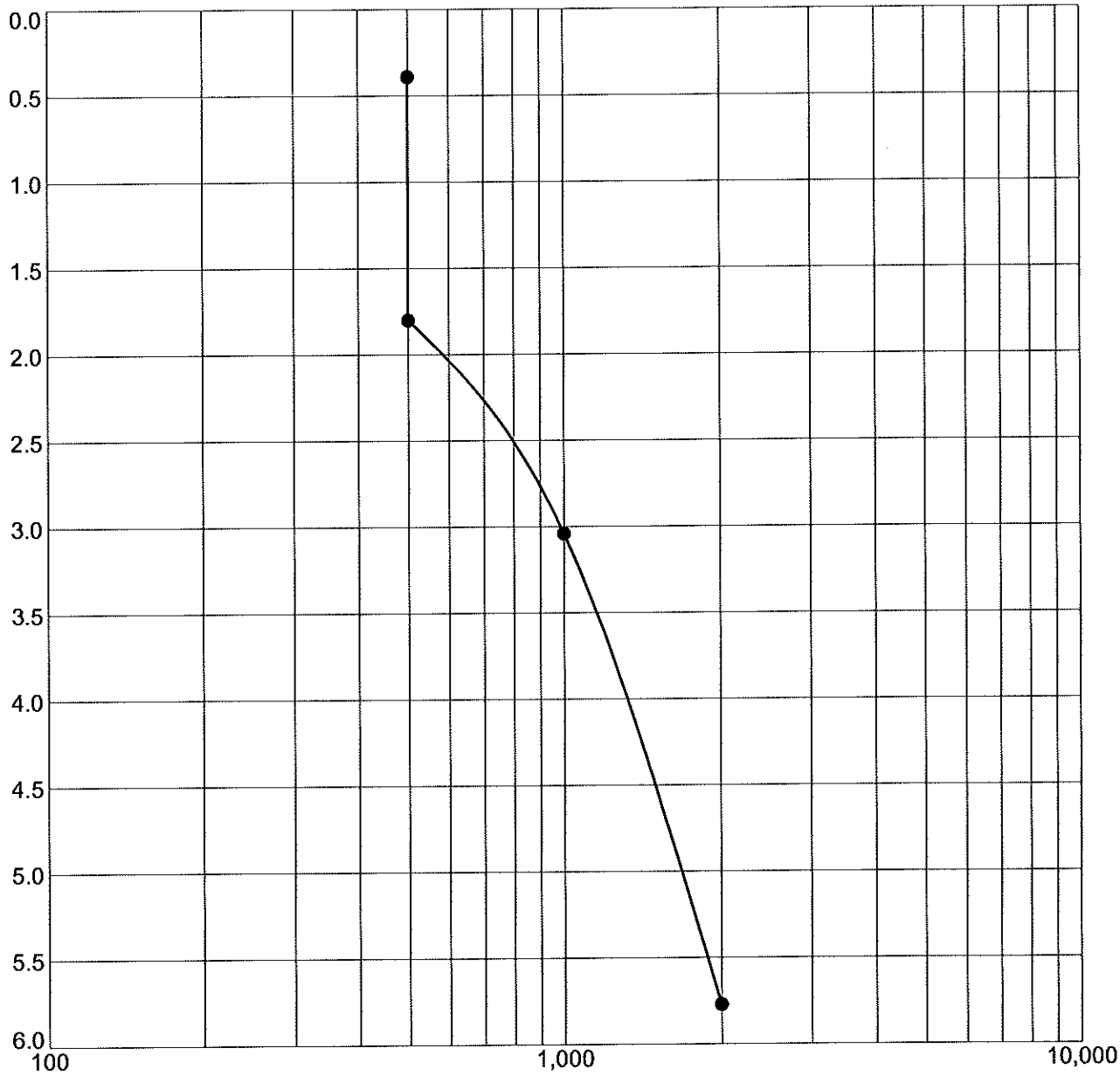
CONSOLIDATION TEST

CLIENT River City Consultants
PROJECT NAME Regional Law Enforcement Training Facility
PROJECT NUMBER 00456-0009
PROJECT LOCATION Mesa County, CO



Specimen Identification		Classification	γ_d	MC%
● B-1	2.0		78	18

CONSOL STRAIN 00456-0009 REGIONAL LAW ENFORCEMENT TRAINING FAC.GPJ GINT US LAB.GDT 2/16/12

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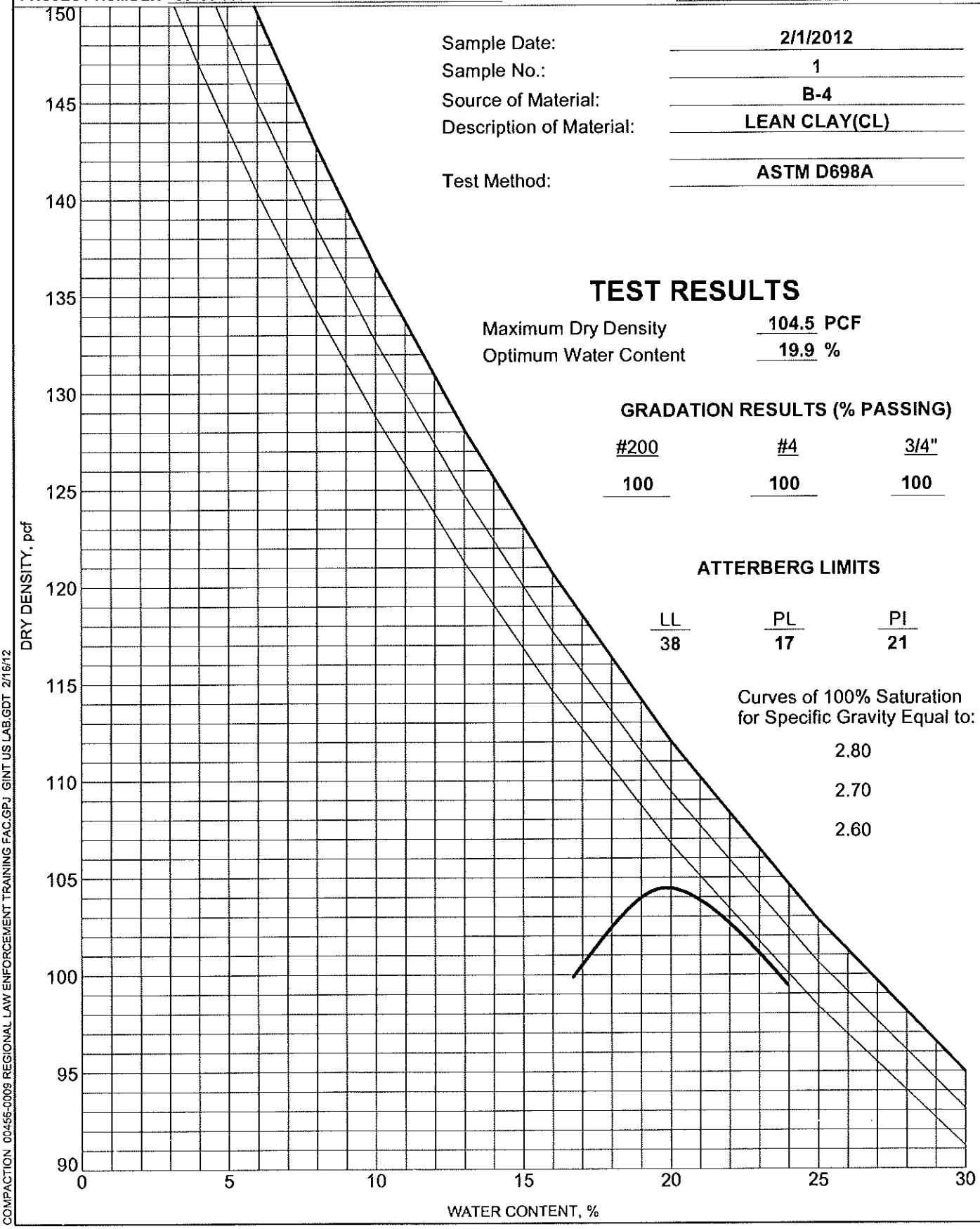
970-255-6818

MOISTURE-DENSITY RELATIONSHIP

CLIENT River City Consultants
PROJECT NAME Regional Law Enforcement Training Facility
PROJECT NUMBER 00456-0009
PROJECT LOCATION Mesa County, CO



Sample Date:	2/1/2012
Sample No.:	1
Source of Material:	B-4
Description of Material:	LEAN CLAY(CL)
Test Method:	ASTM D698A

TEST RESULTS

Maximum Dry Density	104.5 PCF
Optimum Water Content	19.9 %

GRADATION RESULTS (% PASSING)

#200	#4	3/4"
100	100	100

ATTERBERG LIMITS

LL	PL	PI
38	17	21

Curves of 100% Saturation for Specific Gravity Equal to:

2.80

2.70

2.60

Huddleston-Berry
Engineering & Testing, LLC

CALIFORNIA BEARING RATIO
ASTM D1883

Project No.:	00456-0009		**Authorized By:**	Client	**Date:** 02/01/12
Project Name:	Regional Law Enforcement Training Facility		**Sampled By:**	AS	**Date:** 02/01/12
Client Name:	River City Consultants		**Submitted By:**	AS	**Date:** 02/01/12
Sample Number:	12-0065	**Location:** B-4, GB1	**Reviewed By:**	MAB	**Date:** 02/29/12

Compaction Method ASTM D698, Method A

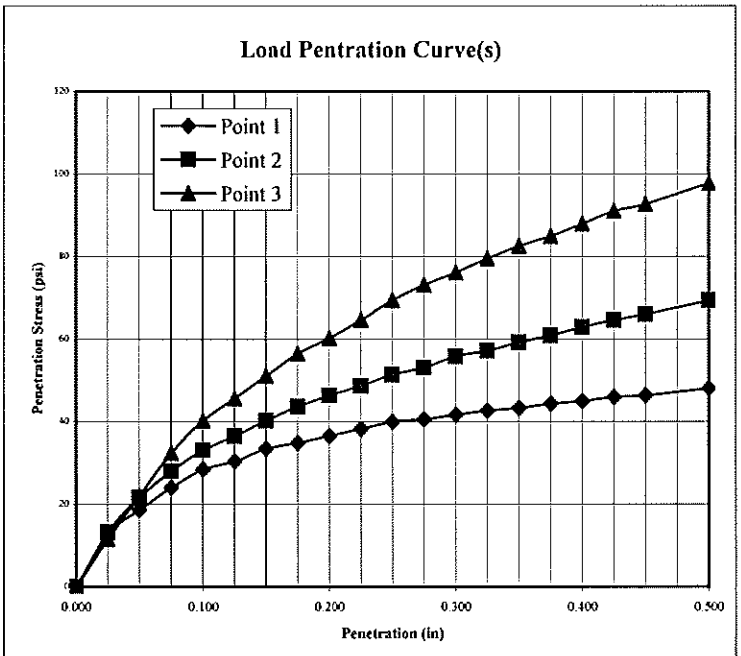
Maximum Dry Density (pcf): 104.5

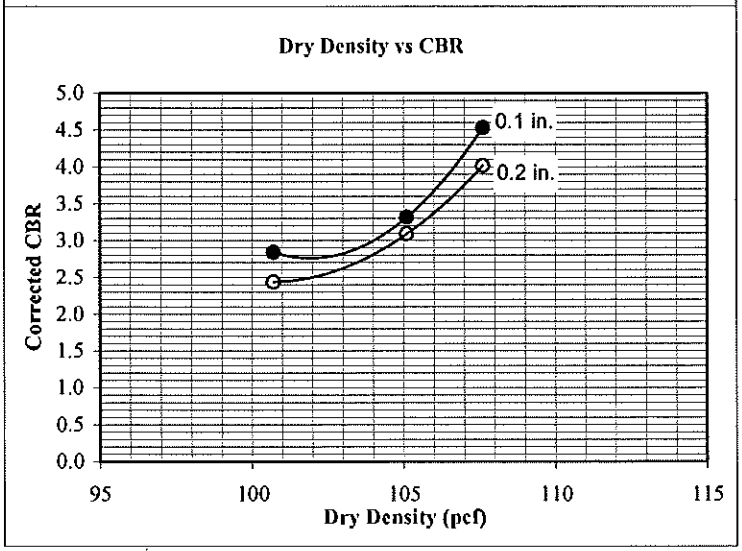
Opt. Moisture Content (%): 19.9

Sample Condition: Soaked

Remarks:

		Sample Data		
		Point 1	Point 2	Point 3
Blows per Compacted Lift:		15	25	56
Surcharge Weight (lbs):		10.0	10.0	10.0
Dry Density Before Soak (pcf):		100.7	105.1	107.6
Dry Density After Soak (pcf):		99.3	104.1	106.2
Moisture Content (%)	Bottom Pre-Test	18.4	18.7	20.4
	Top Pre-Test	18.7	18.1	19.5
	Top 1" After Test	26.1	24.6	23.2
	Average After Soak:	24.8	22.7	21.0
Percent Swell After Soak:		1.4	1.0	1.3





Penetration Data								
Point 1			Point 2			Point 3		
Dist. (in)	Load (lbs)	Stress (psi)	Dist. (in)	Load (lbs)	Stress (psi)	Dist. (in)	Load (lbs)	Stress (psi)
0.000	0	0	0.000	0	0	0.000	0	0
0.025	38	13	0.025	39	13	0.025	34	12
0.050	55	19	0.050	64	22	0.050	65	22
0.075	71	24	0.075	83	28	0.075	96	32
0.100	84	28	0.100	98	33	0.100	119	40
0.125	90	30	0.125	108	37	0.125	135	46
0.150	99	33	0.150	119	40	0.150	151	51
0.175	103	35	0.175	129	44	0.175	167	56
0.200	108	37	0.200	137	46	0.200	178	60
0.225	113	38	0.225	144	49	0.225	191	65
0.250	118	40	0.250	152	51	0.250	205	69
0.275	120	41	0.275	157	53	0.275	216	73
0.300	123	42	0.300	165	56	0.300	225	76
0.325	126	43	0.325	169	57	0.325	235	80
0.350	128	43	0.350	175	59	0.350	244	83
0.375	131	44	0.375	180	61	0.375	251	85
0.400	133	45	0.400	186	63	0.400	260	88
0.425	136	46	0.425	191	65	0.425	269	91
0.450	137	46	0.450	195	66	0.450	274	93
0.500	142	48	0.500	205	69	0.500	289	98

Corrected CBR @ 0.1"		
2.8	3.3	4.5
Corrected CBR @ 0.2"		
2.4	3.1	4.0

Penetration Distance Correction (in)		
0.000	0.000	0.000

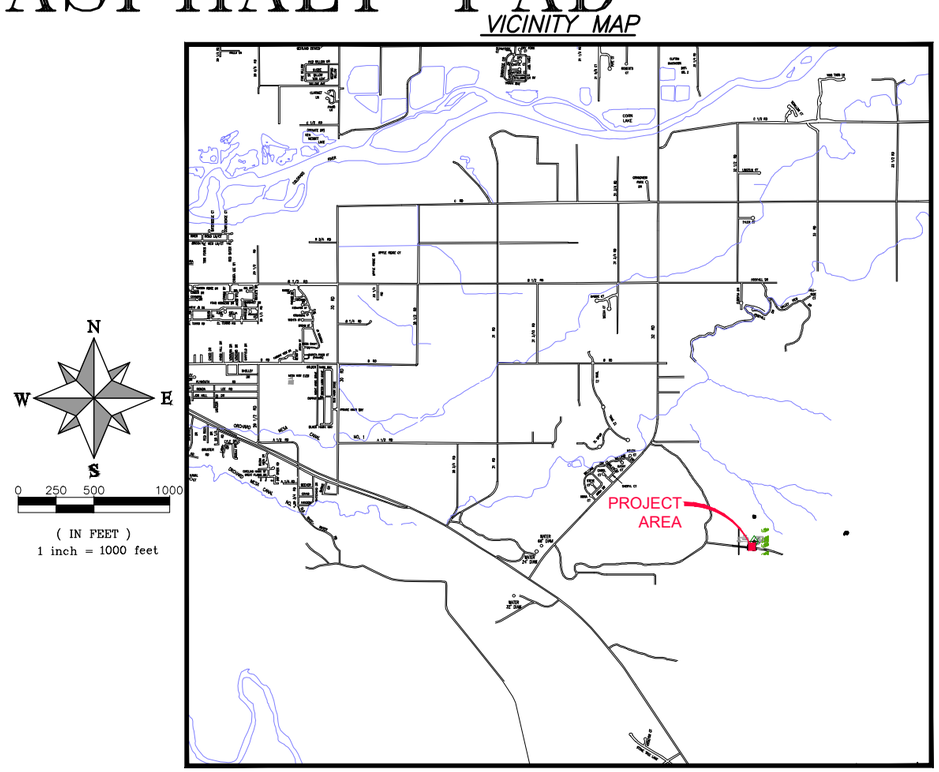
Figure: ____

APPENDIX C
Construction Drawings

PROJECT NO. _____

GRAND JUNCTION FIRE DEPT.
FIRE TRAINING FACILITY
SHED FOUNDATION & ASPHALT PAD

VICINITY MAP



UTILITIES AND AGENCIES								
AGENCY	NAME	POSITION	ROLE	MAILING ADDRESS	STREET ADDRESS	CITY, STATE	VOICE–WK	FAX
CITY OF GRAND JUNCTION	KIRSTEN ARMBRUSTER	PROJECT ENGINEER	PROJECT ENGINEER	333 WEST AVE BLDG C	333 WEST AVE BLDG C	GRAND JCT., CO 81501	(970) 244–1421	(970) 256–4022
CITY OF GRAND JUNCTION	GUS HENDRICKS	DEPUTY FIRE CHIEF	PROJECT MANAGER	625 UTE AVE	625 UTE AVE	GRAND JCT., CO 81501	(970) 549–5802	
GRAND JUNCTION MODELERS	LEE SIMCOX	--	--	--	--	--	(970)216–8073	
GRAND VALLEY POWER	PERRY RUPP	FIELD ENGINEER	ELECTRIC	PO BOX 190	845 22 ROAD	GRAND JCT., CO 81502	(970) 244–0400	(970) 244–0612
CENTURYLINK	CHRIS JOHNSON	ENGINEER	TELEPHONE	2524 BLICHMANN AVE	2524 BLICHMANN AVE	GRAND JCT., CO 81504	(970) 244–4311	(970) 240–4349
CLIFTON WATER	DAVE REINERTSON	SYSTEM SUPERVISOR	WATER	510 34 ROAD	510 34 ROAD	GRAND JCT., CO 81502	(970) 434–7328	
MESA COUNTY	GREG LINZA	PARKS & LANDSCAPE MANAGER	MANAGER	200 S SPRUCE ST	200 S SPRUCE ST	GRAND JCT., CO 81501	(970) 244–3232	(970) 244–3240

CITY OF Grand Junction COLORADO

Public Works
Engineering Division

811 Know what's **below.** **Call** before you dig.

NOTE: NOTIFY AFFECTED UTILITY VENDOR 48 HOURS PRIOR TO EXCAVATIONS THAT WILL EXPOSE UTILITY LINES. THE COVER SHEET WILL HAVE A LISTING OF UTILITY VENDORS AND TELEPHONE NUMBERS.

	DESCRIPTION	DATE
REVISION 1	XXX	201X
REVISION 2	XXX	201X
REVISION 3	XXX	201X
REVISION 4	XXX	201X

DRAWING STATUS:
- PROGRESS
- FINAL CONSTRUCTION DRAWINGS
- ASBUILT

DESIGNED BY:
KIRSTEN ARMBRUSTER, PROJECT ENGINEER FEBRUARY 25, 2019

REVIEWED BY:
TRENT C. PRALL, PUBLIC WORKS DIRECTOR 201X

AUTHORIZED FOR CONSTRUCTION
TRENT C. PRALL, PUBLIC WORKS DIRECTOR 201X

ACCEPTED AS CONSTRUCTED
KIRSTEN ARMBRUSTER, PROJECT ENGINEER 201X

GRAND JUNCTION FIRE DEPT. FIRE TRAINING FACILITY SHED FOUNDATION & ASPHALT PAD

PROJECT NO. _____

ABBREVIATIONS

AASHTO	AMERICAN ASSOCIATION OF STATE HIGHWAY & TRANSPORTATION OFFICIALS
ABC	AGGREGATE BASE COURSE
AC	ASBESTOS CEMENT
AP	ANGLE POINT
ASB	ANCHORED STRAW BALES
ASP	ALUMINIZED STEEL PIPE
ASTM	AMERICAN SOCIETY FOR TESTING MATERIALS
AWWA	AMERICAN WATER WORKS ASSOCIATION
BC	BACK OF CURB
BF	BUTTERFLY VALVE
BOW	BACK OF WALK
BCR	BEGIN CURB RETURN
BOT	BOTTOM
BSWMP	BETTER STORM WATER MANAGEMENT PRACTICES
CH	CHORD
CAP	CORRUGATED ALUMINUM PIPE
CDOT	COLORADO DEPARTMENT OF TRANSPORTATION
CI	CAST IRON
C,G,& SW	CURB, GUTTER & SIDEWALK
℄	CENTER LINE
CL	CLEAR
CMP	CORRUGATED METAL PIPE
CO	CLEAN OUT
COMB	COMBINATION (AS IN STORM SEWER AND SANITARY SEWER)
CONC	CONCRETE
CSM	CITY SURVEY MONUMENT
CSP	CORRUGATED STEEL PIPE
CU	COPPER
DI	DUCTILE IRON
DWY	DRIVEWAY
E	ELECTRIC
ECR	END CURB RETURN
EG	EDGE OF GUTTER
EL	ELEVATION
EP	EDGE OF PAVEMENT
EX	EXISTING
FB	FULL BODY
FC	FACE OF CURB
FG	FINISHED GRADE
℄	FLOW LINE
FL	FLANGE
FM	FORCE MAIN
FO	FIBER OPTICS
FS	FAR SIDE
FTG	FOOTING
G	GAS
GB	GRADE BREAK
GM	GAS METER
GV	GATE VALVE
HBP	HOT BITUMINOUS PAVEMENT
HDPE	HIGH DENSITY POLYETHYLENE
INV	INVERT
IRR	IRRIGATION
L	LENGTH OF ARC
LC	LONG CHORD
LF	LINEAR FEET
LL	LONG ARC
LS	SHORT ARC
LT	LEFT
MB	MAILBOX
MCSM	MESA COUNTY SURVEY MONUMENT
MH	MANHOLE
MJ	MECHANICAL JOINT
MW	MILL WRAP
N/A	NOT APPLICABLE
NIC	NOT IN CONTRACT
NOP	NO ONE PERSON
NRCP	NON-REINFORMATIONRCED CONCRETE PIPE
NS	NEAR SIDE
NTS	NOT TO SCALE
OHP	OVERHEAD POWER
OHT	OVERHEAD TELEPHONE
PC	POINT OF CURVATURE
PCC	POINT OF COMPOUND CURVATURE
PE	POLYETHYLENE
PERF	PERFORATED
PI	POINT OF INTERSECTION
PIP	PLASTIC IRRIGATION PIPE
POC	POINT ON CURVE
POT	POINT ON TANGENT
PR	PROPOSED
PRC	POINT OF REVERSE CURVATURE
PT	POINT OF TANGENCY
PVC	POLYVINYL CHLORIDE
R	RADIUS
RCP	REINFORMATIONRCED CONCRETE PIPE
REQ'D	REQUIRED
RG	RESTRAINED GLANDS
RL	LONG RADIUS
ROW	RIGHT OF WAY
RP	RADIUS POINT
RR	RAIL ROAD
RS	SHORT RADIUS
RT	RIGHT
S	SLOPE
SAN	SANITARY
SC	SHORT CHORD
SCD	STANDARD CONTRACT DOCUMENTS
SCH	SCHEDULE
SF	SILT FENCE
SL	SECTION LINE
SSRB	STANDARD SPECIFICATIONS FOR ROAD & BRIDGE CONSTRUCTION
SSUU	STANDARD SPECIFICATIONS FOR CONSTRUCTION OF UNDERGROUND UTILITIES
STA	STATION
STL	STEEL
STM	STORM
T	TELEPHONE
TAN	LENGTH OF TANGENT
TC	TOP OF CURB
TH	TEST HOLE
TV	TELEVISION
(TYP)	TYPICAL
UU	UNDERGROUND UTILITIES
VC	VERTICAL CURVE
VCP	VITRIFIED CLAY PIPE
VPC	VERTICAL POINT OF CURVATURE
VPCC	VERTICAL POINT OF COMPOUND CURVATURE
VPRC	VERTICAL POINT OF REVERSE CURVATURE
VPI	VERTICAL POINT OF INTERSECTION
VPT	VERTICAL POINT OF TANGENCY
W	WATER
△	DELTA ANGLE

LEGEND

BSWMP DRAINAGE BASIN BOUNDARY

BSWMP ANCHORED STRAW BALES — ASB ASB ASB ASB ASB ASB ASB

BSWMP SILT FENCE — SF SF SF SF SF SF SF

BUILDING

CONCRETE CURB AND GUTTER — 2' CURB AND GUTTER

CONCRETE CURB,GUTTER, & SIDEWALK — 7' C, G, & SW

CONCRETE DITCH — CONCRETE

CONCRETE SIDEWALK — 4' SW

CULVERT — 18" RCP

EARTH DITCH — EARTH EARTH EARTH

EDGE OF GRAVEL

EDGE OF PAVEMENT

FENCE (BARBED WIRE)

FENCE (CHAIN LINK)

FENCE (IRON)

FENCE (PLASTIC)

FENCE (TEMPORARY CONSTRUCTION)

FENCE (WOOD)

FENCE (WOVEN WIRE)

GUARD RAIL — 9' 9'

HATCHING: INDICATES ASPHALT REMOVAL

HATCHING: INDICATES CONCRETE REMOVAL

HATCHING: INDICATES STAGING AREA — ~STAGING AREA~

LINE (CENTER OF IMPROVEMENTS) — CENTERLINE

LINE (CITY LIMITS) — CITY LIMITS

LINE (CONTROL) — CONTROL LINE

LINE (EASEMENT)

LINE (MONUMENT/SECTION) — MONUMENT/SECTION LINE

LINE (PROPERTY)

LINE (RIGHT OF WAY)

MATCH LINE — MATCH LINE SEE SHEET NO ?

PIPE (IRRIGATION) — 4" IRR

PIPE (SIPHON) — 4" SIPHON

PROPOSED CONCRETE CURB AND GUTTER

PROPOSED CONCRETE CURB,GUTTER,& SIDEWALK

PROPOSED CONCRETE SIDEWALK

PROPOSED "WET" UTILITIES (CONSTRUCTION NOTE WILL INDICATE TYPE, SIZE, AND MATERIAL OF NEW MAIN) — 8" PVC SANITARY SEWER

ALL PROPOSED FEATURES NOT SHOWN IN LEGEND WILL BE SHOWN THE SAME AS THEIR EXISTING COUNTERPART, BUT INDICATED BY BOLDER LINETYPE

RAIL ROAD

RETAINING WALL — 1' RETAINING WALL

STRIPING (CONTINUOUS WHITE) — WHITE

STRIPING (DASHED WHITE) — WHITE

STRIPING (CONTINUOUS YELLOW) — YELLOW

STRIPING (DASHED YELLOW) — YELLOW

TOP OF SLOPE — 4580

CONTOUR LINES (SHOWN BETWEEN TOP & TOE)

TOE OF SLOPE — 4570

TRAFFIC DETECTOR LOOP

UTILITY LINE (ABANDON) (THIS CASE A WATER LINE) — W (ABANDON) 8" W

UTILITY LINE (CABLE TV) — TV TV

UTILITY LINE (ELECTRIC) — E E

UTILITY LINE (FIBER OPTIC) — FO QWEST FO

UTILITY LINE (GAS) — G 1 1/4" MW G

UTILITY LINE (HIGH VOLTAGE OVERHEAD POWER) — HVOHP

UTILITY LINE (OVERHEAD POWER) — OHP

UTILITY LINE (OVERHEAD TELEPHONE) — OHT

UTILITY LINE (SANITARY SEWER) — 8" SAN

UTILITY LINE (SANITARY SEWER FORCE MAIN) — 8" FM

UTILITY LINE (SANITARY SEWER SERVICE) — SS SS

UTILITY LINE (STORM SEWER) — 8" STM

UTILITY LINE (STORM SEWER, PERFORATED) — 6" PERF

UTILITY LINE (STORM/SANITARY SEWER SEWER COMBINATION) — 18" COMB

UTILITY LINE (TELEPHONE) — T T

UTILITY LINE (WATER) — W 8" W

SYMBOLS

BENCH MARK — (A)

CATCH BASIN

CLEAN OUT — SSCO

CURB STOP

FIRE HYDRANT

GUY WIRE ANCHOR

HEADGATE

IRRIGATION PUMP — P

MAILBOX — MB

MANHOLE (ELECTRIC) — (E)

MANHOLE (GAS) — (G)

MANHOLE (SANITARY/STORM)

MANHOLE (TELEPHONE) — (T)

MANHOLE (TV) — (TV)

MANHOLE (WATER) — (W)

METER (GAS) — GM

METER (WATER)

PEDESTAL (TELEPHONE)

PEDESTAL (TV) — TV

PROPERTY PIN

PULL BOX

REDUCER FITTING

SIGN OR POST (SIGN TYPE NOTED) — STOP

SPRINKLER HEAD

STREET LIGHT

SURVEY MONUMENT (CITY) — CSM

SURVEY MONUMENT (TYPE NOTED) — MCSM

TEST HOLE — TH #1

TRAFFIC PAINT MARKING

TRAFFIC SIGNAL POLE AND MAST ARM

UTILITY POLE

VALVE (GAS) — GV

VALVE (IRRIGATION) — IRR

VALVE (WATER)

VEGETATION (HEDGE OR BUSH)

VEGETATION (TREE STUMP)

VEGETATION (TREE) (CALIPER SIZE NOTED) — 18" WIL

WATER HYDRANT

WEIR

YARD LIGHT

BAR SCALE:

0 5 10 20

(IN FEET)
1 inch = 20 feet

NORTH ARROW:

N W E S

DESCRIPTION	DATE
REVISION △	–
REVISION △	–
REVISION △	–
REVISION △	–

DRAWN BY JCS DATE 4-02
DESIGNED BY _____ DATE _____
CHECKED BY _____ DATE _____
APPROVED BY _____ DATE _____

SCALES: PLAN & PROFILE
HORIZONTAL: 1" =
VERTICAL: 1" =

CITY OF Grand Junction COLORADO

PUBLIC WORKS
ENGINEERING DIVISION

CITY OF GRAND JUNCTION
STANDARD ABBREVIATIONS, LEGEND, AND SYMBOLS

2

N:\Landproj\(FIRE TRAINING CENTER)\2018 Whitewater Hill Fire Training Center\dwg\E1802 Shed FNDN & Asphalt.dwg, 11x17 Legend (2), 2/28/2020 8:21:21 AM

PROJECT NO. _____

GENERAL STRUCTURAL NOTES & SPECIFICATIONS

1. GENERAL.

A. These general structural notes and specifications supplement the project written technical specifications and the project structural drawings.
B. The Contractor is responsible for all construction bracing, temporary shoring, and other site safety controls required during construction in accordance with all applicable Local, State and Federal regulations, to insure the stability and safety of all construction until it is completed and self-supporting.
C. The Contractor is responsible for all water, both above and below ground, runoff and other environmental controls required during construction to insure the site is maintained in compliance with all applicable Local, State and Federal regulations.
D. Details on these plans are intended to depict the general construction details and methods for this structure. Connection details and conditions not specifically shown that are similar in nature to those that are specified shall be assumed one and the same. If questions regarding the application of details are encountered, notify the Architect/Engineer for clarification or instruction.
E. Prior to implementing any changes to these plans, the Architect/Engineer shall be notified in writing for their written approval. Changes implemented without the Architect/Engineers written approval shall relieve the Architect/Engineer of any claim or liability resulting from that portion of the structure changed or affected by the change.
F. ENGINEERING SEAL: This certification covers the reinforced concrete slab and foundation only, and excludes those structural parts manufactured and delivered by the steel building manufacturer, such as the anchor bolts and base plates.

2. CONTRACTOR RESPONSIBILITY FOR COORDINATION

A. It is the Contractors Prime responsibility to coordinate the work shown on all of the Project Drawings, general, special and technical specifications.
B. The Contractor is responsible to verify all existing construction material types dimensions, elevations and conditions.
C. The Contractor shall verify and coordinate the dimensions among all drawings and in the field prior to proceeding with any work or fabrication, any discrepancy shall be immediately reported to the Architect/Engineer.
D. It is the Contractor's responsibility to carefully study and coordinate the construction requirements shown on both the Architectural and the Structural Drawings. When conflicts or discrepancies are found between these plan sets and/or within these drawings, the Contractor shall report them immediately to the project Architect/Engineer for direction and/or clarification.
E. Any construction work done by the Contractor before obtaining such clarification from the Project Architect/Engineer shall be at the Contractor's own risk and cost. Furthermore; any work required to correct, replace and/or restore the work as directed by the Architect/Engineer shall be at the Contractor's own risk and cost.

3. CODES.

A. International Building Code, IBC 2018 Edition.
B. Minimum Design Loads for Buildings and Other Structures, ASCE 7; current edition.
C. American Concrete Institute, ACI 318, Building Code Requirements for Structural Concrete; current edition.
D. American Concrete Institute, ACI 350, Code Requirements for Environmental Engineering Concrete Structures; current edition.
E. American Concrete Institute, ACI 530, Building Code Requirements and Specifications for Masonry Structures; current edition.
F. American Concrete Institute, ACI 301, Specifications for Structural Concrete.
G. American Institute of Steel Construction, AISC 13th Edition, Steel Construction Manual.
H. American Institute of Steel Construction, AISC 360, Specifications for Structural Steel Buildings; current edition.
I. American Institute of Steel Construction, AISC 341, Seismic Provisions for Structural Steel Buildings; current edition.
J. American Welding Society, AWS D1.1 current edition, Structural Welding Code.
K. American Iron and Steel Institute, AISI, Specifications for the Design of Cold-Formed Steel Structural Members, 1996 Edition and current addenda.
L. National Design Specifications, NDS For Wood Construction; current edition.

4. SPECIAL INSPECTIONS. Special Inspections per IBC Chapter 17 are required for the following items: C indicates Continuous, P indicates Periodic.

	Frequency
A. Soils.	
i Site preparation:	P
B. Concrete.	
i Reinforcement placement:	P
ii Verification of use of required mix:	P
iii Concrete placement:	C
C. Structural steel.	
i Fabrication of structural elements:	P
ii Material verification of structural steel:	P

iii Special Inspector shall submit a final report to the local building official detailing the results of all structural steel inspections prior to final building inspection.
D. All special inspections shall be performed by IBC certified inspectors.

5. SUBMITTALS.

A. Submit required copies, four (4) minimum, of product or material design information to the Architect/Engineer for review for the following items:
i Concrete mix designs and admixtures.
ii Non-shrink grout.

6. DEFERRED SUBMITTALS. The following items to be designed by others are considered "Deferred Submittals". Deferred submittals shall be accompanied by design drawings, shop drawings and structural calculations, stamped and signed by a Professional Structural Engineer currently registered in the State of Idaho.

A. Pre-engineered and shop fabricated wood and trusses.
B. Pre-engineered and shop fabricated metal buildings and components.

7. SHOP DRAWINGS:

A. Submit required copies of shop drawings, four (4) minimum, to the Architect/Engineer for review prior to fabrication of the following items:
i Reinforcing steel for all concrete.

8. WELDING OF STRUCTURAL STEEL.

A. All welding shall conform to the requirements of the current AWS Structural Welding Code D1.1-02
B. Weld Metal: Fexx=70 ksi, typical unless otherwise noted or required by AWS.
C. All welders shall be tested and certified by an independent testing agency.
D. Qualification of welders shall be in accordance with the Specifications for Standard Qualification Procedure of the AWS.

9. DESIGN CRITERIA.

A. OCCUPANCY OR USE; IBC Table 1607.1: MANUFACTURING (LIGHT)
B. LIVE LOADS:
i Minimum Roof Live Load: 30 psf (snow)
ii Ground Snow Load, P_g: 42.86 psf
iii Unbalanced Snow per ASCE-7, Chapter 7
C. DEAD LOADS:
i Roof Dead Load:
Per Building Manufacturers drawings and calculations.
D. WIND:
i Ultimate Wind Speed: 105 mph
ii Basic Wind Speed: 70 mph
ii Site Exposure: C
ii Importance Factor, I_w: 1.0

B. SEISMIC:
i Earthquake Spectral Response Acceleration:
(a) Short Period, S_s: 23%
(b) 1-Second, S_1: 6%
ii Spectral Response Coefficients:
(a) Short Period, S_{DS}: .247g
(b) 1-Second, S_{D1}: .093g
iii Importance factor, I_e: 1.0
iv Soil Class: D
v Seismic Use Group: II
vi Seismic Design Category: B

C. MECHANICAL LOADS: Refer to framing plans and mechanical plans for special mechanical equipment loads.

10. FOUNDATIONS.

A. All footings to be placed on firm undisturbed, inorganic material. Proof roll sub-grade prior to placing concrete where the material has been disturbed by the excavating equipment.
B. All piers and footings outside or at the perimeter of the structure, or in other unheated areas shall be set to a depth of at least 24" below finish grade, unless other wise noted on the plans.
C. All foundations and retaining walls below finish grade shall receive an approved damp-proof coating
D. Allowable bearing pressure for all footings Qa = 1,500 psf
E. Local areas of soft and/or unacceptable material encountered at bottom of footing elevations indicated on the plans must be over-excavated and brought up to design grade with compacted "structural fill" or "lean concrete fill".
F. All structural fill and/or backfill shall be granular, free draining, material; Unified Soils Classification GW, GP, GM or SW; maximum aggregate size of 3-in. and no more than 7% passing a number 200 sieve. Material shall be placed in lifts no greater than 6-in. in depth and compacted to 95% of maximum density as determined per ASTM D1557.
G. Design for the mitigation of subsurface water flow and/or perched water tables shall be the responsibility of others.
H. The Engineer shall be notified in writing if any ground water, clay type soils, debris or unconsolidated materials are encountered during excavations for foundations.

11. STRUCTURAL MATERIALS.

A. STRUCTURAL STEEL:
i PLATES, BARS, CHANNELS & ANGLES: ASTM A36, Fy=36 ksi.
ii SQUARE, RECTANGULAR HSS, STEEL TUBING: ASTM A500 Grade B, Fy=46 ksi.

B. ANCHOR RODS: Anchor Rods (bolts set into concrete) shall be ASTM F1554, Fy=105 ksi Heavy Hex Anchor Stud with threads. Embedment and Diameter for the studs shall be as called out in the details Nuts for anchor rods shall conform to ASTM A563, Grade A, Heavy Hex.
C. WASHERS: All washers shall conform to ASTM F436.
D. PROJECT CONCRETE MIX TYPES: Concrete shall be proportioned and furnished for the various project uses as indicated on the plans and as follows:
i M4500-fnd: Standard exterior concrete mix for building footings/foundation Walls: fc = 4,500 psi, Absolute water-cement ratio by weight = 0.45, Air Content = 6% (+/- 1.5%)
E. CONCRETE MIX COMPONENTS.
i A water-reducing admixture conforming to ASTM C494, used in strict conformance with the manufacturer's instructions, shall be incorporated in all concrete mix designs.
ii Higher water-cement ratios than shown above may be used if substantiated in accordance with ACI 318-89, Chapter 5.
iii Cement: ASTM C150 Type I or II.
iv Water: Clean & Potable.
v Air entraining agent: ASTM C260. Except where noted non-air entrained.
vi Aggregate: 0.75-inch Maximum aggregate per ASTM C33. Unless noted otherwise.
vii Mix Proportioning: ACI 211.1 and 350R.
F. CONCRETE ACCESSORIES:
i REINFORCING STEEL: Reinforcing steel shall conform to ASTM A615 Grade 60; #3 bars may be Grade 40.
ii WIRE: Plain wire shall conform to ASTM A 82. Deformed wire shall conform to ASTM A 496.
iii JOINTING MATERIALS: In accordance with ACI 350 Section 4.5.2. All jointing materials including water-stops, expansion joints and sealants, shall be resistant to chemical attack for the design life of the facility. Sealants shall conform to ACI 350 and Federal Specification TT-S-00277E and PVC Water-stop shall conform to ASTM D 570, ASTM D 746, STM D 1149 and CRD-C572.
G. LUMBER: Grading shall be to the Standard Grading Rules of the WWPA. Typical structural lumber shall be Number 2 Douglas-Fir/Larch or better. Members noted as wood beams, posts or columns shall be Number 1 Douglas-Fir/Larch or better.
H. BOLTS & LAG SCREWS FOR WOOD CONSTRUCTION: Conform to ANSI/ASME Standards B18.2.1-1981 and the National Design Specification for Wood Construction (NDS) 2005 Edition Part XI for Bolts and Screws.
I. NAILS & SPIKES: Conform to Federal Specification FF-N-105B and the National Design Specification (NDS) 2005 Edition Part XI.
J. NAILING: Where not otherwise specified on the plans, nailing shall conform to IBC Table 2304.9.1, Fastening Schedule. All nails shall be common wire nails or pneumatically driven nails with an equivalent cross-section and penetration, unless noted otherwise.
K. LUMBER HARDWARE: Wood construction connectors shall be as manufactured by Simpson Strong-Tie Company; current catalog, or an approved equal. Hardware exposed to weather or view, in unheated portions of the structure, or as indicated on the drawings or in the specifications shall be hot-dipped galvanized with galvanized fasteners.
L. ROOF SHEATHING: All roof sheathing shall be 5/8" nominal, Exterior APA rated Sheathing {32/16} installed with ply-clips.
M. NON-SHRINK GROUT: All non-shrink grout noted on the plans shall be non-shrink, non-metallic grout with a minimum 28-day compressive strength of 7,000 psi.

13. CONCRETE QUALITY AND DETAILS.

A. GENERAL. Concrete shall be proportioned to provide an average compressive strength, fc, as prescribed in ACI 318/350 Section 5.3.2 and shall satisfy the durability criteria of ACI 318/350 Chapter 4.
B. CONCRETE PROPORTIONS.
i Concrete mix proportioning shall be in accordance with ACI 211.1; Standard Practice for Selecting Proportions for Normal, Heavyweight, and Mass Concrete.
ii Concrete mix proportioning for lightweight concrete shall be in accordance with ACI 211.2; Standard Practice for Selecting Proportions for Lightweight Concrete.
C. CONCRETE MIX VERIFICATION: Concrete mix designs shall be verified by standard 28-day cylinder tests per ASTM C39.
D. EVALUATION AND ACCEPTANCE OF CONCRETE. Concrete shall be tested in accordance with the requirements of ACI 318/350 Section 5.6.
E. MIXING & PLACING CONCRETE. Concrete shall be prepared, mixed, placed and consolidated in accordance with ACI 318/350 Sections 5.7 through 5.10 and as follows:
i ACI 304; Guide for Measuring, Mixing, Transporting, and Placing Concrete.
ii ACI 309; Guide for Consolidation of Concrete.
F. CONCRETE CURING. Concrete shall be maintained above 50-degrees F and in a moist condition for at least 7 days after placement, except when cured in accordance with ACI 318 Section 5.11.3.
i Curing of concrete shall be per the recommendations given in ACI 308; Guide to Curing Concrete.
G. COLD WEATHER REQUIREMENTS. Adequate equipment shall be provided for heating concrete materials and protecting concrete during freezing or near-freezing weather. The recommended procedures listed in ACI 306; Cold Weather Concreting shall be followed.
i Cold weather is defined as a period when, for more than 3 consecutive days, the following conditions exist:
(a) The average daily air temperature is less than 40-degrees F and
(b) The air temperature is not greater than 50-degrees F for more than one-half of any 24-hour period.

DESCRIPTION	DATE
REVISION △	
REVISION △	
REVISION △	
REVISION △	

DRAWN BY KA DATE 02/20/2020
DESIGNED BY KA DATE 02/20/2020
CHECKED BY XXX DATE 201X
APPROVED BY XXX DATE 201X

SCALES: PLAN & PROFILE
HORIZONTAL: 1" =
VERTICAL: 1" =

CITY OF Grand Junction COLORADO

PUBLIC WORKS
ENGINEERING DIVISION

GJFD FIRE TRAINING FACILITY – SHED FNDN
GENERAL NOTES & INFORMATION

PROJECT NO. _____

GENERAL STRUCTURAL NOTES & SPECIFICATIONS (CONT.)

13. CONCRETE QUALITY AND DETAILS. (CONT.)

H. Hot Weather Requirements. During hot weather, proper attention shall be given to ingredients, production methods, handling, placing, protection, and curing to prevent excessive concrete temperatures or water evaporation that could impair required strength or serviceability of the member or structure. The recommended procedures listed in ACI 305; Hot Weather Concreting shall be followed.
   I Hot weather is any combination of the following conditions that tends to impair the quality of freshly mixed or hardened concrete by accelerating the rate of moisture loss and rate of cement hydration, or otherwise causing detrimental results:
      (a) High ambient temperature.
      (b) High concrete temperature.
      (c) Low relative humidity.
      (d) Wind speed.
      (e) Solar radiation.

14. FORMWORK.

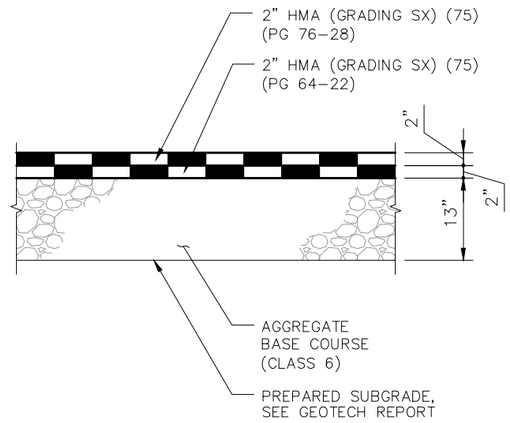
A. Forms shall result in a final structure that conforms to shapes, lines, and dimensions of the members as required by the design drawings and specifications.
   I Design of formwork shall be in accordance with ACI 318/350 Section 6.1.
   II Formwork shall be in accordance with ACI 347; Guide to Formwork for Concrete.
B. Tolerances for finished concrete surfaces shall meet the following requirements, class of surface is per Table 3.4:
   I Footings: CLASS C
   II Foundation walls: CLASS B
   III Above grade concrete not visible to sight: CLASS B
   IV Above-grade concrete visible to sight: CLASS A
C. Embedments in concrete.
   I Conduits, pipes, and sleeves of any material not harmful to concrete and within limitations of ACI 318/350 Section 6.3 shall be permitted to be embedded in concrete with approval of the project engineer, provided they are not considered to replace structurally the displaced concrete, except as provided in Section 6.3.6.
   II Conduits and pipes of aluminum shall not be embedded in structural concrete unless effectively coated or covered to prevent aluminum-concrete reaction or electrolytic action between aluminum and steel.
D. Construction joints.
   I Construction joints shall only be placed where indicated on the project drawings or as approved by the project engineer.
   II Construction joints shall be constructed in accordance with ACI 318/350 Section 6.4

15. DETAILS OF REINFORCEMENT.

A. Placement of all reinforcing steel within concrete structures shall be in conformance with ACI 318/350 Chapter 7.
B. Reinforcing steel hooks, bends, ties, splices and other reinforcement details shall be in accordance with ACI 315; Details and Detailing of Concrete Reinforcement.
C. Spacing limits for reinforcement shall be in conformance with ACI 318/350 Section 7.6.
D. Concrete protection for reinforcement. Unless noted elsewhere on the drawings, all reinforcing steel shall have the following concrete cover:
   I Per ACI 318 Section 7.7:
      (a) Concrete cast against earth: 3.00 inch
      (b) Concrete exposed to earth or weather;
         • No. 5 or smaller bars: 2.00-inch
         • No. 6 or larger bars: 2.00-inch
      (a) Concrete not exposed to earth or weather;
         • No. 11 or smaller bars: 2.00-inch
         • No. 14 or larger bars: 2.00-inch
      (a) Pilasters
         • Primary reinforcement, ties, stirrups or spirals: 1.50-inch
E. Concrete blocks or plastic-coated bar chairs shall be provided for support of all slab reinforcing steel, sufficient in number to prevent settlement or sagging, but in no case shall such support be continuous. Metal clips or supports shall not be placed in contact with the forms or the sub-grade.
F. Dowels and anchor bolts shall be wired or otherwise held in correct position prior to placing concrete. Care shall be taken to insure that dowels and anchor bolts remain plum after concrete is poured and vibrated. In no case shall dowels or anchor bolts be stabbed into freshly poured concrete!
G. Provide dowels in footings and at construction joints to match vertical reinforcing bar size and spacing, unless otherwise noted on the drawings.
H. All bar bends, hooks, splices and other reinforcing steel details shall conform to the requirements of ACI 315.
I. Unless otherwise noted on the plans all bars shall be spliced with a minimum class a lap splice; Lap splices of deformed bars and deformed wire in tension zones shall be class b splices.
J. At all corners and wall intersections provide bent bars to match the horizontal reinforcing steel and in accordance with the typical corner reinforcing details.
K. Chamfer all exposed corners and fillet entrant angles 3/4" unless otherwise noted on the drawings.

16. CONCRETE FINISHING. All concrete surfaces shall be finished in accordance with ACI 301.

A. FORMED CONCRETE SURFACES. After removal of forms, give each formed surface one or more of the following finishes:
   I Concrete structures:
      (a) Concrete footings and foundations not exposed to view. Provide an as-cast finish per Section 5.3.3.3A.
      (b) Foundation wall and other surfaces below grade and not exposed to view. Provide a rough-form finish per Section 5.3.3.3.A.
      (c) Interior, exterior and top surfaces exposed to view to 6-inches below grade. Provide a smooth-form finish per Section 5.3.3.3.B.
      (d) Pilaster, surfaces that are exposed to view. Provide a smooth rubbed finish per Section 5.3.3.5.A.
   II Special or architectural finishes: Refer to the architectural specifications for special or architectural finish requirements.
B. UNFORMED CONCRETE SURFACES. Unformed concrete surfaces including the top surface of all concrete roof and floor slabs shall be finished in accordance with ACI 301 Section 5.3.4 and ACI 302 Chapter 8.
      (a) For the top surfaces of walls, provide a "scratched finish" per Section 5.3.4.2.A.
      (b) Interior areas receiving only light foot traffic shall receive a troweled finish per Section 5.3.4.2C.
      (c) Provide a nonslip finish for exterior surfaces and where indicated on the plans.
C. SAWED CONTRACTION JOINTS. Conform to ACI 301 Section 5.3.5.



TYPICAL SECTION – ASPHALT PAD

Schedule: Fire Training Facility - Shed Foundation & Asphalt

Item No.	CDOT, City Ref.	Description	Quantity	Units
1	201-00000	Clearing and Grubbing	0.6	ACRE
2	202-04060	Dust Abatement	Lump Sum	
3	203-00000	Unclassified Excavation	74.	CY
4	206-00100	Structural Backfill (Class 1)	74.	CY
5	208-00045	Concrete Washout Structure	1.	EA
6	208-00070	Vehicle Tracking Pad	1.	EA
7	208	Erosion Control	Lump Sum	
8	304	Aggregate Base Course (Class 6)	1,016.	CY
9	403	Hot Mix Asphalt (Grading SX, PG 76 - 28)	278.	TON
10	403	Hot Mix Asphalt (Grading SX, PG 64 - 22)	278.	TON
11	420	Geotextile (Class 1)	258.	SY
12	601-03020	Concrete Class GV-B	71.	CY
13	602-00010	Reinforcing Steel (Black)	8,300.	LB
14	603-01125	12 Inch Reinforced Concrete Pipe (Complete In Place)	65.	LF
15	608	Bituminous Curb (6" Wide, 6" High)	520.	LF
16	620-00020	Sanitary Facility	1.	EA
17	625-00000	Construction Surveying	Lump Sum	
18	626-00000	Mobilization	Lump Sum	

DESCRIPTION	DATE
REVISION △	–
REVISION △	–
REVISION △	–
REVISION △	–

DRAWN BY KA DATE 02/20/2020
DESIGNED BY KA DATE 02/20/2020
CHECKED BY XXX DATE 201X
APPROVED BY XXX DATE 201X

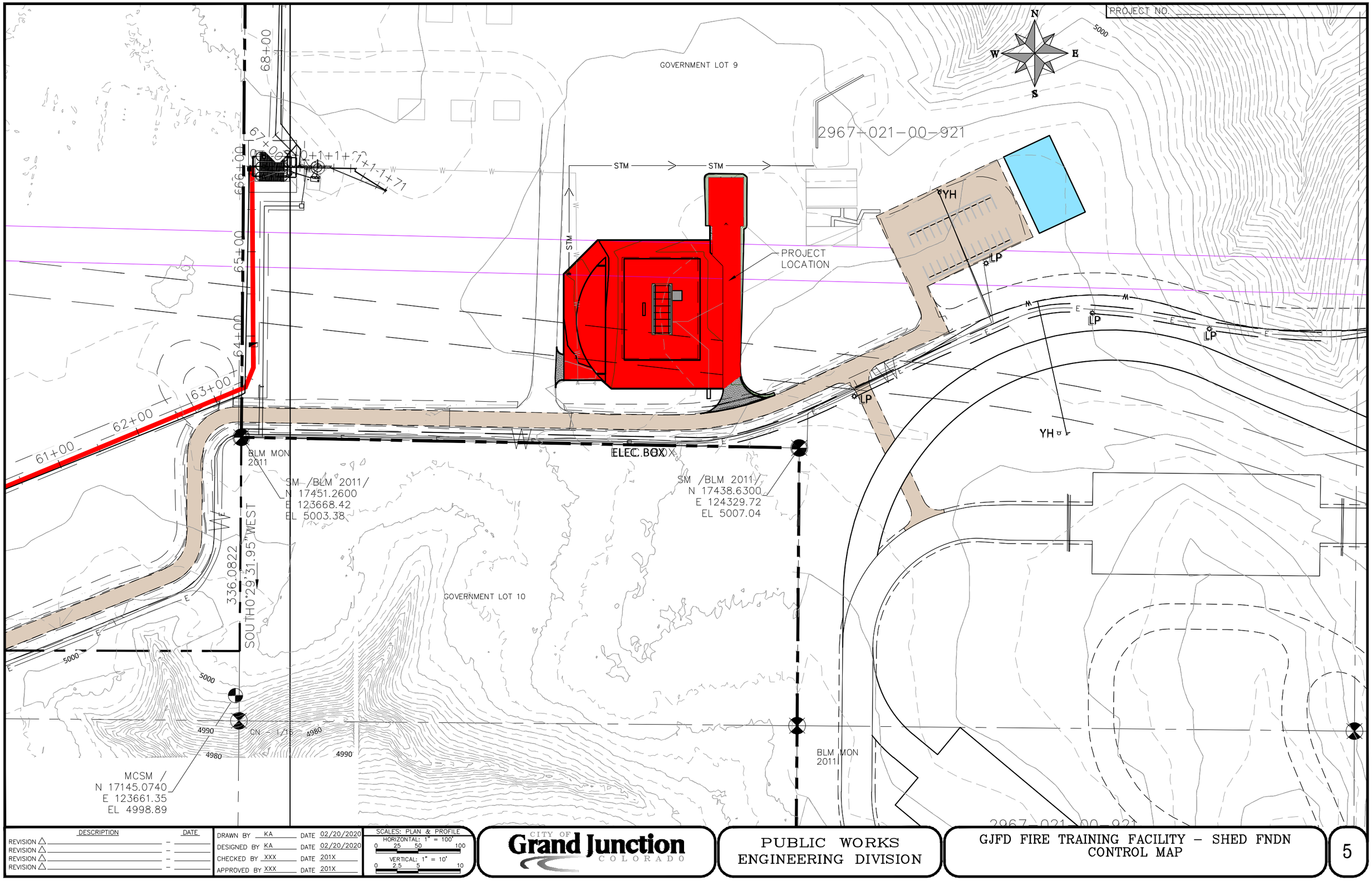
SCALES: PLAN & PROFILE
HORIZONTAL: 1" =
VERTICAL: 1" =

CITY OF Grand Junction COLORADO

PUBLIC WORKS
ENGINEERING DIVISION

GJFD FIRE TRAINING FACILITY – SHED FNDN
GENERAL NOTES & INFORMATION
SUMMARY OF APPROXIMATE QUANTITIES

PROJECT NO. _____



DESCRIPTION	DATE
REVISION △	
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REVISION △	

DRAWN BY	KA	DATE	02/20/2020
DESIGNED BY	KA	DATE	02/20/2020
CHECKED BY	XXX	DATE	201X
APPROVED BY	XXX	DATE	201X

SCALES: PLAN & PROFILE
HORIZONTAL: 1" = 100'
0 25 50 100
VERTICAL: 1" = 10'
0 2.5 5 10

CITY OF Grand Junction COLORADO

PUBLIC WORKS
ENGINEERING DIVISION

GJFD FIRE TRAINING FACILITY – SHED FNDN
CONTROL MAP

5

PROJECT NO. _____

N 17758.06
E 124222.74
EL 5006.71

PROPOSED STORAGE SHED

N 17758.32
E 124262.74
EL 5006.71

DRAIN

PROPOSED CONCRETE SLAB TO SUPPORT STORAGE SHED (SEE DETAILS)

N 17702.06
E 124223.11
EL 5006.71

N 17702.32
E 124263.10
EL 5006.71

N 17702.09
E 124223.99
EL 5006.71

N 17702.30
E 124259.64
EL 5006.71

17.36'

DEMO TO CRACK IN CONCRETE & REPLACE WITH ASPHALT

N 17684.73
E 124090.74
EL 5004.96

ASPHALT CURB

N 17684.73
E 124223.99
EL 5006.29

17.26'

21.74'

-1.01%

-1.63%

49.51'

21.93'

90'

15.03'

32.65'

EXISTING ELECTRIC LINE, PROTECT IN PLACE

EXISTING CONCRETE FOOTING, PROTECT IN PLACE

EXISTING CONCRETE SLAB-ON-GRADE, PROTECT IN PLACE

EXISTING CONCRETE STRIP FOOTING, PROTECT IN PLACE

-1.01%

120'

PROPOSED ASPHALT DRIVE TO STORAGE SHED

N 17644.50
E 124050.52
EL 5004.75

EXISTING GRAVEL DRIVE

-0.41%

ASPHALT CURB

N 17549.00
E 124050.52
EL 5005.21

EARTH QUANTITIES	
CUT (CY)	FILL (CY)
620.9	42.7

NOTE: EARTH QUANTITIES SHOWN ABOVE ARE FOR INFORMATION ONLY. ALL EARTHWORK REQUIRED FOR THIS PROJECT WILL NOT BE PAID FOR SEPARATELY BUT SHALL BE INCLUDED IN THE WORK. EXCESS MATERIAL MAY BE STOCKPILED ON SITE AS DIRECTED BY THE ENGINEER.

EXISTING GAS LINE, PROTECT IN PLACE

N 17528.77
E 124259.64
EL 5006.07

-0.50%

33.97'

EXISTING MANUFACTURED BURN BUILDING, PROTECT IN PLACE

PROPANE

-7.45%

-2.74%

PROPOSED ASPHALT PAVEMENT

EXISTING CONCRETE SLAB-ON-GRADE SUPPORTING PROPANE TANK, PROTECT IN PLACE

-7.24%

EXISTING GRAVEL DRIVE

N 17519.00
E 124050.52
EL 5005.36

N 17519.01
E 124095.54
EL 5005.17

N 17509.01
E 124107.06
EL 5005.51

EXISTING CONCRETE SLAB-ON-GRADE, PROTECT IN PLACE

EXISTING FIRE HYDRANT, P.I.P.

ASPHALT CURB

N 17509.02
E 124239.64
EL 5005.76

N 17479.17
E 124212.68
EL MATCH EXISTING

12" RCP

N 17500.94
E 124300.84
EL MATCH EXISTING

N
W E
S

DESCRIPTION	DATE
REVISION △	
REVISION △	
REVISION △	
REVISION △	

DRAWN BY HMC DATE 02/2020
DESIGNED BY KA DATE 02/2020
CHECKED BY _____ DATE _____
APPROVED BY _____ DATE _____

SCALES: PLAN & PROFILE
HORIZONTAL: 1" = 30'
0 7.5 15 30
VERTICAL: 1" =

CITY OF Grand Junction COLORADO

PUBLIC WORKS
ENGINEERING DIVISION

GJFD FIRE TRAINING FACILITY – SHED FNDN
ASPHALT PAVING PLAN

6

N:\Landproj\FIRE TRAINING CENTER\2018 Whitewater Hill Fire Training Center\dwg\E1802 Shed FNDN & Asphalt.dwg, 11x17 Shed Fndn & Asphalt Pad (6), 2/28/2020 8:22:14 AM

PROJECT NO. _____

STM STM STM STM STM STM STM STM STM STM STM STM STM STM STM STM STM

EXISTING STORM SEWER PIPE, PROTECT IN PLACE

TOP OF SLAB AT NORTH END OF BUILDING, EL. 5006.99

#4 X 11'-0" HAIRPINS, (1) EACH AT EACH ENDWALL SUPPORT

TOP OF STEM WALL, EL. 5007.21

CONTROL JOINT IN 6" CONCRETE MONOSLAB, SEE DETAILS ON SHEET 8.

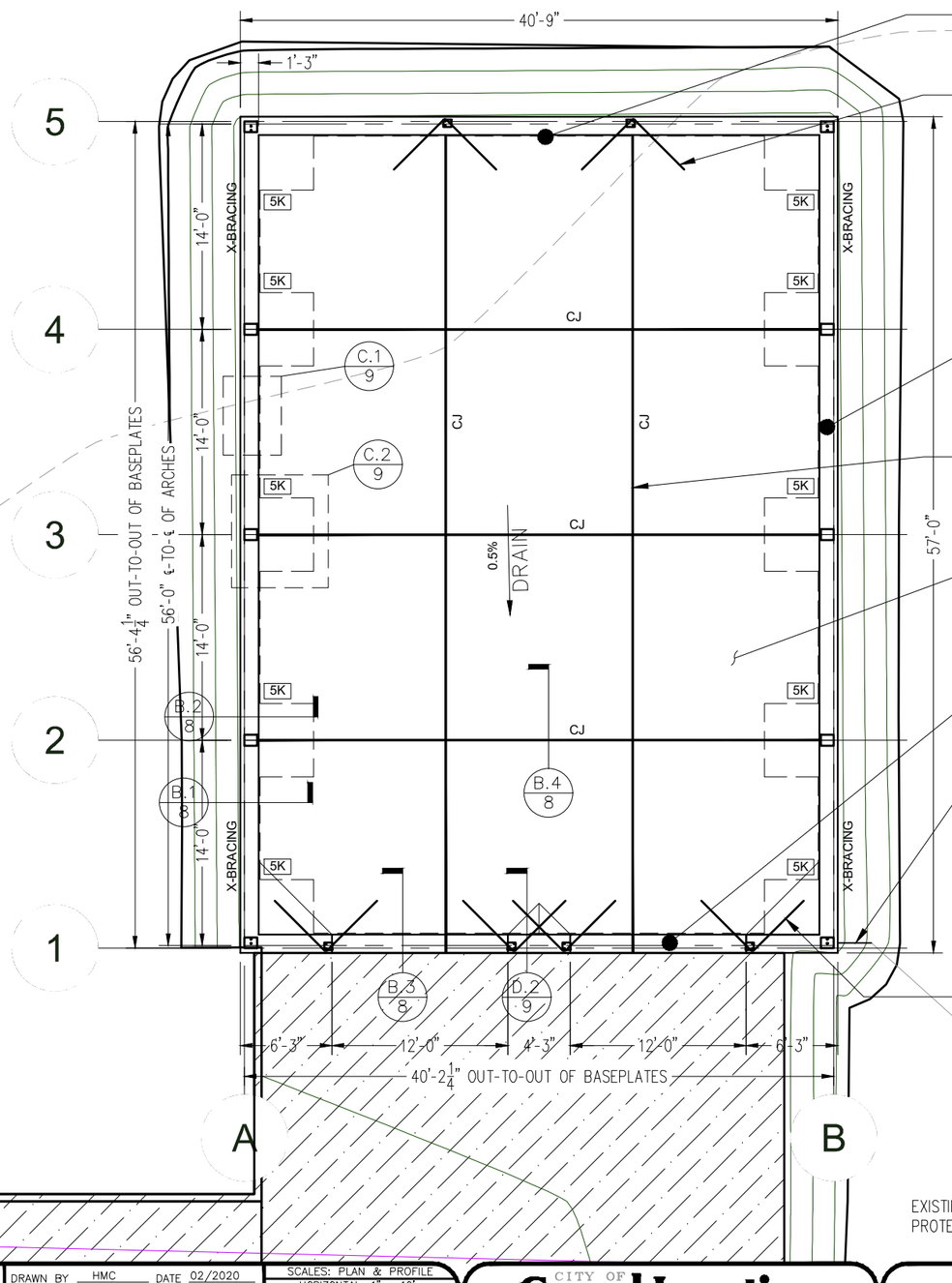
PROPOSED CONCRETE MONOSLAB TO SUPPORT STORAGE SHED - 6" THICK ON 12" ABC CLASS 6, REINFORCE WITH #5 BARS @ 12" O.C. E.W. (SEE DETAILS)

TOP OF SLAB AT DOOR OPENING (TYP.), EL. 5006.71

COORDINATE ELECTRICAL CONDUIT INSTALLATION WITH MCATLIN ELECTRICAL CORP PRIOR TO CONCRETE FOUNDATION POUR

AT HAIRPINS, PROVIDE 1½" CLEARANCE ABOVE & 3" CLEARANCE BELOW REINF., TYP.

EXISTING ELECTRIC LINE, PROTECT IN PLACE



FOUNDATION PLAN NOTES

1. COORDINATE & VERIFY ALL DIMENSIONS WITH METAL BLDG. PLANS.
2. CJ INDICATES CONCRETE FLOOR JOINT.
3. MK INDICATES FOOTING TYPE PER FOOTING SCHEDULE.
4. SEE SITE CIVIL PLANS FOR SIDEWALKS AND EXTERIOR PAVING.

FOOTING SCHEDULE

MK	Size	Reinforcing
5K	5'-0" SQ x 18"	(6) #5 EA. WAY

N:\Landproj\(FIRE TRAINING CENTER)\2018 Whitewater Hill Fire Training Center\dwg\E1802 Shed FNDN & Asphalt.dwg, 11x17 Shed Fndn (7), 3/31/2020 10:29:01 AM

DESCRIPTION	DATE
REVISION △	–
REVISION △	–
REVISION △	–
REVISION △	–

DRAWN BY HMC DATE 02/2020
DESIGNED BY KA DATE 02/2020
CHECKED BY DATE
APPROVED BY DATE

SCALES: PLAN & PROFILE
HORIZONTAL: 1" = 10'
0 2.5 5 10
VERTICAL: 1" =

CITY OF Grand Junction COLORADO

PUBLIC WORKS
ENGINEERING DIVISION

GJFD FIRE TRAINING FACILITY – SHED FNDN
SHED FOUNDATION PLAN

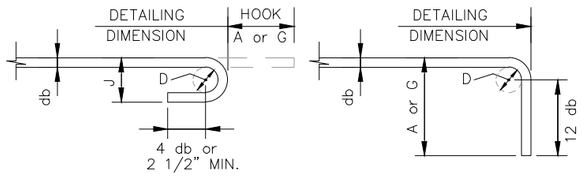
7

PROJECT NO. _____

TYPICAL LAP SPLICE LENGTHS IN INCHES, PER ACI 318									
BAR SIZE	LAP CLASS	f'c=3,000 psi		f'c=4,000 psi		f'c=5,000 psi		f'c=6,000 psi	
		CAT.1	CAT.2	CAT.1	CAT.2	CAT.1	CAT.2	CAT.1	CAT.2
#4	A	22	33	19	28	17	25	15	23
	B	28	43	25	37	22	33	20	30
#5	A	27	41	24	36	21	32	19	29
	B	36	53	31	46	28	41	25	38
#6	A	33	49	28	43	25	38	23	35
	B	43	64	37	55	33	50	30	45
#7	A	48	72	42	62	37	56	34	51
	B	62	93	54	81	48	72	44	66
#8	A	55	82	47	71	42	64	39	58
	B	71	106	61	92	55	83	50	76
#9	A	62	92	53	80	48	72	44	65
	B	80	120	69	104	62	93	57	85

NOTES:
1. FOR GRADE 60 REINFORCING STEEL BARS.
2. ALL LAP SPLICES SHALL BE CLASS B, UNLESS NOTED OTHERWISE.
3. CATEGORY 1: CLEAR COVER >= db & CLR. SPACING >= db, AND STIRRUPS OR TIES THROUGHOUT Ld ARE PROVIDED.
   CATEGORY 1: CLEAR COVER >= db & CLR. SPACING >= 2db.
   CATEGORY 2: CLEAR COVER < db OR CLR. SPACING < 2db.
4. FOR TOP BARS MULTIPLY LAP LENGTH LISTED BY 1.30
   TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW THE BARS.

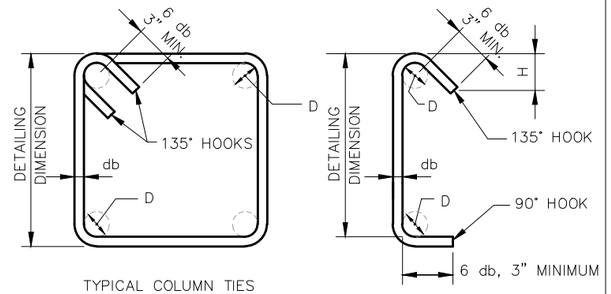
A.1 TYPICAL REBAR LAP SCHEDULE



BAR SIZE	D	180° HOOKS		90° HOOKS
		A or G	J	A or G
#3	2 1/4"	5"	3"	6"
#4	3"	6"	4"	8"
#5	3 3/4"	7"	5"	10"
#6	4 1/2"	8"	6"	1'-0"
#7	5 1/4"	10"	7"	1'-2"
#8	6"	11"	8"	1'-4"
#9	9 1/2"	1'-3"	11 3/4"	1'-7"

NOTES: db = NOMINAL BAR DIAMETER.
D = FINISHED INSIDE BEND DIAMETER.
MINIMUM D = 6 db FOR #3 TO #8 BARS
MINIMUM D = 8 db FOR #9 TO #11 BARS
MINIMUM D = 10 db FOR #14 AND #18 BARS
TYPICAL MINIMUM END HOOKS, ALL GRADES OF STEEL.

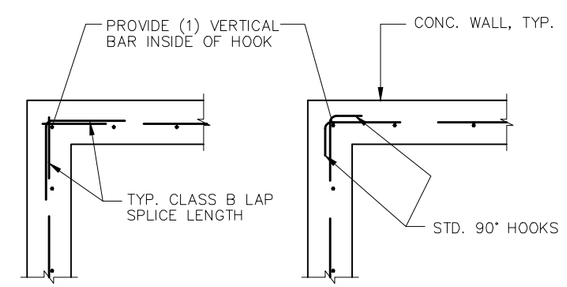
A.2 TYPICAL REBAR HOOK/BEND DETAIL



BAR SIZE	D	135° HOOKS		90° HOOKS
		A or G	H **	A or G
#3	1 1/2"	4 1/4"	3"	4"
#4	2"	4 1/2"	3"	4 1/2"

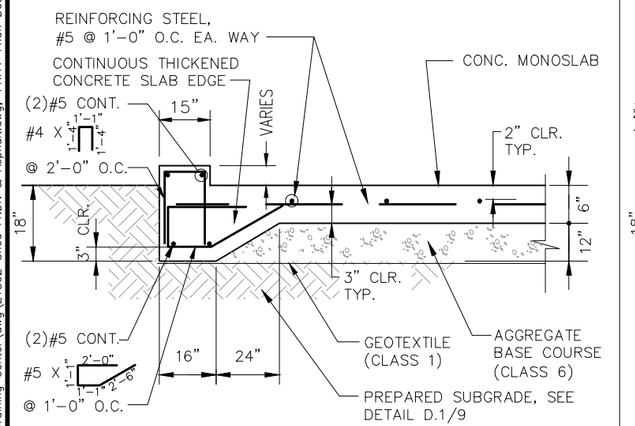
NOTES: db = NOMINAL BAR DIAMETER.
D = FINISHED INSIDE BEND DIAMETER.
MINIMUM D = 6 db FOR #3 & #4 BARS
TYPICAL MINIMUM END HOOKS, ALL GRADES OF STEEL.
H ** DIMENSION IS APPROXIMATE

A.3 TYPICAL REBAR HOOP & TIE DETAIL



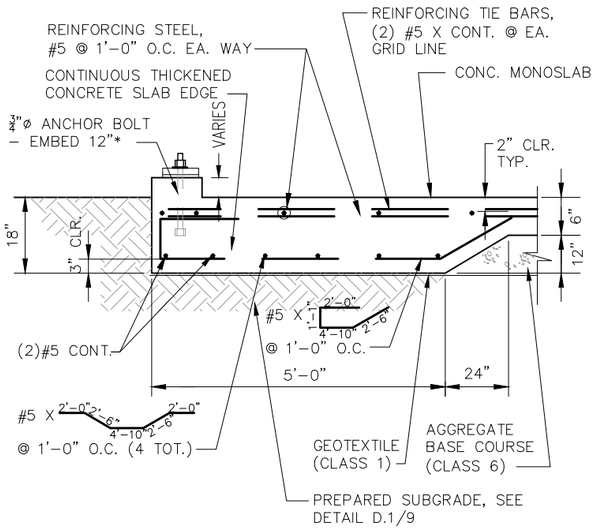
NOTES:
CORNER & INTERSECTION BARS TO MATCH SIZE & SPA. OF HORIZ. BARS. CENTER VERTICAL BARS IN WALL UNLESS NOTED OTHERWISE. REFER TO OTHER DETAILS FOR REQUIRED BAR SIZE AND SPACING. THIS IS A TYPICAL DETAIL FOR WALLS WITH (1) ONE MATT OF REBAR. THIS IS A TYPICAL DETAIL FOR NON-WATER RETAINING WALLS.

A.4 TYPICAL REBAR WALL CORNER DETAIL



NOTE: REFER TO FOUNDATION PLAN FOR SLAB SLOPE AND CONTROL/CONSTRUCTION JOINT LOCATIONS.

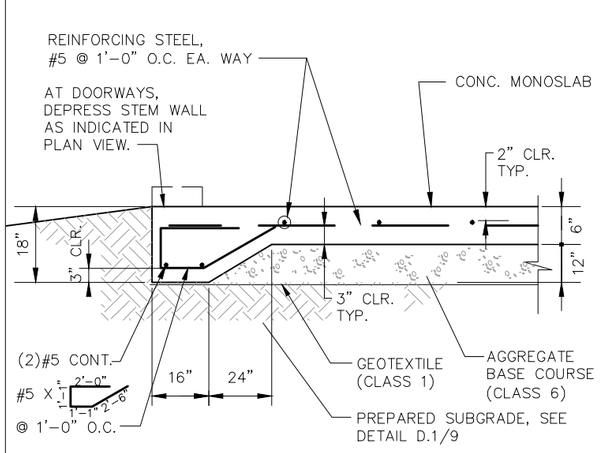
B.1 TYPICAL SLAB DETAIL THICKENED EDGE WITH STEM WALL



NOTE: REFER TO FOUNDATION PLAN FOR SLAB SLOPE AND CONTROL/CONSTRUCTION JOINT LOCATIONS.

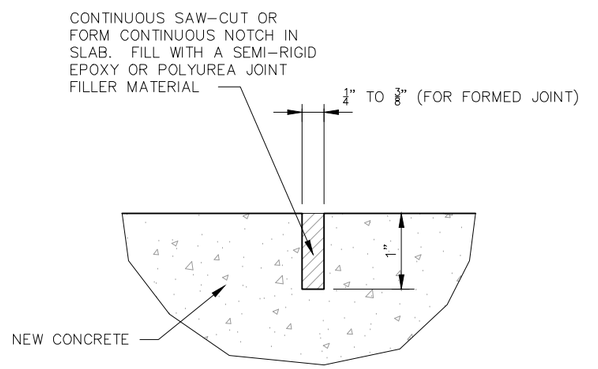
B.2 TYPICAL EXTERIOR WALL FOOTING WITH STEM WALL

* CONTRACTOR SHALL PROVIDE AND INSTALL ANCHOR BOLTS IN ACCORDANCE WITH GENERAL NOTE 11.B AND PREMANUFACTURED BUILDING.



NOTE: REFER TO FOUNDATION PLAN FOR SLAB SLOPE AND CONTROL/CONSTRUCTION JOINT LOCATIONS.

B.3 TYPICAL SLAB DETAIL AT EXTERIOR DOORS



NOTE: REFER TO OTHER TYPICAL DETAILS FOR SLAB REINFORCING REQUIREMENTS. PROVIDE METZGER/McQUIRE MM-80 SEMI-RIGID EPOXY JOINT FILLER OR REZI-WELD FLEX, SEMI-RIGID EPOXY JOINT FILLER (FILLER WILL NOT BE PAID SEPARATELY BUT SHALL BE INCLUDED IN THE WORK).

B.4 TYPICAL SLAB DETAIL CONTROL JOINT

DESCRIPTION	DATE
REVISION 1	
REVISION 2	
REVISION 3	
REVISION 4	

DRAWN BY KA DATE 9/24/2019
DESIGNED BY KA DATE 9/24/2019
CHECKED BY XXX DATE 201X
APPROVED BY XXX DATE 201X

SCALES: PLAN & PROFILE
HORIZONTAL: 1" =
VERTICAL: 1" =

CITY OF Grand Junction COLORADO

PUBLIC WORKS
ENGINEERING DIVISION

GJFD FIRE TRAINING FACILITY - BURN SLAB DETAILS
BURN SLAB

8

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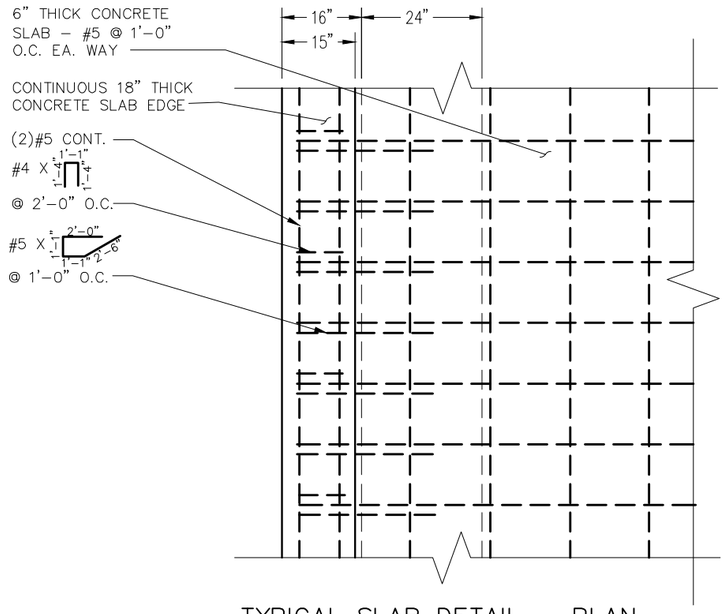
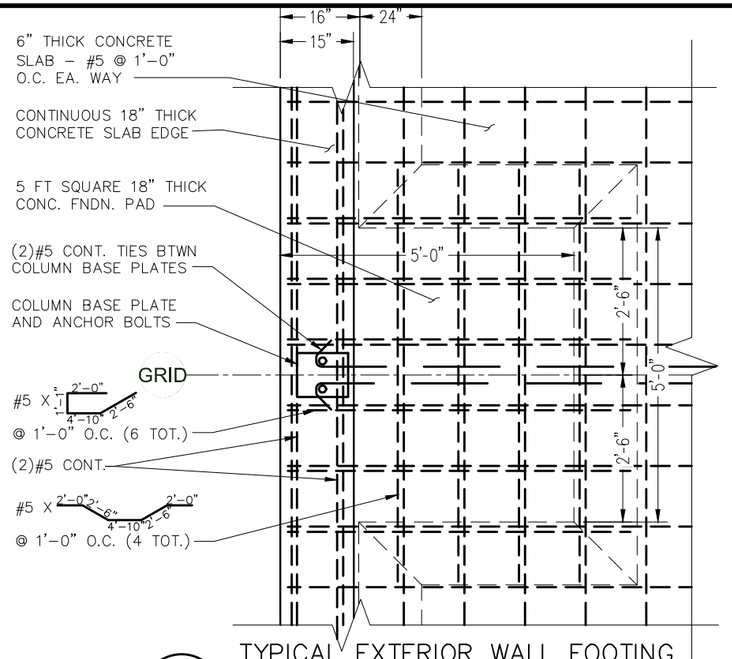
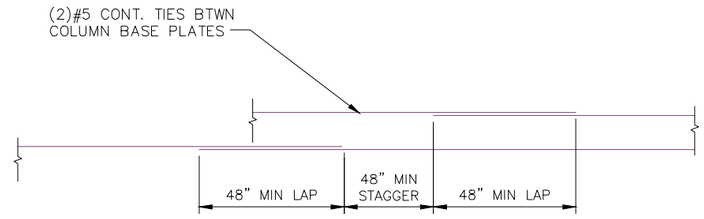
PROJECT NO. _____



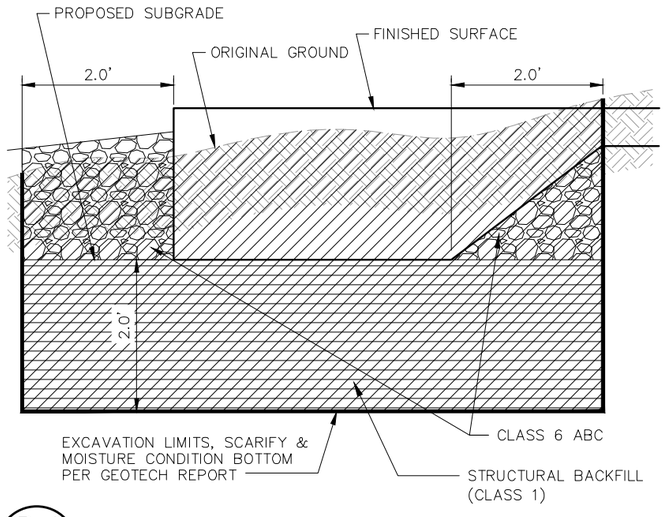
C.1 TYPICAL SLAB DETAIL – PLAN
THICKENED EDGE WITH STEM WALL



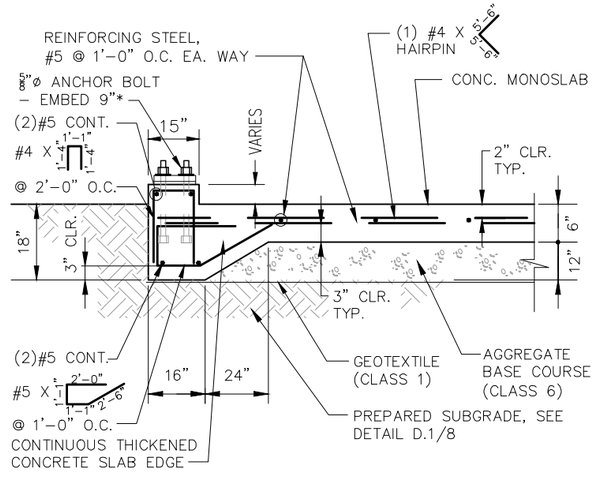
C.2 TYPICAL EXTERIOR WALL FOOTING
WITH STEM WALL – PLAN



C.3 TYPICAL TENSION TIE DETAIL – PLAN



D.1 CONCRETE EXT. WALL FTG. EXCAVATION DTL.



NOTE: REFER TO FOUNDATION PLAN FOR SLAB SLOPE AND CONTROL/CONSTRUCTION JOINT LOCATIONS.

D.2 TYPICAL SLAB DETAIL
END WALL ANCHORAGE WITH STEM WALL

* CONTRACTOR SHALL PROVIDE AND INSTALL ANCHOR BOLTS IN ACCORDANCE WITH GENERAL NOTE 11.B AND PREMANUFACTURED BUILDING.

DESCRIPTION	DATE
REVISION △	–
REVISION △	–
REVISION △	–
REVISION △	–

DRAWN BY	XXX	DATE	201X
DESIGNED BY	XXX	DATE	201X
CHECKED BY	XXX	DATE	201X
APPROVED BY	XXX	DATE	201X

SCALES: PLAN & PROFILE
HORIZONTAL: 1" = 40'
0 10 20 40
VERTICAL: 1" = 10'
0 2.5 5 10

CITY OF Grand Junction COLORADO

PUBLIC WORKS
ENGINEERING DIVISION

GJFD FIRE TRAINING FACILITY – FNDN & SLAB DETAILS
BURN BUILDING FOUNDATION AND BURN SLAB

9

N:\Landpro\(FIRE TRAINING CENTER)\2018 Whitewater Hill Fire Training Center\dwg\E1802 Shed FNDN & Asphalt.dwg, 11x17 Fndn Details (9), 3/31/2020 10:45:18 AM