Riverside Parkway



FINAL DRAINAGE & IRRIGATION DESIGN REPORT: EAST SECTION

August 15, 2005

Prepared by:

Carter & Burgess, Inc. 707 17th Street Suite 2300 Denver, Colorado 80202

Ref: 071514.403



Riverside Parkway



Preliminary Drainage Report

TABLE OF CONTENTS

		<u>Page No.</u>
I,	A. Location B. Description	1 1
11.	 DRAINAGE BASINS AND SUB-BASINS LOCATION AND EVALUATION A. Major Basin Description B. Sub-Basin Description 	3 3
111.	 DRAINAGE DESIGN CRITERIA A. Project References B. Hydrologic Criteria C. Hydraulic Criteria, Discussion and Method Reference 	4 4 5
IV.	DRAINAGE FACILITY DESIGN A. General Concept B. Specific Details	5 6
V.	CONCLUSIONS A. Compliance With Standards B. Drainage Concept	9 9
VI.	IRRIGATION CONVEYANCEA.OverviewB.Irrigation Flow & AnalysisC.Irrigation Lateral SummaryD.Summary	10 10 11 18
VII.	REFERENCES	19
Арре	A. Hydrologic Computation Summary Floodplain Map 2-Yr Hydrology 100-Yr Hydrology Basin Maps	
	B. Hydraulic Computations Summary Storm CAD Output &Maps Culvert Calculations Indian Wash Backwater Calculations Riprap Calculations	
	C. Irrigation Computations	



East Section

I. LOCATION AND DESCRIPTION OF PROJECT

A. <u>Location</u>

The City of Grand Junction proposes to build a new Riverside Parkway on the south side of the city to address both local and regional transportation needs. The Parkway is part of a larger loop road system being jointly developed by the City and Mesa County.

In general, the Riverside Parkway project area lies within the City bounded by 24 Road (Redlands Parkway) on the west, 29 Road on the east, the Union Pacific Railroad tracks on the north, and the Colorado River on the south (see **Figure 1: Project Location Map**). The project includes a proposed interchange with US 50 and modifications to the existing US 50 bridge spans over the Colorado River, resulting in a narrow extension of the project area across the river. The land within the project area is a mixture of open space, vacant lands, and residential, commercial, and industrial development. The proposed roadway traverses all or portions of Sections 8, 9, 10, 15, 22, 23 and 24, Township 1 South, Range 1 West, of the Ute Principle Meridian, in a northwesterly to southeasterly direction, more or less parallel to the Union Pacific Railroad. The Parkway then follows the Section line between Sections 18 and 19, and 17 and 20, Township 1 South, Range 1 East, of the Ute Principle Meridian, along the alignment of existing D Road.

In addition to Riverside Parkway, this project will include reconstruction of a portion of 29 Road from north of the intersection with D Road south to the approach to the bridge over the Colorado River. The 29 Road alignment is along the Section line between Sections 20 and 21, Township 1 South, Range 1 East of the Ute Principle Meridian.

B. <u>Description</u>

The Riverside Parkway project is divided into three Sections, labeled A, B and C for convenience, which shall be hence forth known as the West Section, Lower Downtown Section and East Section, respectively.

The East Section, or Section C, extends 2.7 miles from the approximate intersection of S. 9th Street and Struthers. The alignment bends north at 12th Street and then east again along D Road to 29 Road. The typical section has two through lanes, a center turn lane, bike lanes, curb and gutter, and a detached sidewalk on the south. The majority of this Section is in Mesa County. Major drainageway crossings include the HumpYard Drain, Indian Wash, the State Home Drain and the No Name Drain.

Also included in the East Section is approximately 1 mile of 29 Road. The 29 Road typical section consists of two through lanes, a center turn lane, bike lanes, curb and gutter, and sidewalk on each side. This roadway is completely within Mesa County. The only major drainageway crossing of 29 Road in this vicinity is the Odelburg Drain. In addition, the portion of 29 Road south of C-1/2 Road is historically within the floodplain of the Colorado River.



East Section

Throughout the length of Sections A and C there are agricultural irrigation laterals and drains owned by the Grand Valley Irrigation Company and the D Road Mutual Irrigation Company. Occasionally the systems cross under the roadway to serve users on the south side of the Parkway. This Project will include replacing existing irrigation crossings with new pipe and relocating headgates, ditches, pump sets, and pipelines on adjacent properties.









II. DRAINAGE BASINS AND SUB-BASINS LOCATION AND EVALUATION

A. <u>Major Basin Description</u>

The major drainage basins of Riverside Parkway are all tributary to the Colorado River. The Riverside Parkway project will replace and/or relocate portions of the Buthorn Drain, the Ligrani Drain, the Humpyard Drain, the State Home Drain, the No Name Drain and the Odelburg Drain within the project area. The major drainage basins affecting this project encompass a large portion of the City as well as areas that are located within Mesa County. The major drainage basin areas range in size from 48 acres to over 12 square miles. Most of these basins have been master planned by the City.

Land uses include commercial, residential, and light industry. In general, the major basins slope from northeast to southwest toward the Colorado River although, much of the runoff is conveyed within the washes and drains that cross the proposed Riverside Parkway alignment at nearly perpendicular angles.

In the East Section of the project, the Colorado River is the only FEMA regulated 100-year floodplain. While Indian Wash is FEMA regulated within the City of Grand Junction, in Mesa County it is not. For this reason, the northern panel is referenced. The flood insurance rate map (FIRM) for Indian Wash is Community Panel Number 080117 0007E. The map panel is included in the Appendix B as "Firmette", created through the FEMA online map store.

There are also numerous irrigation ditches and pipelines located throughout the entire project. The East Section of the Riverside Parkway project will replace and/or relocate the Humpyard Drain, the State Home Drain, the Indian Wash Drain, and the Odelburg Drain.

B. <u>Sub-Basin Description</u>

Sub-basins were delineated based on contour mapping prepared for this project as well as 2foot contour mapping provided by the City. Sub-basins are shown on the drainage basin maps in Appendix B at the back of this report. Each basin map depicts the proposed alignment of Riverside Parkway and the associated basin boundaries resulting from the typical section and proposed roadway profile. Aerial photography is overlain with the base file for reference.

In general, the topography along the Parkway alignment is quite flat and the roadway profile has been designed with a minimum 0.5% grade, which causes some undulation of the profile. Roadway sub-basins are also flat and drain to inlets located in sags. The typical section includes curb and gutter and sidewalks along with a small landscaping barrier located intermittently along the roadway. Because the amount of impervious area is greatly increased for the roadway projects, the corresponding runoff coefficients should be high as discussed later in the hydrologic criteria.



East Section

The offsite basins are comprised of various land uses but in general contain mostly commercial, residential, and light industry properties. Offsite flows are intercepted by the proposed roadway drainage system to be conveyed to the nearest drainage culvert.

III. DRAINAGE DESIGN CRITERIA

The basis for analysis of the storm drainage system is the City of Grand Junction / Mesa County Stormwater Management Manual (**SWMM**) (Reference 1). Where necessary guidance was not provided, the Urban Storm Drainage Criteria Manual was used (Reference 2). The following are a summary of several of the more notable parameters:

- 1. Minor Storm 2-year storm.
- 2. Major Storm 100-year storm.
- 3. Maximum gutter depth for the minor storm is 6".
- 4. Maximum gutter depth for the major storm is 12".
- 5. Arterial roads (Riverside Parkway) shall have at least one 8-foot wide traffic lane in each direction and the center turning lane free of ponding during the minor event.
- 6. Minimum pipe diameter is 12".
- 7. All pipe is to be reinforced concrete.
- 8. Minimum pipe velocity is 2.5 fps.

A. <u>Project References</u>

Several prior studies exist regarding offsite drainage. The prior studies mainly involve portions of the West and Lower Downtown sections of this project. Those studies are listed in the References Section. Recommendations and design flows were adhered to where appropriate.

B. <u>Hydrologic Criteria</u>

The minor drainage system analyzed by this study is a result of the 2-year recurrent storm event. The major drainage system (cross culverts) is designed to convey runoff generated by the 100-year storm event.

The for runoff calculation method used for onsite and offsite basins is the Rational Method. The Rational Formula is:

Q = CIA

Q = storm runoff in CFS;

- C = runoff coefficient based on surface impermeability;
- I = rainfall intensity in inches per hour; and
- A = drainage basin area in acres.



East Section

Rainfall intensities were obtained from the Intensity-Duration-Frequency curves in the SWMM, Appendix A, Table A-1a. This figure is included in the Appendix A of this report for reference.

The Time of Concentration, T_c , is the sum of overland flow (T_o) and shallow concentrated flow (T_s):

Where:

 $T_o = \frac{1.8(1.1-C)L^{0.5}}{S^{0.33}}$ C = Rational method runoff coefficient L = Length of the flow plane in feet (300 feet maximum); and S = Average percent slope of the overland plane.

$$T_s = L$$

60V

Where:

L = Length of flowline in feet; and V = velocity in feet per second.

The Rational Method runoff coefficients, or C factors, were computed using the techniques described in the Mesa County SWMM, Appendix B – Rational Method C Values. A summary of the coefficients distributed by hydrologic soil type was used to weight a final "basin-average" runoff coefficient for each basin. These calculations are included in Appendix B in the HEC-12 inlet calculations. For the most part the basins can be considered 100% impervious.

C. <u>Hydraulic Criteria, Discussion and Method Reference</u>

The proposed storm sewer system was designed using StormCAD software developed by Haestad Methods. Street capacity calculations are included in Appendix B with the HEC-12 inlet calculations. The gutter section algorithm uses FHWA HEC-12 methodology to compute capacity. Finally, StormCAD input and output listings are also located Appendix B.

The culverts were designed to convey the 100-year storm without overtopping the roadway. All relevant charts, nomographs and tables used in the analysis and design of the storm drainage system were obtained from the SWMM and are included Appendix B.

IV. DRAINAGE FACILITY DESIGN

A. <u>General Concept</u>

The Riverside Parkway and 29 Road storm sewer systems will outfall into existing roadside ditches or drains, major drainage crossings, or existing or proposed drainage outfall systems. Roadway runoff is intercepted by inlets located at roadway low points and curb returns. In some locations, offsite runoff enters the proposed storm sewer system through inlets or



East Section

culverts connected to the roadway storm sewer system, as shown on the preliminary plans. At these locations the receiving storm sewer has been sized to receive the additional runoff.

On-grade inlets are provided at roadway intersections, and at superelevation transitions. Intermediate inlets are placed to meet project criteria, such as spread and ponding depths.

B. <u>Specific Details</u>

The roadways and accompanying offsite basins can be divided based on the major drainageway they contribute to. The far western portions of the East Section of Riverside Parkway drain to the IDI Drain where a proposed manhole will connect the two systems. Moving east, drainage begins to collect around the existing Water Quality Pond and outfall. Three 7' x 3' concrete box culverts are proposed for the Water Quality Pond and outfall. Drainage extends north to 27 $\frac{1}{2}$ Road where the direction is split by a high point in the intersection.

Once Riverside Parkway connects to existing D Road, roadway drainage patterns vary according to grades before draining to the Colorado River. The first several storm lines drain east to the Humpyard Drain, a 6' x 2' CBC. The sizing of the Humpyard Drain was based on a utility conflict with a proposed 24" sanitary sewer. In this case the sewer line crosses above the storm. Clearance between the utilities is minimal so that the crossing shall be encased.

Moving east, the next storm lines drain into Indian Wash, which crosses the roadway through a 20' x 8' 3-sided box culvert. This pattern extends just beyond the intersection of 28 Road. All drainage systems between 28 Road and 29 Road drain directly to the State Home Drain which sub sequentially outfalls into the Indian Wash. The State Home Drain is assumed to convey 7 cfs as a base flow condition. This assumption accounts for irrigation waste flows returning to that system.

The last system to cross Riverside Parkway is the No Name Drain. The No Name drain intercepts runoff from the intersection of Riverside Parkway and 29 Road to the eastern end of project limits. Also connected to the No Name Drain is an existing 12" storm pipe which intercepts approximately 2 cfs of irrigation base flows. Design flows for this system have been acquired from the Leigh & Whitehead Preliminary Drainage Report for 29 Road (Reference 10). Based on this study the proposed sewer will receive 12 cfs of runoff during the 2-yr event.

Finally, the 1-mile reach of 29 Road to be constructed as part of this project drains to the Odelburg Drain from both the north and the south. Runoff from the Odelburg eventually drains into the Colorado River.

The Odelburg Drain is located just south of C-1/2 Road. Offsite basin OS-2 (basins 20, 21, 23, 25 and 26 of the Leigh & Whitehead Preliminary Drainage Report Reference 10) is tributary to the Odelburg Drain. However, the Odelburg Drain culvert has been sized to provide relief from backwater effects of the 100-year flood in the Colorado River. The size of the culvert is



East Section

controlled by 2 - 12" irrigation crossings along the west side of the roadway and the invert elevations of the existing system. The proximity of the two pipes requires the Odelburg Drain Culvert to be an 36" RCP pipe to provide clearance between the two utilities.

South of C-1/2 Road, the 29 Road centerline profile is set at a minimum of 1.5 feet above the FEMA 100-year water surface elevation of the Colorado River. The 29 Road bridge design performed by Leigh Whitehead & Associates (Reference 10) reports a 100-year water surface elevation of 4587.5' (NAVD29 Datum). The City of Grand Junction GIS webpage reports a datum correction of 0.99 meters to get to NAVD88. However, reference marker RM45 listed on the FEMA map shows an elevation of 4587.36', but surveyed data for this same marker gives an elevation of 4592.98'. This is greater than the 0.99-meter datum correction. According to the surveyed shot on RM45, the datum correction is actually 1.71 meters, or 5.62 feet. Applying this correction to the 100-year water surface elevation of 4593.12. When this elevation is plotted on the City's 2-foot mapping, it very closely matches the 100-year FEMA floodplain mapping. Therefore, the design 100-year elevation for 29 Road is 4593.12.

Cross culvert and major drainage outfall design information is listed for all major drainage outfalls in Table 1.

DESIGN POINT	ROADWAY STATION	DESIGN FLOW (CFS)	STRUCTURE (SIZE, TYPE)	END TREATMENT	LENGTH (FT)	REFERENCE	ALLOWABLE HW ELEV.
175	Humpyard Drain 324+30	107	2' x 6' CBC	CONNECT TO EXISTING PIPE WITH 12'x4' JUNCTION BOX	124	CESP-Basin 14 Drainage District Facility	4588.4
176	Indian Wash 340+59	1030	20'x8' CBC	W/HEADWALLS & WINGWALLS RIPRAP AT OUTLET	75	FEMA FIS plus LOCAL FLOWS Match Existing Channel	4595.3
177	Indian Wash Access Driveway to 28 Rd	1030	2-10'x8' CBC	W/HEADWALLS & WINGWALLS RIPRAP AT OUTLET	32	FEMA FIS plus LOCAL FLOWS Match Existing Channel	4595.0
178	State Home Drain 354+00	7	24" RCP	EXISTING DITCH TO BE DRAINED AND FILLED PUT IN NEW PIPELINE	725	Drainage District Facility Verify irrigation & storm flows	. _
179	No Name Drain 390+60	12	24" RCP	REPLACE EXISTING CMP WITH RCP	146	29 Rd Drainage Report, June 1998 Hydrograph from DP 22-C	-
180	Odelburg Drain 109+80	47	36" RCP	NEW CULVERT W/RIPRAP SLOPE PROTECTION	115	29 Rd Drainage Report, June 1998 Hydrograph from DP 21-B	4593.12

TABLE 1 Cross Culvert Outfall Summary



East Section

The City Flood Insurance Study (FIS), as prepared by the Federal Emergency Management Agency (FEMA), tabulates peak discharges for Indian Wash.

The FIS documents a 100-year peak discharge of 890 cfs below the confluence of the 29-1/2 Road Channel. The corresponding drainage area is 15.3 square miles. Approximately 0.71 square miles of additional drainage area, below the 29-1/2 Road, was estimated to contribute to the Indian Wash Crossings. Using the SCS Method of analysis, a peak flow for this area was estimated to be 220 cfs. The soils in this area consists of BcS-Sagers Silty Clay Loam and is estimated to be a Type C Soil. As with the preliminary report, this flow was added to the flow of 890 cfs for a total flow of 1110 cfs. However, this value is very conservative because the additional area would increase the time of concentration, which would likely produce a lower peak than the 1110 cfs.

A second method was evaluated from the United States Geological Survey (USGS) publication, Analysis of the Magnitude & Frequency of Floods in Colorado. The method used from this publication estimates peak flow rates based on a ratio of the drainage areas with a known flow rate on the same stream. Results from this analysis produced a peak discharge of 916 cfs. This value reflected peak discharge rates from the FIS Report. For example, upstream from the Confluence with 29-1/2 Road, at U.S. Highway 6 and 24, a peak discharge of 820 cfs was determined from the FEMA Study. The corresponding drainage area for this peak is 13.5 cfs, a difference in drainage area of nearly 2 square miles when compared with the design point below the confluence of 29-1/2 Road. A comparison with these two locations indicates that there is roughly an additional 35 cfs added to the peak for every one square mile of additional area. The majority of the tributaries that contribute flow to Indian Wash are upstream from these locations, so this magnitude of additional flow would appear to be normal. When comparing this ratio to the additional 0.71 square miles of drainage added to the Indian Wash Crossings, an estimated additional flow rate added to the crossings would be 0.71(35 cfs) or 25 cfs. The estimated peak of 916 cfs produces an increase of 26 cfs, which compares well with the FEMA Study. For analysis, however, the two peak discharge rates of 1136 cfs and 927 cfs were averaged to produce a design peak discharge of 1013 cfs. The average peak was rounded to 1030 cfs. Averaging the two peak discharge estimates produces a conservative estimate and provides a small safety factor.

Indian Wash comprises two crossings. One at Riverside Parkway and a short distance downstream from the Riverside Crossing under a private driveway access. The crossings were analyzed using HY-8. For the Riverside Crossing, a 20' x 8' concrete box culvert with a grouted riprap flowline is proposed that will be placed on concrete pipe caps. Concrete pipe caps were chosen to provide access for a sanitary sewer through the structure, without compromising the sanitary sewer or the proposed structure. The caps also reflect the poor foundational soils in the area. At the private driveway access, $2 - 10' \times 8'$ concrete boxes will be placed to convey the peak discharge of 1030 cfs. For each crossing, outlet control governed the hydraulics. Outlet control appears to be a function of the fairly flat grade on Indian Wash, roughly 0.0031 feet per foot.



East Section

Riprap basins were incorporated at the outfall locations of the proposed concrete box culverts. A D_{50} of 9 inches was sized for these locations with a riprap depth of 1.5 feet.

Also in Section C, a new culvert will be placed under the roadway at the Humpyard Drain in accordance with the City Master Plan (Reference 5). The proposed culvert is a dual 29" x 45" elliptical pipe. The culvert was sized to avoid a 24" sewer line crossing in the roadway. The cover between the two utilities is minimal, therefore the 24" sewer line will be encased over the Humpyard Drain culverts.

The culvert outlets into a large junction box structure. A detail of this structure can be found in Appendix B. At this time this structure will release to an existing 15" storm sewer however, future provisions are such that a 48" outlet shall be added to the structure. The invert of the proposed 48" line shall be one foot higher than the 15" pipe invert to ensure that pipe flows full on occasion.

The final improvement to be made in Section C is to the State Home Drain. All open channel sections of the State Home Drain on the south side of the roadway east of 28 Road will be replaced with concrete pipe as part of the project. In addition, all portions of the drain that cross Riverside Parkway are improved to RCP.

Hydraulic calculations for each of the major drainageways is located in Appendix B.

V. CONCLUSIONS

A. <u>Compliance with Standards</u>

This report was prepared in accordance with the procedures and concepts outlined in the City of Grand Junction / Mesa County Stormwater Management Manual. Hydraulic Criteria as defined in the SWMM was adhered to, and no variances are required.

B. Drainage Concept

The drainage design allows the 2-year minor storm to be collected before adversely impacting street capacity. The roadway storm design conveys the minor storm event and the cross culvert design conveys the major storm event in a safe manner to protect adjacent property from damage.

Riverside Parkway



VI. IRRIGATION CONVEYANCE

A. <u>Overview</u>

Irrigation conveyance along the project corridor is a predominant feature to the proposed project. Irrigation conveyance is distributed by a number of headgates located on the Grand Valley Canal. The following headgate designations convey flow to share holders along the proposed project corridor.

- MC075
- MC072
- MC070
- MC060
- MC050
- MCO45
- MCO41

Conveyance from these headgates is a mixture of earth ditch flow, concrete ditch flow, and pipeline conveyance. Proposed construction will place the major ditch conveyances into a closed conduit. Where possible, existing pipelines will be maintained and corresponding structures adjusted to compensate for proposed fill elevations.

B. Irrigation Flow & Analysis

Flow for each lateral is quantified in terms of shares. Each recipient of irrigation flow owns a certain number of shares, which translates into a flow rate. The number of shares varies with each landowner depending on the amount of capital invested. Shares are converted to a volumetric quantity by the following two conversion factors:

- 1 share is equal to 4.48 gallons per minute.
- 1 gallon is equal to 0.134 cubic feet per second (cfs).

For analysis purposes, shares were converted to cubic feet per second. Conversion from share to cubic feet per second requires multiplying the share by 0.01 cfs.

Each lateral has a set number of shares that can be converted to a flow. However, this volume of water is often times smaller than historical flows taken from the canal. Flow information from the individual headgate locations was supplied from The Grand Valley Irrigation Company (GVIC). Flow rates are periodically measured at the turnout location on the canal for record keeping purposes. For design purposes, flow from the individual laterals was estimated, based on the number of shares. This flow rate was then compared to the GVIC value and the higher of the two was used for design. The Natural Resources Conservation Service (NRCS) recently designed the main conveyance for MC075. Flow, based on the number of shares, is 62 cfs.



East Section

The peak flow from this headgate, measured by the Grand Valley Irrigation Company, was 1.50 cfs. Consequently, the NRCS designed the system to convey the flow rate of 1.50 cfs.

Table 2 summarizes the flow rates associated with the different headgates.

ingation neaugate riow Rate Summary							
HEADGATE	SHARE FLOW	GVIC FLOW	DESIGN FLOW				
MC072	2.62	2.10	2.62				
MC070	2.16	2.61	2.61				
MC060	1.68	2.16	2.16				
MC050	0.45	2.00	2.00				
MC045	1.45	1.80	1.80				

TABLE 2 Irrigation Headgate Flow Rate Summary

Irrigation laterals were analyzed using the Hydraulic Program EPANet. EPANet computes hydraulic grade lines for pipes under high-pressure and low-pressure conditions. For each lateral, the required head was computed that would satisfactorily meet the delivery elevations. Typically, head elevations greater than 2 feet at the beginning of the system were considered to great. Analysis focused on providing an adequate hydraulic grade line with minimal energy input. Most of the lateral systems have the necessary elevation head to meet the delivery elevations, since they are currently being conveyed in existing ditches.

C. <u>Irrigation Lateral Summary</u>

LATERAL NO: 75

DESCRIPTION: Flow from MC075 is conveyed in a pipe that transports flow to D-Road (Approximate Station 340+13). Flow is redirected west until it crosses D-Road at Station 314+67. The Natural Resources Conservation Service designed this section of pipe for the D-Road Mutual Irrigation Company. Lateral crossings are located at Stations 314+67, 322+79, 327+89, 334+12, and 337+72.

At each crossing, flow is diverted from the main pipe via a gate valve that feeds a 45-degree dogleg pipe section that rises roughly 2 to 3 feet higher than the mainline flow elevation. The actual elevation varies based on the elevation of the existing pipe that the lateral connects to. At the end of the 45-degree dogleg section is an air vent to release air that might build at this high point and inhibit flow (*See Figure 2: MC075 TAKEOUT SECTION*).



East Section



HIGURE 2 MC075 Takeout Section

At each crossing, an attempt was made to match the existing alignment with new pipe. However, at certain locations, the road template was close to the main irrigation pipe. Consequently, if the air release valve were extended to the surface it would intersect the pavement section of the roadway. For these locations, the 45-degree dog leg and air release valve were moved to the other side of the road near the location where the proposed pipe connects to the existing pipe. Existing valves were maintained in their present location.

Flow in this lateral was modeled to determine if the existing system produced a hydraulic grade line that would meet the delivery elevations. Analysis indicates that there is sufficient energy to deliver the water to the existing locations. The D-Road Mutual Irrigation Company has indicated that the system has adequate pressure.

LATERAL NO: 72

DESCRIPTION: Flow from Headgate MC072 is conveyed to the State of Colorado Grand Junction Regional Center. The number of shares supplied from this lateral equals 262 or 2.62 cfs. All of these 262 shares belong to the Regional Center and are utilized for supplying water to irrigation sprinkler heads for lawn watering purposes. Water is pumped from a sump at Station 348+51.

From Headgate MC072, flow is transported south along the east boundary of the Regional Center. At approximate Station 359+18 (LT.), flow enters an existing irrigation access box and is redirected west through a 12 inch irrigation PVC Pipe, which parallels D-Road. Located along the existing 12-inch pipeline are concrete irrigation access boxes that are used as air vents. Near the end of the pipeline at approximate Station 348+51, flow enters a concrete access structure that provides a sump for a pump to draw flow from. Excess water flows out of

Riverside Parkway



the access box and flows under D-Road. A new irrigation access box is proposed. The existing structure is relatively large and would be difficult to reset. The proposed structure incorporates a standpipe to transport water from the inlet chamber to the pump out chamber. The existing pump will need to be reset.

Table 3 summarizes the existing and proposed components of Lateral 72.

Eatorative Summary							
STATION	OWNER	SHARES	EXISTING	PROPOSED	DESCRIPTION		
348+51.47	Regional Center	262	Access Box	Irr. Access Box	Irr. access box is 7'x4' and provides a place for pump to draw water from.		
349+64.65	Regional Center	262	Concrete Air Vent	2" Air Vent			
352+33.48	Regional Center	262	Access Box	Irr. Access Box	Provides access to system		
357+30.92	Regional Center	262	Access Box	2" Air Vent			
359+18.07	Regional Center	262	Access Box	Irr. Access Box	Redirects flow to the west.		

7	TABI	LE	3
Lateral	70 -	S	ummary

LATERAL NO: 70-W & 70-S

DESCRIPTION: Flow from Headgate 70 encompasses 216 shares. Flow is conveyed in a southerly direction within a concrete ditch to Station $367+40^{+/-}$. Flow enters a diversion structure that diverts approximately 50 shares across D-Road and west within a concrete ditch. The remaining 166 shares is diverted across D-Road, where it enters a concrete ditch and flows south to C $\frac{1}{2}$ Road (Lateral 70-S). Conveyance along D-Road is transported in a concrete ditch. Proposed for this lateral is to place the flow into a pipe with irrigation diversion boxes and irrigation access boxes placed where necessary to provide turnout for existing irrigation flow.

Lateral 70-W, as shown on the plans, conveys flow west. Proposed conveyance consists of a 12-inch PVC SDR-26 irrigation pipe. The proposed pipe will connect to the existing diversion box at Station 376+40. The existing irrigation diversion box is complex in function, therefore it was determined to not remove this structure, but rather utilize it for the proposed design. The condition of the structure appears to be in good condition and should function well over the life of the project. Upon crossing the proposed Riverside Parkway alignment, water enters an irrigation diversion box. Twenty shares (20) are diverted south to a 4-foot diameter sump that provides irrigation water for residences on 398 and 399 Evergreen Road. The remaining 30 shares are diverted west. It should be noted that there are occasions when the 20 shares are not diverted south to the 4-foot sump, but rather conveyed west in the 12-inch pipe.

Riverside Parkway



Table 4 summarizes the existing and proposed components of Lateral 70.

TABLE 4							
Lateral 70- Summary							
STATION	OWNER	SHARES	EXISTING	PROPOSED	DESCRIPTION		
354+84.79	Darter LLC	8	6" Slide Gate Off of Concrete Ditch	Irr. Diversion Box w/ 6" access to existing pipe.	Irr. Diversion Box will divert flow south to Darter LLC and allow flow to enter existing concrete ditch to the west for Cooper & Tucker. Dry Well Drain proposed.		
356+89.76	Darter LLC	8	Assume 6" Slide Gate	Irr. Diversion Box	At present, feeds a disconnected pump set, but may be used in future.		
358+86.43	Farley	4	Assume 6" Slide Gate	Irr. Diversion Box	Existing gate plugged field inspection.		
360+51.07	Farley	4	Assume 6" Slide Gate	Irr. Access Box	Reset Pump		
363+12.28	Cardin	1	Sump for Pump	Irr. Access Box	Reset Pump		
361+31.50	Cardin	1	Slide Gate	Need to Add Irr. Diversion Box	Add To Plans		
367+30.30	398 & 399 Evergreen	20	Slide Gate	Irr. Diversion Box	Allows flow to be diverted south to 4" diameter sump. Feeds Pine Estates.		

LATERAL NO: 60

DESCRIPTION: Flow from Headgate 60 encompasses 168 shares. Flow is conveyed in a southerly direction within an earth lined ditch to Station 387+25.62^{+/-}. Flow enters a concrete access structure and is directed under D-Road. Upon crossing D-Road, flow is directed west where shareholders draw water from the concrete irrigation ditch. Proposed for Lateral 60 is to place the current concrete ditch conveyance into a pipeline.

Riverside Parkway



Table 5 summarizes the existing and proposed components of Lateral 60.

TABLE 5						
		Ĺ	Lateral 60- Sun	nmary		
STATION	OWNER	SHARES	EXISTING	PROPOSED	DESCRIPTION	
377+37.48	White Willow Subdivision	10	Irr. Diversion Box w/ 10" pipe diversion south. Excess water is wasted west along earth ditch.	Irr. Diversion Box w/ 10" access to existing pipe. 12" outfall to the west for waste.	Proposed earth ditch to tie into existing waste ditch.	
380+53.79	Skyler Subdivision	25	4" Slide Gate	Irr. Diversion Box	Connect existing 4" pipe that feeds sump for pump.	
383+95.00	Jenson	14	Siphon Irrigation	Turnout Section	May replace with Irr. Diversion Box so Owner has greater control of diversion to gated pipe. (See Note 1)	
385+54.34	Jenson		Siphon Irrigation	Turnout Section	May replace with Irr. Diversion Box so Owner has greater control of diversion to gated pipe. (See Note 2)	
386+30.00	Powell	3	Pump set	Irr. Access Box	Reset Pump: Owner can pump from Irr. Access Box	
387+17.74	Powell		Concrete pan/sump	Irr. Diversion Box	Flow is diverted south to 6" gated pipe. Remaining flow is diverted west.	

Note 1: Owner will connect to 10" gated irrigation pipe (320 L.F.) to be furnished as part of r/w negotiation. Note 2: Owner will connect to 8" gated irrigation pipe (130 L.F.) to be furnished as part of r/w negotiation.

LATERAL NO: 50-S & 50-W

DESCRIPTION: Flow from Headgate 50 encompasses 44 shares. Flow is conveyed in a southerly direction within an earth-lined ditch to Station 393+22.27^{+/-}, where flow is split between southerly and westerly conveyance.

Riverside Parkway



Table 6 summarizes the existing and proposed components of Lateral 50.

TABLE 6								
	Lateral 50- Summary							
STATION	OWNER	SHARES	EXISTING	PROPOSED	DESCRIPTION			
	water en				in a de Ceneral de Color de Line			
143+28.86			Ditch	15" Headwall w/ Trash Guard. Begin pipe conveyance	Existing conveyance is earth ditch. Proposed will be to have ditch flow transition to pipe flow.			
138+05.83 = 393+22.27 Lateral 50-S		44	Diversion plate	Irr. Diversion Box	Irr. Diversion Box will Divert flow west (50-W) and south (50-S).			
134+75.06	Berry	9	Diversion plate	Irr. Diversion Box	Divert flow from Irr. Diversion Box to field. At Stations 131+75.58 & 128+33.87, 12"x8" Tees & 8" End Cap Plug for future use.			
125+41.78		18	Ditch	Lateral Outfall w/ Dry Well Drain	Flow will transition from pipe conveyance to ditch conveyance.			
390+59.41	Grillos	2	Irr. Diversion Structure feeds Ditch (12" Pipe)	Irr. Diversion Box	Flow is diverted into a 12" PVC SDR 26 (Approximately 160 L.F.) Will connect to existing ditch.			
390+04.17	Johnson	1	frr. Diversion Structure	Irr. Diversion Box	Flow is diverted south and west. Also placed approximately 4 yard taps to water.			
388+56.94	ljams	1	Irr. Access Box	Irr. Access Box	Reset pump.			
388+06.80	Rodriguez	0	Ditch	Yard Tap	Tree Watering			

LATERAL NO: 45

DESCRIPTION: Flow from Headgate 45 encompasses 149 shares. Flow is conveyed in a southerly direction within an earth lined ditch to Station 144+99.33, where flow transitions from ditch flow to pipe flow.

Riverside Parkway



Table 7 summarizes the existing and proposed components of Lateral 45.

Lateral 45- Summary						
STATION	OWNER	SHARES	EXISTING	PROPOSED	DESCRIPTION	
					For A marked and a second second	
144+94.26	Stephens	***	Irr. Access Box	Irr. Access Box		
143+52.85	Oakes		Ditch Inlet	Small Area Inlet	Intake ditch runoff	
142+74.45	Oakes		Irr. Access Box	Irr. Access Box	Reset pump	
139+52.00	Feuerborn	****	Irr. Access Box	Irr. Access Box	Pump outlet	
124+73.5		93	Irr. Diversion	Irr. Diversion Box	Divert 43 shares west and 50 shares south.	
124+59.48		50		Irr. Diversion Box	Divert 50 shares cross 29 road. Also provide waste runoff.	
118+92.50		50	Irr. Diversion Box	Connect to existing diversion box		

TABLE 7

LATERAL NO: 40

DESCRIPTION: Flow along MC040 is conveyed from the east to west, intersecting 29 Road at approximate Station 98+90. Water is conveyed west under 29 Road or south, along the east side of 29 Road. Water that is conveyed west under 29-Road enters a ditch and flows south to a pump. Flow continues south along the ditch to approximately Station 93+04, where the ditch changes in direction to the west.

Flow that is not diverted west at Station 98+90 is diverted south along 29 Road to Station 95+20, where it crosses 29 Road and enters a ditch where it is conveyed south and eventually west.

Proposed changes to this existing system include connecting to the existing 8" irrigation pipe at Station 98+90, approximately 21 feet east of the existing pipe that parallels 29 Road. The existing pipe was within the proposed road template, so it needed to be relocated. At station 96+49.08 an 8" pipe is connected into the main line and feeds an irrigation access box located at station 96+47.15. The existing pump will be reset to this location. Valve boxes and flow meters will be reset to the new pipe location.

Riverside Parkway



D. <u>Summary</u>

Within the proposed irrigation diversion boxes, gates and weirs will require adjustments to correlate the demand with existing diversion. Irrigation diversion boxes have a headgate for adjusting as well as wood planks to develop water surface elevations within the structure to meet turnout elevations. The planks also act as weir flow. Rather than design a system that was fixed with no adjustments, it was determined to provide a dynamic system that provides a means of regulating and adjusting the flow to meet share holder needs.

At the end of laterals, where flow transitions from pipe flow to ditch flow, an alfalfa valve will be installed, which will provide access to the surface. There will be a dry well drain installed at these locations, so the pipe can be drained during the winter.

Riverside Parkway



VII. REFERENCES

- 1. <u>Stormwater Management Manual</u>; City of Grand Junction/Mesa County; May, 1996.
- 2. <u>Urban Storm Drainage Criteria Manual</u>; Vol. 1, 2& 3, DRCOG, Denver, Colorado, June 2001 (with revisions).
- 3. *Drainage Design Manual;* Colorado Department of Transportation; 1995.
- 4. <u>Grand Valley Stormwater Management Master Plan;</u> Grand Junction Drainage District, Mesa County, City of Grand Junction; Dec. 1998 (Updated May 2000).
- 5. <u>Combined Sewer Separation & Stormwater Management Master Plan;</u> City of Grand Junction; Dec. 1998 (Updated May 2001).
- 6. *Final Drainage Report for the Grand Mesa Center;* G.R. Williams Engineering, Inc.; March 2001.
- 7. *Final Drainage Report for the Rimrock Marketplace, Grand Junction, Colorado;* Rolland Engineering, Inc.; Jan. 15, 2002 (Rev. May 9, 2002).
- 8. *Final Drainage Report (for the) Charlene Giebler Community Ice Facility, 2515 River Road;* Blythe Design & Co.; Aug. 2003.
- 9. *Flood Insurance Study, City of Grand Junction, Colorado;* Federal Emergency Management Agency; Revised July 1992.
- 10. <u>Preliminary Drainage Report and Plan and Hydraulics Study for 29 Road;</u> Leigh, Whitehead & Assoc.; June 1998.
- 11. <u>Analysis of the Magnitude and Frequency of Floods in Colorado;</u> Water Resrouces Investigation Report 99-4190, United States Geological Survey, 2000.

J:_Transportation\071514.402\manage\report\Riverside Parkway Drainage Design Report.doc

Riverside Parkway

action.

And the second se



Appendix A

Hydrologic Computation Summary





states

Service and service servic

an-1001-100

(100 million)

Description:	Inlet Hydrology and Capacity Analysis
	for City of Grand Junction Intet Types
Methodology:	UDFCD Rational Method (1984), and
	FHWA "Drainage of Highway Pavements" HEC No. 12
Template by:	George K. Cotton, PE
	Carter & Burgess, Inc.
Last Revised:	1-Jul-05

Inlet Location:

ID No.: DP+100			
Station: 25+14,0	Grate Elevation	n: 4588.01	
Offset: 48,00 L	t WS EI @ Grate	e: 0.00	
Inlet Hydrology:			
Area Paved:	0.53 Ac	C _{paved} :	0.93
Area Unpaved:	0.00 Ac	Curpav:	0
Total Area:	0.53 Ac	C _{comp} :	0.93
Overland Length:	44.0 ft	Sover:	2.00%
Channel Length:	472.0 ft	H:	0.5
Overland C (5-yr):	0.93 Cł	annel Type:	1
$t_i =$	1.61 minutes	t, =	12.50 minutes
t _{c =}	12.9 minutes	$t_{o_check} =$	12.9 minutes
One-hour Rainfall	0.34 inches	Freq.	2 yr.
Rainfall Intensity	0.84 in/hr	• ******	******
Discharge:	0.41 cfs		
Upstream Flowby:	cfs		
Total Inlet Q:	0.41 cfs		

Description:	Inlet Hydrology and Capacity Analysis					
	for City of Grand J	unction Inlet Types				
Methodology:	UDFCD Rational M	tethod (1984), and				
	FHWA "Drainage of	of Highway Pavements"	HEC No. 12			
Template by:	George K. Cotton,	PE				
	Carter & Burgess,	Carter & Burgess, Inc.				
Last Revised:	: 1-Jul-05					
Inlet Location:	****					
ID No.:	DP+101					
Station:	25+14.00	Grate Elevation	n: 4588.01			
Offset:	48.00 Rt	WS EI @ Grate	e: 0.00			
Inlet Hydrology:						
Area Pave	d:	0,53 Ac	C _{paved} :	0.93		
Area Unpa	ved:	0.00 Ac	C _{unpav} :	0		
Total Area	•	0.53 Ac	C _{comp} :	0.93		
Overland L	_ength:	65.0 ft	S _{over} :	2.00%		
Channel Lo	ength:	463.0 ft	H:	0,5		
Overland (C. (5-yr):	0.93 CI	hannel Type: 💹	1		
t, =		1.96 minutes	$t_t =$	12.22 minutes		
t _{e =}		12.9 minutes	t _{c_check} =	12.9 minutes		
One-hour l	Rainfall	0.34 inches	Freq.	2 yr.		
Rainfall Int	tensity	0.84 in/hr				
Discharge:	1	0.41 cfs				
Upstream I	Flowby:	cfs				
Total Inlet	Q:	0.41 cfs				

1

ï





Antonio antonio

611-01-0-000

100- 10000112

a la constanta da constante da co

and a second second

e-contractive of the

and the second s

41 Contractor South of the State

ge/AnnuesandAct

THEET DEVICE OREVVERTION	
Description: Inlet Hydrology	and Capacity Analysis
for City of Gran	nd Junction Inlet Types
Methodology: UDFCD Ration	nal Method (1984), and
FHWA "Draina	age of Highway Pavements" HEC No. 12
Template by: George K. Cott	ton PF
Carter & Burge	ass Inc
Last Revised: 30-Jun-05	
Inlet Location:	
ID No.: 221+1	
Station: 22+76.60	Grate Elevation: 1500 FF
Offect: 11.8011	WS EL@ Croto: 4500.62
Inlet Hydrology:	WO LI @ GIALE. 4090.02
Area Daved:	040 Ac C ::::::002
Area Innoved	D TO AC Opaved. U Sa
Alea Olipaved:	Unpav. U
Total Area:	$0.10 \text{ AC} \qquad C_{\text{comp}}: \qquad 0.93$
Overland Length:	43.0 ft S _{over} : 2.00%
Channel Length:	186.0 ft H: 0.5
Overland C (5-yr):	0.93 Channel Type:
$t_i =$	1.59 minutes $t_t = 4.26$ minutes
t _{c =}	5.9 minutes $t_{c_{check}} = 11.3$ minutes
One-hour Rainfall	0.34 inches Freq. 2 yr.
Rainfall Intensity	1.07 in/hr
Discharge:	0.10 cfs
Upstream Flowby:	0.00 cfs
Total Inlet Q:	0.10 cfs
Gutter Hydraulics:	
Slope:	0.50% Street Roughness: 0.016
Cross-Slope:	2.00% Spread (allowable): 9.50 ft
	Actual Spread: 3.47 ft
Gutter Capacity:	1.47 cfs
Actual Depth:	0.07 ft
Actual Velocity:	0.83 ft/s
Inlet Hydraulics:	
Class: G	(G - on grade or S - sag)
Inlet Type: C	(G - grate, O - curb opening, C - combined)
WSEL @ Grate: 4590.62	
Combined:	
Type:	Single Standard
U/S Curb Opening:	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
Length:	3.0 ft Lt: 5.82
Blockage:	100% E: 0.00%
D/S Grate:	••••••••••••••••••••••••••••••••••••••
Grate:	L 4 Eo: 0.78
Width:	1.5 ft Rf: 1.00
Lenoth:	3.0 ft Rs: 0.19
Blockage	63% F [.] 82 16%
Vo	3.9 fps 0.0 caught: 0.08 cfs
E=	82.16% IQ flowby' 0.02 cfs



April 1 - 1 - 1 - 1 - 1 - 1 - 1

dinakaanan d

(0000 to the transformed

-datatatikanidele

AND Annual Control of the Annual Control of

And the second s

ACCUMUNTARY AND

artitizzatikkizzatikka v · · · · · · · · · · · · ·

anteriorationalization

pri-usianation-ing

Al-manadalanan se

Description: Intet Hydrolog	y and Capacity Analysis		
for City of Gr	and Junction		
Methodology: UDFCD Ratio	onal Method (1984), and		
FHWA "Drain	nage of Highway Pavemen	ts" HEC. No., 12	
Template by: George K. Co	otton, PE		
Carter & Burg	jess, Inc.		
Last Revised: 30-Jun-05			
Inlet Location:			
ID No.: 2561-1	*		
Station: 256+22 10	Grate Elevati	on: 4573.46	
Offset: 24.50 L	WS EI @ Gr	ate: 4573.56	
Inlet Hydrology;	** ···· • • • • •		
Área Paved:	0.34 Ac	Conved:	0.93
Area Unnaved	0.00 Ac	C	ĥ
Total Area	0.34 Ac	C.	n 93
Overland Length:	207 70 200 A	Comp.	2000
Channel Longth	5010 IL 521 0 0	U.	6.UU/0
Overland C (5. un)	0.02		C.D
t =	0.90 1.50 minutor	Channel Type.∭	14.20 minutes
ų — +		s 4 –	14.32 minutes
	13.2 minutes	S lc_check =	13.2 minutes
		Freq.	∠z yr.
Raintali Intensity	0.83 m/nr		
Discharge:	U.26 CIS		
Total lolat O			
Fotal milet Q:	0.26 CTS		
Sienes	0 P0% 01	-1 Danaharan 3	
Sidpe.	0.00% Stre	et Rougnness:	0.016
Cross-Stope:	2.00% Spre	ad (allowable):	9.50 ft
Cutton Consolity	4 47 -4-	Actual Spread:	4.99 ft
Actual Danthy	1.47 CIS		
Actual Velocity	0.10 π		
	1.06 17/5		
Intel Hydraulics:	(0		
Class: G	(G - on grade or S	- sag)	
	(G - grate, O - curt	opening, C - coi	nbined)
VVSEL @ Grate: 45/3.56	1		
Type:	Single Standa	d	
Lengin:	υ.υ.π 1008		8.73
Biockage:	1 1	E:	0.00%
]		0.00
Grate:	L 4	E0:	0.62
vviatn:	1.5 Π	KI:	1.00
Length:	3:0 ft	Ks:	0.13
Blockage:	53%	E:	66.67%
<u>Vo:</u>	39 fps	Q caught:	0.17 cfs
E _{net} =	66.67%	Q flowby:	0.09 cfs



Children and Child

ihinanan in the

NATION CONTRACTOR

2002/04/2010/04

with the second state of the second s

shired thereafty

childrend@million.cg

yourned a

și constanți constanți

presentatudetternes

Description: Inlet Hydrolo	y, and Capacity Analysis				
for City of G	and Junction				
Methodology: UDFCD Rat	Methodology: UDFCD. Rational Method (1984), and				
FHWA "Drai	FHWA "Drainage of Highway Pavements", HEC No. 12				
Template by: George K. C	George K. Cotton, PE				
Carter & Bur	jess, Inc.				
Last Revised: 30-Jun-05					
Inlet Location:					
ID No.: 2651-1					
Station: 265+51.0	Grate Elevation	· 1576 10			
Offset 24.50 l	t WS EL@ Grate	· <u>4575 22</u>			
Inlet Hydrology:		·. +0/J.ZZ			
Area Paved	047 Ac	c ·	0.02		
Area Unpayed:		Opaved-	0.00		
Total Area:		C _{unpav} .			
Total Alea.	0.47 AC	Comp.	0.93		
Overland Length:	37.U ft	S _{over} :	2.00%		
Channel Length:	343.0 ft	H:	0.5		
Overland C (5-yr):	0.93 Ch	iannel Type:	1		
$t_i =$	1.48 minutes	t, =	8.64 minutes		
t _{c =}	10.1 minutes	$t_{c_check} =$	12.1 minutes		
One-hour Rainfall	0.34 inches	Freq.	2 yr.		
Rainfall Intensity	0.92 in/hr				
Discharge:	0.40 cfs				
Upstream Flowby:	0.00 cfs				
Total Inlet Q:	0.40 cfs				
Gutter Hydraulics:					
Slope:	0.50% Street	Roughness:	0.016		
Cross-Slope:	2.00% Spread	(allowable):	9.50 ft		
	Ac	tual Spread:	5.84 ft		
Gutter Capacity:	1.47 cfs				
Actual Depth:	0.12 ft -				
Actual Velocity:	1.17 ft/s				
Inlet Hydraulics:					
Class: S	(G - on grade or S - s	ag)			
Inlet Type: C	(G - grate, O - curb o	pening, C - comb	pined)		
WSEL @ Grate: 4575.23	2				
Combined:					
Туре:	Single Standard				
U/S Curb Opening	;				
Length:	3.0 ft	Lt:	10.42		
Blockage:	100%	E:	0.00%		
D/S Grate	•				
Grate:	Ç 4	Eo:	0.55		
Width:	1.5 ft	ືRf:	1.00		
Length:	3.0 ft	Rs:	0.14		
Blockage:	59%	E: 6	0.99%		
Vo:	3.9 fps	Q caught:	0.24 cfs		
E _{net} =	• 60.99%	Q flowby:	0.16 cfs		



and the second states

ALTON COMPANY IN

Sitteration and

olitecture and

With the second s

standard and standards

A rest of the second second

INLET DESIGN CALCULATION			
Description:	Inlet Hydrology and Capacity Analysis		
	for City of Grand Junction Inlet Types		
Methodology:	UDFCD Rational Method (1984), and		
	FHWA "Drainage of Highway Pavements". HEC No. 12		
Template by:	George K. Cotton, PE		
	Carter & Burgess, Inc.		
Last Revised:	30-Jun-05		

Inlet Location:

ID No.: 2741-1				
Station: 274+80	Grate Eleva	ation: 4575,27		
Offset: 24.50 Li	WS EI @ G	Grate: 4575.39		
Inlet Hydrology:	~ —			
Area Paved:	0.61 Ac	C _{paved} :	0.93	
Area Unpaved:	0.00 Ac	Curpav:	0	
Total Area:	0.61 Ac	C _{comp} :	0.93	
Overland Length:	37.0 ft	S _{over} :	2.00%	
Channel Length:	504.0 ft	H:	0.5	
Overland C (5-yr):	0.93	Channel Type:	1	
t, =	1.48 minut	$t_t = t_t$	13.48 minutes	
t _{c=}	13.0 minut	es t _{c_check} =	13.0 minutes	
One-hour Rainfall	0.34 inche	s Freq.	2 yr.	
Rainfall Intensity	0.83 in/hr			
Discharge:	0.47 cfs			
Upstream Flowby:	0.00 cfs			
Total Inlet Q:	0.47 cfs			
Gutter Hydraulics:	****			
Slope:	0.50% St	reet Roughness:	0.016	
Cross-Slope:	2.00% Spi	read (allowable):	9.50 ft	
		Actual Spread:	6.22 ft	
Gutter Capacity:	1.47 cfs			
Actual Depth:	0.12 ft			
Actual Velocity: 1.22 ft/s				
Inlet Hydraulics:				
Class: S	(G - on grade or	S - sag)		
Inlet Type: C	(G - grate, O - cu	irb opening, C - c	ombined)	
WSEL @ Grate: 4575.39		,		
Combined:		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
	Single Stand	lard		
U/S Curb Opening:				
Length:	3.0 11	Lt:	11.18	
Biockage:	100%	E:	0.00%	
D/S Grate:]	·····	0.50	
	ц 4 1 с		0.52	
vvigtn:	1.0 N	RC Dec	1.00	
Lengin: Diseksas	0.U I(E00/		U.13 E9.00%	
Biockage:	38%0 0.0 fra		0.00	
V0:	a s tps	Caught:	U.28 CIS	
E _{net} =	00.30%	La llowby:	U.ZU CIS	

.


Substance and the second

All-All Andrew States

and with the second sec

Antibiotic description of the second second

Alternation and

sdatabaataba

dimentioned by

sidente and the second s

community.com

......

Description: Inlet Hydrology	and Capacity Analysis						
for City of Gran	d Junction Inlet Types						
Methodology: UDFCD Ration	al Method (1984), and		-				
FHWA "Draina	FHWA "Drainage of Highway Pavements". HEC No. 12						
Template by: George K. Cott	on, PE						
Carter & Burge	ss. Inc.						
Last Revised: 30-Jun-05							
Inlet Location:							
ID No.: 2831-1							
Station: 283+05.00	Grate Flevation	4575 32					
Offset: 24 50 Lt	WS EI @ Grate:	4575.53					
Inlet Hydrology:		1010.00					
Area Paved:	0.91 Ac	Cover:	0.93				
Area Unpaved:	0.00 Ac	C	ß				
Total Area	0.91 Δc	C ·	0.03				
Overland Length:	0.01 A0	C omp.	0.00				
Channel Longth:	000 A A		2.VU /0 0 F				
	0000 II 0.02 Ch		U.0				
t =	2.72 minutos	anner type.	25 77 minutos				
ц — ŧ	2.72 minutes	4 –	25.77 minutes				
lc= One hour Deinfelt	15.0 minutes		15.6 minutes				
	0.77 in the	Freq.	2 yr.				
Raiman intensity	U.77 In/nr						
Discharge:	0.05 CIS						
Total lalat O							
Fotal met Q:	0.65 CIS						
	O Chroad I	D avarba a a a 3	ANDA				
Slope.		Cougnness:					
Cross-Stope.	o.uu% Spread	(allowable):	5.00 π				
Cuttor Canacity	ACL 4 CC of a	ual Spread:	3.53 N				
Actual Dopthy							
Actual Depth.							
Inlet Hydrautice:	1.75 105						
Class: S	(C on grado or C o	oc)					
Inlet Type: C	(G - on grade of S - Si (G - grate O - curb or	ay) Soning C som	hinod)				
WSEL @ Crate: 4575.53	(G - glate, O - cuip of	Jenning, C - Con	inilied)				
Combined:							
Type:	Single Standard						
1//S Curb Opening:	olligic otalidate						
Length:	2 /1 ff	1.+-	6.62				
Blockage:	100%	E(. E·	0.02				
D/S Grate		L.	0.0070				
Grate:	Сл	Έο·	0.77				
Midth	1 F. ft	_ة يار. Rf	1.00				
f Anath	20 ft	Re ¹	0.10				
Blockage:	400 N	E.	0.13 81 54%				
Vor	2 Q fne		0.53 ofc				
vo. E =	81 54%	O flowby:	0.00 015				
⊾net ¬	U 1.UT /0	l≪ uowny.	V. 12 UIS				



dillourneledwydde

Nutration address

District and a second second

dollarganger of

Affection and the

1000466695004555555

subbergeringeringer

Accession and the second

070711111111111111

1979)1091417-1030y

shinghood and so that the second s

1997, 1997, 1998, 1998, 1998, 1998, 1998, 1998, 1998, 1998, 1998, 1998, 1998, 1998, 1998, 1998, 1998, 1998, 19

TEL DEGIGI CALOULATION			
Description: Inlet Hydrology ar	nd Capacity Analysis		
for. City of Grand	Junction Inlet Types		
Methodology: UDFCD Rational	Method (1984), and		
FHWA "Drainage	of Highway Pavemer	ts". HEC No. 12	
Template by: George K. Cotton	, PE		
Carter & Burgess	, Inc.		,
Last Revised: 30-Jun-05			
Inlet Location:			
ID No.: 2901-1			
Station: 290+26 80	Grate Flevat	ion: 4581.53	
Offset: 26 50 Rt	WS FL@ Gr	ate: 4581.64	
Inlet Hydrology:			
Area Paved:	037 Ac	C	0.93
Area Unnaved	Ac	⊂pavea. C	
Total Area	0.37 Δc	C.	0.03
Overland Length:	0.07 AC	C ·	0.00
Channel Longth:	2010 0	Uover.	2.0070
	0.02		0.0
	0.95 1.40 minuto	onamer rype. _∭	7.52 minutos
. ų— +	P.40 minute	5 y-	1.52 minutes
	0.9 minute	5 L _{c_check}	11.9 minutes
	0.34 mcnes	Freq.	∠ yr.
Raman mensity	0.96 m/nr		
Linetroom Flowbyr	0.33 CIS		
Total lalat O			
Cuttor Undraution	0.33 CIS		
		at Daughuagas 🎆	
Siope.		et Rougnness:	0.010
Cross-Slope.	a shie	au (allowable).	9.50 1
Cuttor Consoliur	4.25	Actual Spread;	5.60 π
Guiler Capacity:	1.35 CIS		
Actual Velocity	0.11 IL 4.40 #/o		
Actual Velocity.	1.10 105		
) on grada av C		
Class, G (G	s - on grade or S	- sag)	na h in a dì
MOREL @ Croto: 4591.64	e - grate, O - cun	b opening, C - co	mbinea)
Combined			
	Cinala Olanda		
	ongle otaliga	iu.	
UIS Curb Opening.	004	1 4.	0.90
Lengen. Blockago:	0.0 H 1000		9.09
	10070	C .	0.00%
		Eat	0.50
	ь 4 в г 4		0,00
VVIQEN:	5 I G I	KI.	1,00
Length:	्र U त ००००	KS:	0.12
вюскаде:	03%		0.00
V0:	a y tps	La caught:	0.20 cts
E _{net} =	01.05%	I W TIOWDY:	0.13 cts





Description:	ption: Inlet Hydrology and Capacity Analysis						
	for City of Grand Junction Inlet Types						
Methodology:	y: UDFCD Rational Method (1984), and						
	FHWA "Drainage of	FHWA "Drainage of Highway Pavements", HEC No. 12					
Template by:	George K. Cotton, F	ΡE					
	Carter & Burgess, Ir	1 C .					
Last Revised:	30-Jun-05						
Inlet Location:	******						
ID No.:	295P-1						
Station:	295+94.00	Grate Elev	ation:	4588.01			
Offset:	42.50 Rt	WS EI @ Grate: 0.00					
Inlet Hydrology:							
Area Pave	d:	1.72 Ac		C _{paved} :	0.93		
Area Unpa	ved:	0.00 Ac		C _{unpav} :	0		
Total Area	•	1.72 Ac		C _{comp} :	0.93		
Overland L	_ength:	241.0 ft		S _{over} : §	2:00%		
Channel Le	ength:	272.0 ft		H:	0.5		
Overland 0	C (5-yr):	0.93	Chan	inel Type:	1		
t, =		3.77 minu	tes	$t_t = $	6.61 minutes		
t _{c=}		10.4 minu	tes	$t_{c_check} =$	12.9 minutes		
One-hour l	Rainfall	0.34 inche	es F	Freq.	2 yr.		
Rainfall Int	ensity	0.91 in/hr		- · · ·	******		
Discharge:		1.45 cfs					
Upstream I	Flowby:	cfs					
Total Inlet	Q:	1.45 cfs					

while does be edd

Childen and a short a s

Abriance daged (4 ft

Compro-Margare

Sold and a second

Substanting.

Alternation and the second sec

-competences

Description:	Inlet Hydrology and Capacity Analysis
	for City of Grand Junction Inlet Types
Methodology:	UDFCD Rational Method (1984), and
	FHWA "Drainage of Highway Pavements". HEC No. 12
Template by:	George K. Cotton, PE
	Carter & Burgess, Inc.
Last Revised:	30-Jun-05

Inlet Location:

ID No.: 299P-1			
Station: 299+76.80	Grate Elevation: 4	588.01	
Offset: 42.50 R	WS EI @ Grate:	0.00	
Inlet Hydrology:			
Area Paved:	1.32 Ac	C _{paved} :	0.93
Area Unpaved:	0 00 Ac	Cunpay:	0
Total Area:	1.32 Ac	C _{comp} :	0.93
Overland Length:	227.0 ft	Sover:	2.00%
Channel Length:	211.0 ft	H:	0.5
Overland C (5-yr):	0.93 Channe	l Type: 💹	1
t, =	3.66 minutes	t, =	4.93 minutes
t _{c =}	8.6 minutes	t _{c_check} =	12.4 minutes
One-hour Rainfall	0,34 inches Fre	q. 🗰	2 yr.
Rainfall Intensity	0.97 in/hr		*
Discharge:	1.19 cfs		
Upstream Flowby:	cfs		
Total Inlet Q:	1.19 cfs		

Description: Ir	niet Hydrology and	Capacity Analy	sis			
fo	for City of Grand Junction Inlet Types					
Methodology: L	UDFCD Rational Method (1984), and					
F	HWA "Drainage o	of Highway Pave	ments". HEC	No. 12		
Template by: G	Beorge K. Cotton,	PE				
, , ,	arter & Burgess.	Inc.				
Last Revised: 3	0-Jun-05					
Inlet Location:						
ID No.:	302P-1					
Station:	302+97,50	Grate Ele	vation:	4588.01		
Offset:	42.50 Rt	WS EI @	Grate:	0.00		
Inlet Hydrology:		-				
Area Paved:	: 🦷	2 47 Ac		C _{paved} ;	0.93	
Area Unpav	ed:	0.00 Ac		Cunpav;	0	
Total Area:	-2000/00	2.47 Ac		C _{comp} :	0.93	
Overland Le	ngth:	120.0 ft		Sover:	2.00%	
Channel Ler	ngth:	612.0 ft		H:	0.5	
Overland C	(5-yr):	0,93	Chann	el Type:	1	
t _i =		2.66 min	utes	t, =	16.87	minutes
t _{c=}		14.1 min	utes	t _{c_check} =	14.1	minutes
One-hour Ra	ainfall 🛛 💹	0.34 inct	ies Fre	əq. 🛛 🎆	2	yr.
Rainfall Inte	nsity	0.81 in/h	r	• • • • • • • • • • • • • • • • • • • •	******	-
Discharge:	-	1.85 cfs				
Upstream Fl	lowby:	cfs				
Total Inlet Q	- ::	1.85 cfs				





dualities and defined

Abide Tomologia (

AMM-Manager

diministration of the second s

Water-control

gaaaadinaatiin.

solling and a second second

www.zerX

Description: Inlet Hydrol	ogy and Capacity Analysis					
for City of C	Frand Junction Inlet Types					
Methodology: UDFCD Ra	tional Method (1984), and					
FHWA "Dr	inage of Highway Pavement	s". HEC No. 12				
Template by: George K.	Cotton, PE					
Carter & Bu	Carter & Burgess, Inc.					
Last Revised: 30-Jun-05						
Inlet Location:						
ID No.: 3041-2						
Station: 2014-50	10 Croto Elovati	on: 1590 D4				
Offect: 720		VII. 4600.01				
		lle: 4000.14				
Area Daved		с . 				
Area Faveo.	0.00 AC	C _{paved} .	0.93			
Area Unpaved:	U UU AC	C _{unpav} :	0			
Total Area:	0.66 Ac	C _{comp} :	0.93			
Overland Length:	34.0 ft	S _{over} :	2.00%			
Channel Length:	952.0 ft	H:	0.5			
Overland C (5-yr):	0.93	Channel Type: 💹	1			
t, =	1.42 minutes	; t _i =	28.11 minutes			
t _{e ≕}	15.5 minutes	; t _{c check} =	15.5 minutes			
One-hour Rainfall	0,34 inches	Freq.	2 vr.			
Rainfall Intensity	0.77 in/hr	• 200/03				
Discharge:	0.48 cfs					
Upstream Flowby:	0.02 cfs					
Total Inlet Q:	0.50 cfs					
Gutter Hydraulics:	1111 010					
Slope:	2.00% Stre	et Roughness [.]	0.016			
Cross-Slope	4.00% Sore	ad (allowable):	5.00.0			
erose erope.		Actual Soread:	3 16 ft			
Gutter Capacity	1.68 cfs	totuar oproau.	0.10 R			
Actual Depth:	0.13 ff					
Actual Velocity:	2 48 ft/c					
Inlot Hydraulice	2.40 103					
Close: G	(Congrado or S	60 0)				
Class. G	(G - on grade of S	- sag)	a h in a al			
MCEL @ Orate: 4589	G - grate, O - curb	opening, C - con	ibinea)			
VVSEL @ Grate: 4508.	4					
Combined:						
	Iriple Standar	a				
U/S Curb Openin	<u>g: </u>					
Length:	9.0 ft	Lt:	11.39			
Blockage:	33%	E:	67.00%			
D/S Grat	<u>e: </u>					
Grate:	L 4	Eo:	0.64			
Width:	1.0 ft	Rf:	1.00			
Length:	9.0 ft	Rs:	0.45			
Blockage:	63%	<u>E:</u>	80.18%			
Vo:	7.5 fps	Q caught:	0.46 cfs			
Enet	= 93.46%	Q flowby:	0.03 cfs			











· · · · · · ·

Sector and the sector of

112231414694W04779

ppplicad mounts

şççanoolaat otda

554.0000011100000155

Addangeocentricae

soulanalaine ang

our contraction of

Annalasian (

.

Description: Inle	t Hydrology and C	apacity An	alysis		
for.	City of Grand Jun	ction Inlet	Types		
Methodology: UD	FCD Rational Me	ihod (1984)	, and		
FH	WA "Drainage of I	Highway Pa	avements". H	HEC No. 12	
Template by: Ge	George K. Cotton, PE				
Ca	rter & Burgess, Inc	C.			
Last Revised: 30-	Jun-05				
Inlet Location:					
ID No.:	3841-2				
Station: 33	4+10.00	Grate F	levation	1503 15	
Offset:	21 50 11	WS FL	@ Grate:	4593 59	
Inlet Hydrology:				4000.00	
Area Paved		0 87 A	c	с .:	nga
Area Unpayor	4.		с с	C -	0.00
Total Areas	4. <u>.</u>			Curpev.	0.00
Total Alea:		U.02 A	C		0.93
Overland Len	gtn:	33.U ft		S _{over} :	2.00%
Channel Leng	th:	641.0 ft		H:	0.5
Overland C (5	i-yr):	0.93	Cha	annel Type:	1
t, =		1.40 m	ninutes	$t_t =$	17.80 minutes
t _{c=}		13.7 m	ninutes	t _{c_check} ≓	13.7 minutes
One-hour Rai	nfall	0.34 in	iches	Freq.	2 yr.
Rainfall Intens	sity	0.82 in	/hr		
Discharge:	2/2/2/2 00/2/2	0.62 c	ſS		
Upstream Flo	wby:	0.00 cl	ÍS		•
Total Inlet Q:		0.62 cl	S		
Gutter Hydraulics:	******				
Slope:		0.50%	Street F	Roughness:	0.016
Cross-Slope:		2.00%	Spread ((allowable):	9.50 ft
			Actu	ual Spread:	6.89 ft
Gutter Capaci	ty:	1.47 cl	ÍS		
Actual Depth:		0.14 ft			
Actual Velocit	y:	1.31 ft	/s		
Inlet Hydraulics:					
Class:	S (G -	on grade	e or S - sa	ag)	
Inlet Type:	C (G-	grate, O	- curb op	ening, C - cor	nbined)
WSEL @ Grate:	4593,59				
Combined:					
Ту	pe:	Single S	tandard		
U/S Curb C	Opening:				
<u> </u>	ength:	3.0 ft		Lt:	12.53
Blo	ckage:	100%		E:	0.00%
D/	S Grate:	****************			
	Grate:	C	4	Eo:	0.48
	Width:	1.5 ft	***************************************	Rf:	1.00
L	enath:	3.0 ft		Rs:	0.12
Blo	ckade:	59%		E:	54.15%
	Vo:	3.9 fr	os	Q cauaht:	0.34 cfs
	E., = 54	4.15%		Q flowby:	0.29 cfs
	-na V				0.20 010











-

*-----

In a submitted before the submitted by t

Appendiation of the second sec

guanna anna

pression of the second se

93000090000000

administra A ,

Description:	Inlet Hydrology	and Canacity	Analycic			
Bessinption.	for City of Gra	and lunction lake	t Tunoe			
Mothodologur	HOECD Dation	a Mothod (109	a Types			
weinodology.	CUPCD Ratio		Prij, anu Devementer			
Turnet to the	rhvvA Diana	ige of Highway	Pavements.	HEC. NO. 12		
i emplate by:	George K. Con	ion, PE				
	Carter & Burge	Carter & Burgess, Inc.				
Last Revised:	30-Jun-05					
Inlet Location:						
ID No.:	3461-2					
Station:	346+50.00	Grate	Elevation	: 4594.62		
Offset:	22 00 Lt	WS E	I @ Grate	: 4594.77	:	
Inlet Hydrology:	*******					
Area Pave	d:	1,03	Ac	C _{paved} :	0.93	
Area Unpa	ved:	0.00	Ac	Cuppay:	0	
Total Area		1.03	Ac	C.m.	0.93	
Overland L	enath:	33.0	ft	Sauri	2.00%	
Channel L	enath:	827.0	ft	H.	0.5	
Overland (C (5-vr)∙	0.93	". Ch	annel Type	4.0	
	o (o-yr).	1 40	minutas	t =	23.80	minutos
ч †		1/ 8	minutoe	4 + ==	14.9	minutos
Ono hour l		14.0 A 34	inchog	C_check	14.0 A	minutes
Dainfall Inf		0.34	inches	Fley.		yı.
Raman m	lensity	0.79	111/111			
Discharge:	Flouburg	0.70	CIS			
Upstream Tetel Inlet	Flowby:	0.00	CIS			
	Q:	0.76	CIS			
Gutter Hydraulics:			<u> </u>	D . 1		
Slope:		0.50%	Street	Roughness:	0.016	<i>c.</i>
Cross-Slop	be:	2.00%	Spread	(allowable):	9.50	ft
- <i></i> -			AC	tual Spread:	7.41	ft
Gutter Cap	pacity:	1.47	cfs			
Actual Dep	oth:	0.15	ft			
Actual Vel	ocity:	1.38	ft/s			
Inlet Hydraulics:						
Class:	S	(G - on gra	de or S - s	ag)		
Inlet Type:	С	(G - grate,	O - curb o	pening, C - c	combined)	
WSEL @ Grate:	4594.77					
	Combined:					
	LU0.0	Type:	Singli	e Standard		
		D Opening:		ж.o	1.4.	40.00
	-	Length:	3) 1 APA	<i>3</i> : П		13.62
	rt	DIOCKAGE:	100%	9	E:	0.00%
	L	U/S Grate:			· •••	o 15
		Grate:	(4	E0:	0.45
		Width:	1.1	5 ft	RI:	1.00
		Length:	3,1) ft	Rs:	0.11
	E	Blockage:	59%	G	E:	51.20%
		Vo:	3.	9 fps	Q caught:	0.39
		E _{net} =	51.20%	, 0	Q flowby:	0.37







Description: I	nlet Hydrology and (apacity Analysis				
f	or City of Grand Jun	ction Inlet Types				
Methodology: L	JDFCD. Rational Me	thod (1984), and				
F	HWA "Drainage of Highway Pavements", HEC No. 12					
Template by: 0	George K. Cotton, Pl	Ξ				
(Carter & Burgess, In	C.				
Last Revised: 3	30-Jun-05					
Inlet Location:						
ID No.:	3561-2					
Station:	356+43.90	Grate Elevat	ion: 4597.38			
Offset:	22.00 Rt	WS EI @ Gr	ate: 4597.50			
Inlet Hydrology:		0				
Area Paved	:	0.32 Ac	C _{paved} :	0.93		
Area Unpav	ed:	0.00 Ac	Cimpavi	0		
Total Area:	*******	0.32 Ac	C:	0.93		
Overland Le	enath:	37.0 ft	S	2.00%		
Channel I er	ngth:	182 0 ft	- over -	05		
Overland C	(5-vr)	0.93	Channel Type	1		
t, =	(•)))	1 48 minute	s = t =	4 16 minutes		
		5.6 minute	s t=	11.2 minutes		
One-hour R	ainfall	0.0 minute	Freq	TT.Z minutos		
Rainfall Inte	annan	1 08 in/hr	ाच्प.	ζ⊊ γι.		
Discharge:	holly	0.32 ofc				
Unstream F	lowby:	0.02 013				
Total Inlet C	10WDy.	0 15 ofe				
Gutter Hydraulics:	ζ.	0.40 013				
Slone		0.60% Stre	et Roughness	0.046		
Cross-Sione	7.	2.00% Sore	et (ollowable):	950.4		
01033-010pc	** ******	e opro	actual Spread:	6 11 ft		
Gutter Cana	acity:	1 /17 ofe	Actual Opicau.	0.11 ft		
Actual Dent	h.	0.12 ft				
Actual Velo	city:	1.71 ft/c				
	лку.	1.21 103				
Class:	S (G-	on grade or S	" sau)			
Inlet Type:	····) 0	arate O - curl	- sagy h opening C - co	mbined)		
WSEL @ Grate:	4597 50	grate, O - oun	o opennig, o - ee	лавшоцу		
Combined:	4007.00					
e	Type:	Sinnia Standa	rd			
U/S Curt) Opening:	onigio oranaci	ru.			
	Length:	20 ft	l f:	10 97		
R	lockage:	100%		0.00%		
	D/S Grate		ه سم	0.0070		
	Grate:	c 1	Fo.	0.53		
	Width	15.0	Rf [.]	1.00		
	Length:	206	Re ¹	0.13		
D	lockage:	400/II	179. E·	50.10		
D		20 for	L.	0.27 of c		
		0 07%		0.27 015		
	u_net − ⊃	J.UT 70	jocnowby:	0,18 015		



Presson C.

Description: Inlet Hydrolo	= nv and Canacily Analysis				
for City of G	and Junction Inlet Types				
Methodology: UDECD Rati	ional Method (1984), and				
FHWA "Drai	nage of Highway Pavements	" HEC No 12			
Template by George K C	ofton PF				
Carter & Bur					
Last Revised: 30- Jun-05	geos, mo.				
Inlet Location:					
ID No.: 3581-2					
Station: 358+51	1 Grate Elevatio	n: 4601.63			
Offset: 22 fl0 F	WS FL@ Grad	e: 4601 74			
inlet Hydrology:					
Area Paved:	0.45 Ac	Carrad:	0.93		
Area Unpaved	0.00 Ac	C ·	0		
Total Area:	045 Ac	- urpav*	0.93		
Overland Length:	33.0 ft	∽ comp∙ S • ∭∭	2.00%		
Channel Length	400.0 ft	vover- Ц∙	0.5		
Overland C (5-vr):	093 C	hannel Tyne	1		
t, =	1 40 minutes	t. =	13 05 minutes		
4 1.	12.9 minutes	ч f=	12.9 minutes		
ne-hour Rainfall	12.0 minutes	Frag	2 vr		
Rainfall Intensity	0.84 in/hr	1104.	2 yı.		
Discharge:	. 0.35 cfc				
Linstream Flowby:	0.00 cfs				
Total Inlet O	0.35 cfs				
Gutter Hydraulics:	0.00 013				
Slope:	0.50% Stree	t Roughness:	0.016		
Cross-Slope:	2.00% Sprea	d (allowable):	9.50 ft		
	A	ctual Spread:	5 55 ft		
Gutter Capacity:	1.47 cfs	oladi oprodal	0.00 ((
Actual Depth:	0.11 ft				
Actual Velocity:	1.14 ft/s				
Inlet Hydraulics:					
Class: G	(G - on grade or S -	sag)			
Inlet Type: C	(G - grate, O - curb	opening, C - con	nbined)		
WSEL @ Grate: 4601.7	4				
Combined:					
Type:	Single Standard	9			
U/S Curb Opening	<u> :</u>				
Length:	3.0 ft	Lt:	9.85		
Blockage:	100%	E:	0.00%		
D/S Grate	:				
Grate:	L 4	Eo:	0.57		
Width:	1.5 ft	Rf:	1.00		
Length:	3 0 ft	Rs:	0.12		
Blockage:	63%	<u>E:</u>	61.98%		
Vo:	3 .9 fps	Q caught:	0.22 cfs		
E _{nel} :	= 61.98%	Q flowby:	0.13 cfs		



HAMPON PROVINCE

UNAMU

Description: Inlet Hydrology a	nd Capacity Analysis	·					
for City of Grand	Junction Inlet Types						
Methodology: UDECD Rationa	Method (1984) and						
FHWA "Drainag	FHM/A "Drainage of Highway Payemente" HEC No. 12						
Template by: George K Cotto	n PE	5. TR. 0. TO. TZ					
Carter & Burgao	n, ru o Ino						
Last Deviced: 20, lup 05	Last Baylood: 20. Jun 05						
Last Revised, 50-Jun-05							
Inlet Location:							
ID No.: 3641-2							
Station: 364+42	Grate Elevatio	on 4601 10					
Offset: 32.50 Rt	WS EL@ Grat	te: 4601.21					
Inlet Hydrology:		1001.21					
Area Paved:	0.22 Ac	Cnaved:	0.93				
Area Unpaved:	0.06 Ac	C	n 2				
Total Area	0.28 Δc	∼unpav- C.	0.77				
Overland Length	6.20 A0	Comp.	0.77				
Channel Longth	1/204	Uover-	6.0070 0 F				
	0.77 0	⊓. Normal Tuna (0.0				
$\overline{\mathbf{Overland}} \subset (\mathbf{5-yr}).$	0.77 C	manner Type:	0.40 minutes				
ų — +	3.03 minutes	ե –	3.12 minutes				
	7.0 minutes	c_check	T1.2 minutes				
One-nour Raintai	U,34 Inches	⊢req.	2 yr.				
	1.03 in/nr						
Discharge:	0.22 Cfs						
Upstream Flowby:	U.UU CTS						
lotal Inlet Q:	0.31 cts						
Gutter Hydraulics:							
Slope:	0.50% Stree	Roughness:	0.016				
Cross-Slope:	200% Sprea	d (allowable):	20.00 ft				
Outline Demosition	A0.70(ctual Spread:	5.32 ft				
Gutter Capacity:	10.72 CIS						
Actual Depth:	0.11 ft						
Actual Velocity:	1.10 ft/s						
inlet Hydraulics:							
	- on grade or S -	sag)					
Inlet Type: C (0	J - grate, O - curb	opening, C - cor	nbined)				
WSEL @ Grate: 4601.21							
Combined:							
I ype:	Single Standard	1					
U/S Curb Opening:		• -					
Length:	3.U ft	Lt:	9.39				
Blockage:	100%	E:	0.00%				
D/S Grate:							
Grate:	C 4	Eo:	0.59				
Width:	1.5 ft	Rt:	1.00				
Length:	3.0 ft	Rs:	0.15				
Blockage:	59%	<u>E:</u>	64.97%				
Vo:	3 .9 fps	Q caught:	0.20 cfs				
E _{net} =	64.97%	Q flowby:	0.11 cfs				







يقدينيه ومحسبين ومرس

(a.....

فالالامة وساساته المحمد فالع

Description: Inlet Hydrology	Description: Inlet Hydrology and Capacity Analysis			
for: City. of: Gran	for City of Grand Junction Inlet Types			
Methodology: UDFCD Rational Method (1984), and				
FHWA "Draina	FHWA "Drainage of Highway Pavements", HEC No., 12			
Template by: George K. Cotto	George K. Cotton, PE			
Carter & Burge	Carter & Burgess, Inc.			
Last Revised: 30-Jun-05	30-Jun-05			
Inlet Location:				
ID No.: 3731-2				
Station: 373+00	Grate Elevation	on: 4575 32		
Offset: 22.00 Rt	WS FI @ Gra	te: 4575.47		
Inlet Hydrology:				
Area Paved:	0.82 Ac	Convert:	0.93	
Area Unpaved	Ac	C'		
Total Area:	0.82 Ac	C · ·	0.02	
Overland Longth:	0,02 AC	Comp.	0.00	
Ovenand Length.	04.0 IL	O _{over} ,	2.0070	
		П. 	U.3	
Ovenand C (5-yr):	0.93 (znannei iype: ∭	05.04 milester	
ι _i =	1.42 minutes	ել –	35.91 minutes	
	16.7 minutes		16.7 minutes	
One-hour Rainfall	Rainfall 0.34 inches Freq. 2 yr.			
Rainfall Intensity	Rainfall Intensity 0.75 in/hr			
Discharge:	: 0.57 cfs			
Upstream Flowby:	0.19 cts			
Total Inlet Q:	0.76 cfs			
Gutter Hydraulics:		2000		
Slope:	0.50% Stree	et Roughness:	0.016	
Cross-Slope:	2.00% Sprea	id (allowable):	9.50 ft	
Actual Spread: 7.42 ft				
Gutter Capacity:	1.47 cfs			
Actual Depth:	0.15 ft			
Actual Velocity:	1.38 ft/s			
Inlet Hydraulics:		_		
Class: S	(G - on grade or S	· sag)		
Inlet Type: C	(G - grate, O - curb	opening, C - co	mbined).	
WSEL @ Grate: 4575.47				
Combined:		*****		
Type:	Single Standar	d		
U/S Curb Opening:				
Length:	3.0 ft	Lt:	13.64	
Blockage:	100%	E:	0.00%	
D/S Grate:				
Grate:	C 4	Eo:	0.45	
Width:	1.5 ft	Rf:	1.00	
Length:	3.0 ft	Rs:	0.11	
Blockage:	59%	<u>E:</u>	51.15%	
Vo:	3.9 fps	Q caught:	0.39 cfs	
E _{net} =	51.15%	Q flowby:	0.37 cfs	

.










Aleman and a second sec

Strates Andrews

Description: Inlet Hydrology a	and Capacity Analysis			
for City of Grand Junction Inlet Types				
Methodology: UDFCD Rational Method (1984), and				
FHWA "Drainag	FHWA "Drainage of Highway Pavements", HEC No. 12			
Template by: George K. Cotto	n, PE			
Carter & Burges	s, Inc.			
Last Revised: 30-Jun-05				
Inlet Location:				
ID No.: 3911-1				
Station: 391±00	Grate Elevatio	un 4606 11		
Offset: 32.84 Rt	WS EL@ Grat	e: 4606.21		
Inlet Hydrology:		(C. 4000.21		
Area Paved	0.24 Ac	c 🕬	n 02	
Area Unnaved:		C ·	0.00 A	
Total Area:		C unpav.	0.02	
Total Area.	0.24 AC	Comp.	0.93	
Ovenand Length:	49.U T	Sover.	2.00%	
Channel Length:	217.0 ft	H:	0.5	
Overland C (5-yr):	0.93 C	hannel Type:		
ų =	1.70 minutes	$t_t =$	5.09 minutes	
l _{c =}	6.8 minutes	$t_{c_{check}} =$	11.5 minutes	
One-hour Rainfall	0.34 inches	Freq.	2 yr.	
Rainfall Intensity	1.04 in/hr			
Discharge:	0.23 cfs			
Upstream Flowby:	0.00 cfs			
Total Inlet Q:	0.23 cfs			
Gutter Hydraulics:		10000	****	
Slope:	0.50% Stree	t Roughness:	0.016	
Cross-Slope:	2.00% Sprea	d (allowable):	20.30 ft	
	A	ctual Spread:	4.75 ft	
Gutter Capacity:	11.15 cfs			
Actual Depth:	0.10 ft			
Actual Velocity:	1.02 ft/s			
Inlet Hydraulics:				
Class: G (G - on grade or S -	sag)		
Inlet Type: C (G - grate, O - curb	opening, C - cor	nbined)	
WSEL @ Grate: 4606.21				
Combined:		~~~~~		
Type:	Single Standard	1		
U/S Curb Opening:				
Length:	3,0 ft	Lt:	8.27	
Blockage:	100%	E:	0.00%	
D/S Grate:				
Grate:	L 4	Eo:	0.64	
Width:	1.5 ft	Rf:	1.00	
Length:	3.0 ft	Rs:	0.14	
Blockage:	63%	<u>E:</u>	68.76%	
Vo:	3 9 fps	Q caught:	0.16 cfs	
E _{net} =	68.76%	Q flowby:	0.07 cfs	











100 miles 14 Miles

*///deliceologica///

About the second

and consideratively

ورسابه والمراسية

.

Description: Inlet Hydrolog	y and Capacity Analysis				
for: City of Gra	ind Junction Inlet Types				
Methodology: UDFCD Ratio	Methodology: UDFCD Rational Method (1984), and				
FHWA "Drain	FHWA "Drainage of Highway Pavements", HEC No. 12				
Template by: George K. Co	tton, PE				
Carter & Burg	ess, Inc.				
Last Revised: 30-Jun-05					
Inlet Location:					
ID No.: 1371-2					
Station: 137+40	Grate Elevatio	on: 4606.31			
Offset: 34.00 R	WS EI @ Gra	te: 4606.41			
Inlet Hydrology:	× •				
Area Paved:	0.28 Ac	C _{paved} :	0.93		
Area Unpaved:	0.00 Ac	C _{ispav} :	0		
Total Area:	0.28 Ac	C	0.93		
Overland Length	50.0 ft	S:	2.00%		
Channel Length	154 A ft	H.	0.5		
Overland C (5-vr)	0.93 (hannel Type:	4		
	1 72 minutes	t. =	3 43 minutes		
1	5.1 minutes	t =	11 1 minutes		
∽= Ope-hour Rainfall	0.1 minutos	Fron	2 vr		
Rainfall Intensity	1 11 in/hr	1154.	4 yı.		
Discharge					
Lipstream Flowby:	0.23 cis				
Total Inlet O:	0.00 cfs				
Gutter Hydraulics	0,20 015				
Slope	0.50% Stree	et Roughness.	0.016		
Cross-Slope	2.00% Sprea	d (allowable):	9.50 ft		
	Δ	ctual Spread:	5 16 ft		
Gutter Capacity:	1.47 cfs	ioraal oproad.			
Actual Depth:	0.10 ft				
Actual Velocity:	1.08 ft/s				
Inlet Hydraulics:	1100 100				
Class: S	(G - on grade or S -	sad)			
Inlet Type: C	(G - grate, O - curb	opening, C - con	nbined)		
WSEL @ Grate: 4606.41		1 00	······································		
Combined:					
Type:	Single Standar	d			
U/S Curb Opening					
Length:	3.0 ft	Lt:	9.07		
Blockage:	100%	E:	0.00%		
D/S Grate					
Grate:	C 4	Eo:	0.60		
Width:	1.5 ft	Rf:	1.00		
Lenath:	3.0 ft	Rs:	0.16		
Blockage:	59%	E:	66.31%		
Vo:	3.9 fps	Q caught:	0.19 cfs		
E _{net} =	66.31%	Q flowby:	0.10 cfs		



Aniversity of the second secon

Comparison Locality

Alfadatastadade

in nicitation and the

.

HI KOLVIJI I I II II

when a sub-

linu o linu

.....

Description: Inlet Hydrology a	nd Capacity Analysis		
for City of Grand	Junction Inlet Types		
Methodology: UDFCD Rationa	l Method (1984), and		
FHWA "Drainag	e of Highway Pavement	s". HEC No. 12	
Template by: George K. Cotto	n, PE		
Carter & Burges	s, Inc.		
Last Revised: 30-Jun-05			
Inlet Location:			
ID No.: 1251-2			
Station: 125+56-60	Grata Elovativ	nn: #E06 90	
		to: 4505.00	
		10. 4595.94	
Area Daved	0.00 / -	<u> </u>	0.00
Area Hansand	0.03 AC	C _{paved} .	0.93
Area Unpaved:	U:UU AC	C _{unpav} :	0
Total Area:	0.83 Ac	C _{comp} :	0.93
Overland Length:	50.0 ft	S _{over} :	2.00%
Channel Length:	958.0 ft	H:	0:5
Overland C (5-yr):	0.93 (Channel Type:	1
t, =	1.72 minutes	t _i =	28.31 minutes
t _{c=}	15.6 minutes	t _{c check} =	15.6 minutes
One-hour Rainfall	0.34 inches	Freg.	2 vr.
Rainfall Intensity	0.77 in/hr	••••••	·····
Discharge:	0.60 cfs		
Upstream Flowby:	0.00 cfs		
Total Inlet O	0.60 cfs		
Gutter Hydraulice	0.00 013		
Slope:	A A AMA Strov	t Doughnoos	0.040
Cross-Slope:	0.0070 Ollet	d (allowable):	0.010
Cross-Slope.	2002 Spies	id (allowable):	9.50 1
Cuttor Concellu	4 47 of o	ctual Spread:	6.78 π
Gutter Capacity:	1.47 CTS		
	0.14 π		
Actual velocity:	1.30 ft/s		
Inlet Hydraulics:	.		
Class: G (G - on grade or S	· sag)	
Inlet Type: C (G - grate, O - curb	opening, C - cor	nbined)
WSEL @ Grate: 4595.94			
Combined:	****	~~~~~~	
Type:	Single Standar	d	
U/S Curb Opening:			
Length:	3.0 ft	Lt:	12.31
Blockage:	100%	E:	0.00%
D/S Grate:			
Grate:	L 4	Eo:	0.49
Width:	1.5 ft	Rf:	1.00
Length:	3.0 ft	Rs:	0.10
Blockage:	63%	E:	53.65%
Vo:	3.9 fps	Q caught:	0.32 cfs
E =	53.65%	Q flowhy:	0.28 cfs
		1	0.20 0.00







And the second s

A demonstration of the second s

.....

Description: Inlet Hydrol	ogy and Capacity Analysis			
for City of C	Frand Junction Inlet Types			
Methodology: UDFCD Rational Method (1984), and				
FHWA "Dr	FHWA "Drainage of Highway Pavements", HEC No. 12			
Template by: George K.	George K. Cotton, PF			
Carter & Bu	irgess, Inc.			
Last Revised: 30-Jun-05				
Inlet Location:				
ID No.: 1081-2				
Station: 108+	47 Grate Elevatio	n 4593 73		
Offset: 21 ND	Rt WS FL@ Grad	le: 4593 39		
Inlet Hydrology:		1000.00		
Area Paved:	0.56 Ac	C	ព្រច្ចន	
Area Unnaved	0.00 Ac	C ·	0.00	
Total Area:		C ·	0.02	
Overland Longth:	0.50 AC		0.93	
Ovenand Length.	33.U II	O _{over} .	2.00%	
Channel Length:	440.0 π		0.5	
Overland C (5-yr):	0.93 C	hannel Type:		
$\mathbf{t}_i =$	1.40 minutes	t, =	11.53 minutes	
t _{e =}	12.6 minutes	$t_{c_{check}} =$	12.6 minutes	
One-hour Rainfall	0.34 inches	Freq.	2 yr.	
Rainfall Intensity	0.84 in/hr			
Discharge:	0.44 cfs			
Upstream Flowby:	0.44 cfs			
Total Inlet Q:	0.88 cfs			
Gutter Hydraulics:				
Slope:	0.50% Stree	t Roughness:	0.016	
Cross-Slope:	2.00% Sprea	d (allowable):	9.50 ft	
	A	ctual Spread:	7.84 ft	
Gutter Capacity:	1.47 cfs			
Actual Depth:	0.16 ft			
Actual Velocity:	1.43 ft/s			
Inlet Hydraulics:				
Class: S	(G - on grade or S -	sag)		
Inlet Type: C	(G - grate, O - curb	opening, C - con	nbined)	
WSEL @ Grate: 4593.3	39	•		
Combined:				
Type:	Single Standard	1		
U/S Curb Openin	g:	****************************		
Length:		Lt:	14.50	
Blockage:	100%	E:	0.00%	
D/S Grat	e:		. – – • •	
Grate:	' C 4	Eo:	0.43	
Width:	1.5 ft	Rf:	1.00	
Length:	80 ft	Rs:	0.10	
Blockage:	50%	F.	49 01%	
Vo	3 Q fns	O caught:	0.43 cfe	
	= 49.01%	O flowby:	0.45 cfc	
L-net	70.0170	loc nowny.	0.40 015	





Description:	Inlet Hydrology and	Capacity Analysis			
	for City of Grand Junction Inlet Types				
Methodology:	UDFCD Rational M	ethod (1984), and			
	FHWA "Drainage of	f Highway Pavements"	HEC No. 12		
Template by:	George K. Cotton, F	ΡĒ			
	Carter & Burgess, I	nc.			
Last Revised:	1-Jul-05				
Inlet Location:					
ID No.:	DP-172				
Station:	93+00.00	Grate Elevatio	n: 4588.01		
Offset:	29.00 Lt	WS EI @ Grate	e: 0.00		
Inlet Hydrology:					
Area Paveo	d: 🧰	0.22 Ac	C _{paved} :	0.93	
Area Unpa	ved:	0,00 Ac	C _{unpav} :	0	
Total Area:		0.22 Ac	C _{comp} ;	0.93	
Overland L	ength:	35.0 ft	S _{over} :	2.00%	
Channel Le	ength:	322.0 ft	H:	0.5	
Overland C) (5-yr):	0.93 Cl	hannel Type:	1	
t, =		1.44 minutes	$t_{t} =$	8.04 minutes	
. t _{e =}		9.5 minutes	t _{c_check} =	12.0 minutes	
One-hour F	Rainfall 🛛 🖉	0.34 inches	Freq.	2 yr.	
Rainfall Inte	ensity	0.94 in/hr		***************************************	
Discharge:		0.19 cfs			
Upstream F	⁼lowby: 🛛 🖉	cfs			
Total Inlet (Q:	0.19 cfs			

ET DESIGN CALC	JULATION			
Description:	Inlet Hydrology and	d Capacity Analysis		
	for Cily of Grand J	unction Inlet Types	·	
Methodology:	UDFCD Rational N	Nethod (1984), and		
	FHWA "Drainage	of Highway Pavemer	its" HEC No. 12	
Template by:	George K. Cotton,	PE		
	Carter & Burgess,	Inc.		
Last Revised:	1-Jul-05			
Inlet Location:				
ID No :	DP-172			
Station	02100.00	Orata Elavat	ion: #600.04	
Offect:	20 00 00		1011. 4200.01	
Inlet Hydrology	29.00 Rt	₩5 EI @ GI	ale: 0.00	
inter nyurology:			- 2002	050505000000000000000000000000000000000
Area Pave	d:	0.22 Ac	C _{paved} :	0.93
Area Unpa	ved:	0.00 Ac	C _{unpav} :	0
Total Area	:	0.22 Ac	C _{comp} :	0.93
Overland L	.ength:	32.0 ft	S _{over} :	2.00%
Channel Le	ength:	323.0 ft	H:	0.5
Overland C) (5-yr):	0.93	Channel Type:	1
t, =		1.37 minute	s t _i =	8.07 minutes
t _{a=}		9.4 minute	s t _{c_check} =	12.0 minutes
One-hour f	Rainfall 🛛 💹	0.34 inches	Freq.	2 yr.
Rainfall Int	ensity	0.94 in/hr		
Discharge:		0.19 cfs		
Upstream I	Flowby:	cfs		
Total Inlet	Q:	0.19 cfs		



McAntereory woontern p. *

Indon-of-theorem

VIII CONTRACTOR N

1000-01-0000

NUNYMIN

and without

Description:	Inlet Hydrology and Capacity Analysis			
	for City of Grand Junction Inlet Types			
Melhodology:	UDFCD Rational Mel	lhod (1984), and		
	FHWA "Drainage of I	Highway Pavements"	HEC No. 12	
Template by:	George K. Cotton, PI	-		
	Carter & Burgess, Inc	o.		
Last Revised:	1-Jul-05			
Inlet Location:				
ID No.:	DP-100			
Station:	25+14.00	Grate Elevation:	4588.01	
Offset:	48.00 Lt	WS EI @ Grate:	0.00	

Inlet Hydrology:

iniet Hydrology:			
Area Paved:	0.53 Ac	C _{paved} :	0:93
Area Unpaved:	0.00 Ac	C _{unpav} :	0
Total Area:	0.53 Ac	C _{comp} :	0.93
Overland Length:	44.0 ft	Sover:	<u>2.00%</u>
Channel Length:	472.0 ft	H:	0.5
Overland C (5-yr):	0.93	Channel Type:	1
t _i ==	1.61 minute	s t _i =	12.50 minutes
t _{c =}	12.9 minute	s t _{c_check} =	12.9 minutes
One-hour Rainfall	1.33 inches	Freq.	100 yr.
Rainfall Intensity	3.31 in/hr		an na mana ang ang ang ang ang ang ang ang ang
Discharge:	1.63 cfs		
Upstream Flowby:	Cfs		
Total Inlet Q:	1.63 cfs		

•

Rainfall Intensity Discharge:

Upstream Flowby: Total Inlet Q:

Description:	Inlet Hydrology a	nd Capacity Analysi	S			
	for City of Grand Junction Inlet Types					
Methodology:	UDFCD Rational	Method (1984), and				
	FHWA "Drainage	of Highway Pavem	ents" HEC No. 1	2		
Template by:	George K. Cotton	, PE				
	Carter & Burgess	, Inc.				
Last Revised:	1-Jul-05					
Inlet Location:						
ID No.:	DP-101					
Station:	25+14.00	Grate Eleval	lion: 4588.)1		
Offset:	48.00 Rt	WS EI @ Gr	ate: 0.0	00		
Inlet Hydrology:						
Area Pave	d:	0.53 Ac	C_{pav}	_{ed} : 0,93		
Area Unpa	ved:	0.00 Ac	Cunp	_{av} : 0		
Total Area:		0.53 Ac	C_{con}	np: 0.93		
Overland L	ength:	44:0 ft	Sov	_{er} :2.00%		
Channel Le	ength:	472.0 ft	l	H:		
Overland C	; (5-yr):	0.93	Channel Typ	e: 👘 👘 1		
t, =		1.61 minute	s t _i	= 12.50	minutes	
t _{e =}		12.9 minute	s t _{c_check}	= 12.9	minutes	
One-hour F	Rainfall 🛛 👬	1.33 inches	Freq.	100	yr.	

3.31 in/hr

1.63 cfs

1.63 cfs

DP-101 (100)






























Description: Inlet Hydrolo	gy and Capacity Analysis		
for City of G	and Junction Inlet Types		
Methodology: UDFCD Rati	ional Method (1984), and	•	
FHWA "Drai	nage of Highway Pavements"	HEC No. 12	
Template by: George K. C	otton, PE		
Carter & Bur	gess, Inc.		
Last Revised: 29-Jun-05			
Inlet Location:			
ID No.: ST-295P-			
Station: 295+94.0	0 Grate Elevation	: 4588.01	
Offset: 42.50 F	WS EI @ Grate	0.00	
Inlet Hydrology:			
Area Paved:	1.72 Ac	C _{paved} :	0.93
Area Unpaved:	0.00 Ac	C _{unpav} :	0
Total Area:	1.72 Ac	C _{comp} :	0.93
Overland Length:	241.0 ft	S _{over} : 🐰	2.00%
Channel Length:	272.0 ft	H:	0.5
Overland C (5-yr):	0.93 Ch	annel Type: 🕷	1
t, =	3.77 minutes	$t_t = 1$	6.61 minutes
t _{e =}	10.4 minutes	t _{c_check} =	12.9 minutes
One-hour Rainfall	1,33 inches	Freq.	100 yr.
Rainfall Intensity	3.60 in/hr		
Discharge:	5.75 cfs		
Upstream Flowby:	cfs		
Total Inlet Q:	5.75 cfs		

.

Description: Inlet Hydrology and Capacity Analysis

for City of Grand Junction Inlet Types

Methodology: UDFCD Rational Method (1984), and

FHWA "Drainage of Highway Pavements" HEC No. 12

Template by: George K. Cotton, PE

Carter & Burgess, Inc.

Last Revised: 29-Jun-05

Inlet Location:

Total Inlet Q:

ID No.:	ST-299P-1
Station:	299+76.80
Offset:	42.50 Rt
Inlet Hydrology:	

ID NO.: 51-298	1P-1		
Station: 299+70	5.80 Grate Elevation	on: 4588.01	
Offset: 42.5	0 Rt WS EI @ Gra	te: 0.00	
lydrology:			
Area Paved:	1,32 Ac	C _{paved} :	0.93
Area Unpaved:	0.00 Ac	C _{unpav} :	0
Total Area:	1.32 Ac	C _{comp} :	0.93
Overland Length:	227.0 ft	S _{over} : 👹	2.00%
Channel Length:	211.0 ft	H:	0.5
Overland C (5-yr):	0.93 0	Channel Type:	1
t, =	3.66 minutes	t _t =	4.93 minutes
t _{e=}	8.6 minutes	$t_{c_check} =$	12.4 minutes
One-hour Rainfall	1,33 inches	Freq.	100 yr.
Rainfall Intensity	3.83 in/hr	1 P. 1	
Discharge:	4.70 cfs		•
Upstream Flowby:	cfs		

4.70 cfs

299P-1 (100)

Description: I	Inlet Hydrology and C	apacity Analysis		
f	for City of Grand Jun	ction Inlet Types		
Methodology: I	UDFCD Rational Met	hod (1984), and		
í	FHWA "Drainage of I	Highway Pavements"	HEC No. 12	
Template by: 0	George K. Cotton, PE	-		
(Carter & Burgess, Ind	Э.		
Last Revised: 2	29-Jun-05			
Inlet Location:	e <u>ta (han eta arkin hido</u> ka)an			
ID No.:	ST-302P-1			
Station:	302+97.50	Grate Elevation	n: 4588.01	
Offset:	42.50 Rt	WS EI @ Grate	e: 0.00	
Inlet Hydrology:				
Area Paved	l:	2.47 Ac	C _{paved} :	0.93
Area Unpav	/ed:	0.00 Ac	C _{unpav} :	0
Total Area:		2.47 Ac	C _{comp} :	0.93
Overland Le	ength:	120,0 ft	S _{over} :	2.00%
Channel Lei	ngth:	612.0 ft	H:	0.5
Overland C	(5-yr):	0.93 Ch	iannel Type: 💹	1
t, =		2.66 minutes	$t_t =$	16.87 minutes
د =		14.1 minutes	$t_{c_{check}} =$	14.1 minutes
One-hour R	ainfall	1,33 inches	Freq.	100 yr.
Rainfall Inte	ensity	3.19 in/hr		
Discharge:		7.33 cfs		
Upstream F	lowby:	cfs		
Total Inlet C) :	7.33 cfs		









































.

UNTERFORM

. Weeesware

Countration.

A THE ADDRESS AND ADDRESS A

division of

A Figure 1

ann ya

WILL THE TWO

MEET DEGION ORECOLATION			
Description: Inlet Hydrology	and Capacity Analysis		
for City of Gran	nd Junction Inlet Types		
Methodology: UDFCD Ration	nal Method (1984), and		
FHWA "Draina	ige of Highway Pavements'	' HEC No. 12	
Template by: George K. Cot	ion, PE		
Carter & Burge	ess, Inc.		
Last Revised: 29-Jun-05			
Inlet Location:			
ID No.: ST-358I-2			
Station: 358+61.1	Grate Elevatio	n. 1601.63	
Offset: 22.00 Rt	MS EL@ Crot	a: 4601.82	
Inlet Hydrology:		5. 4001.02	
Area Payed:	0 4E A o	с · 📖	0.02
Area Elensus de	0.40 AL	Opaved-	0.93
Area Onpaved:	0.00 AC	Cunpav.	U
lotal Area:	0.45 AC	C _{comp} :	0.93
Overland Length:	33.0 ft	S _{over} :	2.00%
Channel Length:	490.0 ft	H:	0.5
Overland C (5-yr):	0.93 Cl	hannel Type:	1
t, =	1.40 minutes	t, =	13.05 minutes
t _{c =}	12.9 minutes	$t_{c, check} =$	12.9 minutes
One-hour Rainfall	1,33 inches	Freq.	100 vr.
Rainfall Intensity	3.31 in/hr	•	******
Discharge:	1.39 cfs		
Upstream Flowby:	0.00 cfs		
Total Inlet O	1 39 cfs		
Gutter Hydraulics:	1.00 010		
Slope.	0.60% Street	Roughness:	0.016
Cross-Slope:	2.00% Soread	l (allowable):	9 50 ft
oroso-orope.		tual Spread:	0.30.ft
Gutter Canacity:	1 17 cfc	iudi Opicau.	3.30 ft
Actual Dooth:	0.10 #		
Actual Valacity:	1 60 ft/c		
International Actual Velocity.	1.00 105		
Class:	(C) on grade on C	202)	
talet Type: C	(G - on grade of S - :	say) soning C	, hin od)
MIELTYPE. C	(G - glate, O - curb c	pening, C - con	ioinea)
VVSEL @ Glate: 4601.82			
	Single Standard		
Length:	3.U ft	Lt:	17.55
Blockage:	100%	E:	0.00%
D/S Grate:			
Grate:	L 4	Eo:	0.37
Width:	1.5 ft	Rf:	1.00
Length:	3.0 ft	Rs:	0.07
Blockage:	63%	<u>E:</u>	41.72%
Vo:	3,9 fps	Q caught:	0.58 cfs
E _{net} =	41.72%	Q flowby:	0.81 cfs







Description: Inlet Hydrology and Capacity Analysis for City of Grand Junction Inlet Types Methodology: UDFCD Rational Method (1984), and FHWA "Drainage of Highway Pavements" HEC No. 12 Template by: George K. Cotton, PE Carter & Burgess, Inc. Last Revised: 29-Jun-05 Infet Location: ID No.: ST-3711-1 371+44.7 Station: Grate Elevation: 4601.63 Offset: 22.00 Rt WS EI @ Grate: 4601.78 Inlet Hydrology: Area Paved: 0.19 Ac 0.93 C_{paved}: Area Unpaved: 0.00 Ac Cunpav: 0 **Total Area:** 0.19 Ac C_{comp}: 0.93 **Overland Length:** 33.0 ft 2.00% S_{over}: Channel Length: 202.0 ft 0.5 H: Overland C (5-yr): 0.93 Channel Type: 11 t, = 1.40 minutes t, = 4.69 minutes t_{e ≖} 6.1 minutes 11.3 minutes $l_{c_{check}} =$ One-hour Rainfall 1.33 inches 100 yr. Freq. Rainfall Intensity 4.22 in/hr Discharge: 0.75 cfs Upstream Flowby: 0.00 cfs Total Inlet Q: 0.75 cfs **Gutter Hydraulics:** Slope: 0.50% Street Roughness: 0.016 Cross-Slope: 2.00% Spread (allowable): 9.50 ft Actual Spread: 7.37 ft Gutter Capacity: 1.47 cfs Actual Depth: 0.15 ft Actual Velocity: 1.37 ft/s Inlet Hydraulics: Class: G (G - on grade or S - sag) Inlet Type: С (G - grate, O - curb opening, C - combined) WSEL @ Grate: 4601.78 Combined: Type: Single Standard U/S Curb Opening: Length: 3.0 ft Lt: 13.52 E: Blockage: 100% 0.00% D/S Grate: Grate: L 4 Eo: 0.46 Width: 1.5 ft Rf: 1.00 Length: 3.0 ft Rs: 0.09 Blockage: E: 63% 50.31% Vo: 3.9 fps Q caught: 0.37 cfs Enet = 50.31% Q flowby: 0.37 cfs












































Description:	Inlet Hydrology and Capacity Analysis
	for City of Grand Junction Inlet Types
Methodology:	UDFCD Rational Method (1984), and
	FHWA "Drainage of Highway Pavements" HEC No. 12
Template by:	George K. Cotton, PE
	Carter & Burgess, Inc.
Last Revised:	1-Jul-05

Inlet Location:

	ID No.:	DP-172					
	Station: 93+00.00		Grate Elevation: 4588.01				
	Offset:	29.00 Lt	WS EI	@ Grate:	0.00		
Inlet Hydrology:							
	Area Paved: Area Unpaved:		🖉 🔆 🖉 0.53 A	'C	C _{paved} :	0.93	
			0.00 A	°C	Cunpav:	÷0	
	Total Area:		0.53 A	(C	C _{comp} :	0.93	
	Overland Length: Channel Length: Overland C (5-yr):		35.0 ft		S _{over} :	2.00%	
			322.0 ft	•	H:	0.5	
			0.93	Chan	nel Type:	<u> 1</u>	
	$\mathbf{t}_i =$		1.44 n	ninutes	t ₁ =	8.04 mini	utes
	. t _{c≠}		9.5 n	ninutes	$t_{c_check} =$	12.0 min	utes
	One-hour R	Rainfall	1.33 ir	iches F	req.	100 yr.	
	Rainfall Inte	ensity	3.71 ir	ı/hr			
	Discharge:		1.83 c	fs			
	Upstream F	Flowby:	¢ se c	fs			
	Total Inlet C	ג:	1.83 c	fs			

Þ

1

~

Description:	Inlet Hydrology and Capacity Analysis
	for City of Grand Junction Inlet Types
Methodology:	UDFCD Rational Method (1984), and
	FHWA "Drainage of Highway Pavements" HEC No. 12
Template by:	George K. Cotton, PE
	Carter & Burgess, Inc.
Last Revised:	1-Jul-05

Inlet Location: ID No.: DP:173

W/##850/0040/661/1

ID NO.:				
Station: 93+00.00		Grate Elevation		
Offset: 29.00 R		WS EI @ Gra	te: 0.00	
Inlet Hydrology	•	-		
Area Paved:		0.53 Ac	C _{paved} :	0.93
Area Unpaved:		0.00 Ac	Cunpay:	· · · · · 0
Total Area:		0.53 Ac	C _{comp} :	0.93
Overland Length:		32.0 ft	S _{over} :	2,00%
Channel Length: Overland C (5-yr): t _i =		323.0 ft	H: 🖗	0.5
		0.93 (Channel Type:	1
		1.37 minutes	$t_t =$	8.07 minutes
t _e ,	=	9.4 minutes	$t_{c_{c_{c_{c_{c_{c_{c_{c_{c_{c_{c_{c_{c_$	12.0 minutes
One-hour	Rainfall	1.33 inches	Freq.	100 yr.
Rainfall In	tensity	3.72 in/hr	- 2000, S20000	ವರ್ಷನ್ ಸಂಸ್ಥೆಯಲ್ಲಿ ಸಂಗ್ರೆಯ ಕೋಯಿಸಿಕ ಕ್ರಿ 📼
Discharge	:	1.83 cfs		
Upstream	Flowby:	cfs		
Total Inlet	Q:	1.83 cfs		

.