

FEE \$ 10.00  
 TCP \$ 1000.00  
 SIF \$ 292.00

**PLANNING CLEARANCE**  
 (Single Family Residential and Accessory Structures)  
Community Development Department

BLDG PERMIT NO. \_\_\_\_\_

Building Address 2064 BASELINE  
 Parcel No. 2697-354-51-004  
 Subdivision Independence Ranch  
 Filing 10 Block 2 Lot 4

No. of Existing Bldgs 0 No. Proposed 1  
 Sq. Ft. of Existing Bldgs 0 Sq. Ft. Proposed 350  
 Sq. Ft. of Lot / Parcel \_\_\_\_\_  
 Sq. Ft. Coverage of Lot by Structures & Impervious Surface (Total Existing & Proposed) \_\_\_\_\_  
 Height of Proposed Structure \_\_\_\_\_

**OWNER INFORMATION:**

Name CALVIN BUILDERS  
 Address 4200 KANNAH CREEK RD.  
 City / State / Zip WHITEWATER CO. 81527

**DESCRIPTION OF WORK & INTENDED USE:**

New Single Family Home (\*check type below)  
 Interior Remodel  Addition  
 Other (please specify): \_\_\_\_\_

**APPLICANT INFORMATION:**

Name MARK CALVIN  
 Address 4200 KANNAH CREEK RD.  
 City / State / Zip WHITEWATER CO. 81527  
 Telephone 260-1455

**\*TYPE OF HOME PROPOSED:**

Site Built  Manufactured Home (UBC)  
 Manufactured Home (HUD)  
 Other (please specify): \_\_\_\_\_

NOTES: \_\_\_\_\_

**REQUIRED: One plot plan, on 8 1/2" x 11" paper, showing all existing & proposed structure location(s), parking, setbacks to all property lines, ingress/egress to the property, driveway location & width & all easements & rights-of-way which abut the parcel.**

**THIS SECTION TO BE COMPLETED BY COMMUNITY DEVELOPMENT DEPARTMENT STAFF**

ZONE PD Maximum coverage of lot by structures 35%  
 SETBACKS: Front 25' from property line (PL) Permanent Foundation Required: YES X NO \_\_\_\_\_  
 Side 10' from PL Rear 25' from PL Parking Requirement 2  
 Maximum Height of Structure(s) 32' Special Conditions Site + structure OK #1 geotechnical investigation, observation & analysis by an engineer prior to Planning Clearance  
 Voting District A Driveway Location Approval GH (Engineer's Initials)

Modifications to this Planning Clearance must be approved, in writing, by the Community Development Department. The structure authorized by this application cannot be occupied until a final inspection has been completed and a Certificate of Occupancy has been issued, if applicable, by the Building Department (Section 305, Uniform Building Code).

I hereby acknowledge that I have read this application and the information is correct; I agree to comply with any and all codes, ordinances, laws, regulations or restrictions which apply to the project. I understand that failure to comply shall result in legal action, which may include but not necessarily be limited to non-use of the building(s).

Applicant Signature [Signature] Date 6-30-05

Department Approval [Signature] Date 7/20/05

Additional water and/or sewer tap fee(s) are required: YES X NO \_\_\_\_\_ W/O No. 18277

Utility Accounting [Signature] Date 7/20/05

VALID FOR SIX MONTHS FROM DATE OF ISSUANCE (Section 2.2.C.1 Grand Junction Zoning & Development Code)  
 (White: Planning) (Yellow: Customer) (Pink: Building Department) (Goldenrod: Utility Accounting)





GRAND JUNCTION  
LINCOLN - DeVORE, Inc.  
GEOTECHNICAL ENGINEERS - GEOLOGISTS

1441 Motor St.  
Grand Junction, CO 81505

TEL: (970) 242-8968  
FAX: (970) 242-1561

July 18, 2005

Mark Calvin  
1879 Deer Park Ct.  
Grand Junction, CO 81503

Re: Building/Slope Setback  
2064 Baseline Rd., Grand Junction, CO

The Grand Junction Lincoln DeVore records regarding the Slope Stability Analysis for the Filing 10 of Independence Ranch Subdivision have been reviewed in light of the site plan provided for the Calvin residence. The site in question is Lot 4, Block 2 of Filing 10, with a physical address of 2064 Baseline Road.

The site grading and structure placement is such that the proposed residential structure on this lot is not affected by the *Area of Special Slope Stability Concern*, as shown on the Thompson Langford Corporation mapping for this subdivision. Attached are appropriate reproductions of the analysis sections S1 and S2, which were prepared by Grand Junction Lincoln DeVore, Job # 89144-GJ, 3-18-03.

The computed building setback from the north boundary line, due to the slope hazard is 27 feet inside the property on the west boundary line and 64 feet along the east 10' setback line. We have determined a buffer zone of at least 20 feet between the main structure and the computed building setback after the subdivision grading and installation of a shallow subsurface drain along the Lot 3 and 4 boundary line. The figures of the Mesa County Topography show the back line of the Calvin residence, as shown on the site plan for the Calvin residence, which was provided to Grand Junction Lincoln DeVore.

It is believed that all pertinent points have been addressed. If any further questions arise regarding this project or if we can be of any further assistance, please do not hesitate to contact this office at any time.

Respectfully Submitted,

GRAND JUNCTION  
LINCOLN DeVORE, Inc.

by: Edward M. Morris PE  
Principal Engineer



GJLD Job No.: 91666-GJ

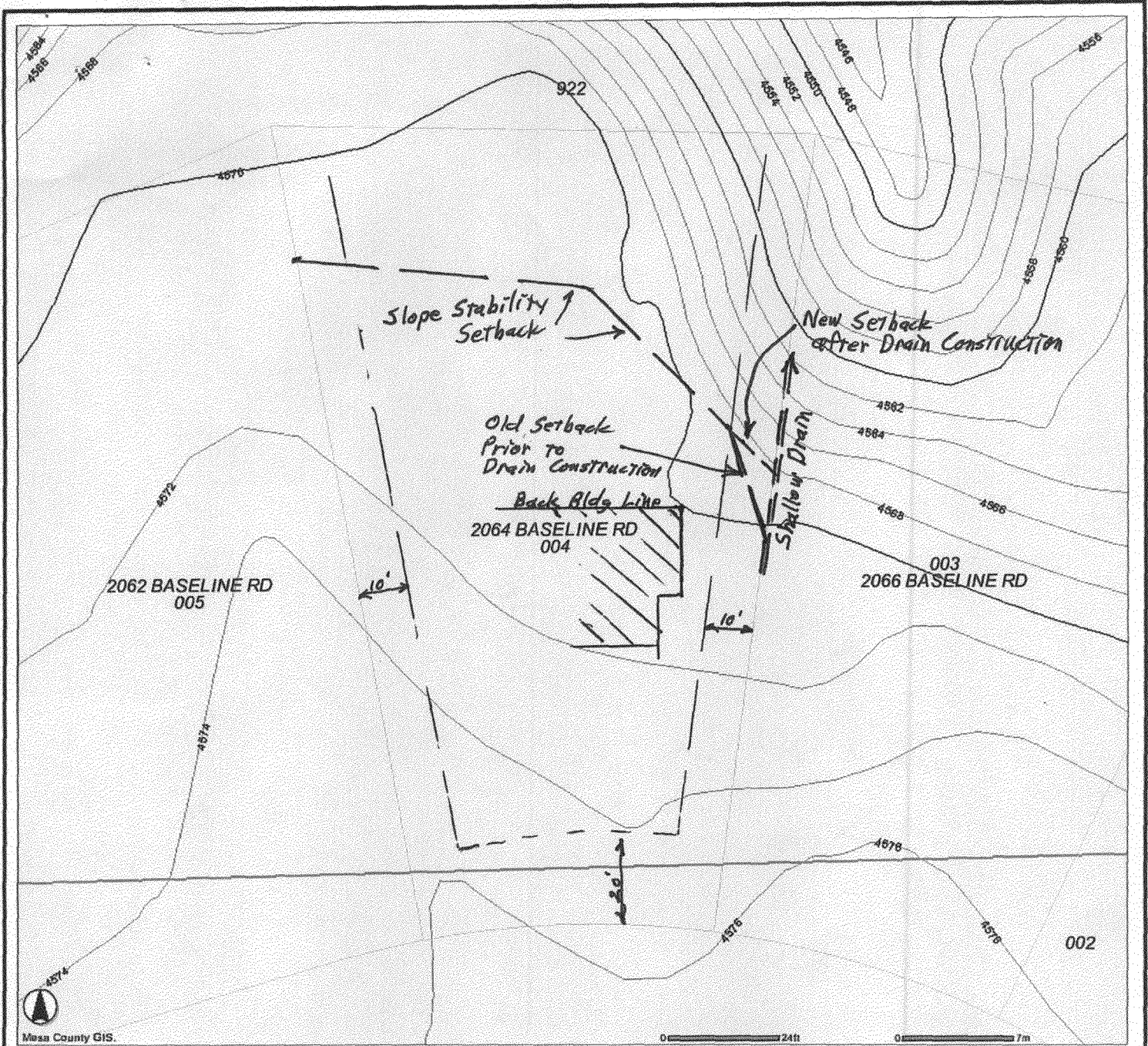


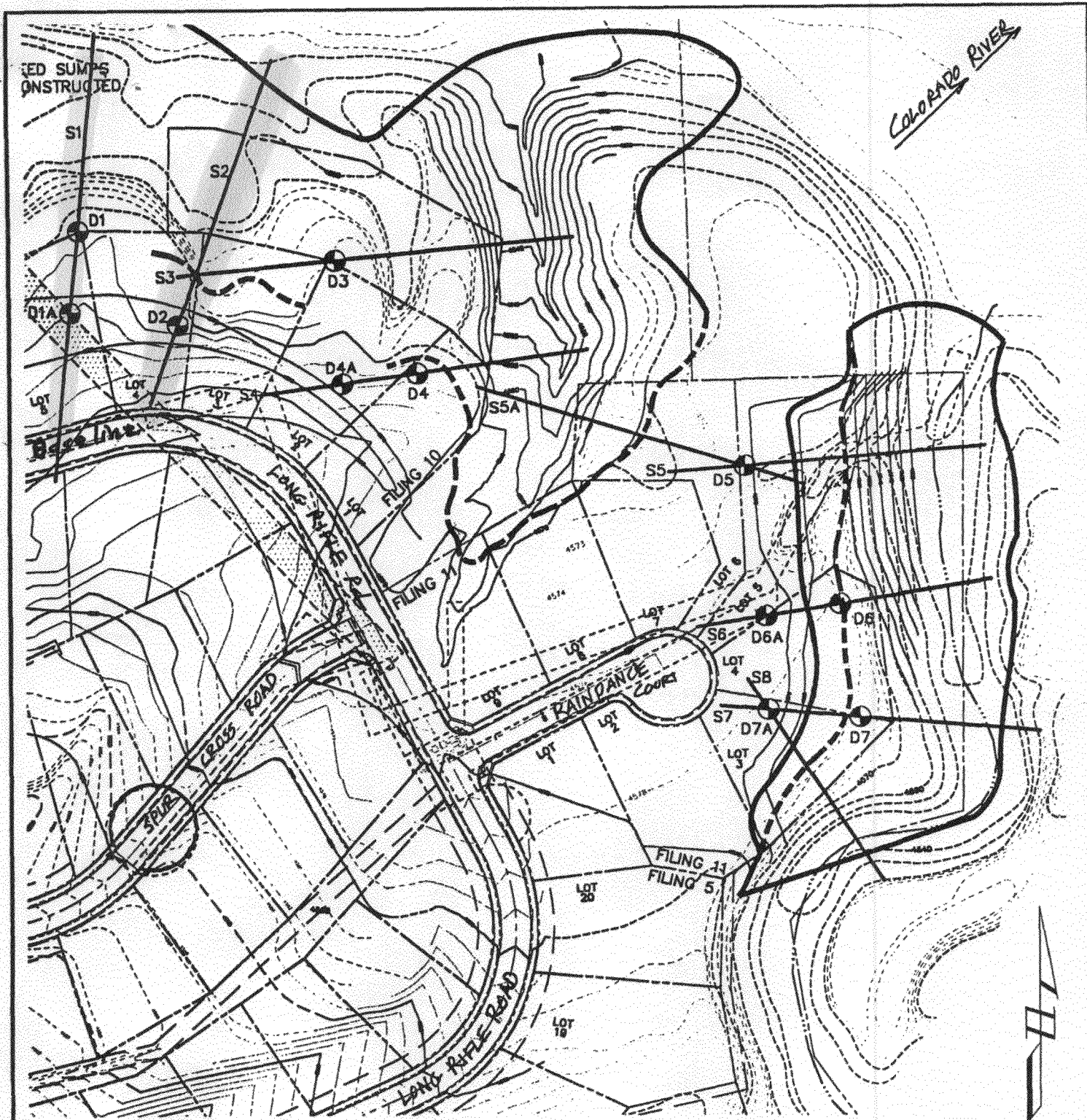
IMAGE from MESA COUNTY GIS Web Site

**MESA COUNTY TOPOGRAPHY**



**GRAND JUNCTION  
LINCOLN - DeVORE, Inc.**  
Geotechnical Consultants  
Grand Junction, Colorado

<b>SLOPE STABILITY CHECK</b>	
2064 BASELINE, Grand Junction, Colorado	
MARK CALVIN Grand Junction, Colorado	Date 7-18-2005
Job No. 91666-GJ	Drawn EMM



 DRILL HOLES  
 STUDY SECTIONS

**BORING LOCATION DIAGRAM**

**SLOPE STABILITY STUDY, Fil. 10 & 11**  
**Independence Ranch Sub., Grand Junction, CO.**

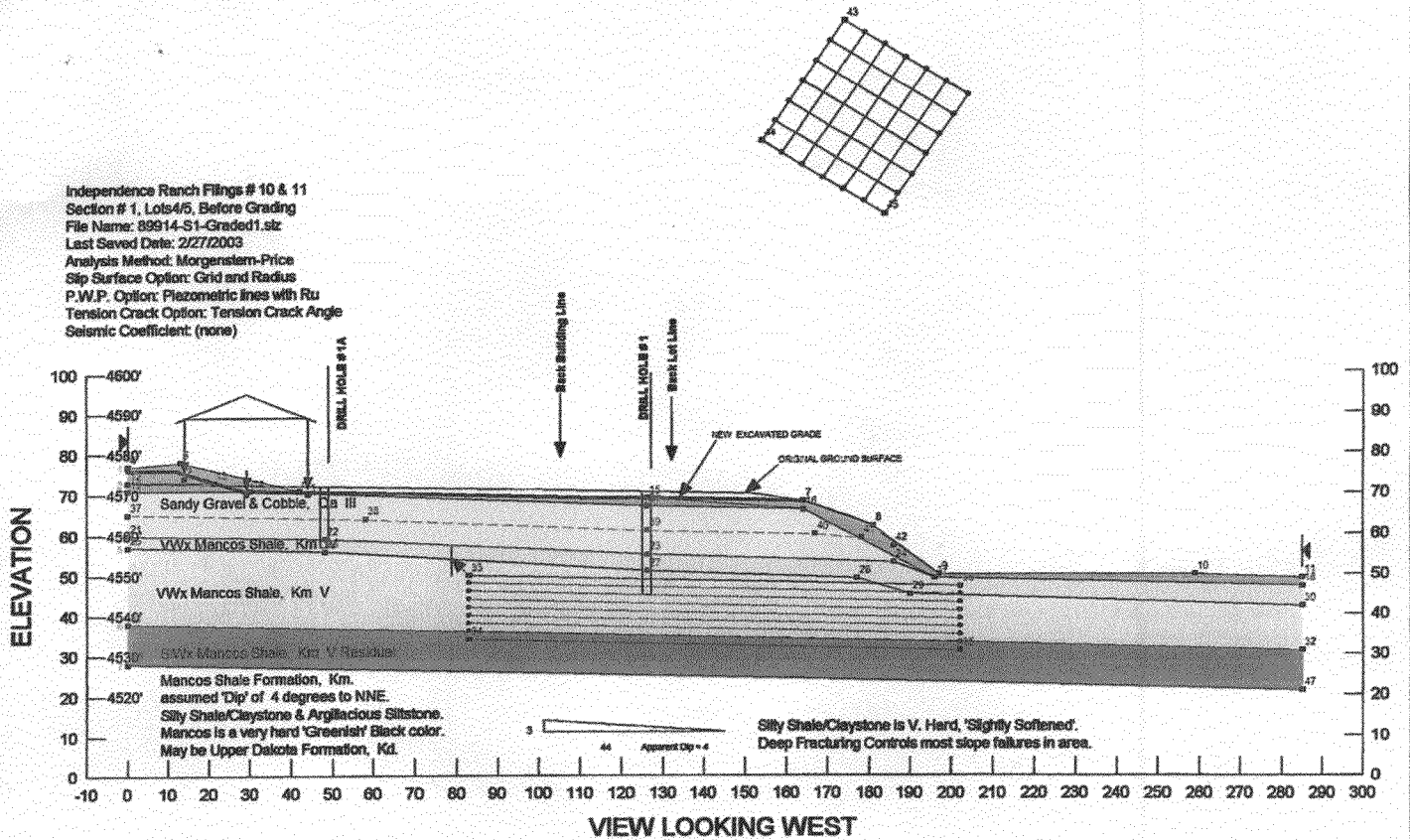


**GRAND JUNCTION**  
**LINCOLN - DeVORE, Inc.**  
 Geotechnical Consultants  
 Grand Junction, Colorado

<b>LAUGHING WATERS, LLC</b> Grand Junction, Colorado		Date 2-22-2003
Job No. 89914-GJ	Drawn EMM	



Independence Ranch Filings # 10 & 11  
 Section # 1, Lots 4/5, Before Grading  
 File Name: 89914-S1-Graded1.stz  
 Last Saved Date: 2/27/2003  
 Analysis Method: Morgenstern-Price  
 Slip Surface Option: Grid and Radius  
 P.W.P. Option: Piezometric lines with Ru  
 Tension Crack Option: Tension Crack Angle  
 Seismic Coefficient: (none)



**This Study is Along Section S1, Overlooking a Small to Medium Depth Gully**

The Site has been Developed, The Site Grading has Removed Less Than 2 Feet of Alluvial Sands.  
 The Structure has been Constructed as a 'Walkout Basement and the Landscaping is irrigated.  
 The Upper Water Table is Elevated to within 5 feet of the Backyard Surface and Seepage is Occurring at the Slope.  
 Building Loads are Modeled at 1500 plf. For the Interior and 2000 plf for the Exterior.  
 Fill is Placed at the Building Area But, Not Toward the Slope Edge..

The Building/Setback is over 25' From the Back Lot Line & over 55' From the New Crest of the Slope.  
 The Building Setback is Very Close to the 3:1 (hor : vert) Limit of the IBC, Chapter 18.

- The Very Weathered Mancos Shale (VWx) IV, is the Former and Existing Erosional Surfaces and is considered to be 'Fully Softened', for this analysis and includes the slope face.
- The Weathered Mancos Shale (Vwx) V, is considered to be 'Softened', for this analysis.
- The Mancos Shale (Vwx) V, Residual Strength is considered to be 'Fully Softened', for this analysis and represents the anticipated Failure Plane..
- The Slightly Weathered Shale & Siltstone Strata are considered to be 'Slightly Softened', for this analysis.

Slope stability calculations were performed on the existing slopes overlooking the Colorado River and the Deeper Gullies. The stability analysis addressed portions of the individual slopes and the 'global' condition of the entire slope height. The analysis was performed using the PC software SLOPE/W, Version 5.11, Geo-Slope International LTD, Calgary, Alberta, Canada. The Limit Equilibrium Theory for the factor of safety, incorporating the Morgenstern-Price Method which uses both Moment and Force Equilibrium Theory, generally considered to be a relatively rigorous analysis.

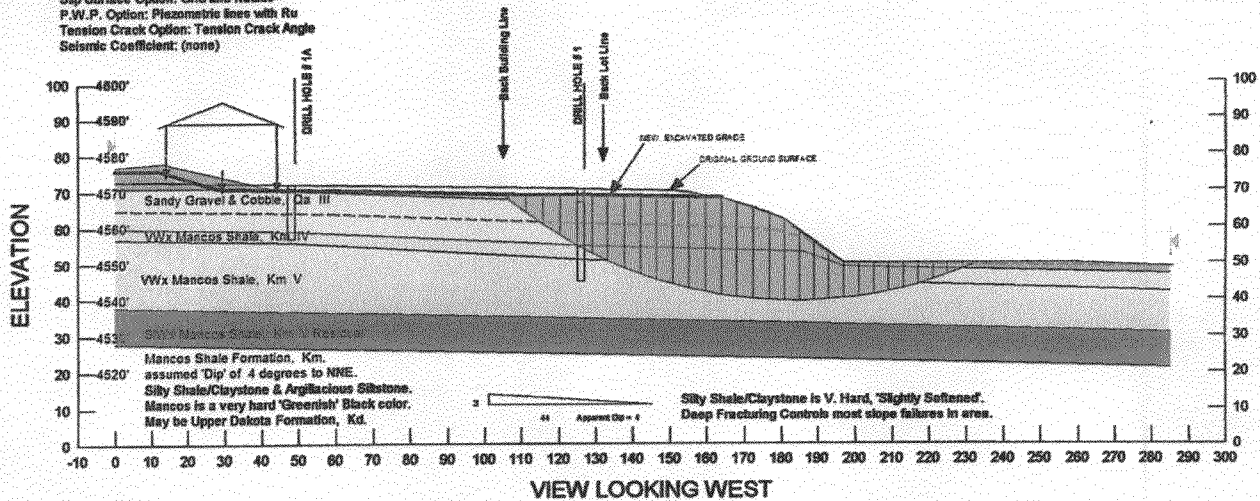


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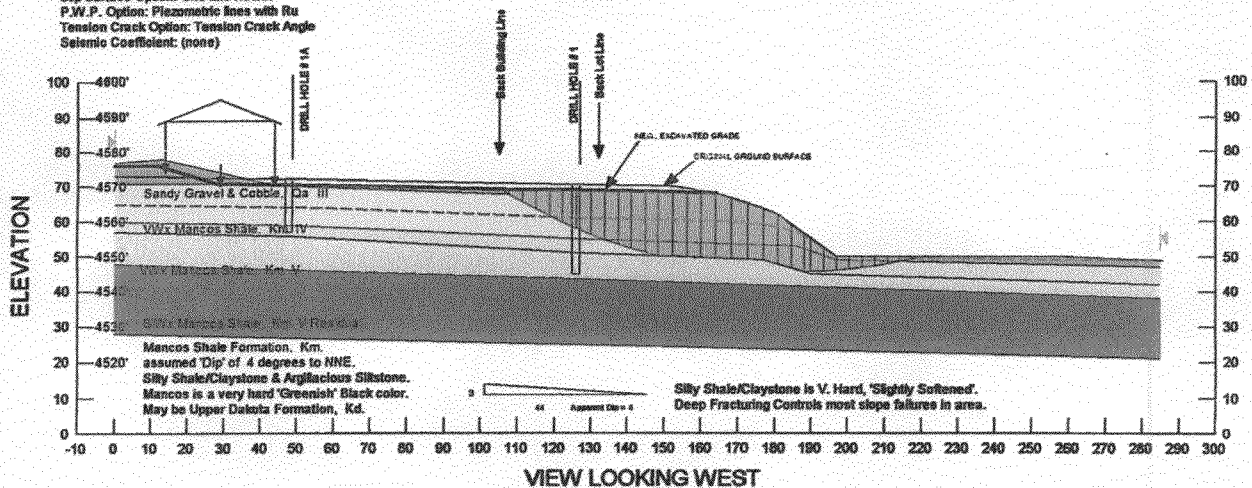
**Figure I-1**

INDEPENDENCE RANCH Sub. Fil. # 10 & 11  
 GJLD # 89144-GJ, March 18, 2003

Independence Ranch Filings # 10 & 11  
 Section # 1, Lots 4/5, Before Grading  
 File Name: 89814-S1-Graded1.siz  
 Last Saved Date: 2/27/2003  
 Analysis Method: Morgenstern-Price  
 Slip Surface Option: Grid and Radius  
 P.W.P. Option: Piezometric lines with Ru  
 Tension Crack Option: Tension Crack Angle  
 Seismic Coefficient: (none)



Independence Ranch Filings # 10 & 11  
 Section # 1, Lots 4/5, Before Grading  
 File Name: 89814-S1-Graded1Bedrock.siz  
 Last Saved Date: 2/27/2003  
 Analysis Method: Morgenstern-Price  
 Slip Surface Option: Grid and Radius  
 P.W.P. Option: Piezometric lines with Ru  
 Tension Crack Option: Tension Crack Angle  
 Seismic Coefficient: (none)

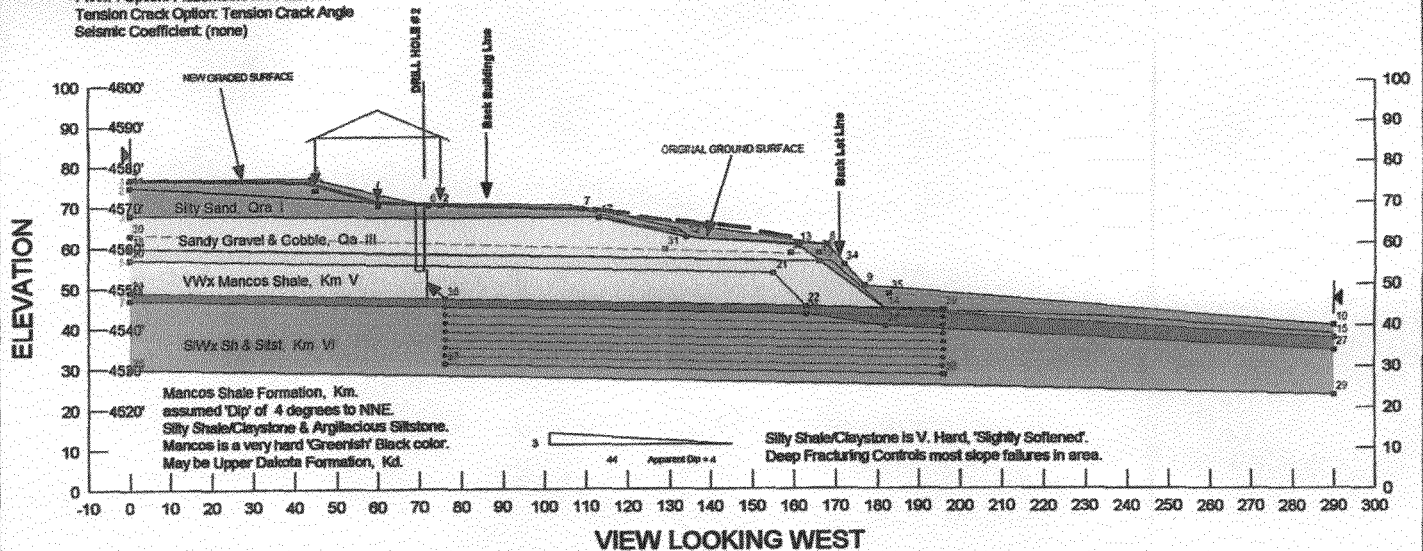
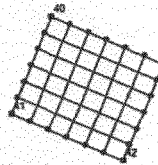


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 GEOTECHNICAL ENGINEERS - GEOLOGISTS

**Figure II-1**

INDEPENDENCE RANCH Sub. Fil. # 10 & 11  
 GJLD # 89144-GJ, March 18, 2003

Independence Ranch Filings # 10 & 11  
 Section # 2, Lot 3/4, Before/After Grading  
 File Name: 89914-S2-Graded1Bedrock.siz  
 Last Saved Date: 2/27/2003  
 Analysis Method: Morgenstern-Price  
 Slip Surface Option: Grid and Radius  
 P.W.P. Option: Piezometric lines with Ru  
 Tension Crack Option: Tension Crack Angle  
 Seismic Coefficient: (none)



**This Study is Along Section S2, Overlooking a Small to Medium Height Slope down to the River Floodplain**

This small drainage appears to have experienced significant seepage within the alluvial soils during previous agricultural activities. The shale encountered in the test boring was very hard.

The Site has been Developed, The Site Grading has Removed Less Than 1 Foot of Alluvial Sands.  
 The Structure has been Constructed as a 'Walkout Basement and the Landscaping is irrigated.  
 The Upper Water Table is Elevated to within 6 feet of the Backyard Surface and Seepage is Occurring at the Slope.  
 Building Loads are Modeled at 1500 plf. For the Interior and 2000 plf for the Exterior.  
 Fill is Placed at the Building Area and Fill is Placed in Some Low Areas Toward the Slope Edge..

The Building/Setback is over 80' From the Back Lot Line & over 75' From the New Crest of the Slope.  
 The Building Setback is Significantly Flatter than the 3:1 (hor : vert) Limit of the IBC, Chapter 18.

The Very Weathered Mancos Shale (VWx) IV, is the Former and Existing Erosional Surfaces and is considered to be 'Fully Softened', for this analysis and includes the slope face.

The Weathered Mancos Shale (Vwx) V, is considered to be 'Softened', for this analysis.

The Mancos Shale (Vwx) V, Residual Strength is considered to be 'Fully Softened', for this analysis and represents the anticipated Failure Plane..

The Slightly Weathered Shale & Siltstone Strata are considered to be 'Slightly Softened', for this analysis.

Slope stability calculations were performed on the existing slopes overlooking the Colorado River and the Deeper Gullies. The stability analysis addressed portions of the individual slopes and the 'global' condition of the entire slope height. The analysis was performed using the PC software SLOPE/W, Version 5.11, Geo-Slope International LTD, Calgary, Alberta, Canada. The Limit Equilibrium Theory for the factor of safety, incorporating the Morgenstern-Price Method which uses both Moment and Force Equilibrium Theory, generally considered to be a relatively rigorous analysis.



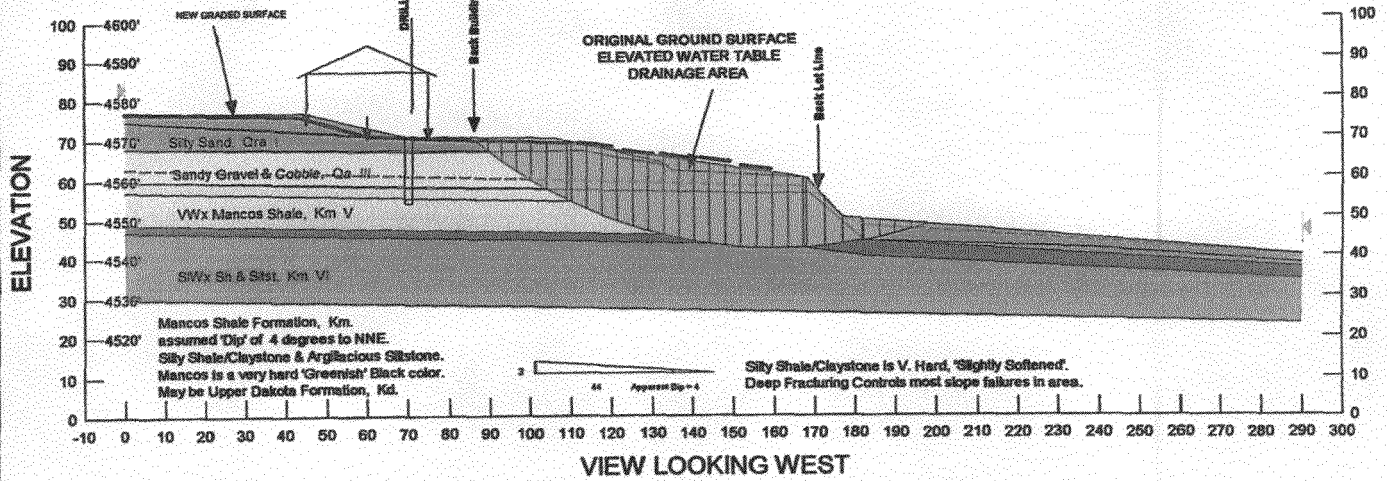
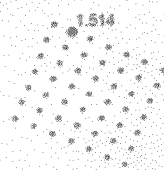
**GRAND JUNCTION  
 LINCOLN DeVORE, Inc.**  
 GEOTECHNICAL ENGINEERS - GEOLOGISTS

**Figure I-2**

INDEPENDENCE RANCH Sub. Fil. # 10 & 11  
 GJLD # 89144-GJ, March 18, 2003

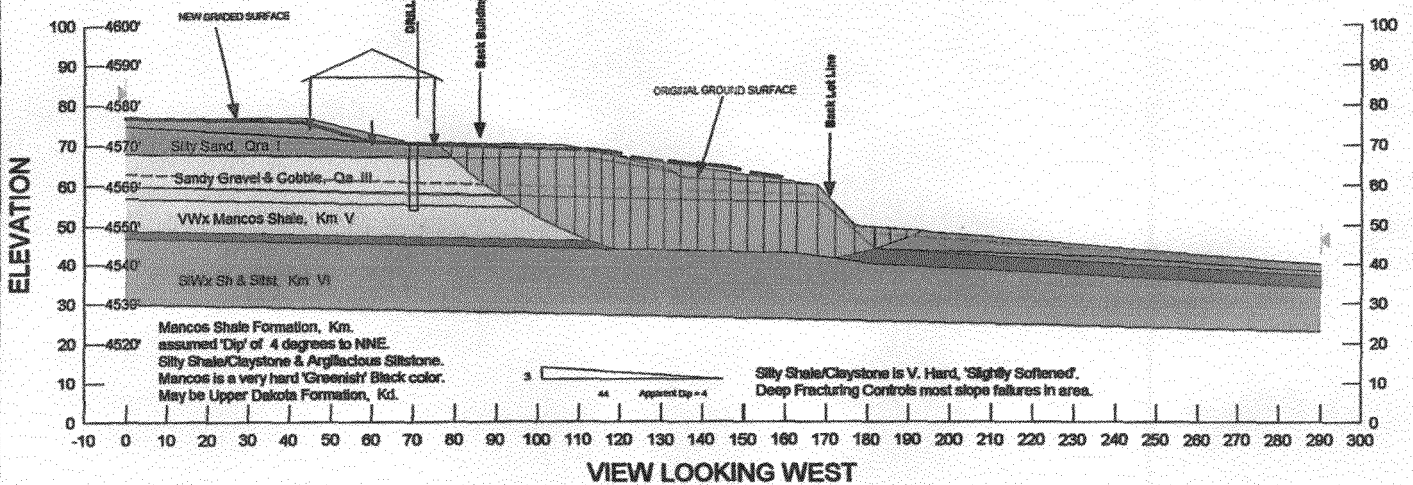
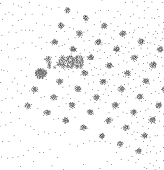


Independence Ranch Filings # 10 & 11  
 Section # 2, Lot 3A, Before/After Grading  
 File Name: 89914-S2-Graded1.stz  
 Last Saved Date: 2/27/2003  
 Analysis Method: Morgenstern-Price  
 Slip Surface Option: Grid and Radius  
 P.W.P. Option: Piezometric lines with Ru  
 Tension Crack Option: Tension Crack Angle  
 Seismic Coefficient: (none)



Probable Failure Mode      Computed S.F. = 1.514

Independence Ranch Filings # 10 & 11  
 Section # 2, Lot 3A, Before/After Grading  
 File Name: 89914-S2-Graded1Bedrock.stz  
 Last Saved Date: 2/27/2003  
 Analysis Method: Morgenstern-Price  
 Slip Surface Option: Grid and Radius  
 P.W.P. Option: Piezometric lines with Ru  
 Tension Crack Option: Tension Crack Angle  
 Seismic Coefficient: (none)



Probable Failure Mode      Computed S.F. = 1.499

*This situation can be mitigated by placing a 14 foot deep drain at the Final North Building Line on the West Side of Lot 3 & the East Side of Lot 4.*



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 LINCOLN DeVORE, Inc.**  
 GEOTECHNICAL ENGINEERS - GEOLOGISTS

Figure II-2

INDEPENDENCE RANCH Sub. Fil. # 10 & 11  
 GJLD # 89144-GJ, March 18, 2003

**STUDY SECTION S1 Building Lot 3 & 4, Filing 10**

*Also 52 445*

**All Soils**

**Soil 1**

Qa/Qc Ib

Soil Model      Mohr-Coulomb

Unit Weight    102

Cohesion       0

Phi 16

Piezometric Line #      2

Ru 0

Pore-Air Pressure       0

**Soil 2**

Silty Sand, Qra I

Soil Model      Mohr-Coulomb

Unit Weight    124

Cohesion       19

Phi 21.3

Unit Wt. above WT      111

Phi B 0

Anisotropic Fn. 0

Piezometric Line #      2

Ru 0

Pore-Air Pressure       0

**Soil 3**

Sandy Gravel & Cobble, Qa III

Soil Model      Mohr-Coulomb

Unit Weight    140

Cohesion       36

Phi 23.2

Unit Wt. above WT      130

Phi B 0

Anisotropic Fn. 0

Piezometric Line #      2

Ru 0

Pore-Air Pressure       0

**Soil 4**

VWx Mancos Shale, Km IV

Soil Model      Mohr-Coulomb

Unit Weight    142

Cohesion       0

Phi 18.8

Unit Wt. above WT      132

Phi B 0

Anisotropic Fn. 0

Piezometric Line #      2

Ru 0

Pore-Air Pressure       0

**Soil 5**

VWx Mancos Shale, Km VI

Soil Model      Mohr-Coulomb

Unit Weight    139

Cohesion       0

Phi 26.6

Unit Wt. above WT      132

Phi B 0

Anisotropic Fn. 0

Piezometric Line #      2

Ru 0

Pore-Air Pressure       0

**Soil 6**

Mancos Shale, Km V Residual

Soil Model      Mohr-Coulomb

Unit Weight    139

Cohesion       0

Phi 18.8

Unit Wt. above WT      132

Phi B 0

Anisotropic Fn. 0

Piezometric Line #      2

Ru 0

Pore-Air Pressure       0

**Soil 7**

SIWx Sh & Sltst, Km VI

Soil Model      Mohr-Coulomb

Unit Weight    142

Cohesion       0

Phi 26.6

Unit Wt. above WT      122

Phi B 0

Anisotropic Fn. 0

Piezometric Line #      0

Ru 0

Pore-Air Pressure       0

**Soil 8**

Bedrock

Soil Model      Bedrock

Piezometric Line #      0

Ru 0

Pore-Air Pressure       0



REVISION	DATE	DESCRIPTION
1	9/9/02	REVISED PER CITY COMMENTS
2	4/14/03	REVISED PER CITY COMMENTS
3	4/30/03	REVISED PER CITY COMMENTS

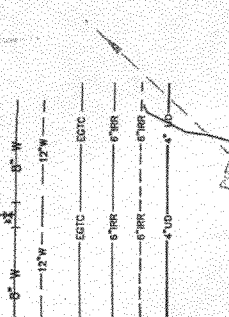
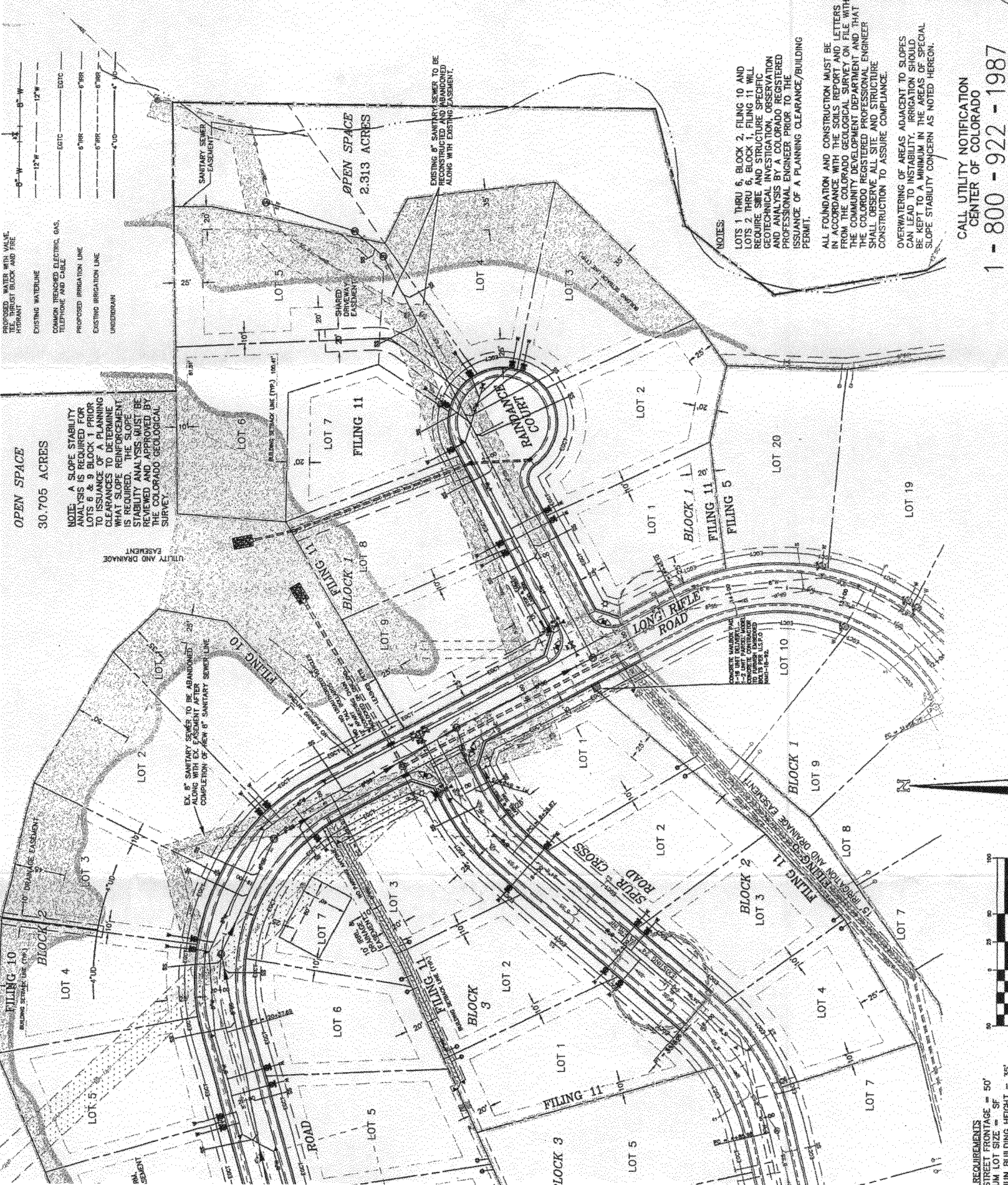
CITY OF GRAND JUNCTION  
 HANS BRUTSCHE

THOMPSON-LANGFORD CORP.  
 ENGINEERS AND LAND SURVEYORS  
 529 25 1/2 RD., SUITE B210  
 GRAND JUNCTION, COLORADO  
 PH. (970) 243-8067  
 FAX (970) 241-2845  
 tl@tlowest.com



CHECKED BY: JEL  
 DRAWN BY: JMS

COMPOSITE SITE PLAN



PROPOSED WATER WITH VALVE, COMMON TRENCH FOR ELECTRIC, GAS, TELEPHONE AND CABLE, PROPOSED IRRIGATION LINE, EXISTING IRRIGATION LINE, UNDERGROUND

OPEN SPACE 30.705 ACRES  
 NOTE: A SLOPE STABILITY ANALYSIS IS REQUIRED FOR ALL LOTS IN BLOCK 1 PLANNING TO ISSUANCE TO DETERMINE CLEARANCE TO DETERMINE WHAT SLOPE REINFORCEMENT IS REQUIRED. THE SLOPE STABILITY ANALYSIS MUST BE REVIEWED AND APPROVED BY THE COLORADO GEOLOGICAL SURVEY.

UTILITY AND DRAINAGE EASEMENT  
 EX. 8" SANITARY SEWER TO BE ABANDONED ALONG WITH EX. EASEMENT AFTER COMPLETION OF NEW 8" SANITARY SEWER LINE

EXISTING 8" SANITARY SEWER TO BE RECONSTRUCTED AND ABANDONED ALONG WITH EXISTING EASEMENT.

EXISTING WATERLINE  
 COMMON TRENCH FOR ELECTRIC, GAS, TELEPHONE AND CABLE  
 PROPOSED IRRIGATION LINE  
 EXISTING IRRIGATION LINE  
 UNDERGROUND

NOTES:  
 LOTS 1 THRU 5, BLOCK 2, FILING 10 AND LOTS 2 THRU 5, BLOCK 3, FILING 11 WILL REQUIRE SITE AND STRUCTURE DESIGN, GEOTECHNICAL INVESTIGATION, OBSERVATION AND ANALYSIS BY A COLORADO REGISTERED PROFESSIONAL ENGINEER PRIOR TO THE ISSUANCE OF A PLANNING CLEARANCE/BUILDING PERMIT.  
 ALL FOUNDATION AND CONSTRUCTION MUST BE IN ACCORDANCE WITH THE SOILS REPORT AND LETTERS FROM THE COLORADO GEOLOGICAL SURVEY ON FILE WITH THE COMMUNITY DEVELOPMENT DEPARTMENT AND THAT THE COLORADO REGISTERED PROFESSIONAL ENGINEER SHALL OBSERVE ALL SITE AND STRUCTURE CONSTRUCTION TO ASSURE COMPLIANCE.  
 OVERWATERING OF AREAS ADJACENT TO SLOPES CAN LEAD TO INSTABILITY. IRRIGATION SHOULD BE KEPT TO A MINIMUM IN THE AREAS OF SPECIAL SLOPE STABILITY CONCERN AS NOTED HEREON.

CALL UTILITY NOTIFICATION CENTER OF COLORADO  
 1-800-922-1987



REQUIREMENTS  
 STREET FRONTAGE = 50'  
 MIN LOT SIZE = SF  
 MIN BUILDING HEIGHT = 35'





DRAWN BY: JMS  
 CHECKED BY: JEL



THOMPSON-LANGFORD CORP.  
 ENGINEERS AND LAND SURVEYORS  
 629 26 1/2 RD., SUITE B210  
 GRAND JUNCTION, COLORADO  
 PH. (970) 243-8067  
 FAX (970) 241-2845  
 tl@tlwest.com

CITY OF GRAND JUNCTION  
 HANS BRUTSCHE  
 INDEPENDENCE PARK - FILMS 10411

COMPOSITE SITE PLAN

REVISION	DATE	DESCRIPTION
1	9/9/02	REVISED PER CITY COMMENTS



NOTES  
 LOTS 4 THRU 6, BLOCK 2, FILING 10 AND LOTS 2 THRU 8, BLOCK 1, FILING 11 WILL BE REDESIGNED AND STRUCTURE SPECIFIC GEOTECHNICAL INVESTIGATION, OBSERVATION AND ANALYSIS BY REGISTERED PROFESSIONAL ENGINEER PRIOR TO THE ISSUANCE OF A PLANNING, CLEARANCE/BUILDING PERMIT.  
 AREA OF SPECIAL SLOPE STABILITY CONCERN. GEOTECHNICAL CONSULTANT MUST REVIEW LINCOLN-DEVORE BLUFF PLAN DATED 10/07/2002.

CALL UTILITY NOTIFICATION CENTER OF COLORADO

See Revised file -

EXISTING SANITARY SEWER WITH MANHOLE AS SHOWN  
 PROPOSED WATER MAIN VALVE, TELE. THIRST BLOCK AND FIRE HYDRANT  
 EXISTING WATERLINE  
 COMMON TRENCHED ELECTRIC, GAS, TELEPHONE AND CABLE  
 PROPOSED IRRIGATION LINE  
 EXISTING IRRIGATION LINE

TRACT RESERVED FOR FUTURE IRRIGATION POND AS NEEDED



LINE CLOSE TABLE

LINE	ARC	PROTUS	DELTA	CHORD BEARING	CHORD
L1	51.48	190.00	15.73 07	N89°13'31"W	51.30
L2	245.62	190.00	78.10 55	S85°42'18"E	27.00
L3	25.74	190.00	857.22 18	S83°32'38"E	13.23
L4	54.48	190.00	4.48 02	N89°09'49"E	31.86
L5	13.24	190.00	11.35 05	N79°38'15"E	20.04
L6	31.95	202.00	5.44 05	N77°11'17"E	63.80
L7	53.95	202.00	18.75 09	N69°55'15"E	54.95
L8	55.28	202.00	18.30 41	S72°45'00"E	69.73
L9	57.40	202.00	18.25 19	S59°43'00"E	97.21
L10	57.71	202.00	18.23 97	S59°26'31"E	99.01
L11	59.36	195.00	23.13 31	S84°56'29"W	16.17
L12	15.18	195.00	5.92 04	N80°34'24"E	16.17
L13	140.05	195.00	50.47 21	N68°54'19"E	136.52

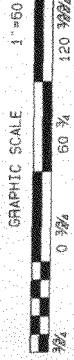
see composite plan.



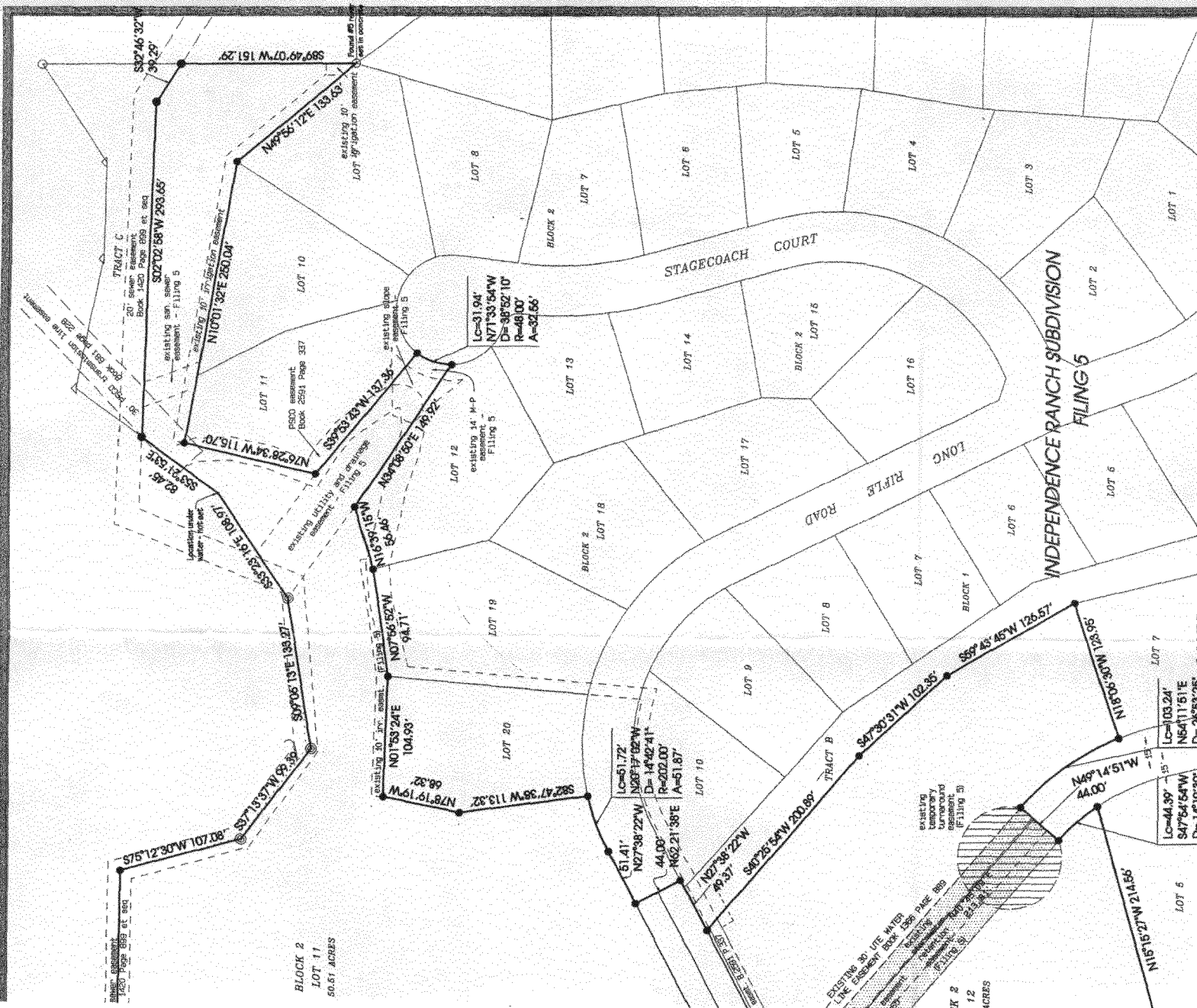
BASIS OF BEARINGS STATEMENT: ALL BEARINGS SHOWN HEREON ARE BASED ON AN ASSUMED BEARING OF N89°06'03"E BETWEEN U.S. G.L.O. BRASS CAPS AT THE SOUTHWEST AND SOUTHEAST CORNERS OF SECTION 35, T.1 N., R.2 E., OF THE UTE MERIDIAN.

LEGEND

- FOUND GLO BRASS CAP
- FOUND CONTROL CORNER AS NOTED
- FOUND MESA COUNTY SURVEY MARKER
- ⊙ FOUND MANHOLE CALLED FOR AS MONUMENT
- ▽ FOUND STEEL POST W/TAG PLS 20677
- FOUND/SET #5 REBAR WALL/RI. CAP THOMPSON-LANGFORD CORP PLS 18478
- ⊠ SET MESA COUNTY SURVEY MARKER IN MONUMENT BOX PLS 18478



INDEPENDENCE RANCH SUBDIVISION  
FLING 10  
HANS BRUTSCHE



BLOCK 2  
LOT 11  
50.51 ACRES

BLOCK 2  
LOT 11  
50.51 ACRES

BLOCK 2  
LOT 11  
50.51 ACRES

BLOCK 2  
LOT 11  
50.51 ACRES