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Fil Da	File       1978-0132A         Date       10/3/00       Project		ct N	Name: <u>Crestview Subdivision – Final Phase I</u>				
P c s e n t	S c a n e d	A few items are denoted with an asterisk (*), which means they are to be scanned for permanent record on the ISYS retrieval system. In some instances, not all entries designated to be scanned are present in the file. There are also documents specific to certain files, not found on the standard list. For this reason, a checklist has been included. Remaining items, (not selected for scanning), will be marked present on the checklist. This index can serve as a quick guide for the contents of each file. Files denoted with (**) are to be located using the ISYS Query System. Planning Clearance will need to be typed in full, as well as other entries such as Ordinances, Resolutions, Board of Appeals, and etc.						
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v	v	Latter from Pon Rich to John Elmer revingermalete week - 5/12/21						
	-	Decent of Final Dist moording						
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X		Final Plat Application	_					
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X		Final Development Plan Application	_					
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X	X	Soil and Foundation Investigation						
X	X	Hydraulic and Hydrologic Analysis of Proposed Dam and Lake						

DEVELOPMENT DEPARTMENT APR 5 1979

### SOIL AND FOUNDATION INVESTIGATION

for

**C-E Maguire, Inc.** Combustion Engineering, Inc. 760 Horizon Drive Grand Junction, Colorado 81501

# CRESTVIEW SUBDIVISION Grand Junction, Colorado



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Architects • Engineers • Planners

## SOIL AND FOUNDATION INVESTIGATION

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### For

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## CRESTVIEW SUBDIVISION Grand Junction, Colorado

For

Henry J. Faussone

and

Noel B. Norris

by

C-E MAGUIRE, INC. 760 Horizon Drive Grand Junction, Colorado

> March 1979 78 2 ARC 0223

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This report presents results of our subsoil investigation between 15th Street and 27-1/2 Road. The purpose of this investigation was to determine those soil conditions and characteristics which would affect the utility of the soils and foundation design of the proposed structures. Data gathered through field and laboratory work are summarized and tabulated in Exhibit Nos. 1 through No. 7 attached.

#### FIELD INVESTIGATION

Test borings were made February, 1979, at the locations shown on Exhibit No. 1, to obtain data concerning existing soil conditions and to obtain samples for laboratory use. 4 holes were excavated with a tractor backhoe which made a hole large enough to enable visual inspection and obtain undisturbed samples for laboratory use. The test excavations were located to best reflect representative general conditions at the site, and to obtain specific data at each location sampled.

Test excavations were taken to the approximate depths below the surface shown on Exhibit No. 2. Undisturbed samples of each soil type encountered were obtained for laboratory analysis. As each excavation progressed, a log was kept on which was recorded such information as field classification of soils, sample locations, depth to groundwater table, if any, and other pertinent data. These logs are reproduced on Exhibit No. 2.

#### LABORATORY INVESTIGATION

The laboratory phase of the investigation included the verification of field soil classification, determination of soil gradations by mechanical

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#### SCOPE

analysis, Atterberg Limits, natural moisture content, and consolidationswell characteristics of foundation material. These test results are summarized on Exhibit No. 7.

#### SITE CONDITIONS

The site under investigation is located in a moderately populated portion of Grand Junction, Colorado. It is bounded on the west by 15th Street and the east by 27-1/2 Road. The topography is considered gently rolling with surface drainage in a southwesterly direction to a drainage ditch which traverses the site northeast to southwest. A low area exists in that portion represented by test pit 50, which is presently cultivated farm land. Additional grading and landscaping will have to be accomplished in order to provide adequate drainage away from the proposed structures.

#### GROUNDWATER

Evidence of groundwater was encountered at the time of the investigation at the depths shown on the accompanying test boring log exhibit. It is anticipated that groundwater may flucuate appreciably during the irrigation season and periods of high precipitation. Accordingly, some means of lowering and stabilizing groundwater will have to be accomplished. Methods of doing this include an underdrain system underneath sanitary sewer mains with perimeter drains located around footings. Perimeter drains should discharge into the underdrain system which may be relieved by a drainage off-site if topography permits.

#### SUBSOILS

Our analysis reveals the soils are uniform over the area investigated. The soils consist of fine grained silts with some sand. These materials are

quite unstable at their present moisture content and are very difficult to compact unless the moisture content is controlled. Capillary rise is very rapid in these soils and thus are subject to detrimental frost heave.

#### BUILDING FOUNDATIONS

Our analysis of field conditions and test results indicates that footings placed below maximum frost penetration and in natural undisturbed soil should be designed for a maximum allowable bearing capacity of 500 pounds per square foot incuding live load. Settlement in the order of three quarters of an inch may be anticipated.

#### FLOOR SLABS AND OTHER SLABS ON GRADE

The foundation soils show no tendency to shrink or swell, however, since little uplift pressure or differential settlement is required to cause unsightly cracks in floor slabs, the following precautionary measures are deemed necessary:

- Compact material underneath floor slabs at or slightly below optimum moisture content.
- 2. Preclude the entrance of an outside water source underneath slabs.
- Eliminate underslab plumbing where possible and where unavoidable, thoroughly pressure test and take other precaution necessary to minimize leaks.
- Separate floor slabs completely from bearing walls, columns and footings.

- 5. A six inch thickness of clean sand covered with a 4 mil. thickness of sealed plastic sheeting should be placed directly beneath the floor slab to act as a vapor barrier and a capillary break for groundwater.
- Appropriate provision should be made in large slabs for shrinkage cracks.

#### TREATMENT OF FOUNDATION SOILS

Precautions should be taken to assure that the moisture content of the foundation soils is maintained at a relatively constant level. Excavations shall not be allowed to remain open long enough to allow appreciable drying below natural moisture content, and the exposed foundation material should be protected from wetting from any outside source.

Wetting of foundation soils should be prevented after construction. Methods of accomplishing this include thorough compaction of all backfill around structures, water collection systems well beyond the limits of all backfill, and any other procedures deemed necessary to maintain a stable moisture content. Excavations should be made only large enough to provide necessary working space in order to hold the area requiring backfilling to a minimum.

#### SULFATE RESISTANT CEMENT

Analysis indicates a sulfate concentration in excess of 0.10 percent water soluble sulfate  $(SO_4)$  in the soil samples. Therefore, type II cement should be used in concrete exposed to this soil.

#### LIMITATIONS

The exploratory data presented in this report were collected to help develop designs and cost estimates for this project, and thus may not represent adequate information for indicating underground conditions for contractor bidding or construction. We recommend considering exploratory work to reveal underground conditions well enough to enable contractors to more accurately evaluate conditions for bidding and execution of work after designs have been prepared.

Professional judgements on design alternatives and criteria are presented in this report. The judgements are based upon our evaluation of actual conditions encountered at the location indicated herein, and upon our extrapolations thereof, together with our interpretations of conditions generally characteristic of this area. We do not warrant the accuracy of such extrapolations and interpretations beyond the limits of the tests performed or where actual physical conditions were not observed.

Excavation for this investigation was located to obtain a reasonably accurate respresentation of subsurface conditions for design purposes. Variations from the conditions disclosed which were not indicated by the test explorations frequently occur and quite often these variations are sufficient to necessitate modifications in design. Therefore, if different materials are encountered the owner or builder should be certain that the foundation conditions are adequate and within the scope of this report prior to proceeding with any construction.

Under the above conditions, it is important that we inspect the subsurface materials exposed in excavations to take advantage of all opportunities to recognize differing conditions and minimize the risk of having undetected conditions which would affect the performance of the facility.

If you have any questions or are in need of further information regarding this report, please feel free to contact us.

Respectfully,

C-E MAGUIRE, INC.

Prepared by:

1 Les rthur F. Uhrich

Approved by:

George B. Kellison, P.E.

AFU/pi





TEST BORING LOCATION MAP

782ARC 0223

EXHIBIT NO. I



## TEST BORING LOGS

782 ARC 0223

## EXHIBIT NO. 2



Tube 66 In-place dry density 105.6 #/ft<sup>3</sup> Natural Moisture 10.7% Voids ratio 0.60 Soil type ML

78 2 ARC 0223



78 2 ARC 0223



Tube 50 In-place dry density 97.2 #/ft<sup>3</sup> Natural moisture 21.2% Voids ratio 0.70 Soil type ML

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APPLIED LOAD

Tube 74 In-place dry density 96.9 #/ft<sup>3</sup> Natural moisture 24.4% Voids ratio 0.71 Soil type SM

78 2 ARC 0223

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#### SOILS INVESTIGATION SUMMARY SHEET

Hole No.	Depth (feet)	Natural Moisture Content(%)	Atterb Liquid Limit	erg Limits Plasticity Index	Percent Passing #200 Sieve	Initial Void Ratio	In-Place Dry Density Lbs./Cu.Ft.	Soil Type (Unified)
50	4	21.2	19.9	2.5		0.70	97.2	ML
55	. 5	18.9	` NV	NP	43.0	0.66	101.5	SM
6_ )	5	10.7	NV	. NP	69.0	0.60	105.6	ML
74	4	24.4	NV	NP	18.0	0.71	96.9	SM

Note: These tests were chosen as representative of all the soil types encountered and grouped as shown on Exhibit No. 2.

Exhibit #7

#### CRESTVIEW SUBDIVISION

#### HYDRAULIC AND HYDROLOGIC ANALYSIS OF PROPOSED DAM AND LAKE

Crestview Subdivision is located in the City of Grand Junction between 27-1/2 Road and 15th Street south of Bellridge Subdivision.

A proposed dam is to be built across the wash which flows into the Subdivision at the northeast corner. The wash continues in a southwest direction where it flows along the southern border of the Subdivision until it enters another channel approximately 80 feet east of 15th Street.

The wash drains an area of  $\pm$  65 acres northeast of the Subdivision. This area is identified as part of Subbasins 44 and 46 of the Patterson Road Basin in the 1975 <u>Master Drainage Plan</u> written by Nelson, Haley, Patterson and Quirk for the City of Grand Junction.

The following hydrologic data (identical to that used in Subbasin 46) was used for determining the peak runoff for the wash:

L = length along stream from study point to upstream limits of basin	.62 miles
L <sub>ca</sub> = length along stream from study point to centroid of basin	.28 miles
C <sub>t</sub> = coefficient reflecting time to peak	.34
C <sub>p</sub> = coefficient relating to peak rate of runoff	.54
100 year, 1 hour storm precipitation	1.6 inches

The Colorado urban hydrograph procedure was used to calculate a peak runoff flow of approximately 98 cfs for the wash basin.

27-1/2 Road bridges the wash approximately 4 feet above the flowline of the channel. A culvert, partially filled with sediment, drains the wash under the road.

The dam will be equipped with an intake bypass structure and channel to divert irrigation water heavily laden with sediment around the lake and back into the original wash, downstream of the dam. A control outlet structure to keep the level of the lake at a constant depth will feed into a separate drain line. This line will be placed to empty the entire lake in case of repairs, etc.

The spillway will be placed across the road that runs on the top of the dam. It will need to have an approximate capacity of 150 cubic feet per second. This flow has a factor of safety of 1.5 over the peak runoff. The spillway will carry the water to the backside of the dam which will be riprapped for bank stabilization. COUNTY PLANNING & DEVELOPMENT PROCESSING WAR AND COUNTY BUILDING PERMIT & INSPECTION

City County Development Department

CITY OF GRAND JUNCTION-MESA COUNTY-COLORADO 81501 359 WHITE AVE -ROOM 60-DIAL (303) 243-9200 EXT. 343

February 1, 1980

Henry J. Faussone 688 26<sup>1</sup>/<sub>2</sub> Rd. Grand Junction, Co 81501

Dear Sir:

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On January 29, 1980 the Grand Junction Planning Commission voted to recommend approval of your petition - Crestview Subdivision - Replat lots 5-14.

This approval is subject to staff and review sheet comments being addressed before the City Council hearing on February 20, 1980, at 7:30 p.m.

Please be present or have a representative in attendance.

Sincerely, UE R Sue Drissel Planning Tech. I

cc file #132-78

Noel B. Norris



May 12, 1981

Naryl Shum

Mr. John Elmer ARIX 760 Horizon Drive Grand Junction, CO 81501

Dear John:

RE: Crestview Subdivision - Filing No. 1

The streets and storm sewers constructed in the above subdivision were finalinspected on August 13, 1980, and November 7, 1980, and my recent reinspection showed that all construction deficiencies have been corrected. There are a few items of incomplete work but according to my recent discussions with Mr. Norris these are to be handled as follows:

- 1. The streetside sidewalks along Crest View Way fronting Lots 16 and 17 will be constructed according to plan when those lots are developed as "Crest View Townhomes".
- 2. A street light post near the northeast corner of Lot 16 will have to be relocated out of the path of the aforementioned sidewalk.
- 3. The storm drainage system through Lot 16 will be constructed as part of Crest View Townhomes and will replace the temporary connection of the storm outlet pipe from Crest View Court which presently is tied into a manhole of the subdrain system. The storm drain system will be in reasonable conformity to the "Grading and Drainage Plan" submitted by Paragon Engineering on February 2, 1981.
- 4. Curbramps were not installed on the corners at Crest View Way and 15th Street. This will be done either when 15th Street is improved or when the sidewalks for Crest View Townhomes are constructed, whichever occurs first.
- 5. Several lots in the subdivision- do not yet have curb cuts and driveway approach aprons. As discussed with Mr. Norris, he is responsible for insuring this work is accomplished and is making arrangements with each property owner as they purchase a lot to have the aprons installed to City Standards. The construction will be controlled and inspected through City Permit system for curbcuts.

Page 2 - Crestview Subdivision - Filing No. 1

We have received the as-built drawings for the improvements which acknowledge the facilities have been constructed in accordance with the approved plans and specifications.

We have received all required construction test results. We also have a letter from Corn Construction Company dated December 12, 1980, whereby they guarantee the uncoated corrugated steel culvert pipe for 10 years from installation. This letter is necessary since City Specifications require coating on corrugated steel pipes.

Powers of Attorney have been recorded for street improvements on 27 1/4 Road (12th Street) and 27 1/2 Road and an easement has been recorded for the storm drain between Lots 10 and 11.

In light of the above, the streets and storm drainage facilities constructed in Crestview Subdivision - Filing No. 1 are accepted by the City, and we are now responsible for maintenance of those facilities.

Very truly yours ish Ø na

Ronald P. Rish, P.E. City Engineer

RPR/rs

cc: Bill Norris
Del Beaver- Paragon
Ed Settle - Corn Construction
John Kenney
Jim Patterson
Daryl Shrum ✓

### REVIEW SHEET SUCAPY

FILE # 132-78	<del></del>	DATE SENT TO REVIEW AGENCIES
ITE: CRESTVIEW	- REVISION	DATE DUE
PC MEETING DATE		
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<u>12/18/79 GJPC</u>

RIDER/GRAHAM/PASSED 4-0 TO RECOMMEND APPROVAL OF THE REVISION.

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Acres 18.2	File # /32
Units <u>20</u> ACTION S	HEET Zone PD-8
Density 8	Tax Area Code
Activity PD-8 Cresturew	
Phase <u>FINAL - PHASE L</u>	Date Neighbors Notified//A
Date Submitted 2 March 19	Date CIC/MCC Legal Ad
Date Mailed Out 2 March 79	PC Hearing Date 27 MARCh 19
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Send	Send
COUNTY ROAD DEPARIMENT	V FIRE City
COUNTY HEALTH DEPARTMENT	
	DRAINAGE G () Proint
COMTRONICS	
GRAND VALLEY RURAL POWER -	
MOUNTAIN BELL	<u>CITY ENGINEER</u>
PUBLIC SERVICE	MACK, LOMA, MESA, COLLBRAN
SOIL CONSERVATION SERVICE	FRUITA, PALISADE
SCHOOL DISTRICT 51	V CITY UTILITIES - JENSEN
STATE HIGHWAY	V P.D. Ed Vander Took
STATE GEOLOGICAL	V Ken Idleman
STATE HEALTH - RADIOLOGICAL	
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Date Submitted <u>S Dec 17</u>	Date CIC/MCC Legal Ad
Date Mailed Out 5 Dec - 99	PC Hearing Date <u>18 Dec 17</u>
Review Agencies 104 Review	ew Period - Return By
Send	Send
COUNTY ROAD DEPARIMENT	FIRE
COUNTY HEALTH DEPARIMENT	IRRIGATION
COUNTY SURVEYOR	DRAINAGE
COMTRONICS	WATER (UTE, CLIFTON)
GRAND VALLEY RURAL POWER	SEWER
MOUNTAIN BELL	CITY ENGINEER/UTILITIES
PUBLIC SERVICE	MACK, LOMA, MESA, COLLBRAN
SOIL CONSERVATION SERVICE	FRUITA, PALISADE
SCHOOL DISTRICT 51	
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Power of Attorney	
Dev. Schedule	

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Cormon Staff	2.20.80 Location Lot Location Lot	Approved 5 5-14 5 5+ 4	$p \neq Cres$	st view 5. 2	5ab. 10 5f F¥y	cated at: Rd.	
	2.20.80 Location Lot Location Lot Comments	Approved 5 5-14 4 57 4	$p \neq Cres$	t view 5. 2	Sab. 10 55 F 44	cated at: Rd.	
	2.20.80 Location Lot Location Lot Comments	Approved 5 5-14 4 54, 4	pf Cres	it view -	5ab. 10 55 FYy	cated at : Rd.	
Cormon Staff	2.20.80 Location Lot Location Lot Comments	Approved 5 5 - 14 4 57 4	$p \neq Cres$	st view 3	5ab. 10 5f FYy	cated at: Rd.	
	2.20.80 Location Lot Location Lot Comments Comments	Approved 5 5-14 4 54 4	$p \neq Cres$	st view 2d, 5. 2	5ab. 10 5f F 44	cated at: Rd.	
	2.20.80 Location Lot Location Lot Comments Comments Agreement	Approved 5 5 - 14 4 57 94 5 5 - 14 5 5 5 5 - 14 5 5 5 5 - 14 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5		1 x .05 = \$_	5ab. 10 5f Fyy	pen Space;	
	2.20.80 Location <u>Lot</u> <u>Location Lot</u> <u>Location JS</u> <u>Comments</u> <u>Al Documents</u> Imp. Agreement Imp. Guarantee	Approved         s       s - 14         b       st         s       s - 14         b       st         s       s - 14         b       st         s       s - 14         s       s - 14         b       st         s       s - 14         s       s - 14         s       s - 14	27 <sup>y</sup> 2 <sup>k</sup>	1 x .05 = \$_ #	5ab./c 5ab./c 5f FYy 0 0 heck #0	pen Space;	
	2.20.80 Location Lot Location Lot Location Lot Location JS Comments Agreements Imp. Agreement Imp. Guarantee Govenants	Approved     s	Appraisa Receipt	1 x .05 = \$_ #Cr ce Dedication	Sab. /0 Sab. /0 Sab	pen Space;	
	2.20.80 Location Lot Location Lot Comments Comments Agreement Imp. Agreement Imp. Guarantee Govenants Power of Attor	Approvid         s       5 - 14         s       5 - 14         t4       54         t4       54         s       5	Appraisa Receipt so Open Space	1 x .05 = \$_ 	Sab. /0 of F <sup>1</sup> /y O O O	cated at: Rd.	