

# Table of Contents

File 1994-0085

Name: Grandview Subdivision - Preliminary Final Plan - 28 and F ¼ Road

<b>P r e s e n t</b>	<b>S c a n n e d</b>	<p>A few items are denoted with an asterisk (*), which means they are to be scanned for permanent record on the ISYS retrieval system. In some instances, items are found on the list but are not present in the scanned electronic development file because they are already scanned elsewhere on the system. These scanned documents are denoted with (**) and will be found on the ISYS query system in their designated categories.</p> <p>Documents specific to certain files, not found in the standard checklist materials, are listed at the bottom of the page.</p> <p>Remaining items, (not selected for scanning), will be listed and marked present. This index can serve as a quick guide for the contents of each file.</p>
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X	X	<b>Table of Contents</b>
		<b>*Review Sheet Summary</b>
X	X	<b>*Application form</b>
X		Review Sheets
X		Receipts for fees paid for anything
X	X	<b>*Submittal checklist</b>
		<b>*General project report</b>
		Reduced copy of final plans or drawings
		Reduction of assessor's map.
		Evidence of title, deeds, easements
		<b>*Mailing list to adjacent property owners</b>
		Public notice cards
		Record of certified mail
X		Legal description
		Appraisal of raw land
		Reduction of any maps – final copy
X	X	<b>*Final reports for drainage and soils (geotechnical reports)</b>
		Other bound or non-bound reports
		Traffic studies
X	X	<b>*Review Comments</b>
X	X	<b>*Petitioner's response to comments</b>
X	X	<b>*Staff Reports</b>
		<b>*Planning Commission staff report and exhibits</b>
		<b>*City Council staff report and exhibits</b>
		<b>*Summary sheet of final conditions</b>

**DOCUMENT DESCRIPTION:**

X	X	Landscaping Standards	X	X	Traffic Study – 6/30/94
X	X	Preliminary Development Plan – 5/1/93	X	X	Planning Commission Minutes – 9/6/94 - **
X		Planning Commission Notice of Public Hearing mail-out – 9/6/94	X		Posting of Public Notice Signs
X		Declaration of Covenants and Restrictions-BK 2143/Pg 753	X	X	Landscape Plan – to be scanned
X		Chicago Title Ins. Co. – Commitment for Title Ins.	X	X	Plat – GIS Historical Maps - **
X	X	Stormwater Management			a. Street Plan
X	X	Roadway Improvements			b. Street Profiles
X	X	Correspondence			c. Street Details
X	X	DIA – Bk 2143/Pg 743 - **			d. Sewer & Water Plan
X	X	Avigation Easement – not recorded – scanned with file			e. Sewer Profiles
X		Preliminary Drainage Report – 5/1/94			f. Sewer Details
X	X	Subsurface Soils Exploration – 6/29/94			g. Water Details
X	X	Hydraulic Calculations			

# SUBMITTAL CHECKLIST

## MAJOR SUBDIVISION: PRELIMINARY

Location: 28 ROAD & (E 1/4) Hawthorne

Project Name: Grand View Subdivision

ITEMS		DISTRIBUTION																				TOTAL REQ'D.						
DESCRIPTION	ID REFERENCE	City Community Development	City Dev. Eng.	City Utility Eng.	City Property Agent	City Parks/Recreation	City Fire Department	City Attorney	City G.J.P.C. (8 sets)	City Downtown Dev. Auth.	City Police	County Planning	Walker Field	School Dist. #51	Irrigation District	Drainage District	Water District <u>UTE</u>	Sewer District	U.S. West	Public Service	GVRP		CDOT	Corps of Engineers	Colorado Geological Survey	US Postal Service	Persigo WWTF	
● Application Fee	VII-1	1																										
● Submittal Checklist*	VII-3	1																										
● Review Agency Cover Sheet*	VII-3	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
● Application Form*	VII-1	1	1	1	1	1	1	1	8	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
● Assessor's Map	VII-1	1	1	1	1	1	1	1	8	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
● Evidence of Title	VII-2	1		1			1																					
● Names and Addresses	VII-3	1																										
● Legal Description	VII-2	1		1																								
● General Project Report	X-7	1	1	1	1	1	1	1	8	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
● Location Map	IX-21	1																										
● Preliminary Plan	IX-26	1	2	1	1																							
● 11"x17" Reduction of Prelim. Plan	IX-26	1			1	1	1	3	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
● Preliminary Drainage Report	X-12	1	2																									

#85 94  
 \$63000 plus Acreage Fees  
 (1500 per acre)  
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- NOTES:
- 1) An asterisk in the item description column indicates that a form is supplied by the City.
  - 2) Required submittal items and distribution are indicated by filled in circles, some of which may be filled in during the pre-application conference. Additional items or copies may be subsequently requested in the review process.
  - 3) Each submitted item must be labeled, named, or otherwise identified as described above in the description column.

PRE-APPLICATION CONFERENCE

Date: 3-31-94

Conference Attendance: TOM Logue / DAVE THORNTON

Proposal: \_\_\_\_\_

Location: ZB ROAD between Hawthorne & Ridge Drive

Tax Parcel Number: \_\_\_\_\_

Review Fee: 630<sup>00</sup> plus 15<sup>00</sup> per acre

(Fee is due at the time of submittal. Make check payable to the City of Grand Junction.)

Additional ROW required? Along ZB ROAD (30' from centerline)

Adjacent road improvements required? Along ZB ROAD AT FINAL APPROVAL

Area identified as a need in the Master Plan of Parks and Recreation? Possible PARK site

Parks and Open Space fees required? YES AT FINAL Estimated Amount: 225<sup>00</sup> per unit

Recording fees required? YES AT FINAL Estimated Amount: \_\_\_\_\_

Half street improvement fees required? YES or (be built if approved) Estimated Amount: \_\_\_\_\_

Revocable Permit required? NA

State Highway Access Permit required? NA

Applicable Plans, Policies and Guidelines NA

Located in identified floodplain? FIRM panel # \_\_\_\_\_

Located in other geohazard area? \_\_\_\_\_

Located in established Airport Zone? Clear Zone, Critical Zone, Area of Influence? YES

Avigation Easement required? YES AT FINAL PLAT

While all factors in a development proposal require careful thought, preparation and design, the following "checked" items are brought to the petitioner's attention as needing special attention or consideration. Other items of special concern may be identified during the review process.

- Access/Parking
- Drainage
- Floodplain/Wetlands Mitigation
- Other \_\_\_\_\_
- Screening/Buffering
- Landscaping
- Availability of Utilities
- Land Use Compatibility
- Traffic Generation
- Geologic Hazards/Soils

Related Files: \_\_\_\_\_

It is recommended that the applicant inform the neighboring property owners and tenants of the proposal prior to the public hearing and preferably prior to submittal to the City.

PRE-APPLICATION CONFERENCE

WE RECOGNIZE that we, ourselves, or our representative(s) must be present at all hearings relative to this proposal and it is our responsibility to know when and where those hearings are.

In the event that the petitioner is not represented, the proposed item will be dropped from the agenda, and an additional fee shall be charged to cover rescheduling expenses. Such fee must be paid before the proposed item can again be placed on the agenda. Any changes to the approved plan will require a re-review and approval by the Community Development Department prior to those changes being accepted.

WE UNDERSTAND that incomplete submittals will not be accepted and submittals with insufficient information, identified in the review process, which has not been addressed by the applicant, may be withdrawn from the agenda.

WE FURTHER UNDERSTAND that failure to meet any deadlines as identified by the Community Development Department for the review process may result in the project not being scheduled for hearing or being pulled from the agenda.

X \_\_\_\_\_  
Signature(s) of Petitioner(s)

X \_\_\_\_\_  
Signature(s) of Representative(s)





PRE-APPLICATION CONFERENCE

Date: 6/30/94
Conference Attendance: Michael Drollinger, Tom Logue
Proposal: Final Plat - Filing # 1
Location: 28 Road N of Patterson Rd.

Tax Parcel Number:
Review Fee: \$ 720 + \$15/acre
(Fee is due at the time of submittal. Make check payable to the City of Grand Junction.)
Related Files: # 85-94

Additional ROW required? Yes
Area identified as a need in the Master Plan of Parks and Recreation? No
Parks and Open Space fees required? Yes - to be paid at time of recording Estimated Amount: \$225/unit
Recording fees required? Yes Estimated Amount:
Adjacent Half street improvements/fees required? TCP - Transportation Capacity Payment
Revocable Permit required? ?
State Highway Access Permit required? No

Applicable Plans, Policies and Guidelines Zoning & Devel. Code
Located in identified floodplain? FIRM panel # No
Located in other geohazard area? ?
Located in established Airport Zone? Clear Zone, Critical Zone, Area of Influence? - Possibly

Avigation Easement required? ?

While all factors in a development proposal require careful thought, preparation and design, the following "checked" items are brought to the petitioner's attention as needing special attention or consideration. Other items of special concern may be identified during the review process.

- Access/Parking, Drainage, Floodplain/Wetlands Mitigation, Other, Screening/Buffering, Landscaping, Availability of Utilities, Land Use Compatibility, Traffic Generation, Geologic Hazards/Soils

It is recommended that the applicant inform the neighboring property owners and tenants of the proposal prior to the public hearing and preferably prior to submittal to the City.

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Signatures: X [Signature] Signature(s) of Petitioner(s) X [Signature] Signature(s) of Representative(s)



**DEVELOPMENT APPLICATION**  
 Community Development Department  
 250 North 5th Street Grand Junction, CO 81501  
 (303) 244-1430

Receipt 1187  
 Date 5-3-94  
 Rec'd By, MP  
 File No. 85 94

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*Preliminary PLAN/PLAT*

We, the undersigned, being the owners of property situated in Mesa County, State of Colorado, as described herein do hereby petition this:

PETITION	PHASE	SIZE	LOCATION	ZONE	LAND USE
<input checked="" type="checkbox"/> Subdivision Plat/Plan	<input type="checkbox"/> Minor <input checked="" type="checkbox"/> Major <input type="checkbox"/> Resub	66.7	NW 28 Rd. & Hawthorn Ave	RSF-5	Residential
<input type="checkbox"/> Rezone				From: To:	
<input type="checkbox"/> Planned Development	<input type="checkbox"/> ODP <input checked="" type="checkbox"/> Prelim <input type="checkbox"/> Final				
<input type="checkbox"/> Conditional Use					
<input type="checkbox"/> Zone of Annex					
<input type="checkbox"/> Text Amendment					
<input type="checkbox"/> Special Use					
<input type="checkbox"/> Vacation					<input type="checkbox"/> Right-of-Way <input type="checkbox"/> Easement

<input checked="" type="checkbox"/> PROPERTY OWNER	<input checked="" type="checkbox"/> DEVELOPER	<input checked="" type="checkbox"/> REPRESENTATIVE
<i>Ben Carnes &amp; Discovery, 76 Corp.</i>	<i>Don Dela Motte</i>	<i>Thomas A. Logue</i>
Name	Name	Name
<i>2499 U.S. Hwy. 50</i>	<i>634 Avalon Dr.</i>	<i>227 So. 9th Street</i>
Address	Address	Address
<i>Grand Jct. CO. 81501</i>	<i>Grand Jct. CO 81504</i>	<i>Grand Jct. CO. 81501</i>
City/State/Zip	City/State/Zip	City/State/Zip
<i>243-6456</i>	<i>234-6224</i>	<i>245-4099</i>
Business Phone No.	Business Phone No.	Business Phone No.

NOTE: Legal property owner is owner of record on date of submittal.

We hereby acknowledge that we have familiarized ourselves with the rules and regulations with respect to the preparation of this submittal, that the foregoing information is true and complete to the best of our knowledge, and that we assume the responsibility to monitor the status of the application and the review comments. We recognize that we or our representative(s) must be present at all hearings. In the event that the petitioner is not represented, the item will be dropped from the agenda, and an additional fee charged to cover rescheduling expenses before it can again be placed on the agenda.

*Thomas A. Logue* Signature of Person Completing Application Date 4/26/94  
*Frank Wisley Jr. Pres Discovery 76* Signature of Property Owner(s) - Attach Additional Sheets if Necessary Date 4/28/94

\*\*\*\*\*

SECTION 6, TOWNSHIP 1 SOUTH, RANGE 1 EAST, UTE MERIDIAN

NE 1/4 SW 1/4 NW 1/4,

SE 1/4 SW 1/4 NW 1/4,

NW 1/4 SW 1/4,

SW 1/4 SW 1/4 NW 1/4, EXCEPT Beginning at a point being the W 1/2 Section 6, Township 1 South, Range 1 East, Ute Meridian, thence North 536.25 feet, thence East 165.0 feet, thence South 210.25 feet, thence 261.8 feet along the arc of a 50 foot radius curve to the right, the chord of which bears South 50 feet, thence South 276.0 feet to the South line of the NW 1/4 Section 6, Township 1 South, Range 1 East, Ute Meridian, thence West 165.00 feet to the point of beginning,

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PRELIMINARY DEVELOPMENT PLAN

FOR:

***GRAND VIEW SUBDIVISION***

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MAY 1, 1993



A LanDesign Partner

**PRELIMINARY PLAN for: GRAND VIEW SUBDIVISION**

**INTRODUCTION** - Grand view Subdivision site is located 1/4 mile north of Patterson Road, east of 28 Road in the City of Grand Junction. The 64.8 acre site was formerly known as "Fox Estates" which gained a change in zoning and Preliminary Plan approval from Mesa County in 1980. The approved development application consisted of a "mixed use" type development. A land use summary of the existing development plan follows:

<b>EXISTING LAND USE SUMMARY</b>		
<b>USE</b>	<b>NO. OF DWELLING UNITS</b>	<b>% OF TOTAL UNITS</b>
SF DETACHED	114	14.7
TOWNHOMES	201	25.9
CONDOMINIUMS	460	59.4
TOTAL	775	
DENSITY: 12.0 DU/AC.		

Included as part of this Preliminary Plan application annexation to the City of Grand Junction is also requested.

**EXISTING LAND USE** - The property under consideration is comprised of 64.9 acres and contains one single family dwelling and two out buildings. The most dominate feature of the site is the commanding view of the Bookcliffs and Grand Mesa. The topography of the site is considered to be "flat" in nature and slopes towards the southwest at a typical rate of one percent. Other than that area surrounding the existing home site, all of the property is being farmed, which is the

historical land use. A small groves of cottonwood trees are evident around the existing house.

The property was zoned PR (planned residential) 12.5 units per acre by Mesa County. The pending annexation will establish a City zone designation of RSF-5.

**SURROUNDING LAND USE** - The dominate land use in the area surrounding Grand View is Spring Valley a single family development on moderately sized lots. Some acreage sized parcel with single family dwellings adjoin the subject site adjacent to 28 Road. Land to the south consists of a large fallow parcel that does not have any plans for development. Matchett Village a new development which is currently in the County process adjoins the easterly boundary of Grand View. The Matchett Village proposal consists of a mixed use development consisting of single family, multi-family and non-residential uses. There are no existing non-residential uses in the surrounding area.

**SURROUNDING LAND USE MATRIX**

NORTH			
USE	AREA in ac.	UNITS	ZONE
Single Family	15.0	4	County R-2
Vacant Land	7.3	0	County R-2
Highline Canal & 1-70			

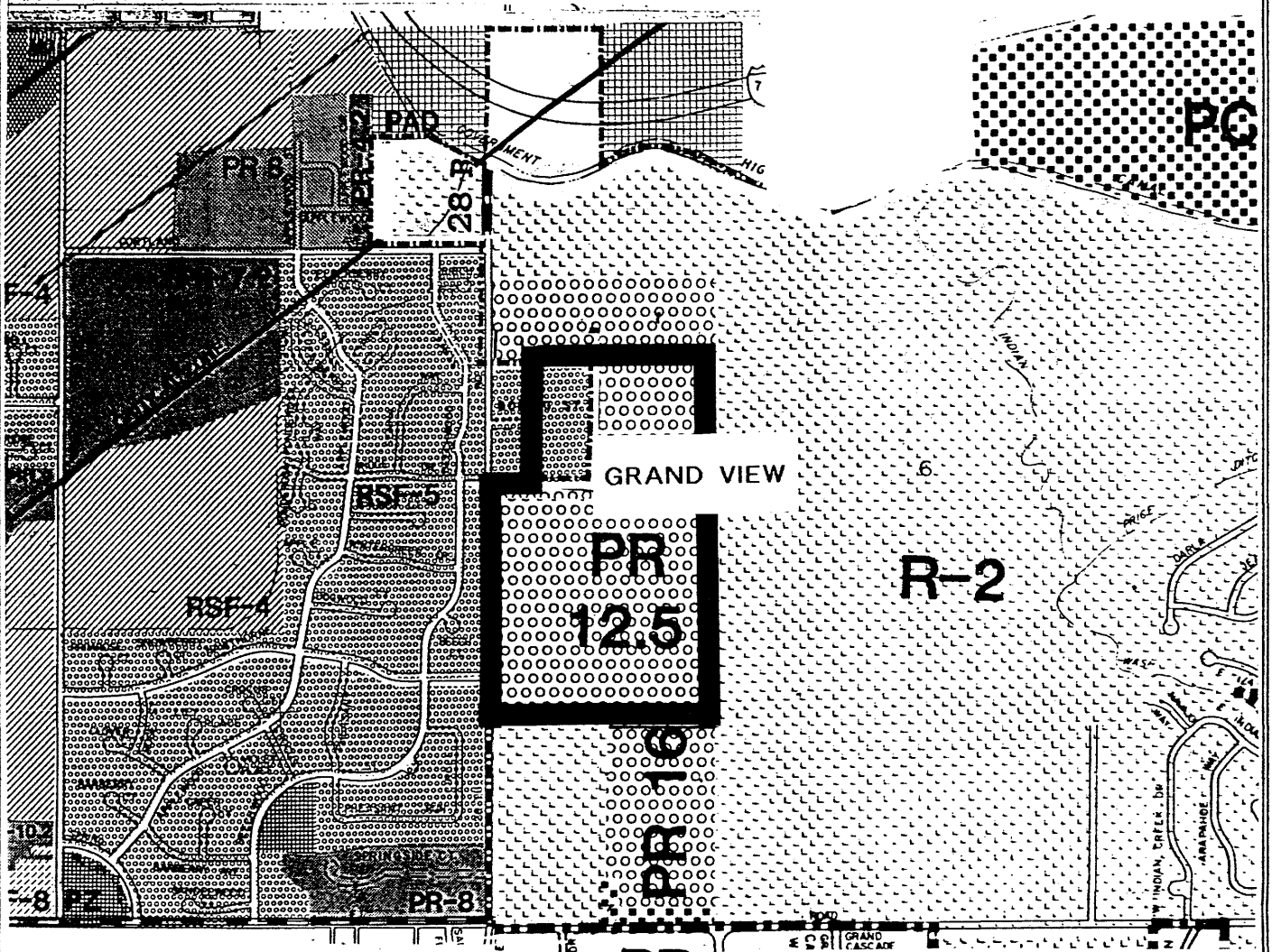
SOUTH			
USE	AREA in ac.	UNITS	ZONE
Vacant Land	20.0	0	County R-2
Matchett Village	20.0	0	County PR-16

EAST			
USE	AREA in ac.	UNITS	ZONE
Matchett Village	160.0	750 proposed	County PR (proposed) County R-2 (existing)

WEST			
USE	AREA in ac.	UNITS	ZONE
Spring Valley	169.8	443	RSF - 5
Pheasant Run Condos.	3.2	17	PR -8
Garfield View Sub.	2.1	4	RSF - 5

A Location Map at the end of this narrative statement illustrates the location of Grand View in relationship to the surrounding land ownership. A reproduction from the Grand Junction and Mesa County Zoning maps follow:

# SURROUNDING ZONING MAP





**PROPOSED LAND USE** - The proposal calls for the ultimate development of 200 single family building sites on 64.8 acres. The resulting density is 3.1 dwelling units per acre. Lots range in size from 8000 square feet to 12,500 square feet. Building Setback requirements for each lot is illustrated on the following chart:

PRINCIPAL BUILDING SETBACK REQUIREMENTS	
FRONT	20 feet
SIDE	10 feet
REAR	20 feet
Maximum Building Height = 32 feet	

The accompanying Preliminary Development Plan depicts the relationship of each dwelling use type to the property boundary, roadway access and Open Spaces.

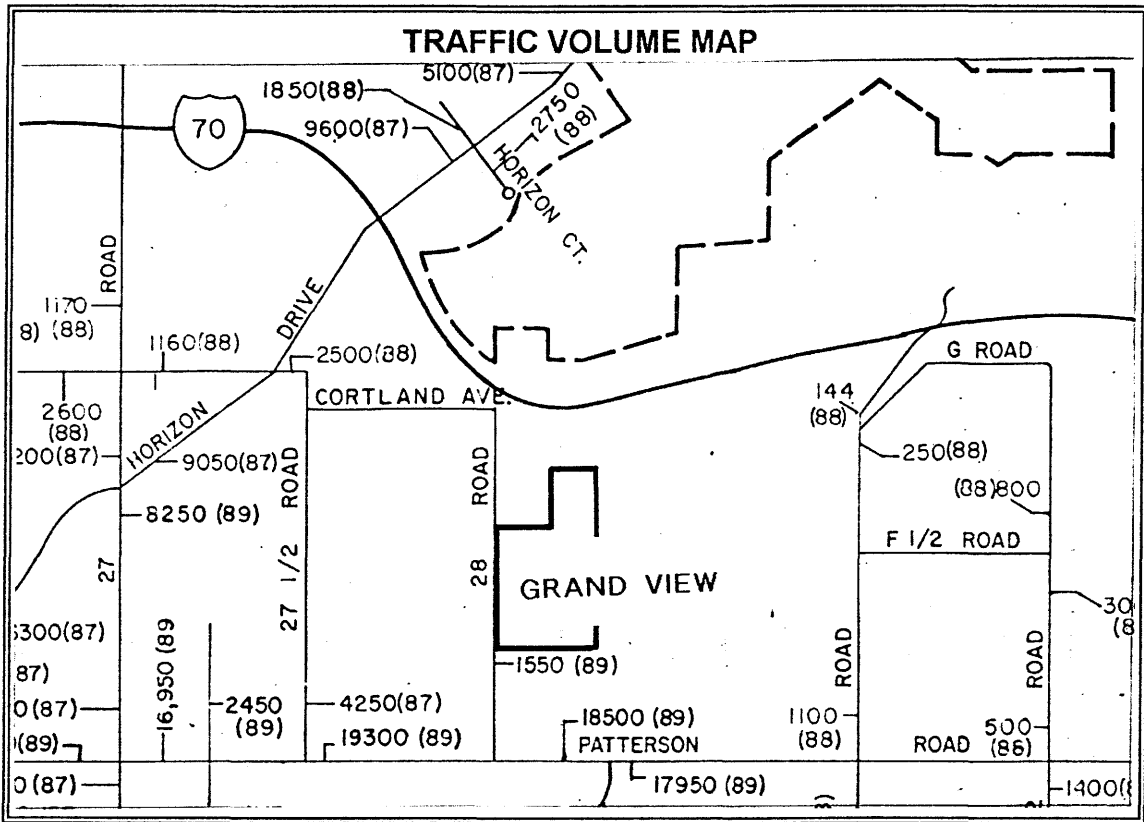
Almost 5% of the total site is designated as two separate Private Open Spaces which includes 2.25 acres site near the south west corner which will be fully landscaped and contain a small pond. This open space will also serve as a buffer between Grand View and the proposed multi-family and non-residential uses which are currently proposed as part of the Matchett Village development. The second private open space is located along 28 Road. This landscaped area will serve as a buffer between the proposed lots and the adjoining roadway.

LAND USE SUMMARY			
USE	UNITS	AREA	% OF TOTAL
SINGLE FAMILY AREA	200	48.1	74.29
ROADS		13.6	21.00
PRIVATE OPEN SPACE		3.1	4.80
<b>TOTAL</b>	<b>200</b>	<b>64.8</b>	<b>100.00</b>
<b>DENSITY: 3.1 DWELLING UNITS PER ACRE</b>			

In addition to the individual lot development standards presented herein, architectural controls will be implemented to insure an aesthetically pleasing and orderly development. To achieve this, covenants, conditions and restrictions (C.C. & R's) will be adopted to insure ongoing protection to the future residents of Grand View and surrounding property owners. The C. C. & R's will also include provisions for ownership and maintenance of the designated irrigation system.

**ACCESS** - Primary access is gained to Grand View from 28 Road. Patterson Road is located 1/4 mile south of the site and serves as a major arterial east/west roadway in Grand Junction. Several other access are also available from Hawthorn Avenue, Ridge Drive, and Courtland Avenue which affords access to 27 1/2 Road And Horizon Drive. Access to the future single family lots within the development will be from either a new "Urban Residential Street" or an "Urban Residential Collector" (as modified). The modified collector roadway proposal includes a 16 foot wide landscaped median. Three "inter-neighborhood" connectors are also provided to adjoining undeveloped properties and will improve the circulation of traffic in the neighborhood. According to the City of Grand Junction's, *Trip Generator*, 1900 average week day trips would be realized when the Grand View is fully developed.

The following map illustrates traffic counts made by the City of Grand Junction major roads in the vicinity of Grand View.



**UTILITY SERVICE**

**DOMESTIC WATER** - All dwellings within Grand View will be served by an public owned domestic water distribution system. An existing 18 inch water main is located within 28 Road and will be used to provide water service to the new dwellings. New 8, 6 and 4 inch mains will be extended within the property. The existing water main has been transferred to the Ute Water Conservancy District. Fire hydrants will be placed throughout the development, as required. Sufficient flows and pressure exist to provide adequate water supply for fire protection.

**SANITARY SEWER** - A new sanitary sewage collection system will be constructed. Sewer service will be extended from an existing main, owned and maintained by the City, located in Hawthorn Avenue within Spring Valley Subdivision. It is estimated that peak sewage flows generated by the lots within the development will be 60,000 gallons per day.

Proposed utility construction will improve the infrastructure in the neighborhood by extending sewer and water lines to adjoining properties. This can allow surrounding undeveloped land to have easier access to improved water and sewer service.

**ELECTRIC, GAS, PHONE & CATV** - Electric, gas, and communication lines will be extended to each lot within the development from existing lines located adjacent to the proposed development. Proposed gas, electric, and communication lines will be located in a "common trench" adjacent to the dedicated road right-of-way.

**IRRIGATION WATER** - Irrigation water will be provided by a zoned pressurized delivery system which will create water conservation. A central pumping facility will be located within the proposed Private Open Space near the southeast property corner, where the petitioners water rights are available.

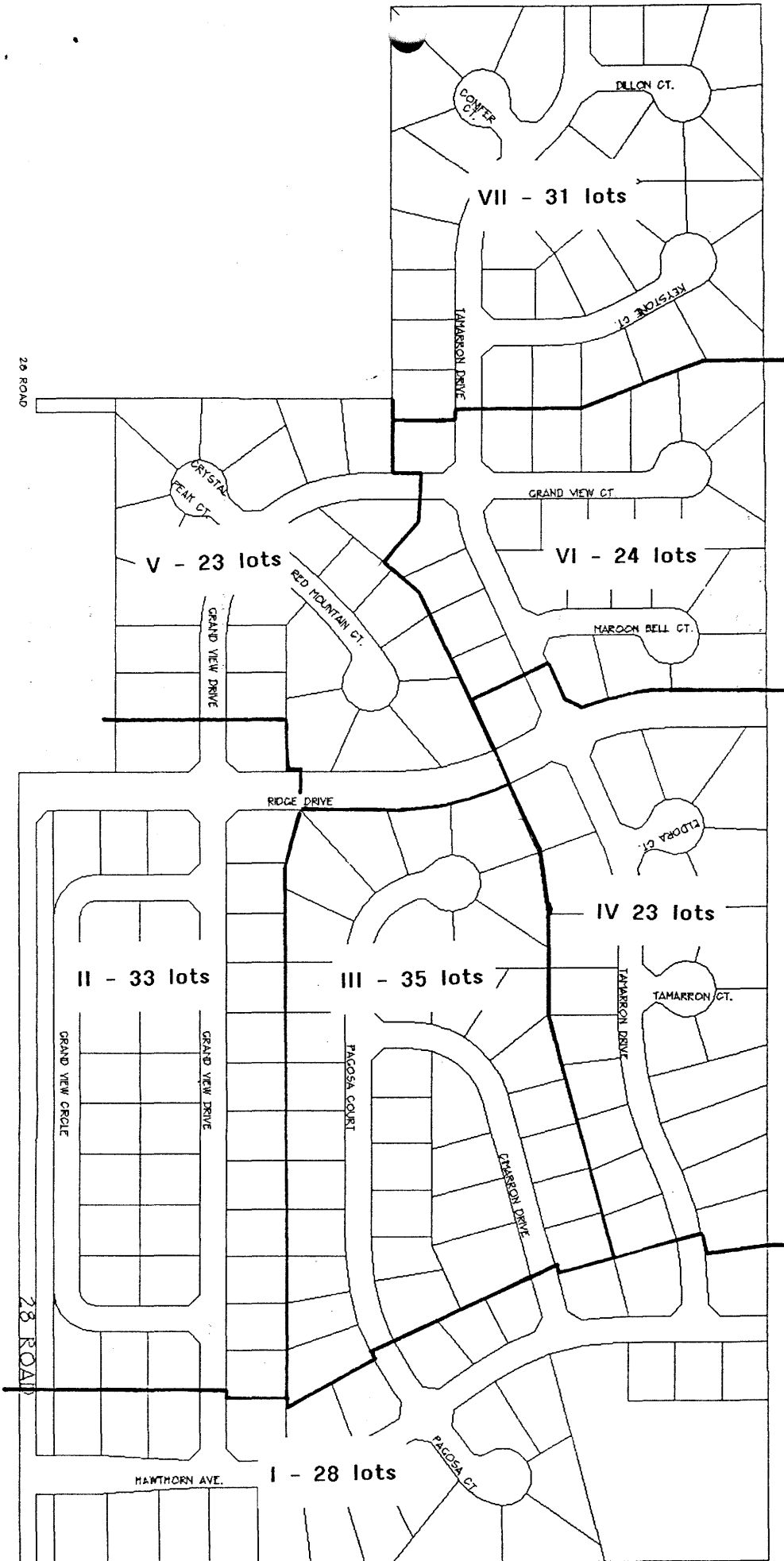
**DRAINAGE** - A Preliminary Drainage Report which evaluates the impacts on existing drainage patterns has been submitted to the City Engineering and Community Development Departments under separate cover. Most of the future drainage will be carried on the ground surface to the proposed street system to three separate detention facilities; the irrigation pond, the buffer strip along 28 Road, and an existing drainage channel along the south property boundary .

**SOILS AND GEOLOGY** - There are no known geologic hazards within Grand View. The Soil Conservation Service (SCS) has identified two soil types within the site:

**BILLINGS SILTY CLAY LOAM, Class IIs Land (Bc)** - This soil is classified as severe for local roads and streets due to poor traffic supporting capacity and moderate to high water tables.

**RAVOLA VERY FINE SANDY LOAM, Class I Land (Rf)** - SCS has not identified any severe limitations for this soil type.

**DEVELOPMENT SCHEDULE** - The rate at which development of Grand View will occur is dependent upon the City's future housing needs. It is anticipated that site development will occur in phases as shown on the Phasing Plan on the next page.



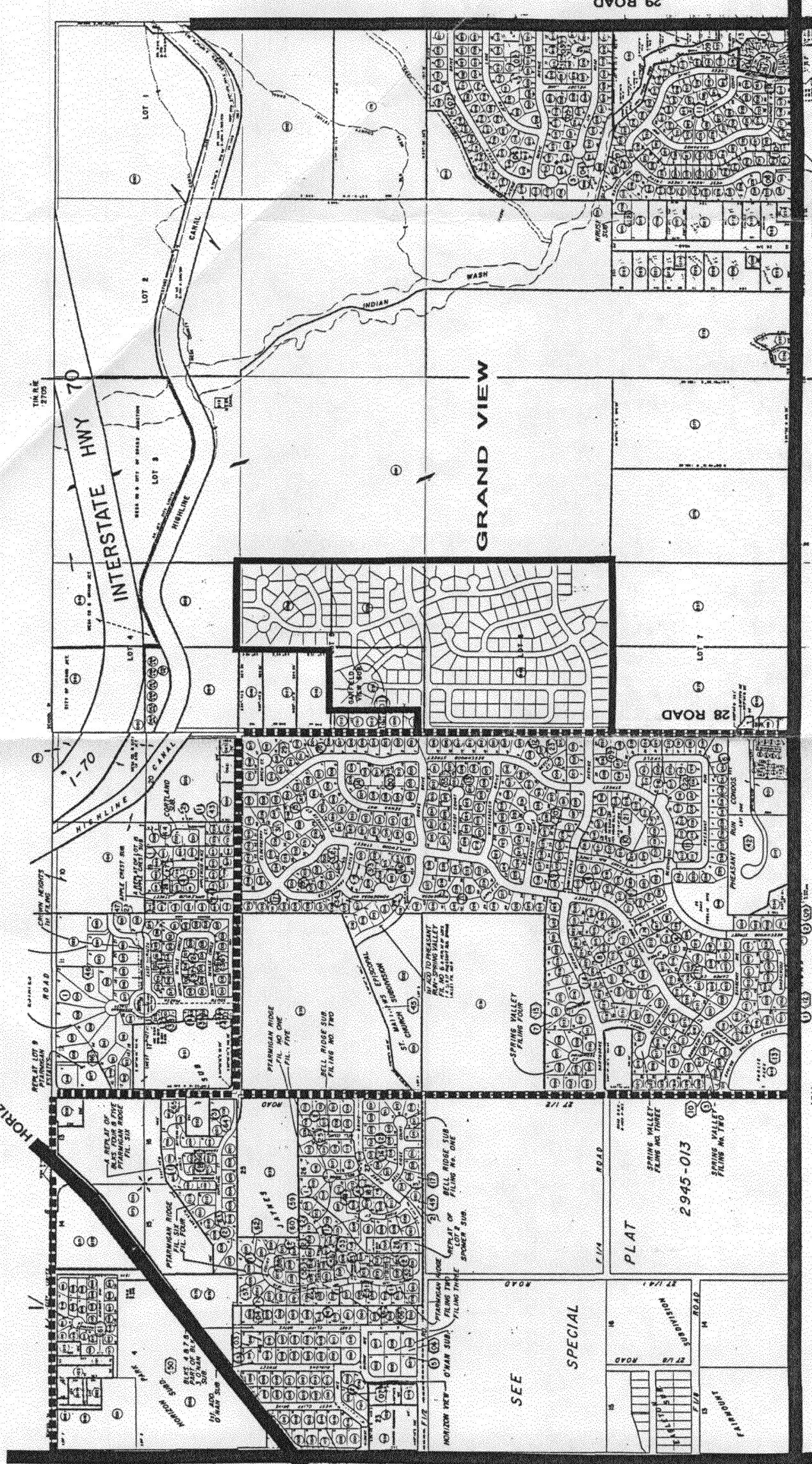
PHASE PLAN

**AIRPORT OVERLAY ZONE** - Grand View is located within the "Area of Influence" according to the *Airport Overlay Map*, within Section 5.11.2B of the *Grand Junction Zoning and Development Code*. The code indicates that the proposal is a compatible use at the density proposed.

HORIZON DRIVE

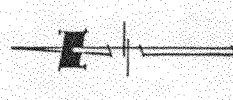
G ROAD

12 TH. STREET




PATTERSON ROAD

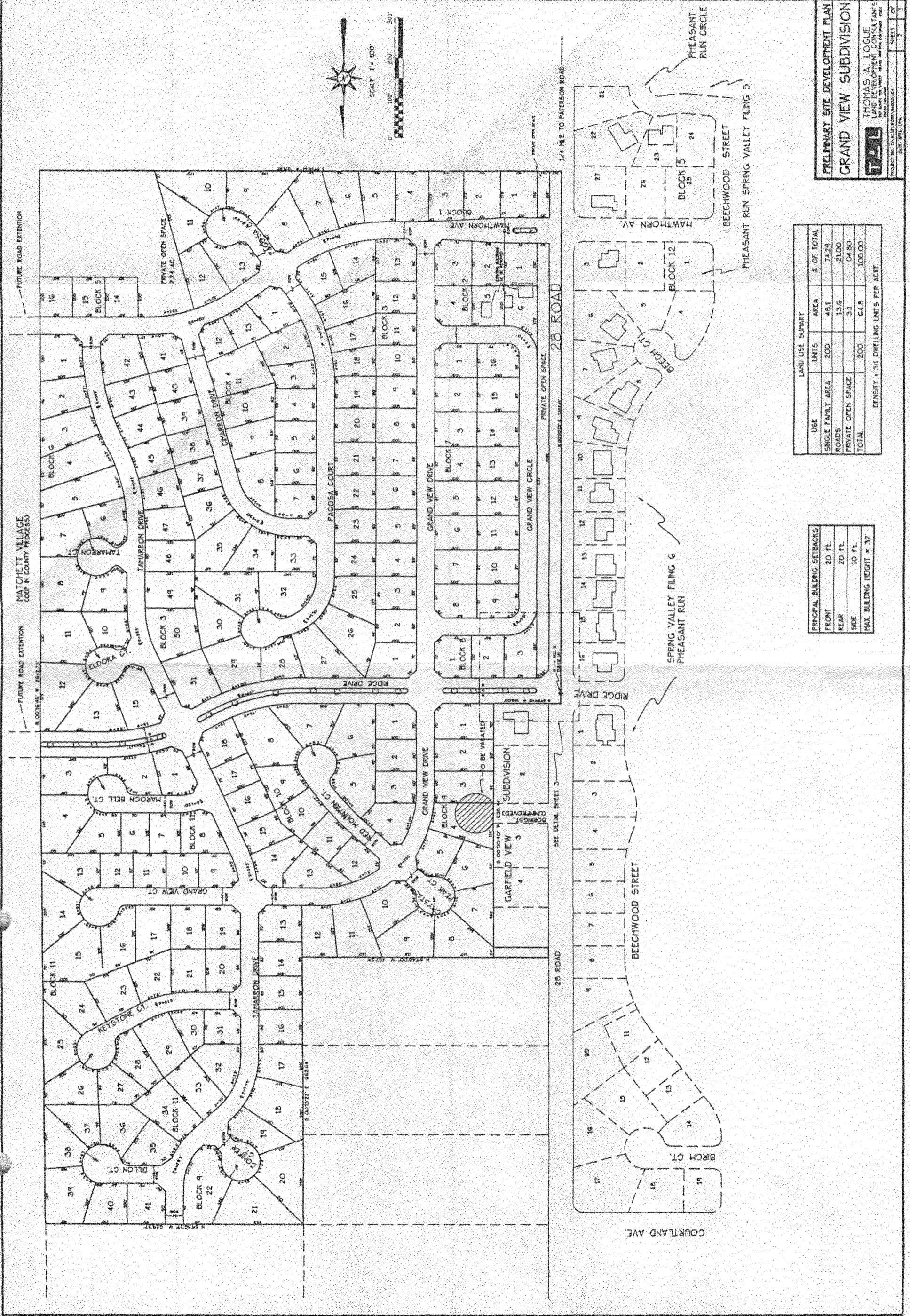
29 ROAD



SCALE: 1"=400'

LOCATION MAP  
**GRAND VIEW SUBDIVISION**  
  
 THOMAS A. LOGUE  
 LAND DEVELOPMENT CONSULTANTS  
 287 NORTH 100 WEST, SUITE 100, JAYHAWK, IOWA 52233  
 PROJECT NO. G-2945-013  
 DATE: APR. 1998  
 SHEET 1 OF 3





LAND USE SUPPLY		
USE	UNITS	% OF TOTAL
SINGLE FAMILY AREA	200	46.1
ROADS	13.6	21.00
PRIVATE OPEN SPACE	3.1	04.80
TOTAL	200	64.8
DENSITY = 3.1 DWELLING UNITS PER ACRE		

PRINCIPAL BUILDING SETBACKS	
FRONT	20 FT.
REAR	20 FT.
SIDE	10 FT.
MAX. BUILDING HEIGHT = 32'	

**PRELIMINARY SITE DEVELOPMENT PLAN**  
**GRAND VIEW SUBDIVISION**

**TAL** THOMAS A. LOGUE  
 LAND DEVELOPMENT CONSULTANTS  
 1000 14th Street, Suite 1000  
 Denver, CO 80202  
 PROJECT NO. 04-01-001-003-01  
 DATE: APR. 1994

SHEET	2	OF	5
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# REVIEW COMMENTS

Page 1 of 3

FILE #85-94

TITLE HEADING: Preliminary Plan/Plat - Grand  
View Subdivision

LOCATION: 28 & F 1/4 Roads

PETITIONER: Ben Carnes & Discovery 76 Corp.

PETITIONER'S ADDRESS/TELEPHONE: 2499 Highway 6 & 50  
Grand Junction, CO 81505  
243-0456

PETITIONER'S REPRESENTATIVE: Tom Logue

STAFF REPRESENTATIVE: Michael Drollinger

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**NOTE: WRITTEN RESPONSE BY THE PETITIONER TO THE REVIEW COMMENTS IS  
REQUIRED ON OR BEFORE 5:00 P.M., MAY 27, 1994.**

---

**U.S. WEST**  
**Leon Peach**

**5/4/94**  
**244-4964**

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New or additional telephone facilities necessitated by this project may result in a "contract" and up-front monies required from developer, prior to ordering or placing of said facilities. For more information, please call: Leon Peach, 244-4964.

**U.S. POSTAL SERVICE**  
**Cheryl Fiegel**

**5/4/94**  
**244-3435**

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1. Delivery options are centralized - preferred - immediate deliver - curbside or behind sidewalk -50% complete prior to delivery.
2. Townhomes will need to be centralized due to parking concerns.
3. Condos must be centralized and customer provides the equipment.

**CITY UTILITY ENGINEER**  
**Bill Cheney**

**5/9/94**  
**244-1590**

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1. Show utilities for "Preliminary Plan" submittal as required in the SSID manual.
2. Sewer lines to be stubbed to the east property line for all future road extensions.
3. A portion of the sewer lines may have to be enlarged to accommodate development to the east. Sewer system will pay for upgrades if needed.
4. More hydrants will be required than are shown on the "Utility Plan" which is O.K. since the submittal is conceptual only.

**GRAND JUNCTION FIRE DEPARTMENT**  
**George Bennett**

**5/6/94**  
**244-1400**

---

The fire hydrant placement for this subdivision needs to be reconfigured to meet with the standards. These standards require that fire hydrants be placed at each intersection and no greater than 500 feet apart thereafter. Please contact our office for assistance in placing the fire hydrants.

**MESA COUNTY PLANNING**  
**Karl G. Metzner**

**5/11/94**  
**244-1867**

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1. A Grand View Drive already exists at C.25 and 31.75.
2. If the cul-de-sac portion of Boeing Street is vacated, all of the street should be vacated.
3. Cortland Avenue right-of-way extension should be provided to connect future 28 1/4 Road to existing Cortland Avenue.
4. Recommend 3 lots adjacent to open space be deleted to create a larger and more useable open space area around the pond.

**UTE WATER DISTRICT**  
**Gary R. Mathews**

**5/11/94**  
**242-7491**

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1. Fire hydrants should be placed at intersections and no more than 500' apart.
2. Water mains are installed 2-3 foot in oil on the North and East side of the street.
3. All valves are installed at the main line and fire hydrants are required to be valved.
4. Policies and fees in effect at the time of application will apply.

**PARKS & RECREATION DEPARTMENT**  
**Don Hobbs**

**5/12/94**  
**244-1542**

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Dedication and development, to City standards, of a minimum of 5 continuous acres for a public park should be part of this development. It would be desirable if this could be adjoining to Matchett Village.

**GRAND JUNCTION POLICE DEPARTMENT**  
**Dave Stassen**

**5/16/94**  
**244-3587**

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In reference to the open space that runs along 28 Road, I would recommend that landscaping be of that type does not totally block vision from 28 Road to the homes (i.e. intermittent trees or shrubs as opposed to a solid wall of green). Keeping with this idea, if fencing is to be used at this location, I would recommend a type that allows for unobstructed vision from the street (wrought iron or similar in nature).

I would also suggest some low, decorative, see through fencing around the irrigation pond to keep children from the water.

**CITY UTILITY ENGINEER**

**5/17/94**

**Jody Kliska**

**244-1591**

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See attached comments, red-lined text and red-lined drawings.

**CITY DEVELOPMENT ENGINEER**

**5/18/94**

**Michael Drollinger**

**244-1439**

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See attached comments.

**MESA COUNTY SCHOOL DISTRICT**

**5/16/94**

**Lou Grasso**

**242-8500**

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See attached comments.

**PUBLIC SERVICE COMPANY**

**5/19/94**

**Dale Clawson**

**244-2695**

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ELECTRIC & GAS: No objections.

## RESPONSE TO REVIEW COMMENTS

May 26, 1994

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Title: GRAND VIEW, Preliminary Plan and Plat

File No: 85-94

Location: 28 Road & F 1/4 Road

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### RESPONSE TO U.S. WEST:

Comments do not require response.

### RESPONSE TO POSTAL SERVICE:

Centralized delivery points will be identified on the Final Plat and Plan.

### RESPONSE TO UTILITY ENGINEER:

Sewer lines will be extended 10 feet beyond the street improvements at all stubs to the east.

Fire Hydrants will be placed as directed in the Fire Department review comments.

Sewer main enlargement will be coordinated with the Department during the final design phase.

### RESPONSE TO FIRE DEPARTMENT:

Fire Hydrants will be reconfigured as requested with the submission of the Final Plat and Plans.

### RESPONSE TO MESA COUNTY PLANNING:

1. Grand View Drive will be renamed.
2. Land owners adjoining Boeing Street will be contacted to determine if they would be willing to vacate Boeing Street.
3. Courtland Avenue and future 28 1/4 Road extensions will occur on property not currently owned by the applicant.
4. The three lots around the private open space are included within the development plan in order to increase the number of people who can observe activities which may occur in the park area.

#### RESPONSE TO UTE WATER:

1. Fire hydrants will be placed as directed by the Fire Department.
2. Water mains will be placed within the roadway as requested.
3. Valves will be placed as requested.

#### RESPONSE TO CITY PARKS:

Discussions with personal from the City Parks and Recreation Department resulted in not including a public park at this time. At such time as development plans for the Matchett property are finalized further discussions may be required to determine if a Public Park can be developed in conjunction with the Matchett site. Further, the \$225.00 per lot open space fees would be in effect for Grand View Subdivision.

#### RESPONSE TO POLICE DEPARTMENT:

The proposal as submitted includes landscaping and screening which will comply with the departments suggestions.

#### RESPONSE TO DEVELOPMENT ENGINEER:

**Site Plan** - The median islands on Ridge and Hawthorne are to be described as separate tract which will be transferred to the Home Owner Association for ownership and maintenance.

The corners on Grand View Circle have been modified to be "bulbed" out similar to a cul-de-sac.

A stub street has been provided to the south opposite of the Grand View Drive and Hawthorn Avenue intersection.

**Street Details** - The applicant has had discussion with the property owner at the northeast corner of 28 Road and Hawthorn Avenue. the property owner indicated a willingness to relocate their existing driveway from 28 Road to a point approximately 130 feet east of 28 Road on Hawthorn Avenue. Since the property owner's home is located near the north right-of-way for proposed Hawthorn Avenue, it would be difficult to request any additional right-of-way dedication for the Hawthorn Avenue extension.

A R4-7 traffic control sign will be installed on both ends of the median planed within Hawthorn Avenue. A traffic analysis will be submitted with the Final Plat and Plan which will address the need for a left turn lane and storage requirement at Hawthorn Avenue and 28 Road.

All "sight zone" requirements will be maintained at all intersections within the development.

The Final Landscape Plan will indicate that trees will be planted an minimum of five feet from the back of the walk.

The entrance sign will be relocated outside of the "sight zone" area and will not exceed 30 inches in height above the curb line.

The Final Plans for the 28 road widening will show any pavement markings.

**Grading and Drainage** - A meeting between the City Department and the project engineer was held to discuss items within the Departments review. Comments raised were addressed.

Developers of the Matchett property have indicated to the applicant that their drainage control plan has been evaluated and have been taken care of on their site.

**Traffic** - A traffic impact analysis will be provided for the entire project with the Final Plan submittal.

**RESPONSE TO COMMUNITY DEVELOPMENT:**

1. A ten foot pressurized irrigated landscaping easement adjacent to both sides of Ridge Drive will be provided at the rear of those lots which adjoin Ridge Drive.
2. Planting details will be provided with the Final Landscape Plans.
3. Ownership and maintenance of all private open spaces will be the responsibility of the Homeowner's Association.
4. Intersection sight distances will not be impaired with landscaping.
5. The entrance feature signs will be relocated to the corner of the intersection outside of the sight distance triangles.
6. The applicant has had discussion with the property owner at the northeast corner of 28 Road and Hawthorn Avenue. The property owner indicated a willingness to relocate their existing driveway from 28 Road to a point approximately 130 feet east of 28 Road on Hawthorn Avenue. The property owner also indicated that they were not interested in any fencing along their south property line adjacent to the Hawthorn Avenue extension.
7. The applicant will attempt to coordinate the entire vacation of Boeing Street prior to the submission of the Final Plat and Plan for that phase.
8. At this time there is no firm development plans for the property east of Dillon Court and Tamarron Court. Therefore, the applicant is reluctant to commit to the

dedication of any pedestrian easements until a plan for development has been submitted for review by a public agency.

9. the Private Open Space will also be label as a "Irrigation and Stormwater Detention" on the Final Plat.

10. It is the applicants desire to maintain a low level landscaped screen area along 28 Road instead of a landscaped berm. A below grade detention structure would be a hardship for maintenance by the Home Owner's Association. The landscaped private open space along 28 Road will be labeled as a Detention Storage and Buffer Area on the Final Plat.

11. The land owner's on either side of the small strip of land between Lot 8, Block 9 will be contacted by the applicant too see if they are interested in ownership.

12. Grand View Drive has been extended to the south on the revised Preliminary Plan.



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SUBSURFACE SOILS EXPLORATION  
GRAND VIEW SUBDIVISION, Fil. 1  
Grand Junction, Colorado

Prepared For:

Mr. Don Dela Motte  
634 Avalon Drive  
Grand Junction, CO.

Prepared By:

LINCOLN-DeVORE, INC.  
1441 Motor Street  
Grand Junction, CO 81505

June 29, 1994

June 29, 1994

Mr. Don Dela Motte  
634 Avalon Drive  
Grand Junction, CO.

117 94

Re: SUBSURFACE SOILS EXPLORATION  
GRAND VIEW SUBDIVISION, Fil. 1  
Grand Junction, Colorado

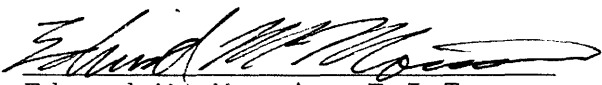
Dear Sir:

Transmitted herein are the results of a Subsurface Soils Exploration for the proposed single family residential Grand View Subdivision, Filing #1.

If you have any questions after reviewing this report, please feel free to contact this office at any time. This opportunity to provide Geotechnical Engineering services is sincerely appreciated.

Respectfully submitted,

LINCOLN-DeVORE, INC.

By:   
Edward M. Morris, E.I.T.  
Western Slope Branch Manager  
Grand Junction, Office

Reviewed by: \_\_\_\_\_  
George D. Morris, P.E.  
Colorado Springs Office

LDTL Job No. 81273-J

EMM/ss

117 94

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## INTRODUCTION

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### PROJECT DESCRIPTION

This report presents the results of our geotechnical evaluation performed to determine the general subsurface conditions of the site applicable to construction of 28 single family residential structures, in Filing #1 of the Grand View Subdivision. A vicinity map is included in the Appendix of this report.

To assist in our exploration, we were provided with a preliminary site development plan prepared by Thomas A. Logue, Land Development Consultant. The Boring Location Plan attached to this report is based on that plan provided to us.

We understand that the proposed structures may consist of one and possibly two story, wood framed structures with the possibility of a partial basement and concrete floor slab on grade. Lincoln DeVore has not seen a full set of building plans, but structures of this type typically develop wall loads on the order of 600 to 1800 plf and column loads on the order of 8 to 16 kips.

The characteristics of the subsurface materials encountered were evaluated with regard to the type of construction described above. Recommendations are included herein to match the described construction to the soil characteristics found. The information contained herein may or may not be valid for other purposes. If the proposed site use is changed or types of construction proposed, other than noted herein, Lincoln

DeVore should be contacted to determine if the information in this report can be used for the new construction without further field evaluations.

## PROJECT SCOPE

The purpose of our exploration was to evaluate the surface and subsurface soil and geologic conditions of the site and, based on the conditions encountered, to provide recommendations pertaining to the geotechnical aspects of the site development as previously described. The conclusions and recommendations included herein are based on an analysis of the data obtained from our field explorations, laboratory testing program, and on our experience with similar soil and geologic conditions in the area.

The scope of our geotechnical exploration consisted of a surface reconnaissance, a geophoto study, subsurface exploration, obtaining representative samples, laboratory testing, analysis of field and laboratory data, and a review of geologic literature.

Specifically, the intent of this study is to:

1. Explore the subsurface conditions to the depth expected to be influenced by the proposed construction.
2. Evaluate by laboratory and field tests the general engineering properties of the various strata which could influence the development.
3. Define the general geology of the site including likely geologic hazards which could have an effect on site development.

4. Develop geotechnical criteria for site grading and earthwork.
5. Identify potential construction difficulties and provide recommendations concerning these problems.
6. Recommend an appropriate foundation system for the anticipated structure and develop criteria for foundation design.

## FIELD EXPLORATION AND LABORATORY TESTING

A field evaluation was performed on June 14, 1994, and consisted of a site reconnaissance by our geotechnical personnel and the drilling of 3 shallow exploration borings. These 3 exploration borings were drilled within the proposed building sites near the locations indicated on the Boring Location Plan. The exploration borings were located to obtain a reasonably good profile of the subsurface soil conditions. All exploration borings were drilled using a CME 45B, truck mounted drill rig with continuous flight auger to depths of approximately 18 to 25 feet. Samples were taken with a standard split spoon sampler, thin wall Shelby tubes, and by bulk methods. Logs describing the subsurface conditions are presented in the attached figures.

Laboratory tests were performed on representative soil samples to determine their relative engineering properties. Tests were performed in accordance with test methods of the American Society for Testing and Materials or other accepted standards. The results of our laboratory tests are included in this report. The in-place moisture content and the standard penetration test values are presented on the attached drilling logs.

## FINDINGS

### SITE DESCRIPTION

The project site is located in the Northwest Quarter of the Southwest Quarter of Section 6, Township 1 South, Range 1 East of the Ute Principal Meridian, Mesa County, Colorado. More specifically the site is located West of 28 Road and will include the Eastward extension of Hawthorne Avenue from the Spring Valley Subdivision. The site is located approximately 1/4 mile North of Patterson Road and is approximately 2 miles Northeast of the main downtown business district of Grand Junction, Colorado.

The topography of the site is relatively flat, with a slight overall gradient to the South. The exact direction of surface runoff on this site will be controlled by the proposed construction and therefore will be variable. In general, surface runoff is expected to travel to the drain ditch along the South boundary, continuing along established drain ditches in the area, eventually entering the Colorado River to the Southwest. Surface and subsurface drainage on this site would be described as poor.

### GENERAL GEOLOGY AND SUBSURFACE DESCRIPTION

The geologic materials encountered under the site consist of low density alluvial soils which overlie the Mancos Shale Formation which is considered to be bedrock in this area. The geologic and engineering properties of the materials found in our 3 exploration borings will be discussed in the following sections.



The surface soils on this site consist of an alluvial deposit placed by the action of the ancient debris fans which originate in the Bookcliffs to the North. This stratification of upper soils results in a layered system of silts and clays with thin, interbedded sand lenses overlying the Mancos Shale Formation. Generally, the silts and clays are soft, wet and of low density. Soil density decreases and the moisture content increases with increasing depth. The upper 2 feet of the soil profile are stiffer and relatively dry due to surface desiccation.

The upper 2 feet of the surface soils have been extensively re-worked by agricultural activity and have been subjected to flood and furrow irrigation for many years.

The surface soils were found to be quite consistent and have been grouped together and designated Soil Type I. This Soil Type was classified as a silty clay (CL) under the Unified Classification System. This material is of low to very low plasticity, of low to moderate permeability, and was encountered in a low to very low density, wet condition. If this soil is found in a relatively dry condition, it may undergo mild expansion with the entry of small amounts of moisture, but will undergo long-term consolidation upon the addition of larger amounts of moisture. This soil will settle after being loaded. The maximum allowable bearing capacity for this soil was found to be 800 psf, with 100 minimum dead load pressure required. The finer grained portion of Soil Type I contains sulfates in detrimental quantities.

The Mancos Shale Formation is considered

to be bedrock in this area. The Mancos Shale is described as a thinbedded, drab, light to dark gray marine shale, with thinly interbedded fine grain sandstone and siltstone layers. Some portions of the Mancos Shale are bentonitic, and therefore, are highly expansive. The majority of the shale, however, has only a moderate expansion potential. Formational shale was encountered in Test Boring No. 1 at a depth of 32 feet. It is anticipated that this formational shale will not affect the construction and the performance of any shallow foundations placed on the site. If a deep foundation system, such as drilled piers, is utilized the Mancos Shale will affect the construction and performance of foundations on this site.

The lines defining the change between soil types or rock materials on the attached boring logs and soil profiles are determined by interpolation and therefore are approximations. The transition between soil types may be abrupt or may be gradual.

The boring logs and related information show subsurface conditions at the date and location of this exploration. Soil conditions may differ at locations other than those of the exploratory borings. If the structure is moved any appreciable distance from the locations of the borings, the soil conditions may not be the same as those reported here. The passage of time may also result in a change in the soil conditions at the boring locations.

**GROUND WATER:**

A free water table came to equilibrium

during drilling at 5 to 7 feet below the present ground surface. This is probably not a true phreatic surface but is an accumulation of subsurface seepage moisture (perched water). In our opinion the subsurface water conditions shown are a permanent feature on this site. The depth to free water would be subject to fluctuation, depending upon external environmental effects.

Because of capillary rise, the soil zone within a few feet above the free water level identified in the borings will be quite wet. Pumping and rutting may occur during the excavation process, particularly if the bottom of the foundations are near the capillary fringe. Pumping is a temporary, quick condition caused by vibration of excavating equipment on the site. If pumping occurs, it can often be stopped by removal of the equipment and greater care exercised in the excavation process. In other cases, geotextile fabric layers can be designed or cobble sized material can be introduced into the bottom of the excavation and worked into the soft soils. Such a geotextile or cobble raft is designed to stabilize the bottom of the excavation and to provide a firm base for equipment.

Data presented in this report concerning ground water levels are representative of those levels at the time of our field exploration. Groundwater levels are subject to change seasonally or by changed environmental conditions. Quantitative information concerning rates of flow into excavations or pumping capacities necessary to dewater excavations is not included and is beyond the scope of this report. If this information is desired, permeability and field pumping tests will be required.

This area has been extensively irrigated in the past for agricultural purposes. It is possible that after agricultural irrigation has ceased in the area, the water table may drop somewhat. It is not anticipated the free water table will rise in the future however, the water table should not be expected to drop more than 2 feet during the Summer and Fall months after area-wide irrigation has ceased or been diminished.

## CONCLUSIONS AND RECOMMENDATIONS

### GENERAL DISCUSSION

No geologic conditions were apparent during our reconnaissance which would preclude the site development as planned, provided the recommendations contained herein are fully complied with. Based on our investigation to date and the knowledge of the proposed construction, the site condition which would have the greatest effect on the planned development is the soft soils and the relatively high water table.

Since the exact magnitude and nature of the foundation loads are not precisely known at the present time, the following recommendations must be somewhat general in nature. Any special loads or unusual design conditions should be reported to Lincoln DeVore so that changes in these recommendations may be made, if necessary. However, based upon our analysis of the soil conditions and project characteristics previously outlined, the following recommendations are made.

### OPEN FOUNDATION OBSERVATION

Since the recommendations in this report are based on information obtained through random borings, it is possible that the subsurface materials between the boring points could vary. Therefore, prior to placing forms or pouring concrete, an open excavation observation should be performed by representatives of Lincoln DeVore. The purpose of this observation is to determine if the subsurface soils directly below the proposed foundations are similar to those encountered in our exploration borings. If the materials below the proposed foundations differ from those encountered, or in our opinion, are not

capable of supporting the applied loads, additional recommendations could be provided at that time.

#### **EXCAVATION & STRUCTURAL FILL:**

##### **Subgrade**

Site preparation in all areas to receive structural fill should begin with the removal of all topsoil, vegetation, and other deleterious materials. Prior to placing any fill, the subgrade should be observed by representatives of Lincoln DeVore to determine if the existing vegetation has been adequately removed and that the subgrade is capable of supporting the proposed fills. The subgrade should then be scarified to a depth of 10 inches, brought to near optimum moisture conditions and compacted to at least 90% of its maximum modified Proctor dry density [ASTM D-1557]. The moisture content of this material should be within + or - 2% of optimum moisture, as determined by ASTM D-1557.

##### **Structural Fill**

In general, we recommend all structural fill in the area beneath any proposed structure or roadway be compacted to a minimum of 90% of its maximum modified Proctor dry density (ASTM D1557). We recommend that fill be placed and compacted at approximately its optimum moisture content (+/-2%) as determined by ASTM D 1557. Structural fill should be a granular, coarse grained, non-free draining, non-expansive soil. This structural fill should be placed in the overexcavated portion of this site in lifts not to exceed 6 inches after compaction. This

Structural Fill must be brought to the required density by mechanical means. No soaking, jetting or puddling techniques of any type should be used in placement of fill on this site.

#### **Non-Structural Fill**

We recommend that all backfill placed around the exterior of the building, and in utility trenches which are outside the perimeter of the building and not located beneath roadways or parking lots, be compacted to a minimum of 80% of its maximum modified Proctor dry density (ASTM D-1557).

#### **Fill Limits**

To provide adequate lateral support, we recommend that the zone of overexcavation extend at least 3 feet beyond the perimeter of the building on all sides. The Structural Fill should be a minimum of 3 feet in final compacted thickness.

No major difficulties are anticipated in the course of excavating into the surficial soils on the site. It is probable that safety provisions such as sloping or bracing the sides of excavations over 4 feet deep will be necessary. Any such safety provisions shall conform to reasonable industry safety practices and to applicable OSHA regulations. The OSHA classification for excavation purposes on this site is Soil Class C.

#### **Field Observation & Testing:**

During the placement of any structural fill, it is recommended that a sufficient amount of field tests

and observation be performed under the direction of the geotechnical engineer. The geotechnical engineer should determine the amount of observation time and field density tests required to determine substantial conformance with these recommendations. It is recommended that surface density tests be taken at maximum 2 foot vertical interval.

The opinions and conclusions of a geotechnical report are based on the interpretation of information obtained by random borings. Therefore the actual site conditions may vary somewhat from those indicated in this report. It is our opinion that field observations by the geotechnical engineer who has prepared this report are critical to the continuity of the project.

#### **Slope Angles**

Allowable slope angle for cuts in the native soils is dependent on soil conditions, slope geometry, the moisture content and other factors. Should deep cuts be planned for this site, we recommend that a slope stability analysis be performed when the location and depth of the cut is known.

#### **DRAINAGE AND GRADIENT:**

Adequate site drainage should be provided in the foundation area both during and after construction to prevent the ponding of water and the saturation of the subsurface soils. We recommend that the ground surface around the structure be graded so that surface water will be carried quickly away from the building. The minimum gradient within 10 feet of the building



will depend on surface landscaping. We recommend that paved areas maintain a minimum gradient of 2%, and that landscaped areas maintain a minimum gradient of 8%. It is further recommended that roof drain downspouts be carried across all backfilled areas and discharged at least 10 feet away from the structure. Proper discharge of roof drain downspouts may require the use subsurface piping in some areas. Planters, if any, should be so constructed that moisture is not allowed to seep into foundation areas or beneath slabs or pavements.

If adequate surface drainage cannot be maintained, or if subsurface seepage is encountered during excavation for foundation construction, a full perimeter drain is recommended for this building. It is recommended that this drain consist of a perforated drain pipe and a gravel collector, the whole being fully wrapped in a geotextile filter fabric. We recommend that this drain be constructed with a gravity outlet. If sufficient grade does not exist on the site for a gravity outlet, then a sealed sump and pump is recommended. Under no circumstances should a dry well be used on this site.

If a half basement type structure is planned on any of these sites, the high water level found across the tract should be controlled to prevent large upward fluctuations of this water surface. For this purpose, we recommend that this be accomplished by construction of an area drain beneath the building area. To control water surface movement, it is recommended that the drain outfall in a free gravity drain. If a gravity outfall is not possible, a sealed sump and pump is recommended to remove the water.

Should an automatic lawn irrigation systems be used on this site, we recommend that the sprinkler heads be installed no less than 5 feet from the buildings. In addition, these heads should be adjusted so that spray from the system does not fall onto the walls of the buildings and that such water does not excessively wet the backfill soils.

It is recommended that lawn and landscaping irrigation be reasonably limited, so as to prevent complete saturation of subsurface soils. Several methods of irrigation water control are possible, to include, but not limited to:

- \* Metering the Irrigation water.
- \* Sizing the irrigation distribution service piping to limit on-site water usage.
- \* Encourage efficient landscaping practices.
- \* Enforcing reasonable limits on the size of high water usage landscaping for each lot and any park areas.

system should be designed as grade beams capable of spanning at least thirteen feet. These "grade beams" should be horizontally reinforced both near the top and near the bottom. The horizontal reinforcement required should be placed continuously around the structure with no gaps or breaks. A foundation system designed in this manner should provide a rather rigid system and, therefore, be better able to tolerate differential movements associated with consolidation of the underlying soft soils.

If increased bearing capacity is required, the Structural Fill recommendations given for Structural Slab Foundation systems could be utilized. When The structural fill is completed, an allowable bearing capacity of 1600 psf maximum may be assumed for proportioning the footings.

#### Structural Slab

If the design of the upper structure is such that loads can be balanced reasonably well, a floating structural slab type of foundation could be used on this site. Such a slab would require heavy reinforcing to resist differential bending along the rim wall. It is possible to design such a slab either as a thickened edge only, a solid or a ribbed slab. A rim wall must be used for confinement purposes. Any such slab must be specifically designed for the anticipated loading.

Such a foundation system may settle to some degree however, the use of a structural fill beneath the slab and rim wall will help reduce settlement and hold differential movement to a minimum. Relatively large slabs will tend to experience minor cracking and heave of lightly loaded interior

portions, unless the slabs are specifically designed with this movement in mind.

The existing low density, metastable soils should be removed to a depth of 2 feet below the proposed bottom footing or rimwall elevation. Once it is felt that adequate soil removal has been achieved, it is recommended that the excavation be closely examined by a representative of Lincoln-DeVore to ensure that an adequate overexcavation depth has indeed occurred and that the exposed soils are suitable to support the proposed structural man-made fill.

Once this examination has been completed, it is recommended that a coarse-grained, non-expansive, non-free draining man-made structural fill be imported to the site. The native soils may be utilized as structural fill, if specifically approved by the Geotechnical Engineer. This imported fill should be placed in the overexcavated portion of this site in lifts not to exceed 6 inches after compaction. A minimum of 90% of the soils maximum Modified Proctor dry density (ASTM D-1557) must be maintained during the soil placement. These soils should be placed at a moisture content conducive to the required compaction (usually Proctor optimum moisture content  $\pm$  2%). The granular material must be brought to the required density by mechanical means. No soaking, jetting or puddling techniques of any type should be used in placement of fill on this site. To ensure adequate lateral support, we must recommend that the zone of overexcavation extend at least 2 feet around the perimeter of the proposed footing. To confirm the quality of the compacted fill product, it is recommended that surface density tests be taken at

maximum 2 foot vertical intervals.

The placement of a geotextile fabric for separation between the native soils and the structural fill is may be recommended to aid the fill placement and to improve the stability of the completed fill.

When The structural fill is completed, an allowable bearing capacity of 1600 psf maximum may be assumed for proportioning the footings.

The placement of the structural fill a minimum of two feet beyond the edge of the structural slab should provide additional support for the eccentrically placed wall loads on the slab edges.

#### **SETTLEMENT:**

We anticipate that total and/or differential settlements for the proposed structures may be considered to be within tolerable limits, provided the recommendations presented in this report are fully complied with. In general, we expect total settlements for the proposed structure to be less than 1-1/2 inch.

#### **FROST PROTECTION**

We recommend that the bottom of all foundation components rest a minimum of 1-1/2 feet below finished grade or as required by the local building codes. Foundation components must not be placed on frozen soils.

DRILLED PIERS:

Under some loading conditions, we recommend that a deep foundation system, consisting of drilled piers be used to carry the weight of the proposed structure. Deep foundations must extend through the low density upper low plastic clay materials and into the underlying clays and shales of the Mancos Shale Formation. The upper soils are very soft and wet with a water table being encountered at approximately 5 feet below the ground surface. Squeezing, caving and seepage moisture are anticipated during installation of these drilled piers, requiring that casing and dewatering equipment be on the site available for use during construction.

We recommend that drilled piers have a minimum shaft length of 38 feet and be embedded at least 6 feet into the relatively unweathered clays of the Mancos Shale Formation. At this level, these piers may be designed for a maximum end bearing capacity of 25000 psf, plus 2000 psf side support considering only the side wall area embedded in the bedrock. Due to the expansive potential of the bedrock, a minimum dead load uplift is required, consisting of a point uplift of 2200 psf and 300 psf side uplift, based on the side wall embedded in the bedrock. The overburden is soft and no supporting or uplift values are assigned to this material. The weight of the concrete in the pier may be incorporated into the required dead load.

It is recommended that the bottoms of all piers be thoroughly cleaned prior to the placement of concrete. The amount of reinforcing in each pier will depend on the

magnitude and nature of loads involved. As a rule of thumb, reinforcing equal to approximately 1/2 of 1% of the gross cross-sectional concrete area should be used. Additional reinforcing should be used if structural conditions warrant. We recommend that reinforcing extend through the full length of pier.

To minimize the possibility of voids developing in the drilled piers, concrete with a slump of 5 to 6 inches is recommended. We recommend that piers be dewatered and thoroughly cleaned of all loose material prior to placing the steel cage and concrete. The pier excavation should contain no more than 2 inches of free water unless the concrete is placed by means of a tremie extending to the bottom of the pier. A free fall in excess of 5 feet is not recommended when placing concrete in drilled piers. We recommend that casing be pulled as the concrete is being placed and that a 5 foot head of concrete be maintained while pulling the casing. It is recommended that drilled piers be plumb with 2% of their length and that the shaft maintain a constant diameter for the full length of the pier and not allowed to "mushroom" at the top.

#### DRILLED PIER OBSERVATION:

The foundation installation for drilled piers should be continuously observed by a representative of Lincoln DeVore to determine that the recommended bearing material has been adequately penetrated and that soil conditions are as anticipated by the exploration. This observation will aid in attaining an adequate foundation system. In addition, abnormalities in the subsurface conditions encountered during foundation

installation can be identified and corrective measures taken as required. Lincoln DeVore requires a minimum of one working day's notice, and a copy of the foundation plan, to schedule any field observation.

**GRADE BEAMS:**

A reinforced concrete grade beam is recommended to carry the exterior wall loads in conjunction with the deep foundation system. We recommend that this grade beam be designed to span from bearing point to bearing point and not be calculated to rest on the ground surface between these points.



## CONCRETE SLABS ON GRADE

Slabs could be placed directly on the natural soils or on a structural fill. We recommend that all slabs on grade be constructed to act independently of the other structural portions of the building. One method of allowing the slabs to float freely is to use expansion material at the slab-structure interface.

It is recommended that slabs on grade be constructed over a capillary break of approximately 6 inches in thickness. We recommend that the material used to form the capillary break be free draining, granular material and not contain significant fines. A free draining outlet is also recommended for this break so that it will not trap water beneath the slab. A vapor barrier is recommended beneath the floor slab and above the capillary break. To prevent difficulty in finishing concrete, a 2 inch sand layer should be placed above the break. An alternate method of reducing finishing problems would be to place the vapor barrier beneath approximately 6 inches of a minus 3/4 inch gravel fill. This method must be very carefully accomplished to minimize excessive puncturing and tearing of the vapor barrier.

It is recommended that floor slabs on grade be constructed with control joints placed to divide the floor into sections not exceeding 360 to 400 square feet, maximum. Also, additional control joints are recommended at all inside corners and at all columns to control cracking in these areas.

## EARTH RETAINING STRUCTURES

The active soil pressure for the design of earth retaining structures may be based on an equivalent fluid pressure of 51 pounds per cubic foot. The active pressure should be used for retaining structures which are free to move at the top (unrestrained walls). For earth retaining structures which are fixed at the top, such as basement walls, an equivalent fluid pressure of 63 pounds per cubic foot may be used. It should be noted that the above values should be modified to take into account any surcharge loads, sloping backfill or other externally applied forces. The above equivalent fluid pressures should also be modified for the effect of free water, if any.

The passive pressure for resistance to lateral movement may be considered to be 183 pcf per foot of depth. The coefficient of friction for concrete to soil may be assumed to be .24 for resistance to lateral movement. When combining frictional and passive resistance, the latter must be reduced by approximately 1/3.

We recommend that the backfill behind any retaining wall be compacted to a minimum of 85% of its maximum modified Proctor dry density, ASTM D-1557. The backfill material should be approved by the Soils Engineer prior to placing and a sufficient amount of field observation and density tests should be performed during placement. Placing backfill behind retaining walls before the wall has gained sufficient strength to resist the applied lateral earth pressures is not recommended.

### REACTIVE SOILS

Since groundwater in the Grand Junction area typically contains sulfates in quantities detrimental to a Type I cement, a Type II or Type I-II or Type II-V cement is recommended for all concrete which is in contact with the subsurface soils and bedrock. Calcium chloride should not be added to a Type II, Type I-II or Type II-V cement under any circumstances.

## FOUNDATIONS

### Footing and Stemwall

Assuming that some amount of differential movement can be tolerated, then a conventional shallow foundation system, either resting on the native alluvial soils or underlain by a structural fill, placed in accordance with the recommendations contained within this report may be utilized. The foundation would consist of continuous spread footings beneath all bearing walls and isolated spread footings beneath all columns and other points of concentrated load. Such a shallow foundation system, resting on the native alluvial soils may be designed on the basis of an allowable bearing capacity of 800 psf maximum. If the shallow foundation system is founded on a properly constructed structural fill at least two feet thick below the foundation, the foundation system may be designed on the basis of an allowable bearing capacity of 1600 psf maximum. Recommendations pertaining to balancing, reinforcing, drainage, and inspection are considered extremely important and must be followed. Contact stresses beneath all continuous walls should be balanced to within + or - 200 psf at all points. Isolated interior column footings should be designed for contact stresses of about 150 psf less than the average used to balance the continuous walls. The criteria for balancing will depend somewhat on the nature of the structure. Single-story, slab-on-grade structures may be balanced on the basis of dead load only. Multi story structures may be balanced on the basis of dead load plus one half live load, for up to three stories.

Stem walls for a shallow foundation

## PAVEMENTS

Samples of the surficial native soils at this property that may be required to support pavements have been evaluated using the Hveem-Carmany method (ASTM D-2844) to determine their support characteristics. The results of the laboratory testing are as follows:

AASHTO Classification - A-4(8)      Unified Classification - CL

	R =	11
Expansion @ 300 psi =		1.3
Displacement @ 300 psi =		4.28

Displacement values higher than 4.00 generally indicate the soil is unstable and may require confinement for proper performance.

No estimates of traffic volumes have been provided to Lincoln DeVore. However, we assume that the roads will be classified as residential. The design procedures utilized are those recognized by the Colorado Department of Highways and the 1986 AASHTO design procedure.

Based upon the existing topography, the anticipated final road grades and the anticipated future irrigation practices in the local area, a Drainage Factor of 0.6 (1986 AASHTO procedure) has been utilized for the section analysis.

## PROPOSED PAVEMENT SECTIONS

Based on the soil support characteristics outlined above, the following pavement sections are recommended:

### **Residential Roadway, 18k EAL = 5 :**

The terminal Serviceability Index of 2.0, a Reliability of 70 and a design life of 20 years have been utilized, based on recommendations by the Highway Department. An 18 kip EAL of 5, also recommended by the Highway Department, was used for the analysis.

#### **Asphalt-Base Coarse**

3 inches of asphaltic concrete pavement  
on 8 inches of aggregate base coarse  
on 8 inches of recompacted native material

#### **Full Depth Asphalt:**

5 inches of asphaltic concrete pavement  
on 12 inches of recompacted native material

Due to anticipated problems of compacting the native subgrade soils and probable soil 'pumping' the use of a Full Depth Asphalt Section is NOT Recommended.

#### **Rigid Concrete:**

Doweled, not tied to shoulder slabs or curbing

6 inches of portland cement pavement  
on 4 inches of aggregate base coarse  
on 8 inches of recompacted native material

### **Collector Roadway, 18k EAL = 25 :**

The terminal Serviceability Index of 2.5, a Reliability of 70 and a design life of 20 years have been utilized, based on recommendations by the Highway Department. An 18 kip EAL of 25, also recommended by the Highway Department, was used for the analysis.

#### **Asphalt-Base Coarse**

3 inches of asphaltic concrete pavement  
on 18 inches of aggregate base coarse  
on 8 inches of recompacted native material

OR

4 inches of asphaltic concrete pavement  
on 12 inches of aggregate base coarse  
on 8 inches of recompacted native material

**Full Depth Asphalt:**

6 inches of asphaltic concrete pavement  
on 12 inches of recompacted native material

Due to anticipated problems of compacting the native subgrade soils and probable soil 'pumping' the use of a Full Depth Asphalt Section is NOT Recommended.

**Rigid Concrete:**

Doweled, not tied to shoulder slabs or curbing

6 inches of portland cement pavement  
on 4 inches of aggregate base coarse  
on 8 inches of recompacted native material

Due to the possibility of very high soil moisture in the subgrade soils, the use of a Geotextile Fabric for separation and minor reinforcement ( such as Mirafi 500-X or 140-N), placed beneath the Aggregate Base Course, may be required in some areas on this site.

**PAVEMENT SECTION CONSTRUCTION**

We recommend that any asphaltic concrete pavement meet the State of Colorado requirements for a Grade C mix. In addition, the asphaltic concrete pavement should be compacted to a minimum of 95% of its maximum Hveem density. The aggregate base coarse should meet the requirements of State of Colorado Class 5 or Class 6 material, and have a minimum R value of 78. We recommend that the base coarse be compacted to a minimum of 95% of its maximum Modified Proctor dry density (ASTM D-1557), at a moisture content within + or -2% of optimum moisture. The native subgrade shall be scarified and recompacted to a minimum of 90% of their maximum Modified Proctor day density (ASTM D-1557) at a moisture content within + or -2% of optimum moisture.

All pavement should be protected from moisture migrating beneath the pavement structure. If surface drainage is allowed to pond behind curbs, islands or other areas of the site and allowed to seep beneath pavement, premature deterioration or possibly pavement failure could result.

Concrete

Pavement

We recommend that any rigid concrete pavement have a minimum flexural strength ( $F_t$ ) of 650 psi at 28 days. This strength requirement can be met using Class P or AX or A or B Concrete as defined in Section 600 of the Standard Specifications for Road and Bridge Construction, Colorado DOT. It is recommended that field control of the concrete mix be made utilizing compressive strength criteria.

Flexural Strength should only be used for the design process. Concrete with a lower flexural strength may be allowed by the agency having jurisdiction however, the design section thicknesses should be confirmed. In addition, the final durability of the pavement should be carefully considered.

Control joints should be placed at a minimum distance of 12 feet in all directions. If it is desired to increase the spacing of control joints, then 66-66 welded wire fabric should be placed in the mid-point of the slab. If the welded wire fabric is used, the control joint spacing can be increased to 40 feet. Construction joints designed so that positive joint transfer is maintained by the use of dowels is recommended.



The concrete should be placed at the lowest slump practical for the method of placement. In all circumstances, the maximum slump should be limited to 4 inches. Proper consolidation of the plastic concrete is important. The placed concrete must be properly protected and cured.

## LIMITATIONS

This report is issued with the understanding that it is the responsibility of the owner, or his representative to ensure that the information and recommendations contained herein are brought to the attention of the individual lot purchasers for the subdivision. In addition, it is the responsibility of the individual lot owners that the information and recommendations contained herein are brought to the attention of the architect and engineer for the individual projects and the necessary steps are taken to see that the contractor and his subcontractors carry out the appropriate recommendations during construction.

The findings of this report are valid as of the present date. However, changes in the conditions of a property can occur with the passage of time, whether they be due to natural processes or the works of man on this or adjacent properties. In addition, changes in acceptable or appropriate standards may occur or may result from legislation or the broadening of engineering knowledge. Accordingly, the findings of this report may be invalid, wholly or partially, by changes outside our control. Therefore, this report is subject to review and should not be relied upon after a period of 3 years.

The recommendations of this report pertain only to the site investigated and are based on the assumption that the soil conditions do not deviate from those described in this report. If any variations or undesirable conditions are encountered during construction or the proposed construction will differ from that planned on the day of this

report, Lincoln DeVore should be notified so that supplemental recommendations can be provided, if appropriate.

Lincoln DeVore makes no warranty, either expressed or implied, as to the findings, recommendations, specifications or professional advice, except that they were prepared in accordance with generally accepted professional engineering practice in the field of geotechnical engineering.

**SOILS DESCRIPTIONS:**

SYMBOL	USCS	DESCRIPTION
		Topsoil
		Man-made Fill
	GW	Well-graded Gravel
	GP	Poorly-graded Gravel
	GM	Silty Gravel
	GC	Clayey Gravel
	SW	Well-graded Sand
	SP	Poorly-graded Sand
	SM	Silty Sand
	SC	Clayey Sand
	ML	Low-plasticity Silt
	CL	Low-plasticity Clay
	OL	Low-plasticity Organic Silt and Clay
	MH	High-plasticity Silt
	CH	High-plasticity Clay
	OH	High-plasticity Organic Clay
	Pt	Peat
	GW/GM	Well-graded Gravel, Silty
	GW/GC	Well-graded Gravel, Clayey
	GP/GM	Poorly-graded Gravel, Silty
	GP/GC	Poorly-graded Gravel, Clayey
	GM/GC	Silty Gravel, Clayey
	GC/GM	Clayey Gravel, Silty
	SW/SM	Well-graded Sand, Silty
	SW/SC	Well-graded Sand, Clayey
	SP/SM	Poorly-graded Sand, Silty
	SP/SC	Poorly-graded Sand, Clayey
	SM/SC	Silty Sand, Clayey
	SC/SM	Clayey Sand, Silty
	CL/ML	Silty Clay

**ROCK DESCRIPTIONS:**

SYMBOL	DESCRIPTION
<u>SEDIMENTARY ROCKS</u>	
	CONGLOMERATE
	SANDSTONE
	SILTSTONE
	SHALE
	CLAYSTONE
	COAL
	LIMESTONE
	DOLOMITE
	MARLSTONE
	GYPSUM
Other Sedimentary Rocks	
<u>IGNEOUS ROCKS</u>	
	GRANITIC ROCKS
	DIORITIC ROCKS
	GABBRO
	RHYOLITE
	ANDESITE
	BASALT
	TUFF & ASH FLOWS
	BRECCIA & Other Volcanics
Other Igneous Rocks	
<u>METAMORPHIC ROCKS</u>	
	GNEISS
	SCHIST
	PHYLLITE
	SLATE
	METAQUARTZITE
	MARBLE
	HORNFELS
	SERPENTINE
Other Metamorphic Rocks	

**SYMBOLS & NOTES:**

SYMBOL	DESCRIPTION
	9/12 Standard penetration drive Numbers indicate 9 blows to drive the spoon 12" into ground.
	ST 2-1/2" Shelby thin wall sample
	W <sub>0</sub> Natural Moisture Content
	W <sub>x</sub> Weathered Material
	Free water table
	γ <sub>d</sub> Natural dry density
	T.B. - Disturbed Bulk Sample
	Soil type related to samples in report
	15' W <sub>x</sub> Form. Top of formation
	Test Boring Location
	Test Pit Location
	Seismic or Resistivity Station. Lineation indicates approx. length & orientation of spread (S = Seismic, R = Resistivity)

Standard Penetration Drives are made by driving a standard 1.4" split spoon sampler into the ground by dropping a 140 lb. weight 30". ASTM test des. D-1586.

Samples may be bulk, standard split spoon (both disturbed) or 2-1/2" I.D. thin wall ("undisturbed") Shelby tube samples. See log for type.

The boring logs show subsurface conditions at the dates and locations shown, and it is not warranted that they are representative of subsurface conditions at other locations and times.

SUMMARY SHEET

Soil Sample SILTY CLAY (CL)  
 Location GRAND VIEW SUB. G-J.  
 Boring No. 1 Depth 3'  
 Sample No. I

Test No. 81273-J  
 Date 6-16-94  
 Test by LRS

Natural Water Content (w) 22.0 %  
 Specific Gravity (Gs) \_\_\_\_\_

In Place Density ( $\rho_o$ ) 98.6 pcf

SIEVE ANALYSIS:

Sieve No.	% Passing
1 1/2"	_____
1"	_____
3/4"	_____
1/2"	_____
4	_____
10	<u>100</u>
20	<u>99</u>
40	<u>99</u>
100	<u>94</u>
200	<u>81</u>

HYDROMETER ANALYSIS:

Grain size (mm)	%
<u>.02</u>	<u>53</u>
<u>.005</u>	<u>32</u>
_____	_____
_____	_____
_____	_____
_____	_____
_____	_____
_____	_____

Plastic Limit P.L. 16 %  
 Liquid Limit L. L. 24 %  
 Plasticity Index P.I. 8 %  
 Shrinkage Limit \_\_\_\_\_ %  
 Flow Index \_\_\_\_\_ %  
 Shrinkage Ratio \_\_\_\_\_ %  
 Volumetric Change \_\_\_\_\_ %  
 Lineal Shrinkage \_\_\_\_\_ %

MOISTURE DENSITY: ASTM METHOD

Optimum Moisture Content -  $w_p$  \_\_\_\_\_ %  
 Maximum Dry Density -  $\rho_d$  \_\_\_\_\_ pcf  
 California Bearing Ratio (av) \_\_\_\_\_ %  
 Swell: \_\_\_\_\_ Days \_\_\_\_\_ %  
 Swell against \_\_\_\_\_ pcf  $W_o$  gain \_\_\_\_\_ %

BEARING:

Housel Penetrometer (av) 800 psf  
 Unconfined Compression (qu) \_\_\_\_\_ psf  
 Plate Bearing: \_\_\_\_\_ psf  
 Inches Settlement \_\_\_\_\_  
 Consolidation 2.4% under 930 psf

PERMEABILITY:

K (at 20°C) \_\_\_\_\_  
 Void Ratio \_\_\_\_\_

Sulfates 1000 ppm.

SOIL ANALYSIS

LINCOLN-DeVORE TESTING LABORATORY  
 COLORADO SPRINGS, COLORADO

SAMPLE: AASHTO A-4 (0)

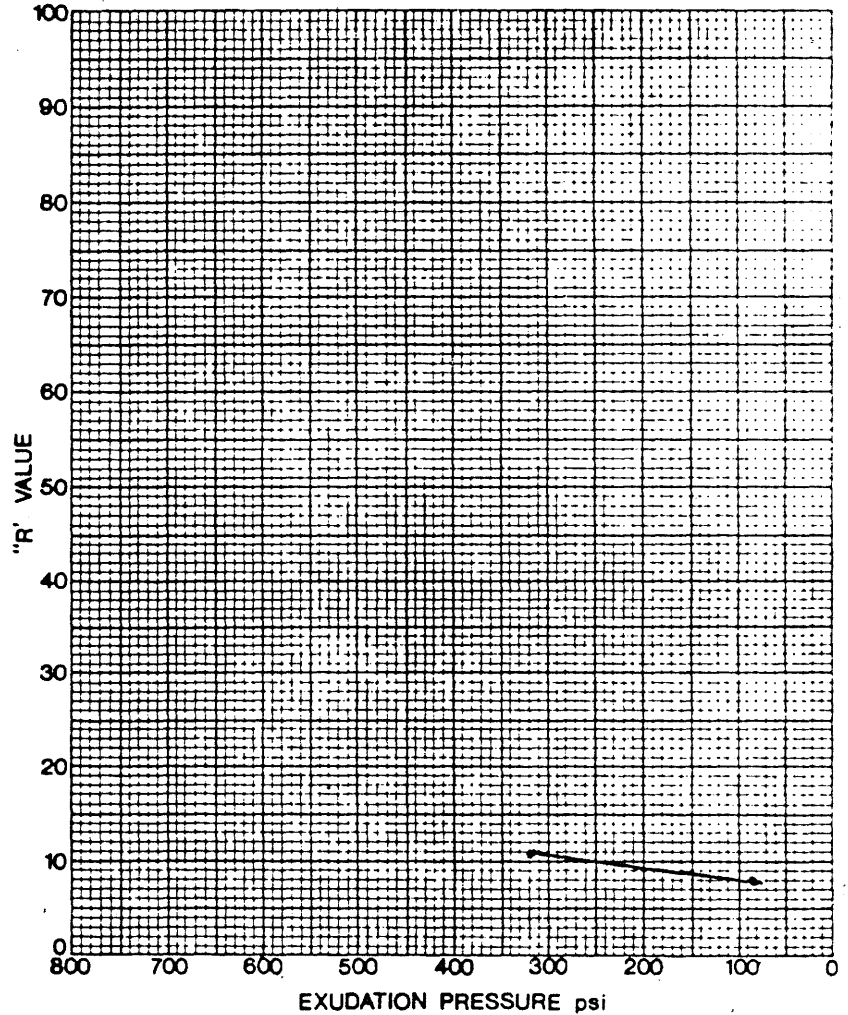
UCL (CL)

TEST SPECIMAN			A	B	C	D	E
DATE TESTED			4/17	4/17	4/17		
SPECIMEN FABRICATION	Compactor Air Pressure	psi					
	Initial Moisture	%	8.2%	8.2	8.2		
	Moisture at Compaction	%	18.2	17.2	16.2		
	Briquette Height	in.	2.58	2.53	2.51		
	Density	pcf	107.9	109.0	111.1		
EXUDATION PRESSURE			84	135	318		
EXPANSION PRESSURE DIAL			0.8	0.6	1.4		
STABIL-O-METER	P <sub>h</sub> at 1000 pounds	psi	66	64	60		
	P <sub>h</sub> at 2000 pounds	psi	138	147	132		
	Displacement	turns	4.05	4.14	4.30		
	"R" Value		9	5	11		
CORRECTED "R" VALUE			8				

EXPANSION @ 300 PSI EXUDATION PRESSURE 1.3  
 DISPLACEMENT @ 300 PSI EXUDATION PRESSURE 4.28  
 "R" VALUE @ 300 PSI EXUDATION PRESSURE 11

1 1/2"	_____
1"	_____
3/4"	_____
1/2"	_____
3/8"	_____
4	100
10	99
20	99
40	99
100	94
200	82
.02 mm	50
.005 mm	32

LIQUID LIMIT	24
PLASTIC LIMIT	17
PLASTICITY INDEX	7
SAND EQUIVALENT	



Lincoln DeVore, Inc.  
Geotechnical Consultants

GRAND VIEW SUB FIL #1 G.J.

DON DELA MOTTE

DATE 7-1-94

JOB NO. 81273-J

DRAWN EHM



		BORING NO. 2					
		BORING ELEVATION:					
DEPTH (FT.)	SOIL LOG	DESCRIPTION		BLOW COUNT	SOIL DENSITY pcf	WATER %	
		Surface Soils reworked by Agriculture					
		Surface is desiccated					
			Debris Fan Deposits				
			Sulfates				
			Increasing Moisture				
5	CL	Low Plastic, Silty Clay	Gray-brown	ST	105.1	17.9%	
		Saturated, capillary frings		5			
		<b>Free Water</b> ▽	compressible				
		Some silt and sand strata					
		Very Soft					
10	CL	Silty Clay		SPT 1/6			
		Very Stratified	Compressible	10	2/12	21.4%	
					4/18		
		Very Soft	Hole is squeezing shut				
15		Silty Clay					
		TD @ 13'		15			
20				20			
25				25			
30				30			
Blow Counts are cumulative for each 6 inches of sampler penetration.							
<b>Free Water @ 6'</b>							
<b>During Drilling 6-14-94</b>							

LOG OF SUBSURFACE EXPLORATION			
<b>LINCOLN - DeVORE, Inc.</b>		<b>GRAND VIEW SUBDIVISION</b>	
		<b>Grand Junction, Colorado</b>	
		Date	7-1-94
Grand Junction, Colorado		Mr. Don Dela Motte	E.M.M.
Job No.	81273-J	Drawn	EMM





**MASTER DRAINAGE STUDY  
OF  
GRAND VIEW SUBDIVISION**

85  
94-2  
Original  
Do NOT Remove  
From Office

**Prepared For:**

**DONADA INC, c/o DON DELA MOTTE  
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(303)-434-6224**

**Prepared By:**

**LANDesign LTD.  
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Grand Junction, Colorado 81501  
(303)-245-4099**

Prepared by: Monty D. Stroup 07/29/94  
Monty D. Stroup

"I hereby certify that this Master Drainage Study of Grand View Subdivision was prepared under my direct supervision".

Reviewed by: Philip M. Hart  
Philip M. Hart P.E.  
State of Colorado, # 19346

NOTICE: The information contained herein is the sole property of LANDesign LTD. and Donada Inc.. It may not be reused or referenced without the expressed written consent of Philip M. Hart P.E. and Don Dela Motte.

## **I. PURPOSE AND SCOPE OF STUDY**

The purpose of this study is to evaluate the affects of development of the Grand View Subdivision and to provide a basis for subsequent analysis and design of proposed phases of the development. A hydrologic analysis of the project's tributary watersheds for historic and developed conditions has been preformed and is presented in this report.

## **II. PROJECT LOCATION AND DESCRIPTION**

### **Location:**

Grand View Subdivision is located West 1/2 of Section 6, T.1 S., R.1 E., of the Ute Meridian.

Streets in the vicinity include 28 Road running from the south to the north defining the west boundary line. Approximately 1/4 mile south is Patterson Road running from the east to the west.

Development in the vicinity includes Spring Valley to the west a single family development of moderate density. Additionally some acreage sized parcels with single family dwellings adjoin the parcel adjacent to 28 Road. To the north lies undeveloped agricultural lands. To the east is Matchett Village a new development which is currently in the County Subdivision process. The Matchett Village area is planned for mixed use consisting of single family, multifamily and non-residential uses. To the south lies undeveloped agricultural lands.

### **Description:**

The project contains approximately 64.9 acres of land and is planned for 200 single family building sites. Lots range in size from 8,000 to 10,000 square feet.

Presently there is one single family dwelling and two out-buildings on the subject property. Agricultural production has occurred on the property for a number of years and is presently readled for seasonal production.

## **III. HYDROLOGIC CALCULATIONS**

### **References and Constraints:**

The policies outlined in the "Interim Outline of Grading and Drainage Criteria, City of Grand Junction" were used as a basis to determine the study methodologies, techniques and hydrologic data presented herein.

The US Army Corps of Engineers "HEC1 Flood Hydrograph Package" was used to estimate the flowrates within the tributary watershed as a result of development of the property.

### **Watershed Description:**

The tributary watershed contains an estimated 91.31 acres or 0.1426 square miles being entirely located east of 28 Road. The watershed length is approximately 3,200 feet or 0.61 miles. The elevation at the headwater is approximately 4,760 and the elevation at the southwest corner of the project is approximately 4,711. This results in a average slope of approximately 1.53%. Hydrologic conditions are considered "good".

Lands within the watershed are best described as flat agricultural and pasture lands tilled to support straight row crops.

**Watershed Vegetation:**

Vegetation consists primarily of row crops and pockets of grass ground cover. Isolated pockets of wetland vegetation is found to exist within the existing irrigation and drainage channel along the south boundary line of the project.

**Watershed Soils:**

The "Soil Survey, Grand Junction Area" defines soils within the watershed as being (Bc), Billings silty clay loam, 0 to 2 percent slopes, hydrological soils group "C", (Rf), Ravola very fine sandy loam, 0 to 2 percent slopes, hydrological soils group "B" and (Fc) Fruita and Ravola loams, 2 to 5 percent slopes, hydrological soils group "B".

As indicated on the "Area Soils Map" the watershed soils are classified as being in the Hydrologic Soils Group "B" and "C". Based on this information (CN) Curve Numbers assigned to sub-areas within the watershed range from 88 to 96 based on existing and proposed land use and are tabulated on the Sub-Basin Summary forms contained in the appendix of this report. These (CN)s represent a soil Antecedent Moisture Condition III for use with the "24 Hour Storm" (DDF) Depth-Duration-Frequency data. Runoff Curve Number Tables 408 and 409 for Mesa County are provided in the appendix of this report show relative changes in curve numbers based on site specific conditions.

**Watershed Geometry:**

Historic topography divides the watershed into 6 distinct sub-basins defined as offsite sub-basins OF1, OF2 and OF3 and onsite sub-basins A, B and C as shown on the "Historic Watershed and Routing Map" contained in the Appendix of this report.

Runoff from all of these sub-basins is conveyed via well defined irrigation and drainage ditches from the north to the south directly to the southwest corner of the site at 28 Road. From this point runoff is directed south along 28 Road within a deep drainage ditch to an existing drainage infrastructure within Spring Valley Ranch and subsequently underground west to an existing major drainageway west of 28 1/2 Road.

The proposed land plan divides the onsite watershed into 16 sub-basins defined as A1 thru A3, B1 thru B5, C1 thru C3, D1 thru D4 and E1.

Runoff from these sub-basins will continue to be directed to the southwest corner of the site via proposed lot grading, roadways, swales and storm sewer as shown on the "Master Drainage Study Grading, Basin and Routing Plan".

**IV. HEC1 METHODOLOGY****Precipitation Method:**

2 Year and 100 Year Synthetic Storms were simulated based on rainfall (DDF) Depth-Duration-Frequency data for the Grand Junction Urbanized, Area (Table 403b, Reference 2).

**Loss Rate Method:**

The effects of interception and infiltration were analyzed using the SCS Curve Number Method.

**Basin Model:**

Flow from each of the sub-basins is analyzed as it converges with southwest corner of the site using the Muskingum-Cunge Routing Method.

**Runoff Transformation Method:**

Based on watershed geometry the SCS Dimensionless Unit Hydrograph method was used.

**Element Application:**

Each sub-basin was analyzed using 3 elements, overland flow, shallow concentrated flow and channel flow. Travel times (Tt) for each of these elements were calculated individually and combined to define the Time of Concentration (Tc) for each sub-basin. The Lag Time (TLAG) for each basin was calculated based on the relationship of  $TLAG = 0.6 * Tc$  as defined in Reference 3.

**V. HEC1 RESULTS**

**Historic Condition:**

The resultant runoff hydrograph for the historic condition at southwest corner of the site (CP4) indicates a Peak Flow (Q2) of 8 CFS and a (Q100) of 87 CFS as shown on the computer printouts labeled Run #1 and Run #2.

**Developed Condition:**

The resultant runoff hydrograph for the developed condition at same location (CP11) indicates a Peak Flow (Q2) of 7 CFS and a (Q100) of 84 CFS as shown on the computer printouts labeled Run #3 thru Run #6.

**Detention Requirements:**

The proposed outflow hydrograph is obtained by routing runoff through a series of detention ponds defined as Res1 thru Res5 on the printouts. Res1 thru Res3 are located adjacent to 28 Road within sub-basins B1, B2 and B3 respectively and combine to form "System 1". Res4 is to be located in the southeast corner of the site within sub-basin D4. Res5 is located along the south boundary line of the site within sub-basin E1. Res4 and Res5 combine to form "System 2".

**Detention Summary**

<u>I.D.</u>	<u>2 YEAR C.F.</u>	<u>*W.S. ELEV.</u>	<u>100 YEAR C.F.</u>	<u>**W.S. ELEV.</u>	<u>TYPE OF DISCHARGE ELEMENT</u>
Res1	N/A	N/A	19,940	23.60	Spillway crest
Res2	N/A	N/A	12,704	21.00	Spillway crest
Res3	<u>14,374</u> ✓ 14,374	15.75	<u>28,889</u> ✓ 61,533	17.20	Outlet structure-dual stage Ex. Channel 28 Road
System 1 Totals					

\*Water surface elevation at maximum stage prior to 100 Year discharge.

\*\*Maximum water surface elevation including head above outlet element.

**Detention Summary**

<u>I.D.</u>	<u>2 YEAR C.F.</u>	<u>*W.S. ELEV.</u>	<u>100 YEAR C.F.</u>	<u>**W.S. ELEV.</u>	<u>TYPE OF DISCHARGE ELEMENT</u>
Res4	39,640	23.50	47,189	24.00	Outlet structure-dual stage
Res5	<u>22,651</u>	16.00	<u>34,365</u>	17.00	Outlet structure-dual stage
	62,291		81,554		Ex. Channel 28 Road
<b>System 2 Totals</b>					
	<u>76,665 C.F.</u>		<u>143,087 C.F.</u>		Ex. Channel 28 Road
	=1.76 Ac.Ft.		=3.28 Ac.Ft.		

**Site Totals**

\*Water surface elevation at maximum stage prior to 100 Year discharge.

\*\*Maximum water surface elevation including head above outlet element.

**VI. DETENTION ELEMENT ALTERNATIVES:**

As requested by the City of Grand Junction the potential use of a "Infiltrator" underground retention storage system has be evaluated as an alternative to above ground detention as follows:

**Cost**

The estimated cost for construction of Res1, Res2 and Res3 = \$30,000 including landscaping.

Total Required Storage Volume System 1 = 61,533 C.F.

Number of "Infiltrator" Units = 61,533 C.F. / 21.75 C.F./ Unit = 2,829 Units

Estimated Construction Cost = 2,829 Units X \$64.00 / Unit = \$181,056.00

The use of underground retention versus surface detention increase construction cost by a factor of 6.

**Maintenance**

Underground retention systems are prone to clogging due to sedimentation and other debris. Manufacture recommendations are that storm water delivered to the underground system be free of suspended solids. This would require the installation of sedimentation basins upstream of the infiltration system. Sedimentation basins would in effect emulate the above ground detention ponds resulting in redundancy. Further, the manufacture recommends that the system be inspected after each storm event to assure functional reliability.

**Ground Water**

The "Subsurface Soils Exploration" report for the site indicates the presence of ground water at depths of 5 to 6 feet. This would eliminate the effectiveness of a underground infiltration system below these depths. The result is a net increase in the infiltration bed area by 30% which equates to loss of buildable land and yet additional cost.

In summary the use of underground storage as a viable option is not feasible and is not recommended for future consideration.

## **VII. IRRIGATION IMPACTS AND THE 2 YEAR STORM EVENT**

### **Historic Conditions:**

The site is bound along its west and east lines by irrigation ditches originating at the Highline Canal. These ditches flow from the north to the south and are the source of irrigation water for lands surrounding the project as well as the project itself. The existing drainage ditch adjacent to the south boundary line of the site is the sole conveyance element for tailwater from these ditches. The ditch is currently owned and maintained by the Grand Valley Water Users Association (GVWUA). The association has agreed to grant to Grand View Subdivision the right to detain water within this ditch (aka Res5), with the stipulation that the conveyance of irrigation tailwater by the ditch continue perpetually.

### **The 2 Year Storm:**

Analysis of the minor storm event presented herein is based on attenuation and release at historic levels excluding irrigation water. Maintaining the historic release rate during the 2 Year event from Res3, Res4 and Res5 will require the construction of a 3-inch by 3-inch square orifice blockout in each of the pond's dual stage release structures. This extremely small orifice creates the potential for continual maintenance problems due to clogging. The practical application of screening devices or filters capable of maintaining continuous flow through such a small conveyance element is in itself marginal.

For the reasons indicated above the proposed detention ponds, lowflow conveyance elements and release structures shall be designed to accommodate the aforementioned irrigation tailwater. The ultimate size of the lowflow discharge elements shall be based on the sum of 2 year historic release rates and calculated irrigation flow rates.

## **VIII. CONCLUSIONS**

The calculated developed runoff hydrographs and required minimum detention volumes presented herein are to be used as the basis for design of individual phases within the total development. The proposed detention elements defined herein will attenuate developed flows to historic rates. The final size and configuration of individual detention and conveyance elements shall be verified by hydrologic and hydraulic design for each phase as development proceeds.



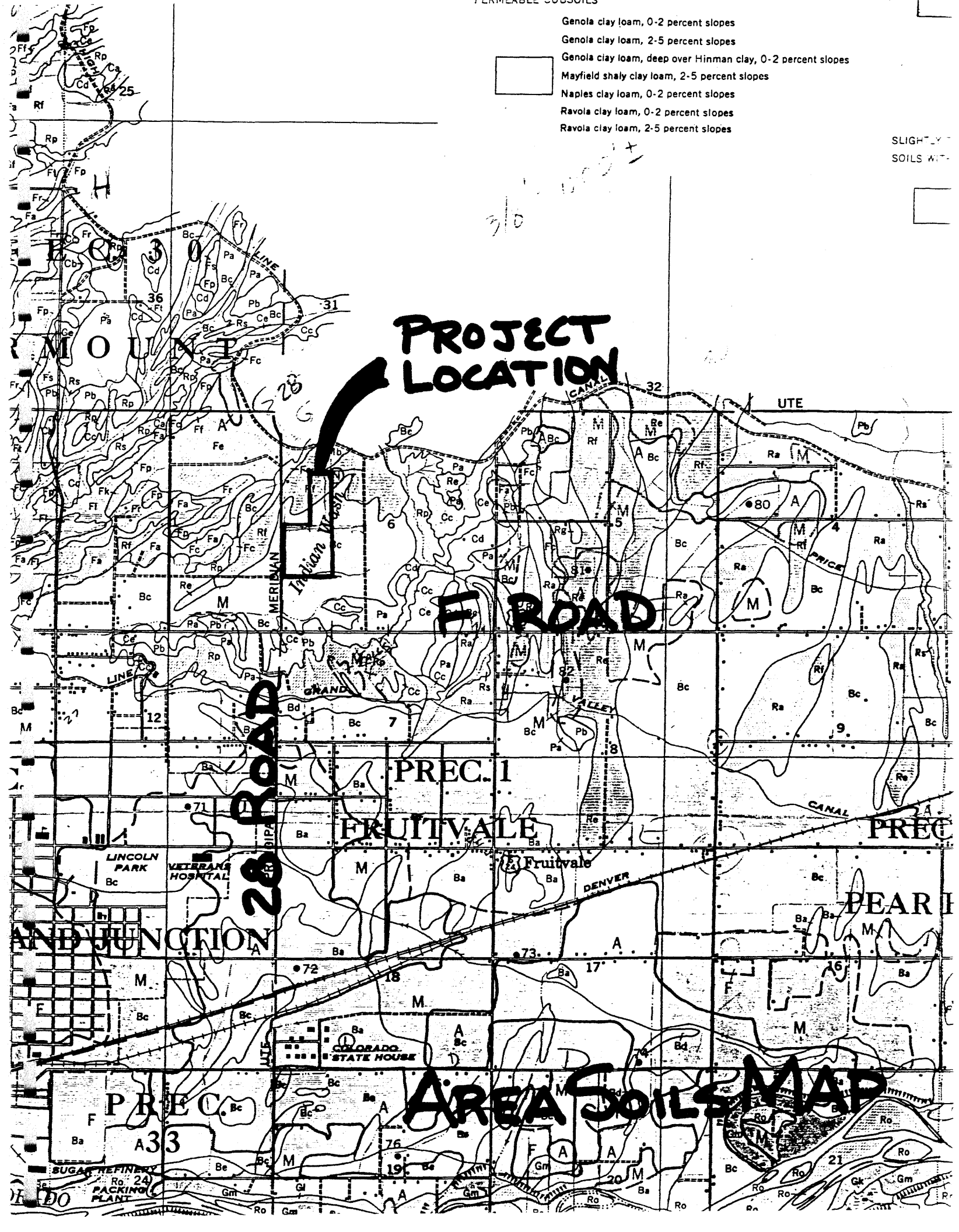
## **References**

- 1. Interim Outline of Grading and Drainage Criteria, City of Grand Junction, July, 1992.**
- 2. Mesa County Storm Drainage Criteria Manual, Final Draft, Mesa County, Colorado, March 1992.**
- 3. HEC1 Flood Hydrograph Package, Hydrologic Engineering Center, US Army Corps of Engineers, Davis, CA., September, 1990.**
- 4. Soil Survey, Grand Junction Area, Colorado, Series 1940, No. 19, U.S. Department of Agriculture, Issued November, 1955.**
- 5. Concrete Pipe Design Manual, American Concrete Pipe Association, Fifth Printing (revised) June, 1980.**
- 6. Flowmaster I, Version 3.16, Haestad Methods, Inc., Copyright 1990.**
- 7. Subsurface Soils Exploration, Grand View Subdivision, Filing 1, Lincoln-DeVORE, Inc., Grand Junction, CO., June 29, 1994.**

**APPENDIX**

- Genola clay loam, 0-2 percent slopes
- Genola clay loam, 2-5 percent slopes
- Genola clay loam, deep over Hinman clay, 0-2 percent slopes
- Mayfield shaly clay loam, 2-5 percent slopes
- Naples clay loam, 0-2 percent slopes
- Ravola clay loam, 0-2 percent slopes
- Ravola clay loam, 2-5 percent slopes

SLIGHTLY  
SOILS WITH



**PROJECT  
LOCATION**

**F ROAD**

**28  
29  
30  
31  
32**

**PREC. 1**

**FRUITVALE**

**BEAR I**

**AREA SOILS MAP**

**PREC. A33**

**SUGAR REFINERY  
PACKING PLANT**

**VETERANS HOSPITAL**

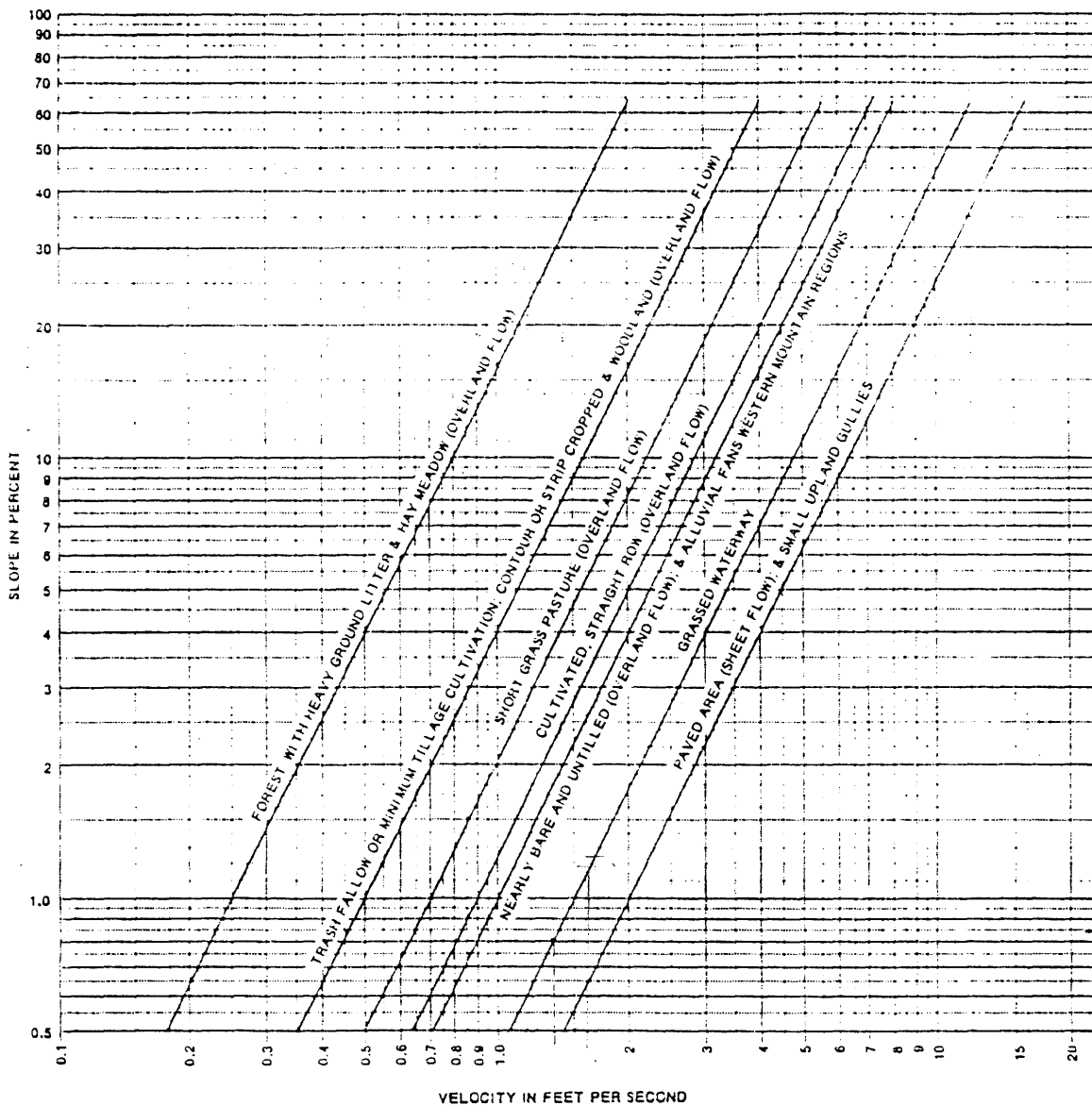
**COLORADO STATE HOUSE**

**DENVER**

**DO**

# MESA COUNTY STORM DRAINAGE CRITERIAL MANUAL

FIGURE 402



**AVERAGE VELOCITIES  
FOR OVERLAND FLOW**

# MESA COUNTY STORM DRAINAGE CRITERIAL MANUAL

TABLE 403 b

## DEPTH-DURATION-FREQUENCY TABLE FOR GRAND JUNCTION URBANIZED AREA (REFERENCE: Sabol 1991, TM-1)

DURATION	2-YR	10-YR	100-YR
5-MIN	0.10	0.18	0.39
10-MIN	0.15	0.28	0.60
15-MIN	0.19	0.36	0.76
30-MIN	0.27	0.50	1.06
1-HR	0.34	0.63	1.34
2-HR	0.42	0.72	1.40
3-HR	0.47	0.77	1.44
6-HR	0.55	0.87	1.56
12-HR	0.64	0.98	1.69
24-HR	0.70	1.12	2.01

## INTENSITY-DURATION-FREQUENCY TABLE FOR GRAND JUNCTION URBANIZED AREA (REFERENCE: Sabol 1991, TM-1)

DURATION	2-YR	10-YR	100-YR
5-MIN	1.20	2.16	4.68
10-MIN	0.90	1.68	3.60
15-MIN	0.76	1.44	3.04
30-MIN	0.54	1.00	2.12
1-HR	0.34	0.63	1.34
2-HR	0.21	0.36	0.70
3-HR	0.16	0.26	0.48
6-HR	0.09	0.15	0.26
12-HR	0.05	0.08	0.14
24-HR	0.03	0.05	0.08

# MESA COUNTY STORM DRAINAGE CRITERIAL MANUAL

TABLE 108

## HISTORIC CONDITIONS

\*\*\*USE ONLY FOR 2 HOUR STORM\*\*\*

RUNOFF CURVE NUMBERS FOR HYDROLOGIC  
SOIL COVER COMPLEXES - RURAL CONDITIONS  
(Antecedent Moisture Condition III, and Ia = 0.2S)  
(FROM: USDA SCS 1977)

LAND USE	COVER	HYDROLOGIC CONDITION	RUNOFF CURVE NUMBER BY HYDROLOGIC SOIL GROUP			
			A	B	C	D
Fallow	Straight Row	----	89	94	97	98
Row crops	Straight Row	Poor	86	92	95	97
	Straight Row	Good	83	90	94	96
	Contoured	Poor	85	91	93	95
	Contoured	Good	82	88	92	94
	Cont and Terraced	Poor	82	88	91	92
	Cont and Terraced	Good	79	86	90	92
Small grain	Straight Row	Poor	82	89	93	95
	Straight Row	Good	80	88	93	95
	Contoured	Poor	80	88	92	94
	Contoured	Good	78	87	92	93
	Cont and Terraced	Poor	78	86	91	92
	Cont and Terraced	Good	77	85	91	92
Close seeded	Straight Row	Poor	82	89	94	96
legumes 1/ or rotation	Straight Row	Good	76	86	92	94
meadow	Contoured	Poor	81	88	93	94
	Contoured	Good	74	84	90	93
	Cont and Terraced	Poor	80	87	91	93
	Cont and Terraced	Good	70	83	89	91
Pasture or Range		Poor	84	91	94	96
		Fair	69	84	91	93
		Good	59	78	88	91
	Contoured	Poor	67	83	92	95
	Contoured	Fair	64	77	88	93
	Contoured	Good	15	55	85	91
Meadow		Good	50	76	86	90
Woods		Poor	65	82	89	93
		Fair	56	78	87	91
		Good	43	74	85	89
Farmsteads		----	77	88	92	94
Roads (dirt) 2/ (hard surface) 2/		----	86	92	95	96
		----	88	93	96	97

1/ Close drilled or broadcast

2/ Including right-of-way



Worksheet 2: Runoff curve number and runoff

Project GRAND VIEW SUBDIVISION By MS Date 06/25/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: (Present) Developed SUB-BASIN A

1. Runoff curve number (CN)

Soil name and hydrologic group (appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN <sup>1/</sup>			Area <input checked="" type="checkbox"/> acres <input type="checkbox"/> mi <sup>2</sup>	Product of CN x area
		Table 2-1	Fig. 2-3	Fig. 2-4		
(R <sub>2</sub> ) Ravola VERY FINE SANDY LOAM "B"	ROW CROPS STRAIGHT, CONDITION GOOD.	90			8.60	774.00
(B <sub>2</sub> ) Billings SILTY CLAY LOAM "C"	ROW CROPS STRAIGHT, CONDITION GOOD	94			17.47	1642.18

<sup>1/</sup> Use only one CN source per line. Totals = 26.07 2416.18

CN (weighted) =  $\frac{\text{total product}}{\text{total area}} = \frac{2416.18}{26.07} = 92.68$  Use CN = 93

2. Runoff

Frequency ..... yr  
 Rainfall, P (24-hour) ..... in  
 Runoff, Q ..... in  
 (Use P and CN with table 2-1, fig. 2-1, or eqs. 2-3 and 2-4.)

Storm #1	Storm #2	Storm #3
N/A		
N/A		
N/A		



# GRAND VIEW ESTATES

ON-SITE FARM LAND USE 0.22

OFF-SITE PASTURE USE 0.35

Table 3.5

Resistance Factor for Overland Flow

Surface	N value	Source
Asphalt/Concrete*	0.05 - 0.15	a
Bare Packed Soil Free of Stone	0.10	c
Fallow - No Residue	0.008 - 0.012	b
Conventional Tillage - No Residue	0.06 - 0.12	b
Conventional Tillage - With Residue	0.16 - 0.22	b
Chisel Plow - No Residue	0.06 - 0.12	b
Chisel Plow - With Residue	0.10 - 0.16	b
Fall Disking - With Residue	0.30 - 0.50	b
No Till - No Residue	0.04 - 0.10	b
No Till (20-40 percent residue cover)	0.07 - 0.17	b
No Till (60-100 percent residue cover)	0.17 - 0.47	b
Sparse Rangeland with Debris:		
0 Percent Cover	0.09 - 0.34	b
20 Percent Cover	0.05 - 0.25	b
Sparse Vegetation		
Short Grass Prairie	0.053 - 0.13	f
Short Grass Prairie	0.10 - 0.20	f
Poor Grass Cover On Moderately Rough	0.30	c
Bare Surface		
Light Turf	0.20	a
Average Grass Cover	0.4	c
Dense Turf	0.17 - 0.80	a,c,e,f
Dense Grass	0.17 - 0.30	d
Bermuda Grass	0.30 - 0.48	d
Dense Shrubbery and Forest Litter	0.4	a

Legend: a) Harley (1975), b) Engman (1986), c) Hathaway (1945), d) Palmer (1946), e) Ragan and Duru (1972), f) Woolhiser (1975). (See Hjermfelt, 1986)

\*Asphalt/Concrete n value for open channel flow 0.01 - 0.016

Appendix A.—TABLES

*Design charts for open channels from HDS No 3 US Dept of Transportation Aug 1961, Reprinted 1977*

Table 1.—Manning roughness coefficients,  $n^1$

	Manning's $n$ range <sup>1</sup>		Manning's $n$ range <sup>1</sup>
<b>I. Closed conduits:</b>		<b>IV. Highway channels and swales with maintained vegetation <sup>1,2</sup></b>	
A. Concrete pipe	0.011-0.013	(values shown are for velocities of 2 and 6 f.p.s.):	
<b>B. Corrugated-metal pipe or pipe-arch:</b>		A. Depth of flow up to 0.7 foot:	
1. 2½ by ½-in. corrugation (riveted pipe): <sup>3</sup>		1. Bermudagrass, Kentucky bluegrass, buffalograss:	
a. Plain or fully coated	0.024	a. Mowed to 2 inches	0.07-0.045
b. Paved invert (range values are for 25 and 50 percent of circumference paved):		b. Length 4-6 inches	0.09-0.05
(1) Flow full depth	0.021-0.018	2. Good stand, any grass:	
(2) Flow 0.8 depth	0.021-0.016	a. Length about 12 inches	0.18-0.09
(3) Flow 0.6 depth	0.019-0.013	b. Length about 24 inches	0.30-0.15
2. 6 by 2-in. corrugation (field bolted)	0.03	3. Fair stand, any grass:	
C. Vitrified clay pipe	0.012-0.014	a. Length about 12 inches	0.14-0.08
D. Cast-iron pipe, uncoated	0.013	b. Length about 24 inches	0.25-0.13
E. Steel pipe	0.009-0.011	B. Depth of flow 0.7-1.5 feet:	
F. Brick	0.014-0.017	1. Bermudagrass, Kentucky bluegrass, buffalograss:	
<b>G. Monolithic concrete:</b>		a. Mowed to 2 inches	
1. Wood forms, rough	0.015-0.017	b. Length 4 to 6 inches	0.05-0.035
2. Wood forms, smooth	0.012-0.014	2. Good stand, any grass:	
3. Steel forms	0.012-0.013	a. Length about 12 inches	0.12-0.07
<b>H. Cemented rubble masonry walls:</b>		b. Length about 24 inches	0.20-0.10
1. Concrete floor and top	0.017-0.022	3. Fair stand, any grass:	
2. Natural floor	0.019-0.025	a. Length about 12 inches	0.10-0.06
I. Laminated treated wood	0.015-0.017	b. Length about 24 inches	0.17-0.09
J. Vitrified clay liner plates	0.015	<b>V. Street and expressway gutters:</b>	
<b>II. Open channels, lined <sup>4</sup> (straight alignment): <sup>4</sup></b>		A. Concrete gutter, troweled finish	
A. Concrete, with surfaces as indicated:		B. Asphalt pavement:	
1. Formed, no finish	0.013-0.017	1. Smooth texture	
2. Trowel finish	0.012-0.014	2. Rough texture	
3. Float finish	0.013-0.015	C. Concrete gutter with asphalt pavement:	
4. Float finish, some gravel on bottom	0.015-0.017	1. Smooth	
5. Gunite, good section	0.016-0.019	2. Rough	
6. Gunite, wavy section	0.018-0.022	D. Concrete pavement:	
B. Concrete, bottom float finished, sides as indicated:		1. Float finish	
1. Dressed stone in mortar	0.015-0.017	2. Broom finish	
2. Random stone in mortar	0.017-0.020	E. For gutters with small slope, where sediment may accumulate, increase above values of $n$ by	
3. Cement rubble masonry	0.020-0.025		
4. Cement rubble masonry, plastered	0.016-0.020		
5. Dry rubble (riprap)	0.020-0.030		
C. Gravel bottom, sides as indicated:		<b>VI. Natural stream channels: <sup>1</sup></b>	
1. Formed concrete	0.017-0.020	A. Minor streams <sup>1</sup> (surface width at flood stage less than 100 ft.):	
2. Random stone in mortar	0.020-0.023	1. Fairly regular section:	
3. Dry rubble (riprap)	0.023-0.033	a. Some grass and weeds, little or no brush	
D. Brick	0.014-0.017	b. Dense growth of weeds, depth of flow materially greater than weed height	
E. Asphalt:		c. Some weeds, light brush on banks	
1. Smooth	0.013	d. Some weeds, heavy brush on banks	
2. Rough	0.016	e. Some weeds, dense willows on banks	
F. Wood, planed, clean	0.011-0.013	f. For trees within channel, with branches submerged at high stage, increase $n$ above values by	
G. Concrete-lined excavated rock:		2. Irregular sections, with pools, slight channel meander; increase values given in 1a-e about	
1. Good section	0.017-0.020	3. Mountain streams, no vegetation in channel, banks usually steep, trees and brush along banks submerged at high stage:	
2. Irregular section	0.022-0.027	a. Bottom of gravel cobbles, and few boulders	
<b>III. Open channels, excavated <sup>4</sup> (straight alignment, <sup>4</sup> natural lining):</b>		b. Bottom of cobbles, with large boulders	
A. Earth, uniform section:		B. Flood plains (adjacent to natural streams):	
1. Clean, recently completed	0.016-0.018	-1. Pasture, no brush:	
2. Clean, after weathering	0.018-0.020	a. Short grass	
3. With short grass, few weeds	0.022-0.027	b. High grass	
4. In gravelly soil, uniform section, clean	0.022-0.025	2. Cultivated areas:	
B. Earth, fairly uniform section:		a. No crop	
1. No vegetation	0.022-0.025	b. Mature row crops	
2. Grass, some weeds	0.025-0.030	c. Mature field crops	
3. Dense weeds or aquatic plants in deep channels	0.030-0.035	3. Heavy weeds, scattered brush	
4. Sides clean, gravel bottom	0.025-0.030	4. Light brush and trees: <sup>10</sup>	
5. Sides clean, cobble bottom	0.030-0.040	a. Winter	
C. Dragline excavated or dredged:		b. Summer	
1. No vegetation	0.028-0.033	5. Medium to dense brush: <sup>10</sup>	
2. Light brush on banks	0.035-0.050	a. Winter	
D. Rock:		b. Summer	
1. Based on design section	0.035	6. Dense willows, summer, not bent over by current	
2. Based on actual mean section:		7. Cleared land with tree stumps, 100-150 per acre:	
a. Smooth and uniform	0.035-0.040	a. No sprouts	
b. Jagged and irregular	0.040-0.045	b. With heavy growth of sprouts	
E. Channels not maintained, weeds and brush uncut:		8. Heavy stand of timber, a few down trees, little undergrowth:	
1. Dense weeds, high as flow depth	0.08-0.12	a. Flood depth below branches	
2. Clean bottom, brush on sides	0.05-0.08	b. Flood depth reaches branches	
3. Clean bottom, brush on sides, highest stage of flow	0.07-0.11	C. Major streams (surface width at flood stage more than 100 ft.): Roughness coefficient is usually less than for minor streams of similar description on account of less effective resistance offered by irregular banks or vegetation on banks. Values of $n$ may be somewhat reduced. Follow recommendation in publication cited <sup>4</sup> if possible. The value of $n$ for larger streams of most regular section, with no boulders or brush, may be in the range of	
4. Dense brush, high stage	0.10-0.14		

Footnotes to table 1 appear at the top of page 101.

Table 13-3  
MANNING'S ROUGHNESS COEFFICIENTS

Lining Category	Lining Type	Depth Ranges		
		0 - 0.5 (ft)	0.5 - 2.0 (ft)	> 2.0 (ft)
Rigid	Concrete	0.015	0.013	0.013
	Grouted Riprap	0.040	0.030	0.028
	Stone Masonry	0.042	0.032	0.030
	Soil Cement	0.025	0.022	0.020
	Asphalt	0.018	0.016	0.016
Temporary	Woven Paper Net	0.016	0.015	0.015
	Jute Net	0.028	0.022	0.019
	Fiberglass Roving	0.028	0.021	0.019
	Straw and Erosion Net	0.065	0.033	0.025
	Curled Wood Mat	0.066	0.035	0.028
	Nylon Mat	0.036	0.025	0.021
Gravel	1-inch, D <sub>50</sub>	0.044	0.033	0.030
	2-inch, D <sub>50</sub>	0.066	0.041	0.034
Rock Riprap	6-inch, D <sub>50</sub>	0.104	0.069	0.035
	12-inch, D <sub>50</sub>	---	0.078	0.040

Streets 1016 - 1015

ARAPAHOE CO., COLORADO  
1985

EROSION & SEDIMENTATION CONTROL

Table 2-1 Values of  $n$  to be used with the Manning equation [2]

Surface	Best	Good	Fair	Bad
(2-24) Uncoated cast-iron pipe	0.012	0.013	0.014	0.015
Coated cast-iron pipe	0.011	0.012 <sup>a</sup>	0.013 <sup>a</sup>	
Commercial wrought-iron pipe, black	0.012	0.013	0.014	0.015
Commercial wrought-iron pipe, galvanized	0.013	0.014	0.015	0.017
Smooth brass and glass pipe	0.009	0.010	0.011	0.013
Smooth lockbar and welded "OD" pipe	0.010	0.011 <sup>a</sup>	0.013 <sup>a</sup>	
Riveted and spiral steel pipe	0.013	0.015 <sup>a</sup>	0.017 <sup>a</sup>	
Vitrified sewer pipe	{ 0.010 0.011 }	0.013 <sup>a</sup>	0.015	0.017
Common clay drainage tile	0.011	0.012 <sup>a</sup>	0.014 <sup>a</sup>	0.017
(2-25) Glazed brickwork	0.011	0.012	0.013 <sup>a</sup>	0.015
Brick in cement mortar; brick sewers	0.012	0.013	0.015 <sup>a</sup>	0.017
Neat cement surfaces	0.010	0.011	0.012	0.013
Cement mortar surfaces	0.011	0.012	0.013 <sup>a</sup>	0.015
Concrete pipe	0.012	0.013	0.015 <sup>a</sup>	0.016
(2-26) Wood stave pipe	0.010	0.011	0.012	0.013
Plank flumes				
Planned	0.010	0.012 <sup>a</sup>	0.013	0.014
Unplanned	0.011	0.013 <sup>a</sup>	0.014	0.015
With battens	0.012	0.015 <sup>a</sup>	0.016	
(2-27) Concrete-lined channels	0.012	0.014 <sup>a</sup>	0.016 <sup>a</sup>	0.018
Cement-rubble surface	0.017	0.020	0.025	0.030
Dry-rubble surface	0.025	0.030	0.033	0.035
(2-27a) Dressed-ashlar surface	0.013	0.014	0.015	0.017
Semicircular metal flumes, smooth	0.011	0.012	0.013	0.015
Semicircular metal flumes, corrugated	0.0225	0.025	0.0275	0.030
Canals and ditches				
Earth, straight and uniform	0.017	0.020	0.0225 <sup>a</sup>	0.025
Rock cuts, smooth and uniform	0.025	0.030	0.033 <sup>a</sup>	0.035
Rock cuts, jagged and irregular	0.035	0.040	0.045	
Winding sluggish canals	0.0225	0.025 <sup>a</sup>	0.0275	0.030
(2-28a) Dredged-earth channels	0.025	0.0275 <sup>a</sup>	0.030	0.033
Canals with rough stony beds, weeds on earth banks	0.025	0.030	0.035 <sup>a</sup>	0.040
Earth bottom, rubble sides	0.028	0.030 <sup>a</sup>	0.033 <sup>a</sup>	0.035
Natural-stream channels				
1. Clean, straight bank, full stage, no rifts or deep pools	0.025	0.0275	0.030	0.033
2. Same as (1), but some weeds and stones	0.030	0.033	0.035	0.040
3. Winding, some pools and shoals, clean	0.033	0.035	0.040	0.045
4. Same as (3), lower stages, more ineffective slope and sections	0.040	0.045	0.050	0.055
5. Same as (3), some weeds and stones	0.035	0.040	0.045	0.050
6. Same as (4), stony sections	0.045	0.050	0.055	0.060
(2-29) 7. Sluggish river reaches, rather weedy or with very deep pools	0.050	0.060	0.070	0.080
(2-29a) 8. Very weedy reaches	0.075	0.100	0.125	0.150

<sup>a</sup>Values commonly used in designing.

H I S T O R I C  
S U B - B A S I N S U M M A R Y

SUB-BASIN HISTORIC	AREA AC.	AREA SM.	HYDRO. SOIL GROUP	SCS CN
OF1	11.22	0.0175	B	91
OF2	12.00	0.0187	B	91
OF3	3.09	0.0048	B	88
A	26.07	0.0407	B & C	93
B	23.25	0.0363	C	94
C	15.68	0.0245	C	94
TOTAL	91.31	0.1426		

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed \_\_\_\_\_  
 Circle one:  $T_c$   $T_t$  through subarea 2 YEAR STORM  
SUB-BASIN OF 1

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

<u>Sheet flow</u> (Applicable to $T_c$ only)	Segment ID		
1. Surface description (table 3-1) .....	A-B	PASTURE	
2. Manning's roughness coeff., n (table 3-1) ..	0.35		
3. Flow length, L (total L < 300 ft) .....	150	ft	
4. Two-yr 24-hr rainfall, $P_2$ .....	0.70	in	
5. Land slope, s .....	0.055	ft/ft	
6. $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ .....	0.635	hr	0.635

<u>Shallow concentrated flow</u>	Segment ID		
7. Surface description (paved or unpaved) .....	B-B1	PASTURE	
8. Flow length, L .....	510	ft	
9. Watercourse slope, s .....	0.008	ft/ft	
10. Average velocity, V (figure 3-1) .....	1.3	ft/s	
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	0.109	hr	0.109

<u>Channel flow</u>	Segment ID		
12. Cross sectional flow area, a .....	B1-C	SEE	
13. Wetted perimeter, $p_w$ .....	CALL		
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	SHEET		
15. Channel slope, s .....	0.014	ft/ft	
16. Manning's roughness coeff., n .....	0.025		
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	3.14	ft/s	
18. Flow length, L .....	360	ft	
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	0.032	hr	0.032
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....		hr	0.776

$T_{LAG} = 0.60 (0.776) = 0.466$  HOURS  
 CHECK COMPUTATION INTERVAL  $0.29 * T_{LAG} = 8.11$  MINUTES  
 (210-VI-TR-55, Second Ed., June 1986) SET CI @ 5 MIN.

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_t$  through subarea SUB-BASIN OF 1

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

<u>Sheet flow</u> (Applicable to $T_c$ only)	Segment ID		
1. Surface description (table 3-1) .....	A-B	PASTURE	
2. Manning's roughness coeff., n (table 3-1) ..	0.35		
3. Flow length, L (total L $\leq$ 300 ft) .....	150	ft	
4. <del>100</del> <sup>100</sup> yr 24-hr rainfall, $P_2$ .....	2.01	in	
5. Land slope, s .....	0.055	ft/ft	
6. $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$ Compute $T_c$ .....	0.375	hr	0.375

<u>Shallow concentrated flow</u>	Segment ID		
7. Surface description (paved or unpaved) .....	B-B1	PASTURE	
8. Flow length, L .....	510	ft	
9. Watercourse slope, s .....	0.008	ft/ft	
10. Average velocity, V (figure <del>41</del> <sup>402</sup> ) .....	1.3	ft/s	
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	0.109	hr	0.109

<u>Channel flow</u>	Segment ID		
12. Cross sectional flow area, a .....	B1-C	SEE	
13. Wetted perimeter, $p_w$ .....	CALL		
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	SHEET		
15. Channel slope, s .....	0.014	ft/ft	
16. Manning's roughness coeff., n .....	0.025		
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	4.45	ft/s	
18. Flow length, L .....	360	ft	
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	0.023	hr	0.023
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....		hr	0.507

$T_{LAG} = 0.60 (0.507) = 0.304 \text{ HOURS}$

HISTORIC

Triangular Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH B1 TO C

Comment: 2 YEAR DITCH FLOW

Solve For Depth

Given Input Data:

Left Side Slope..	1.00:1 (H:V)
Right Side Slope.	1.00:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0140 ft/ft
Discharge.....	2.24 cfs $\approx$ 0.20 CFS/AC. OF BASIN AREA

Computed Results:

Depth.....	0.84 ft
Velocity.....	<u>3.14 fps</u> - INITIAL EST.
Flow Area.....	0.71 sf
Flow Top Width...	1.69 ft
Wetted Perimeter.	2.39 ft
Critical Depth...	0.79 ft
Critical Slope...	0.0197 ft/ft
Froude Number....	0.85 (flow is Subcritical)



HISTORIC

Triangular Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH B1 TO C

Comment: 100 YEAR DITCH FLOW

Solve For Depth

Given Input Data:

Left Side Slope..	1.00:1 (H:V)
Right Side Slope.	1.00:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0140 ft/ft
Discharge.....	8.98 cfs <i>2 0.80 CFS/AC. OF BASIN AREA</i>

Computed Results:

Depth.....	1.42 ft
Velocity.....	<u>4.45 fps</u> - INITIAL EST.
Flow Area.....	2.02 sf
Flow Top Width...	2.84 ft
Wetted Perimeter.	4.02 ft
Critical Depth...	1.38 ft
Critical Slope...	0.0164 ft/ft
Froude Number....	0.93 (flow is Subcritical)

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

Circle one:  $T_c$   $T_t$  through subarea

2 YEAR STORM  
SUB-BASIN OF 2

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

Segment ID

1. Surface description (table 3-1) .....
2. Manning's roughness coeff., n (table 3-1) ..
3. Flow length, L (total L  $\leq$  300 ft) ..... ft
4. Two-yr 24-hr rainfall,  $P_2$  ..... in
5. Land slope, s ..... ft/ft
6.  $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

<u>D-E</u>	
<u>PASTURE</u>	
<u>0.35</u>	
<u>250</u>	
<u>0.70</u>	
<u>0.032</u>	
<u>1.186</u> +	

1.186

Shallow concentrated flow

Segment ID

7. Surface description (paved or unpaved) .....
8. Flow length, L ..... ft
9. Watercourse slope, s ..... ft/ft
10. Average velocity, V (figure ~~A~~) ..... ft/s
11.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr

<u>E-F</u>	
<u>PASTURE</u>	
<u>210</u>	
<u>0.005</u>	
<u>1.10</u>	
<u>0.053</u> +	

0.053

Channel flow

Segment ID

12. Cross sectional flow area, a ..... ft<sup>2</sup>
13. Wetted perimeter,  $p_w$  ..... ft
14. Hydraulic radius,  $r = \frac{a}{p_w}$  Compute r ..... ft
15. Channel slope, s ..... ft/ft
16. Manning's roughness coeff., n .....
17.  $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
18. Flow length, L ..... ft
19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

<u>F-G</u>	
<u>SEE</u>	
<u>CALL</u>	
<u>Sheet</u>	
<u>0.024</u>	
<u>0.025</u>	
<u>3.91</u>	
<u>895</u>	
<u>0.064</u> +	

0.064

1.303

$T_{LAG} = 0.60 (1.303) = 0.781 \text{ HOURS}$

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_t$  through subarea SUB-BASIN OF Z

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to $T_c$ only)	Segment ID
1. Surface description (table 3-1) .....	D-E PASTURE
2. Manning's roughness coeff., n (table 3-1) ..	0.35
3. Flow length, L (total L $\leq$ 300 ft) ..... ft	250
4. <del>100-</del> <sup>100-</sup> 24-hr rainfall, $P_2$ ..... in	2.01
5. Land slope, s ..... ft/ft	0.032
6. $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$ Compute $T_c$ ..... hr	0.699 + <span style="border: 1px solid black; padding: 2px;"> </span> = <span style="border: 1px solid black; padding: 2px;">0.699</span>

Shallow concentrated flow	Segment ID
7. Surface description (paved or unpaved) .....	E-F PASTURE
8. Flow length, L ..... ft	210
9. Watercourse slope, s ..... ft/ft	0.005
10. Average velocity, V (figure <del>4</del> ) ..... ft/s	1.10
11. $T_t = \frac{L}{3600 V}$ Compute $T_t$ ..... hr	0.053 + <span style="border: 1px solid black; padding: 2px;"> </span> = <span style="border: 1px solid black; padding: 2px;">0.053</span>

Channel flow	Segment ID
12. Cross sectional flow area, a ..... ft <sup>2</sup>	F-G SEE
13. Wetted perimeter, $p_w$ ..... ft	CALL.
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r ..... ft	SHEET
15. Channel slope, s ..... ft/ft	0.024
16. Manning's roughness coeff., n .....	0.025
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V ..... ft/s	5.53
18. Flow length, L ..... ft	895
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ ..... hr	0.045 + <span style="border: 1px solid black; padding: 2px;"> </span> = <span style="border: 1px solid black; padding: 2px;">0.045</span>
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) ..... hr	0.797

$T_{LAG} = 0.60 (0.797) = 0.478 \text{ HOURS}$

HISTORIC

Triangular Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH F TO G

Comment: 2 YEAR DITCH FLOW

Solve For Depth

Given Input Data:

Left Side Slope..	1.00:1 (H:V)
Right Side Slope.	1.00:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0240 ft/ft
Discharge.....	2.40 cfs $\approx$ 0.120 CFS/AC. OF BASIN AREA

Computed Results:

Depth.....	0.78 ft
Velocity.....	3.91 fps - INITIAL EST.
Flow Area.....	0.61 sf
Flow Top Width...	1.57 ft
Wetted Perimeter.	2.22 ft
Critical Depth...	0.81 ft
Critical Slope...	0.0195 ft/ft
Froude Number....	1.10 (flow is Supercritical)

HISTORIC

Triangular Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH F TO G

Comment: 100 YEAR DITCH FLOW

Solve For Depth

Given Input Data:

Left Side Slope..	1.00:1 (H:V)
Right Side Slope.	1.00:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0240 ft/ft
Discharge.....	9.60 cfs $\approx$ 0.80 CFS/AC. OF BASIN AREA

Computed Results:

Depth.....	1.32 ft
Velocity.....	<u>5.53 fps</u> INITIAL EST.
Flow Area.....	1.74 sf
Flow Top Width...	2.63 ft
Wetted Perimeter.	3.73 ft
Critical Depth...	1.42 ft
Critical Slope...	0.0162 ft/ft
Froude Number....	1.20 (flow is Supercritical)

# HISTORIC

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

2 YEAR STORM  
SUB-BASIN OF 3

Circle one:  $T_c$   $T_t$  through subarea

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to $T_c$ only)	Segment ID		
1. Surface description (table 3-1) .....		R-S	
2. Manning's roughness coeff., n (table 3-1) ..		RESIDENTIAL LAWNS	
3. Flow length, L (total L < 300 ft) .....	ft	0.35	
4. Two-yr 24-hr rainfall, $P_2$ .....	in	150	
5. Land slope, s .....	ft/ft	0.70	
6. $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ .....	hr	0.010	
		1.255	- 1.255

Shallow concentrated flow	Segment ID		
7. Surface description (paved or unpaved) .....		N/A	
8. Flow length, L .....	ft		
9. Watercourse slope, s .....	ft/ft		
10. Average velocity, V (figure <del>3</del> ) .....	ft/s		
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	hr		- N/A

Channel flow	Segment ID		
12. Cross sectional flow area, a .....	ft <sup>2</sup>	S-K	
13. Wetted perimeter, $p_w$ .....	ft	SEE	
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	ft	CALL	
15. Channel slope, s .....	ft/ft	SHEET	
16. Manning's roughness coeff., n .....		0.012	
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	ft/s	0.025	
18. Flow length, L .....	ft	2.30	
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	hr	640	
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....	hr	0.077	- 0.077
			1.332

TLAG = 0.60 (1.332) = 0.799 HOURS

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

100 YEAR STORM  
SUB-BASIN OF 3

Circle one:  $T_c$   $T_t$  through subarea

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

- Segment ID
1. Surface description (table 3-1) .....
  2. Manning's roughness coeff., n (table 3-1) ..
  3. Flow length, L (total L  $\leq$  300 ft) ..... ft
  4. ~~100~~ <sup>100</sup> yr 24-hr rainfall,  $P_{100}$  ..... in
  5. Land slope, s ..... ft/ft
  6.  $T_c = \frac{0.007 (nL)^{0.8}}{P_{100}^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

<u>R-S</u>	
<u>RESIDENTIAL LAWNS</u>	
<u>0.35</u>	
<u>150</u>	
<u>2.01</u>	
<u>0.010</u>	
<u>0.741</u>	+

0.741

Shallow concentrated flow

- Segment ID
7. Surface description (paved or unpaved) .....
  8. Flow length, L ..... ft
  9. Watercourse slope, s ..... ft/ft
  10. Average velocity, V (figure ~~2-1~~ <sup>402</sup>) ..... ft/s
  11.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr

<u>N/A</u>	

N/A

Channel flow

- Segment ID
12. Cross sectional flow area, a ..... ft<sup>2</sup>
  13. Wetted perimeter,  $p_w$  ..... ft
  14. Hydraulic radius,  $r = \frac{a}{p_w}$  Compute r ..... ft
  15. Channel slope, s ..... ft/ft
  16. Manning's roughness coeff., n .....
  17.  $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
  18. Flow length, L ..... ft
  19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
  20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

<u>S-K</u>	
<u>SEE</u>	
<u>CALL.</u>	
<u>SHEET</u>	
<u>0.012</u>	
<u>0.025</u>	
<u>3.25</u>	
<u>640</u>	
<u>0.055</u>	+

0.055

0.796

$T_{LAG} = 0.60 (0.796) = 0.478 \text{ HOURS}$

HISTORIC

Triangular Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH S TO K

Comment: 2 YEAR DITCH FLOW

Solve For Depth

Given Input Data:

Left Side Slope..	1.00:1 (H:V)
Right Side Slope.	1.00:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0050 ft/ft
Discharge.....	3.02 cfs

2 0.20 CFS/AC. OF BASIN AREA

Computed Results:

Depth.....	1.15 ft
Velocity.....	2.30 fps - INITIAL EST.
Flow Area.....	1.31 sf
Flow Top Width...	2.29 ft
Wetted Perimeter.	3.24 ft
Critical Depth...	0.89 ft
Critical Slope...	0.0189 ft/ft
Froude Number....	0.54 (flow is Subcritical)



HISTORIC

Triangular Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH S TO K

Comment: 100 YEAR DITCH FLOW

Solve For Depth

Given Input Data:

Left Side Slope.. 1.00:1 (H:V)  
Right Side Slope. 1.00:1 (H:V)  
Manning's n..... 0.025  
Channel Slope.... 0.0050 ft/ft  
Discharge..... 12.07 cfs

~ 0.80 CFS/AC. OF BASIN AREA

Computed Results:

Depth..... 1.93 ft  
Velocity..... 3.25 fps INITIAL EST.  
Flow Area..... 3.71 sf  
Flow Top Width... 3.85 ft  
Wetted Perimeter. 5.45 ft  
Critical Depth... 1.55 ft  
Critical Slope... 0.0157 ft/ft  
Froude Number.... 0.58 (flow is Subcritical)

# HISTORIC

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

Circle one:  $T_c$   $T_t$  through subarea

2 YEAR STORM

SUB-BASIN A

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

Segment ID

- |   |       |                     |
|---|-------|---------------------|
| 1. Surface description (table 3-1) .....                    | ..... |                     |
| 2. Manning's roughness coeff., n (table 3-1) ..             |       |                     |
| 3. Flow length, L (total L $\leq$ 300 ft) .....             | ft    |                     |
| 4. Two-yr 24-hr rainfall, $P_2$ .....                       | in    |                     |
| 5. Land slope, s .....                                      | ft/ft |                     |
| 6. $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ ..... | hr    | Compute $T_c$ ..... |

H-I	
CULTIVATED	
0.22	
300	
0.70	
0.008	
1.648	+ [ ] - 1.648

Shallow concentrated flow

Segment ID

- |  |       |                     |
|--|-------|---------------------|
| 7. Surface description (paved or unpaved) .....        | ..... |                     |
| 8. Flow length, L .....                                | ft    |                     |
| 9. Watercourse slope, s .....                          | ft/ft |                     |
| 10. Average velocity, V (figure <del>3-1</del> ) ..... | ft/s  |                     |
| 11. $T_t = \frac{L}{3600 V}$ .....                     | hr    | Compute $T_c$ ..... |

I-J	
CULTIVATED	
460	
0.012	
1.70	
0.075	+ [ ] - 0.075

Channel flow

Segment ID

- |  |                 |                     |
|--|-----------------|---------------------|
| 12. Cross sectional flow area, a .....   | ft <sup>2</sup> |                     |
| 13. Wetted perimeter, $p_w$ .....  | ft              |                     |
| 14. Hydraulic radius, $r = \frac{a}{p_w}$ .....                                  | ft              | Compute r .....     |
| 15. Channel slope, s .....   | ft/ft           |                     |
| 16. Manning's roughness coeff., n .....  |                 |                     |
| 17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ .....                                   | ft/s            | Compute V .....     |
| 18. Flow length, L .....   | ft              |                     |
| 19. $T_c = \frac{L}{3600 V}$ .....   | hr              | Compute $T_c$ ..... |
| 20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) ..... | hr              |                     |

J-K	
SEE	
CALL	
SHEET	
0.010	
0.025	
3.74	
1,390	
0.103	+ [ ] - 0.103

$T_{LAG} = 0.60 (1.826) = 1.096 \text{ HOURS}$

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

Circle one:  $T_c$   $T_t$  through subarea

100 YEAR STORM  
SUB-BASIN A

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

Segment ID	
1. Surface description (table 3-1) .....	H-I CULTIVATED
2. Manning's roughness coeff., n (table 3-1) ..	0.22
3. Flow length, L (total L $\leq$ 300 ft) .....	300 ft
4. <del>100-</del> yr 24-hr rainfall, $P_2$ .....	2.01 in
5. Land slope, s .....	0.008 ft/ft
6. $T_t = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$ Compute $T_t$ .....	0.973 hr = <span style="border: 1px solid black; padding: 2px;">0.973</span>

Shallow concentrated flow

Segment ID	
7. Surface description (paved or unpaved) .....	I-I CULTIVATED
8. Flow length, L .....	460 ft
9. Watercourse slope, s .....	0.012 ft/ft
10. Average velocity, V (figure <del>2</del> ) .....	1.70 ft/s
11. $T_t = \frac{L}{3600 V}$ Compute $T_t$ .....	0.075 hr = <span style="border: 1px solid black; padding: 2px;">0.075</span>

Channel flow

Segment ID	
12. Cross sectional flow area, a .....	J-K SEE
13. Wetted perimeter, $p_w$ .....	CALL.
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	SHEET
15. Channel slope, s .....	0.010 ft/ft
16. Manning's roughness coeff., n .....	0.025
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	5.29 ft/s
18. Flow length, L .....	1.390 ft
19. $T_t = \frac{L}{3600 V}$ Compute $T_t$ .....	0.073 hr = <span style="border: 1px solid black; padding: 2px;">0.073</span>
20. Watershed or subarea $T_c$ or $T_t$ (add $T_t$ in steps 6, 11, and 19) .....	<span style="border: 1px solid black; padding: 2px;">1.121</span> hr

$T_{LAG} = 0.60 (1.121) = 0.673 \text{ HOURS}$

HISTORIC

Triangular Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH J TO K

Comment: 2 YEAR DITCH FLOW

Solve For Depth

Given Input Data:

Left Side Slope..	1.00:1 (H:V)
Right Side Slope.	1.00:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0100 ft/ft
Discharge.....	7.46 cfs

2 0.20 CFS/AC. OF BASIN AREA

Computed Results:

Depth.....	1.41 ft
Velocity.....	3.74 fps
Flow Area.....	1.99 sf
Flow Top Width...	2.82 ft
Wetted Perimeter.	3.99 ft
Critical Depth...	1.28 ft
Critical Slope...	0.0168 ft/ft
Froude Number....	0.78 (flow is Subcritical)

INITIAL EST.

HISTORIC

Triangular Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH J TO K

Comment: 100 YEAR DITCH FLOW

Solve For Depth

Given Input Data:

Left Side Slope..	1.00:1 (H:V)
Right Side Slope.	1.00:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0100 ft/ft
Discharge.....	29.83 cfs

~ 0.80 cfs/AC. OF BASIN AREA

Computed Results:

Depth.....	2.37 ft
Velocity.....	<u>5.29 fps</u> INITIAL EST.
Flow Area.....	5.64 sf
Flow Top Width...	4.75 ft
Wetted Perimeter.	6.72 ft
Critical Depth...	2.23 ft
Critical Slope...	0.0139 ft/ft
Froude Number....	0.86 (flow is Subcritical)

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

2 YEAR STORM

Circle one:  $T_c$   $T_t$  through subarea

SUB-BASIN B

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

	Segment ID	
1. Surface description (table 3-1) .....	L-M	
2. Manning's roughness coeff., n (table 3-1) ..	CULTIVATED	
3. Flow length, L (total L < 300 ft) .....	0.22	
4. Two-yr 24-hr rainfall, $P_2$ .....	300	ft
5. Land slope, s .....	0.70	in
6. $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_t$ .....	0.009	ft/ft
	1.572	hr
	+ [ ]	- 1572

Shallow concentrated flow

	Segment ID	
7. Surface description (paved or unpaved) .....	M-N	
8. Flow length, L .....	CULTIVATED	
9. Watercourse slope, s .....	1,060	ft
10. Average velocity, V (figure <del>34</del> ) .....	0.012	ft/ft
11. $T_t = \frac{L}{3600 V}$ Compute $T_t$ .....	1.66	ft/s
	402	hr
	0.178	hr
	+ [ ]	- 0.178

Channel flow

	Segment ID	
12. Cross sectional flow area, a .....	N-O	
13. Wetted perimeter, $p_w$ .....	SEE	ft <sup>2</sup>
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	CALL	ft
15. Channel slope, s .....	SHEET	ft
16. Manning's roughness coeff., n .....	0.005	ft/ft
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	0.025	ft/s
18. Flow length, L .....	2.56	ft/s
19. $T_t = \frac{L}{3600 V}$ Compute $T_t$ .....	400	ft
20. Watershed or subarea $T_c$ or $T_t$ (add $T_t$ in steps 6, 11, and 19) .....	0.043	hr
	+ [ ]	- 0.043
		1.793

$T_{LAG} = 0.60 (1.793) = 1.076 \text{ HOURS}$

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

Circle one:  $T_c$   $T_c$  through subarea

100 YEAR STORM  
SUB-BASIN B

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

- Segment ID
1. Surface description (table 3-1) .....
  2. Manning's roughness coeff., n (table 3-1) ..
  3. Flow length, L (total L  $\leq$  300 ft) ..... ft
  4. ~~100~~-yr 24-hr rainfall,  $P_2$  ..... in  
~~100~~
  5. Land slope, s ..... ft/ft
  6.  $T_t = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

L-M	
CULTIVATED	
0.22	
300	
2.01	
0.009	
0.928	- 0.928

Shallow concentrated flow

- Segment ID
7. Surface description (paved or unpaved) .....
  8. Flow length, L ..... ft
  9. Watercourse slope, s ..... ft/ft
  10. Average velocity, V (figure ~~41~~) ..... ft/s  
402
  11.  $T_t = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr

M-N	
CULTIVATED	
1,060	
0.012	
1.66	
0.178	- 0.178

Channel flow

- Segment ID
12. Cross sectional flow area, a ..... ft<sup>2</sup>
  13. Wetted perimeter,  $p_w$  ..... ft
  14. Hydraulic radius,  $r = \frac{a}{p_w}$  Compute r ..... ft
  15. Channel slope, s ..... ft/ft
  16. Manning's roughness coeff., n .....
  17.  $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
  18. Flow length, L ..... ft
  19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
  20. Watershed or subarea  $T_c$  or  $T_c$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

N-O	
SEE	
CALL	
SHEET	
0.005	
0.025	
3.62	
400	
0.031	- 0.031
	1.137

$T_{LAG} = 0.60 (1.137) = 0.682 \text{ HOURS}$

HISTORIC

Triangular Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH N TO O

Comment: 2 YEAR DITCH FLOW

Solve For Depth

Given Input Data:

Left Side Slope..	1.00:1 (H:V)
Right Side Slope.	1.00:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0050 ft/ft
Discharge.....	4.65 cfs

Z 0.70 CFS/AC OF BASIN AREA

Computed Results:

Depth.....	1.35 ft
Velocity.....	2.56 fps INITIAL EST.
Flow Area.....	1.81 sf
Flow Top Width...	2.69 ft
Wetted Perimeter.	3.81 ft
Critical Depth...	1.06 ft
Critical Slope...	0.0179 ft/ft
Froude Number....	0.55 (flow is Subcritical)



HISTORIC

Triangular Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH N TO O

Comment: 100 YEAR DITCH FLOW

Solve For Depth

Given Input Data:

Left Side Slope..	1.00:1 (H:V)
Right Side Slope.	1.00:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0050 ft/ft
Discharge.....	18.60 cfs

*2 0.80 cfs/AC. OF BASIN AREA*

Computed Results:

Depth.....	2.27 ft
Velocity.....	<u>3.62 fps</u> - INITIAL EST.
Flow Area.....	5.13 sf
Flow Top Width..	4.53 ft
Wetted Perimeter.	6.41 ft
Critical Depth...	1.85 ft
Critical Slope...	0.0149 ft/ft
Froude Number....	0.60 (flow is Subcritical)

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

2 YEAR STORM  
SUB-BASIN C

Circle one:  $T_c$   $T_t$  through subarea

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

<u>Sheet flow</u> (Applicable to $T_c$ only)	Segment ID	
1. Surface description (table 3-1) .....		P-Q CULTIVATED
2. Manning's roughness coeff., n (table 3-1) ..		0.22
3. Flow length, L (total L < 300 ft) .....	ft	300
4. Two-yr 24-hr rainfall, $P_2$ .....	in	0.70
5. Land slope, s .....	ft/ft	0.011
6. $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ .....	hr	1.451 + <span style="border: 1px solid black; padding: 2px;"> </span> = <span style="border: 1px solid black; padding: 2px;">1.451</span>
 <u>Shallow concentrated flow</u>		
	Segment ID	
7. Surface description (paved or unpaved) .....		Q-O CULTIVATED
8. Flow length, L .....	ft	1,640
9. Watercourse slope, s .....	ft/ft	0.010
10. Average velocity, V (figure <del>3A</del> ) .....	ft/s	1.50
11. $T_t = \frac{L}{3600 V}$ Compute $T_t$ .....	hr	0.304 + <span style="border: 1px solid black; padding: 2px;"> </span> = <span style="border: 1px solid black; padding: 2px;">0.304</span>
 <u>Channel flow</u>		
	Segment ID	
12. Cross sectional flow area, a .....	ft <sup>2</sup>	N/A
13. Wetted perimeter, $p_w$ .....	ft	
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	ft	
15. Channel slope, s .....	ft/ft	
16. Manning's roughness coeff., n .....		
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	ft/s	
18. Flow length, L .....	ft	
19. $T_t = \frac{L}{3600 V}$ Compute $T_t$ .....	hr	N/A + <span style="border: 1px solid black; padding: 2px;"> </span> = <span style="border: 1px solid black; padding: 2px;">N/A</span>
20. Watershed or subarea $T_c$ or $T_t$ (add $T_t$ in steps 6, 11, and 19) .....	hr	<span style="border: 1px solid black; padding: 2px;">1.755</span>

$T_{LAG} = 0.60 (1.755) = 1.053 \text{ HOURS}$

## Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW ESTATES By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

Circle one:  $T_c$   $T_t$  through subarea

100 YEAR STORM  
SUB-BASIN C

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

- Segment ID
1. Surface description (table 3-1) .....
  2. Manning's roughness coeff., n (table 3-1) ..
  3. Flow length, L (total L  $\leq$  300 ft) ..... ft
  4. ~~100-yr~~ 24-hr rainfall,  $P_{\frac{100}{100}}$  ..... in
  5. Land slope, s ..... ft/ft
  6.  $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

<u>P-Q</u>	
<u>CULTIVATED</u>	
<u>0.22</u>	
<u>300</u>	
<u>2.01</u>	
<u>0.011</u>	
<u>0.856</u>	+

0.856

Shallow concentrated flow

- Segment ID
7. Surface description (paved or unpaved) .....
  8. Flow length, L ..... ft
  9. Watercourse slope, s ..... ft/ft
  10. Average velocity, V (figure ~~2~~) ..... ft/s
  11.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr

<u>Q-O</u>	
<u>CULTIVATED</u>	
<u>1,640</u>	
<u>0.010</u>	
<u>1.50</u>	
<u>0.304</u>	+

0.304

Channel flow

- Segment ID
12. Cross sectional flow area, a ..... ft<sup>2</sup>
  13. Wetted perimeter,  $p_w$  ..... ft
  14. Hydraulic radius,  $r = \frac{a}{p_w}$  Compute r ..... ft
  15. Channel slope, s ..... ft/ft
  16. Manning's roughness coeff., n .....
  17.  $v = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
  18. Flow length, L ..... ft
  19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
  20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

<u>N/A</u>	
<u>N/A</u>	+

N/A  
1.160

$T_{LAG} = 0.60 ( 1.160 ) = 0.696 \text{ HOURS}$

Trapezoidal Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH K TO O

Comment: 2 YEAR DITCH FLOW - ALONG 28 ROAD

Solve For Depth

Given Input Data:

Bottom Width.....	1.00 ft
Left Side Slope..	1.50:1 (H:V)
Right Side Slope.	1.50:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0070 ft/ft
Discharge.....	10.48 cfs

*2 0.20 CFS/AC. BASIN AREA.*

Computed Results:

Depth.....	1.12 ft
Velocity.....	<u>3.51 fps</u> INITIAL EST.
Flow Area.....	2.98 sf
Flow Top Width...	4.35 ft
Wetted Perimeter.	5.02 ft
Critical Depth...	0.97 ft
Critical Slope...	0.0130 ft/ft
Froude Number....	0.75 (flow is Subcritical)

Trapezoidal Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: DITCH K TO O

Comment: 100 YEAR DITCH FLOW - ALONG 28 ROAD

Solve For Depth

Given Input Data:

Bottom Width.....	1.00 ft
Left Side Slope..	1.50:1 (H:V)
Right Side Slope.	1.50:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0070 ft/ft
Discharge.....	41.90 cfs

*2 0.80 CFS/AC. OF BASIN AREA*

Computed Results:

Depth.....	2.06 ft
Velocity.....	<u>4.97 fps</u> - INITIAL EST.
Flow Area.....	8.43 sf
Flow Top Width...	7.18 ft
Wetted Perimeter.	8.43 ft
Critical Depth...	1.87 ft
Critical Slope...	0.0110 ft/ft
Froude Number....	0.81 (flow is Subcritical)

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
*
* RUN DATE 06/27/1994 TIME 19:03:59 *
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

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X X XXXXXXX XXXXX X
X X X X X XX
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X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,  
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HISTORIC 2 YEAR STORM  
 Run # 1

R1-1

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1         ID  GRAND VIEW SUBDIVISION
2         ID  HISTORIC CONDITION
3         ID  2 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)
4         IT   5 22JUN94   1200   300
5         IO   5      2      0
          * *****
6         KK   OF1
7         KM   Basin runoff calculation for   OF1
8         KO   3      1      0      1      21      110      300
9         BA   0.0175
10        PH           0      0.10   0.19   0.34   0.42   0.47   0.55   0.64   0.70
11        LS           91
12        UD   0.466
          * *****
13        KK   DITCH2
14        KM   Muskingum-Cunge channel routing from   CP1 to   CP3
15        KO   3      1      0      1      21      110      300
16        RD
17        RC   0.035   0.025   0.035   1770   0.010
18        RX   100     110     111     113     113.5   115.5   116.5   126.5
19        RY   4742   4742   4741.5  4739.5  4739.5  4741.5  4742   4742
          * *****
20        KK   A
21        KM   Basin runoff calculation for   A
22        KO   3      1      0      1      21      110      300
23        BA   0.0407
24        PH           0      0.10   0.19   0.34   0.42   0.47   0.55   0.64   0.70
25        LS           93
26        UD   1.096
          * *****
27        KK   A
28        KM   Combining two hydrographs at control point   CP3
29        KO   3      1      0      1      21      110      300
30        HC   2
          * *****
31        KK   OF2
32        KM   Basin runoff calculation for   OF2
33        KO   3      1      0      1      21      110      300
34        BA   0.0187
35        PH           0      0.10   0.19   0.34   0.42   0.47   0.55   0.64   0.70
36        LS           91
37        UD   0.781
          * *****
38        KK   DITCH1
39        KM   Muskingum-Cunge channel routing from   CP2 to   CP3
40        KO   3      1      0      1      21      110      300
41        RD
42        RC   0.035   0.025   0.035   700   0.012
43        RX   100     110     111     113     113.5   115.5   116.5   126.5
    
```

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
44	RY	4734	4734	4733.5	4731.5	4731.5	4733.5	4734	4734		
		* *****									
45	KK	OF3									
46	KM	Basin runoff calculation for OF3									
47	KO	3	1	0	1	21	110	300			
48	BA	0.0048									
49	PH		0	0.10	0.19	0.34	0.42	0.47	0.55	0.64	0.70
50	LS		88								
51	UD	0.799									
		* *****									
52	KK	OF3									
53	KM	Combining two hydrographs at control point									CP3
54	KO	3	1	0	1	21	110	300			
55	HC	2									
		* *****									
56	KK	OF3									
57	KM	Combining two hydrographs at control point									CP3
58	KO	3	1	0	1	21	110	300			
59	HC	2									
		* *****									
60	KK	DITCH3									
61	KM	Muskingum-Cunge channel routing from									CP3 to CP4
62	KO	3	1	0	1	21	110	300			
63	RD										
64	RC	0.035	0.025	0.035	1480	0.007					
65	RX	100	105	110	113	114	117	122	127		
66	RY	4726	4726	4725.5	4723.5	4723.5	4725.5	4726	4726		
		* *****									
67	KK	B									
68	KM	Basin runoff calculation for B									
69	KO	3	1	0	1	21	110	300			
70	BA	0.0363									
71	PH		0	0.10	0.19	0.34	0.42	0.47	0.55	0.64	0.70
72	LS		94								
73	UD	1.076									
		* *****									
74	KK	B									
75	KM	Combining two hydrographs at control point									CP4
76	KO	3	1	0	1	21	110	300			
77	HC	2									
		* *****									
78	KK	C									
79	KM	Basin runoff calculation for C									
80	KO	3	1	0	1	21	110	300			
81	BA	0.0245									
82	PH		0	0.10	0.19	0.34	0.42	0.47	0.55	0.64	0.70
83	LS		94								
84	UD	1.053									
		* *****									



LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

85	KK	C								
86	KM	Combining two hydrographs at control point								CP4
87	KO	1	2	0	1	21	110	300		
88	HC	2								
		* *****								
89	ZZ									

R1-4

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
*
* RUN DATE 06/27/1994 TIME 19:03:59 *
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

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GRAND VIEW SUBDIVISION  
HISTORIC CONDITION  
2 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)

5 IO OUTPUT CONTROL VARIABLES

IPRNT 5 PRINT CONTROL  
IPLOT 2 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA

NMIN 5 MINUTES IN COMPUTATION INTERVAL  
IDATE 22JUN94 STARTING DATE  
ITIME 1200 STARTING TIME  
NO 300 NUMBER OF HYDROGRAPH ORDINATES  
NDDATE 23JUN94 ENDING DATE  
NDTIME 1255 ENDING TIME  
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .08 HOURS  
TOTAL TIME BASE 24.92 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES  
PRECIPITATION DEPTH INCHES  
LENGTH, ELEVATION FEET  
FLOW CUBIC FEET PER SECOND  
STORAGE VOLUME ACRE-FEET  
SURFACE AREA ACRES  
TEMPERATURE DEGREES FAHRENHEIT

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*
* OF1 *
*
*****

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8 KO OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .083 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

9 BA SUBBASIN CHARACTERISTICS  
TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.10	.19	.34	.42	.47	.55	.64	.70	.00	.00	.00	.00

STORM AREA = .02

11 LS SCS LOSS RATE

RI-5

12 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .47 LAG

\*\*\*

UNIT HYDROGRAPH  
 30 END-OF-PERIOD ORDINATES

1.	4.	8.	13.	16.	17.	16.	14.	12.	9.
6.	5.	4.	3.	2.	2.	1.	1.	1.	1.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

\*\*\* \*\*

HYDROGRAPH AT STATION OF1

TOTAL RAINFALL = .70, TOTAL LOSS = .53, TOTAL EXCESS = .17

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW (CFS)	6-HR	24-HR	72-HR	24.92-HR
1.	12.58	0.	0.	0.	0.	0.
		(INCHES)	.148	.169	.169	.169
		(AC-FT)	0.	0.	0.	0.

SUB-BASIN CUMULATIVE AREA = .02 SQ MI

"OF1 INTO PROJECT SITE"

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 \* \*  
 \* DITCH2 \*  
 \* \*  
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15 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	110	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.083	TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

16 RD MUSKINGUM-CUNGE CHANNEL ROUTING

17 RC NORMAL DEPTH CHANNEL

ANL	.035	LEFT OVERBANK N-VALUE
ANCH	.025	MAIN CHANNEL N-VALUE
ANR	.035	RIGHT OVERBANK N-VALUE
RLNTH	1770.	REACH LENGTH
SEL	.0100	ENERGY SLOPE
ELMAX	.0	MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVERBANK ---	+	----- MAIN CHANNEL -----	+	--- RIGHT OVERBANK ---
19 RY ELEVATION	4742.00		4741.50 4739.50 4739.50 4741.50		4742.00 4742.00
18 RX DISTANCE	100.00		110.00 111.00 113.00 113.50 115.50		116.50 126.50

\*\*\*

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.00	.01	.01	.02	.03	.04	.05	.07	.08
OUTFLOW	.00	.10	.35	.76	1.35	2.13	3.13	4.37	5.87	7.64
ELEVATION	4739.50	4739.63	4739.76	4739.89	4740.03	4740.16	4740.29	4740.42	4740.55	4740.68
STORAGE	.10	.11	.13	.15	.17	.20	.22	.25	.28	.31
OUTFLOW	9.70	12.08	14.78	17.83	21.24	25.03	30.02	35.72	41.95	48.73
ELEVATION	4740.81	4740.94	4741.08	4741.21	4741.34	4741.47	4741.60	4741.73	4741.86	4742.00

COMPUTED MUSKINGUM-CUNGE PARAMETERS

ELEMENT	ALPHA	M	COMPUTATION TIME STEP		PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
			DT (MIN)	DX (FT)				
MAIN			3.00	221.25	1 10	765.00	17	2 40

R1-6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 5.00 1.10 765.00 .17

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1577E+00 EXCESS= .0000E+00 OUTFLOW= .1577E+00 BASIN STORAGE= .5110E-03 PERCENT ERROR= -.3

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HYDROGRAPH AT STATION DITCH2

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.92-HR
1.	12.75	0.	0.	0.	0.
		(CFS)	0.	0.	0.
		(INCHES)	.149	.169	.169
		(AC-FT)	0.	0.	0.

CUMULATIVE AREA = .02 SQ MI

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\* \*  
\* A \*  
\* \*  
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22 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	110	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.083	TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

23 BA SUBBASIN CHARACTERISTICS  
TAREA .04 SUBBASIN AREA

PRECIPITATION DATA

24 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.10	.19	.34	.42	.47	.55	.64	.70	.00	.00	.00	.00

STORM AREA = .04

25 LS SCS LOSS RATE

STRTL	.15	INITIAL ABSTRACTION
CRVNR	93.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

26 UD SCS DIMENSIONLESS UNITGRAPH  
TLAG 1.10 LAG

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UNIT HYDROGRAPH  
68 END-OF-PERIOD ORDINATES

0.	1.	2.	3.	5.	6.	9.	11.	13.	15.
16.	17.	17.	17.	17.	16.	16.	15.	14.	12.
11.	9.	8.	7.	6.	6.	5.	5.	4.	4.
3.	3.	2.	2.	2.	2.	2.	1.	1.	1.
1.	1.	1.	1.	1.	1.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

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HYDROGRAPH AT STATION A

TOTAL RAINFALL = .70. TOTAL LOSS = .47. TOTAL EXCESS = .23

PEAK FLOW TIME MAXIMUM AVERAGE FLOW

RI-7

2. 13.33 (CFS) 1. 0. 0. 0.  
 ↑ (INCHES) .200 .230 .230 .230  
 (AC-FT) 0. 0. 0. 0.  
 SUB-BASIN CUMULATIVE AREA = .04 SQ MI  
 "A"

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 \* \*  
 27 KK \* A \*  
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29 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .083 TIME INTERVAL IN HOURS

30 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION A

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW
3.	13.08	24.92-HR
(CFS)	(HR)	6-HR 24-HR 72-HR
1.		0. 0. 0.
(INCHES)		.185 .212 .212
(AC-FT)		1. 1. 1.

Sum of "OF1" and "A"

CUMULATIVE AREA = .06 SQ MI

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 \* \*  
 31 KK \* OF2 \*  
 \* \*  
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33 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .083 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

34 BA SUBBASIN CHARACTERISTICS  
 TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

35 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.10	.19	.34	.42	.47	.55	.64	.70	.00	.00	.00	.00

STORM AREA = .02

36 LS SCS LOSS RATE  
 STRTL .20 INITIAL ABSTRACTION  
 CRVNB 91.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

R1-8

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UNIT HYDROGRAPH  
49 END-OF-PERIOD ORDINATES

0.	1.	2.	3.	5.	7.	9.	10.	11.	11.
11.	10.	9.	8.	7.	6.	5.	4.	3.	3.
3.	2.	2.	2.	1.	1.	1.	1.	1.	1.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

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HYDROGRAPH AT STATION OF2

TOTAL RAINFALL = .70, TOTAL LOSS = .53, TOTAL EXCESS = .17

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.92-HR
1.	13.00	0.	0.	0.	0.
		(CFS)	(CFS)	(CFS)	(CFS)
		(INCHES)	(INCHES)	(INCHES)	(INCHES)
		(AC-FT)	(AC-FT)	(AC-FT)	(AC-FT)

CUMULATIVE AREA = .02 SQ MI

"OF2" INTO  
PROJECT SITE

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\* \*  
\* DITCH1 \*  
\* \*  
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40 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	110	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.083	TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

41 RD MUSKINGUM-CUNGE CHANNEL ROUTING

42 RC NORMAL DEPTH CHANNEL

ANL	.035	LEFT OVERBANK N-VALUE
ANCH	.025	MAIN CHANNEL N-VALUE
ANR	.035	RIGHT OVERBANK N-VALUE
RLNTH	700.	REACH LENGTH
SEL	.0120	ENERGY SLOPE
ELMAX	.0	MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

44 RY	ELEVATION	4734.00	4734.00	4733.50	4731.50	4731.50	4733.50	4734.00	4734.00
43 RX	DISTANCE	100.00	110.00	111.00	113.00	113.50	115.50	116.50	126.50

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.00	.00	.01	.01	.01	.02	.02	.03	.03
OUTFLOW	.00	.11	.39	.83	1.47	2.33	3.43	4.79	6.43	8.37
ELEVATION	4731.50	4731.63	4731.76	4731.89	4732.03	4732.16	4732.29	4732.42	4732.55	4732.68
STORAGE	.04	.05	.05	.06	.07	.08	.09	.10	.11	.12
OUTFLOW	10.63	13.23	16.20	19.54	23.27	27.42	32.89	39.13	45.95	53.38
ELEVATION	4732.81	4732.94	4733.08	4733.21	4733.34	4733.47	4733.60	4733.73	4733.86	4734.00

COMPUTED MUSKINGUM-CUNGE PARAMETERS

ELEMENT	ALPHA	M	COMPUTATION TIME STEP		PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
			DT (MIN)	DX (FT)				
MAIN			4.25	350.00	.86	786.25	.17	2.39

R1-9

MAIN 5.00 .86 785.00 .17

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1681E+00 EXCESS= .0000E+00 OUTFLOW= .1679E+00 BASIN STORAGE= .2035E-03 PERCENT ERROR= .0

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HYDROGRAPH AT STATION DITCH1

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.92-HR	
1.	13.08	0.	0.	0.	0.	
		(INCHES)	.146	.168	.168	.168
		(AC-FT)	0.	0.	0.	0.

CUMULATIVE AREA = .02 SQ MI

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\* \*  
\* OF3 \*  
\* \*  
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45 KK

47 KO

OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	110	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.083	TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

48 BA

SUBBASIN CHARACTERISTICS

TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

49 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.10	.19	.34	.42	.47	.55	.64	.70	.00	.00	.00	.00

STORM AREA = .00

50 LS

SCS LOSS RATE

STRTL	.27	INITIAL ABSTRACTION
CRVNBR	88.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

51 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .80 LAG

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UNIT HYDROGRAPH  
50 END-OF-PERIOD ORDINATES

0.	0.	1.	1.	1.	2.	2.	3.	3.	3.
3.	3.	2.	2.	2.	2.	1.	1.	1.	1.
1.	1.	1.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

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HYDROGRAPH AT STATION OF3

.70. TOTAL LOSS = .60, TOTAL EXCESS = .10

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.92-HR	
0.	13.08	0.	0.	0.	0.	
		(INCHES)	.085	.101	.101	.101

"OF 3" INTO PROJECT SITE

R1-10

CUMULATIVE AREA = .00 SQ MI

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\* \*  
\* OF3 \*  
\* \*  
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52 KK

54 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .083 TIME INTERVAL IN HOURS

55 HC

HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION OF3

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.92-HR	
1.	13.08	0.	0.	0.	0.	
		(INCHES)	.134	.155	.155	.155
		(AC-FT)	0.	0.	0.	0.

Sum of "OF2"  
And "OF3" into PROJECT SITE  
CUMULATIVE AREA = .02 SQ MI

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\* \*  
\* OF3 \*  
\* \*  
\*\*\*\*\*

56 KK

58 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .083 TIME INTERVAL IN HOURS

59 HC

HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION OF3

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.92-HR	
4.	13.08	1.	0.	0.	0.	
		(INCHES)	.170	.196	.196	.196
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = .08 SQ MI

Sum of "OF1, OF2, OF3 and A"

R1-11

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 \* DITCH3 \*  
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62 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .083 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

63 RD MUSKINGUM-CUNGE CHANNEL ROUTING

64 RC NORMAL DEPTH CHANNEL  
 ANL .035 LEFT OVERBANK N-VALUE  
 ANCH .025 MAIN CHANNEL N-VALUE  
 ANR .035 RIGHT OVERBANK N-VALUE  
 RLNTH 1480. REACH LENGTH  
 SEL .0070 ENERGY SLOPE  
 ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVERBANK ---	+ --- MAIN CHANNEL ---	+ --- RIGHT OVERBANK ---
66 RY ELEVATION	4726.00	4723.50	4726.00
65 RX DISTANCE	100.00	110.00	127.00

\*\*\*

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.01	.01	.02	.03	.04	.06	.07	.09	.11
OUTFLOW	.00	.18	.60	1.27	2.21	3.45	5.02	6.93	9.23	11.92
ELEVATION	4723.50	4723.63	4723.76	4723.89	4724.03	4724.16	4724.29	4724.42	4724.55	4724.68
STORAGE	.13	.16	.18	.21	.23	.26	.30	.35	.40	.47
OUTFLOW	15.05	18.62	22.66	27.20	32.26	37.86	45.23	53.87	63.69	74.90
ELEVATION	4724.81	4724.94	4725.08	4725.21	4725.34	4725.47	4725.60	4725.73	4725.86	4726.00

COMPUTED MUSKINGUM-CUNGE PARAMETERS  
 COMPUTATION TIME STEP

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN			5.00	493.33	3.88	790.00	.20	2.89

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN			5.00		3.88	790.00	.20
------	--	--	------	--	------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8523E+00 EXCESS= .0000E+00 OUTFLOW= .8502E+00 BASIN STORAGE= .3213E-02 PERCENT ERROR= -.1

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HYDROGRAPH AT STATION DITCH3

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR (INCHES)	72-HR (AC-FT)	24.92-HR (INCHES)
4.	13.17	1.	.170	1.	.195
		1.	.195	1.	.195

CUMULATIVE AREA = .08 SQ MI

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 \* R \*

67 KK

R1-12

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69 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .083 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

70 BA SUBBASIN CHARACTERISTICS  
 TAREA .04 SUBBASIN AREA

PRECIPITATION DATA

71 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
 HYDRO-35 TP-40 TP-49  
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
 .10 .19 .34 .42 .47 .55 .64 .70 .00 .00 .00 .00

STORM AREA = .04

72 LS SCS LOSS RATE  
 STRTL .13 INITIAL ABSTRACTION  
 CRVNBR 94.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

73 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG 1.08 LAG

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UNIT HYDROGRAPH  
67 END-OF-PERIOD ORDINATES

0.	1.	2.	3.	4.	6.	8.	10.	12.	14.
15.	15.	16.	16.	15.	15.	14.	13.	12.	11.
9.	8.	7.	6.	6.	5.	4.	4.	3.	3.
3.	2.	2.	2.	2.	1.	1.	1.	1.	1.
1.	1.	1.	1.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

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HYDROGRAPH AT STATION B

TOTAL RAINFALL = .70, TOTAL LOSS = .43, TOTAL EXCESS = .27

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR (INCHES)	72-HR (AC-FT)	24.92-HR (INCHES)
2.	13.25	1.	.235	0.	.269

*SUB-BASIN "B"*  
 CUMULATIVE AREA = .04 SQ MI

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74 KK \* \*  
 \* B \*  
 \* \*  
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76 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .083 TIME INTERVAL IN HOURS

77 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

R1-13

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HYDROGRAPH AT STATION B

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.92-HR
6.	13.25	2.	1.	1.	1.
		(CFS)	(CFS)	(CFS)	(CFS)
		(INCHES)	.218	.218	.218
		(AC-FT)	1.	1.	1.

CUMULATIVE AREA = .12 SQ MI

Sum of SUB-BASINS "OF 1, OF 2, OF 3, A AND B"

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 \* \*  
 78 KK \* C \*  
 \* \*  
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80 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	110	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.083	TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

81 BA SUBBASIN CHARACTERISTICS

TAREA	.02	SUBBASIN AREA
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PRECIPITATION DATA

82 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35		TP-40						TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.10	.19	.34	.42	.47	.55	.64	.70	.00	.00	.00	.00

STORM AREA = .02

83 LS SCS LOSS RATE

STRTL	.13	INITIAL ABSTRACTION
CRVNBR	94.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

84 UD SCS DIMENSIONLESS UNITGRAPH

TLAG	1.05	LAG
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UNIT HYDROGRAPH

65 END-OF-PERIOD ORDINATES

0.	1.	1.	2.	3.	4.	6.	7.	9.	10.
10.	11.	11.	11.	10.	10.	9.	9.	8.	7.
6.	5.	5.	4.	4.	3.	3.	3.	2.	2.
2.	2.	1.	1.	1.	1.	1.	1.	1.	1.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

\*\*\* \*\*

HYDROGRAPH AT STATION C

TOTAL RAINFALL = .70, TOTAL LOSS = .43, TOTAL EXCESS = .27

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.92-HR
2.	13.25	1.	0.	0.	0.
		(CFS)	(CFS)	(CFS)	(CFS)
		(INCHES)	.236	.269	.269
		(AC-FT)	0.	0.	0.

CUMULATIVE AREA = .02 SQ MI

SUB-BASIN "C"

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 \* \*  
 85 KK \* C \*  
 \* \*  
 \*\*\*\*\*

87 KO OUTPUT CONTROL VARIABLES  
 IPRNT 1 PRINT CONTROL  
 IPLOT 2 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .083 TIME INTERVAL IN HOURS

88 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION C  
 SUM OF 2 HYDROGRAPHS

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DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*
22	JUN	1200	1	0.	*	22	JUN	1815	76	0.	*	23	JUN	0030	151	2.	*	23	JUN	0645	226	1.	*
22	JUN	1205	2	0.	*	22	JUN	1820	77	0.	*	23	JUN	0035	152	3.	*	23	JUN	0650	227	1.	*
22	JUN	1210	3	0.	*	22	JUN	1825	78	0.	*	23	JUN	0040	153	4.	*	23	JUN	0655	228	1.	*
22	JUN	1215	4	0.	*	22	JUN	1830	79	0.	*	23	JUN	0045	154	5.	*	23	JUN	0700	229	1.	*
22	JUN	1220	5	0.	*	22	JUN	1835	80	0.	*	23	JUN	0050	155	6.	*	23	JUN	0705	230	1.	*
22	JUN	1225	6	0.	*	22	JUN	1840	81	0.	*	23	JUN	0055	156	7.	*	23	JUN	0710	231	1.	*
22	JUN	1230	7	0.	*	22	JUN	1845	82	0.	*	23	JUN	0100	157	7.	*	23	JUN	0715	232	1.	*
22	JUN	1235	8	0.	*	22	JUN	1850	83	0.	*	23	JUN	0105	158	8.	*	23	JUN	0720	233	1.	*
22	JUN	1240	9	0.	*	22	JUN	1855	84	0.	*	23	JUN	0110	159	8.	*	23	JUN	0725	234	1.	*
22	JUN	1245	10	0.	*	22	JUN	1900	85	0.	*	23	JUN	0115	160	8.	*	23	JUN	0730	235	1.	*
22	JUN	1250	11	0.	*	22	JUN	1905	86	0.	*	23	JUN	0120	161	8.	*	23	JUN	0735	236	1.	*
22	JUN	1255	12	0.	*	22	JUN	1910	87	0.	*	23	JUN	0125	162	8.	*	23	JUN	0740	237	0.	*
22	JUN	1300	13	0.	*	22	JUN	1915	88	0.	*	23	JUN	0130	163	7.	*	23	JUN	0745	238	0.	*
22	JUN	1305	14	0.	*	22	JUN	1920	89	0.	*	23	JUN	0135	164	7.	*	23	JUN	0750	239	0.	*
22	JUN	1310	15	0.	*	22	JUN	1925	90	0.	*	23	JUN	0140	165	7.	*	23	JUN	0755	240	0.	*
22	JUN	1315	16	0.	*	22	JUN	1930	91	0.	*	23	JUN	0145	166	6.	*	23	JUN	0800	241	0.	*
22	JUN	1320	17	0.	*	22	JUN	1935	92	0.	*	23	JUN	0150	167	6.	*	23	JUN	0805	242	0.	*
22	JUN	1325	18	0.	*	22	JUN	1940	93	0.	*	23	JUN	0155	168	6.	*	23	JUN	0810	243	0.	*
22	JUN	1330	19	0.	*	22	JUN	1945	94	0.	*	23	JUN	0200	169	5.	*	23	JUN	0815	244	0.	*
22	JUN	1335	20	0.	*	22	JUN	1950	95	0.	*	23	JUN	0205	170	5.	*	23	JUN	0820	245	0.	*
22	JUN	1340	21	0.	*	22	JUN	1955	96	0.	*	23	JUN	0210	171	5.	*	23	JUN	0825	246	0.	*
22	JUN	1345	22	0.	*	22	JUN	2000	97	0.	*	23	JUN	0215	172	4.	*	23	JUN	0830	247	0.	*
22	JUN	1350	23	0.	*	22	JUN	2005	98	0.	*	23	JUN	0220	173	4.	*	23	JUN	0835	248	0.	*
22	JUN	1355	24	0.	*	22	JUN	2010	99	0.	*	23	JUN	0225	174	4.	*	23	JUN	0840	249	0.	*
22	JUN	1400	25	0.	*	22	JUN	2015	100	0.	*	23	JUN	0230	175	4.	*	23	JUN	0845	250	0.	*
22	JUN	1405	26	0.	*	22	JUN	2020	101	0.	*	23	JUN	0235	176	3.	*	23	JUN	0850	251	0.	*
22	JUN	1410	27	0.	*	22	JUN	2025	102	0.	*	23	JUN	0240	177	3.	*	23	JUN	0855	252	0.	*
22	JUN	1415	28	0.	*	22	JUN	2030	103	0.	*	23	JUN	0245	178	3.	*	23	JUN	0900	253	0.	*
22	JUN	1420	29	0.	*	22	JUN	2035	104	0.	*	23	JUN	0250	179	3.	*	23	JUN	0905	254	0.	*
22	JUN	1425	30	0.	*	22	JUN	2040	105	0.	*	23	JUN	0255	180	3.	*	23	JUN	0910	255	0.	*
22	JUN	1430	31	0.	*	22	JUN	2045	106	0.	*	23	JUN	0300	181	3.	*	23	JUN	0915	256	0.	*
22	JUN	1435	32	0.	*	22	JUN	2050	107	0.	*	23	JUN	0305	182	2.	*	23	JUN	0920	257	0.	*
22	JUN	1440	33	0.	*	22	JUN	2055	108	0.	*	23	JUN	0310	183	2.	*	23	JUN	0925	258	0.	*
22	JUN	1445	34	0.	*	22	JUN	2100	109	0.	*	23	JUN	0315	184	2.	*	23	JUN	0930	259	0.	*
22	JUN	1450	35	0.	*	22	JUN	2105	110	0.	*	23	JUN	0320	185	2.	*	23	JUN	0935	260	0.	*
22	JUN	1455	36	0.	*	22	JUN	2110	111	0.	*	23	JUN	0325	186	2.	*	23	JUN	0940	261	0.	*
22	JUN	1500	37	0.	*	22	JUN	2115	112	0.	*	23	JUN	0330	187	2.	*	23	JUN	0945	262	0.	*
22	JUN	1505	38	0.	*	22	JUN	2120	113	0.	*	23	JUN	0335	188	2.	*	23	JUN	0950	263	0.	*
22	JUN	1510	39	0.	*	22	JUN	2125	114	0.	*	23	JUN	0340	189	2.	*	23	JUN	0955	264	0.	*
22	JUN	1515	40	0.	*	22	JUN	2130	115	0.	*	23	JUN	0345	190	2.	*	23	JUN	1000	265	0.	*
22	JUN	1520	41	0.	*	22	JUN	2135	116	0.	*	23	JUN	0350	191	2.	*	23	JUN	1005	266	0.	*
22	JUN	1525	42	0.	*	22	JUN	2140	117	0.	*	23	JUN	0355	192	2.	*	23	JUN	1010	267	0.	*
22	JUN	1530	43	0.	*	22	JUN	2145	118	0.	*	23	JUN	0400	193	2.	*	23	JUN	1015	268	0.	*
22	JUN	1535	44	0.	*	22	JUN	2150	119	0.	*	23	JUN	0405	194	1.	*	23	JUN	1020	269	0.	*
22	JUN	1540	45	0.	*	22	JUN	2155	120	0.	*	23	JUN	0410	195	1.	*	23	JUN	1025	270	0.	*
22	JUN	1545	46	0.	*	22	JUN	2200	121	0.	*	23	JUN	0415	196	1.	*	23	JUN	1030	271	0.	*
22	JUN	1550	47	0.	*	22	JUN	2205	122	0.	*	23	JUN	0420	197	1.	*	23	JUN	1035	272	0.	*
22	JUN	1555	48	0.	*	22	JUN	2210	123	0.	*	23	JUN	0425	198	1.	*	23	JUN	1040	273	0.	*
22	JUN	1600	49	0.	*	22	JUN	2215	124	0.	*	23	JUN	0430	199	1.	*	23	JUN	1045	274	0.	*
22	JUN	1605	50	0.	*	22	JUN	2220	125	0.	*	23	JUN	0435	200	1.	*	23	JUN	1050	275	0.	*
22	JUN	1610	51	0.	*	22	JUN	2225	126	0.	*	23	JUN	0440	201	1.	*	23	JUN	1055	276	0.	*
22	JUN	1615	52	0.	*	22	JUN	2230	127	0.	*	23	JUN	0445	202	1.	*	23	JUN	1100	277	0.	*
22	JUN	1620	53	0.	*	22	JUN	2235	128	0.	*	23	JUN	0450	203	1.	*	23	JUN	1105	278	0.	*
22	JUN	1625	54	0.	*	22	JUN	2240	129	0.	*	23	JUN	0455	204	1.	*	23	JUN	1110	279	0.	*

RHIS

22 JUN 1635	56	0.	*	22 JUN 2250	131	0.	*	23 JUN 0505	206	1.	*	23 JUN 1120	281	0.
22 JUN 1640	57	0.	*	22 JUN 2255	132	0.	*	23 JUN 0510	207	1.	*	23 JUN 1125	282	0.
22 JUN 1645	58	0.	*	22 JUN 2300	133	0.	*	23 JUN 0515	208	1.	*	23 JUN 1130	283	0.
22 JUN 1650	59	0.	*	22 JUN 2305	134	0.	*	23 JUN 0520	209	1.	*	23 JUN 1135	284	0.
22 JUN 1655	60	0.	*	22 JUN 2310	135	0.	*	23 JUN 0525	210	1.	*	23 JUN 1140	285	0.
22 JUN 1700	61	0.	*	22 JUN 2315	136	0.	*	23 JUN 0530	211	1.	*	23 JUN 1145	286	0.
22 JUN 1705	62	0.	*	22 JUN 2320	137	0.	*	23 JUN 0535	212	1.	*	23 JUN 1150	287	0.
22 JUN 1710	63	0.	*	22 JUN 2325	138	0.	*	23 JUN 0540	213	1.	*	23 JUN 1155	288	0.
22 JUN 1715	64	0.	*	22 JUN 2330	139	0.	*	23 JUN 0545	214	1.	*	23 JUN 1200	289	0.
22 JUN 1720	65	0.	*	22 JUN 2335	140	0.	*	23 JUN 0550	215	1.	*	23 JUN 1205	290	0.
22 JUN 1725	66	0.	*	22 JUN 2340	141	0.	*	23 JUN 0555	216	1.	*	23 JUN 1210	291	0.
22 JUN 1730	67	0.	*	22 JUN 2345	142	0.	*	23 JUN 0600	217	1.	*	23 JUN 1215	292	0.
22 JUN 1735	68	0.	*	22 JUN 2350	143	0.	*	23 JUN 0605	218	1.	*	23 JUN 1220	293	0.
22 JUN 1740	69	0.	*	22 JUN 2355	144	0.	*	23 JUN 0610	219	1.	*	23 JUN 1225	294	0.
22 JUN 1745	70	0.	*	23 JUN 0000	145	0.	*	23 JUN 0615	220	1.	*	23 JUN 1230	295	0.
22 JUN 1750	71	0.	*	23 JUN 0005	146	0.	*	23 JUN 0620	221	1.	*	23 JUN 1235	296	0.
22 JUN 1755	72	0.	*	23 JUN 0010	147	0.	*	23 JUN 0625	222	1.	*	23 JUN 1240	297	0.
22 JUN 1800	73	0.	*	23 JUN 0015	148	1.	*	23 JUN 0630	223	1.	*	23 JUN 1245	298	0.
22 JUN 1805	74	0.	*	23 JUN 0020	149	1.	*	23 JUN 0635	224	1.	*	23 JUN 1250	299	0.
22 JUN 1810	75	0.	*	23 JUN 0025	150	1.	*	23 JUN 0640	225	1.	*	23 JUN 1255	300	0.

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PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.92-HR
8.	13.25	(CFS) 3.	1.	1.	1.
		(INCHES) .197	.227	.227	.227
		(AC-FT) 1.	2.	2.	2.

CUMULATIVE AREA = .14 SQ MI



SUM OF ALL SUB-BASINS  
 "OF 1, OF 2, OF 3, A, B AND C"  
 FLOW DISCHARGED AT SOUTHWEST CORNER OF PROJECT  
 INTO DRAINAGE AND IRRIGATION DITCH ALONG ZB ROAD.

STATION C

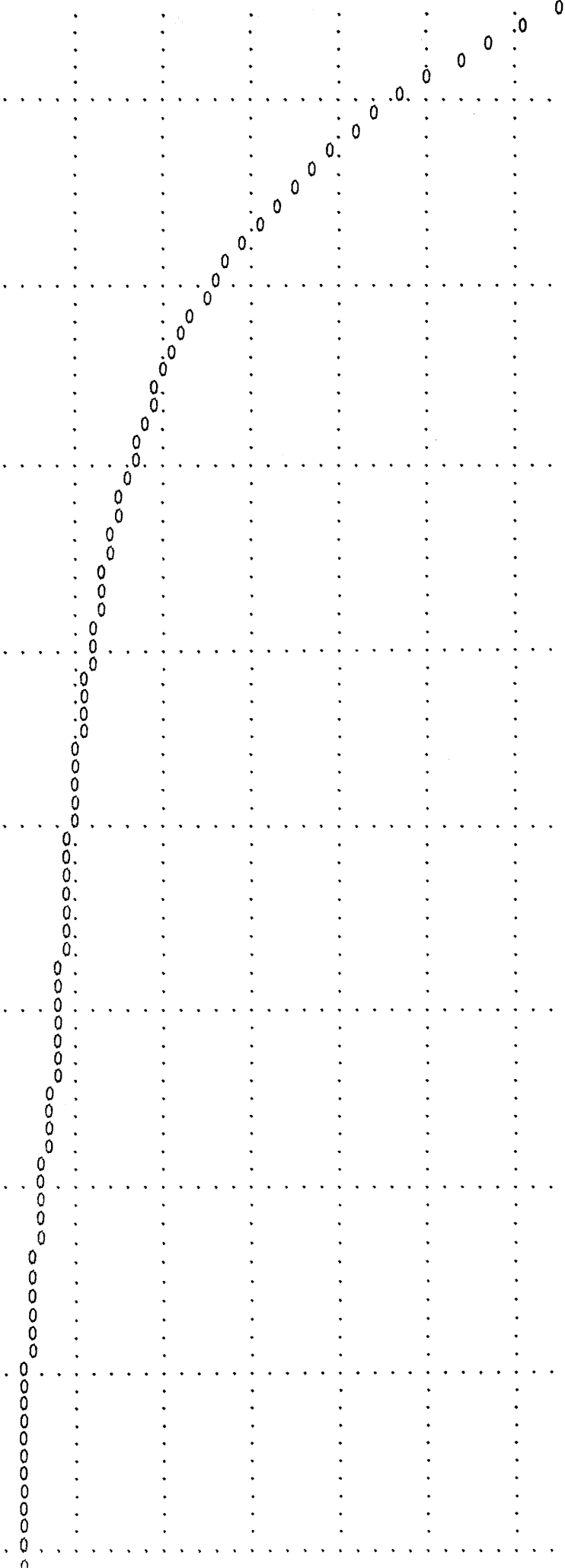
(0) OUTFLOW

DAHRMN PER	0.	1.	2.	3.	4.	5.	6.	7.	8.	0.	0.	0.	0.
221200	10	.	.	.	.	.	.	.	.	.	.	.	.
221205	20	.	.	.	.	.	.	.	.	.	.	.	.
221210	30	.	.	.	.	.	.	.	.	.	.	.	.
221215	40	.	.	.	.	.	.	.	.	.	.	.	.
221220	50	.	.	.	.	.	.	.	.	.	.	.	.
221225	60	.	.	.	.	.	.	.	.	.	.	.	.
221230	70	.	.	.	.	.	.	.	.	.	.	.	.
221235	80	.	.	.	.	.	.	.	.	.	.	.	.
221240	90	.	.	.	.	.	.	.	.	.	.	.	.
221245	100	.	.	.	.	.	.	.	.	.	.	.	.
221250	110	.	.	.	.	.	.	.	.	.	.	.	.
221255	120	.	.	.	.	.	.	.	.	.	.	.	.
221300	130	.	.	.	.	.	.	.	.	.	.	.	.
221305	140	.	.	.	.	.	.	.	.	.	.	.	.
221310	150	.	.	.	.	.	.	.	.	.	.	.	.
221315	160	.	.	.	.	.	.	.	.	.	.	.	.
221320	170	.	.	.	.	.	.	.	.	.	.	.	.
221325	180	.	.	.	.	.	.	.	.	.	.	.	.
221330	190	.	.	.	.	.	.	.	.	.	.	.	.
221335	200	.	.	.	.	.	.	.	.	.	.	.	.
221340	210	.	.	.	.	.	.	.	.	.	.	.	.
221345	220	.	.	.	.	.	.	.	.	.	.	.	.
221350	230	.	.	.	.	.	.	.	.	.	.	.	.
221355	240	.	.	.	.	.	.	.	.	.	.	.	.
221400	250	.	.	.	.	.	.	.	.	.	.	.	.
221405	260	.	.	.	.	.	.	.	.	.	.	.	.
221410	270	.	.	.	.	.	.	.	.	.	.	.	.
221415	280	.	.	.	.	.	.	.	.	.	.	.	.
221420	290	.	.	.	.	.	.	.	.	.	.	.	.
221425	300	.	.	.	.	.	.	.	.	.	.	.	.
221430	310	.	.	.	.	.	.	.	.	.	.	.	.
221435	320	.	.	.	.	.	.	.	.	.	.	.	.
221440	330	.	.	.	.	.	.	.	.	.	.	.	.
221445	340	.	.	.	.	.	.	.	.	.	.	.	.
221450	350	.	.	.	.	.	.	.	.	.	.	.	.
221455	360	.	.	.	.	.	.	.	.	.	.	.	.
221500	370	.	.	.	.	.	.	.	.	.	.	.	.
221505	380	.	.	.	.	.	.	.	.	.	.	.	.
221510	390	.	.	.	.	.	.	.	.	.	.	.	.
221515	400	.	.	.	.	.	.	.	.	.	.	.	.
221520	410	.	.	.	.	.	.	.	.	.	.	.	.
221525	420	.	.	.	.	.	.	.	.	.	.	.	.
221530	430	.	.	.	.	.	.	.	.	.	.	.	.
221535	440	.	.	.	.	.	.	.	.	.	.	.	.
221540	450	.	.	.	.	.	.	.	.	.	.	.	.
221545	460	.	.	.	.	.	.	.	.	.	.	.	.
221550	470	.	.	.	.	.	.	.	.	.	.	.	.
221555	480	.	.	.	.	.	.	.	.	.	.	.	.
221600	490	.	.	.	.	.	.	.	.	.	.	.	.
221605	500	.	.	.	.	.	.	.	.	.	.	.	.
221610	510	.	.	.	.	.	.	.	.	.	.	.	.
221615	520	.	.	.	.	.	.	.	.	.	.	.	.
221620	530	.	.	.	.	.	.	.	.	.	.	.	.
221625	540	.	.	.	.	.	.	.	.	.	.	.	.
221630	550	.	.	.	.	.	.	.	.	.	.	.	.
221635	560	.	.	.	.	.	.	.	.	.	.	.	.
221640	570	.	.	.	.	.	.	.	.	.	.	.	.
221645	580	.	.	.	.	.	.	.	.	.	.	.	.
221650	590	.	.	.	.	.	.	.	.	.	.	.	.
221655	600	.	.	.	.	.	.	.	.	.	.	.	.
221700	610	.	.	.	.	.	.	.	.	.	.	.	.
221705	620	.	.	.	.	.	.	.	.	.	.	.	.
221710	630	.	.	.	.	.	.	.	.	.	.	.	.
221715	640	.	.	.	.	.	.	.	.	.	.	.	.
221720	650	.	.	.	.	.	.	.	.	.	.	.	.
221725	660	.	.	.	.	.	.	.	.	.	.	.	.
221730	670	.	.	.	.	.	.	.	.	.	.	.	.
221735	680	.	.	.	.	.	.	.	.	.	.	.	.
221740	690	.	.	.	.	.	.	.	.	.	.	.	.
221745	700	.	.	.	.	.	.	.	.	.	.	.	.
221750	710	.	.	.	.	.	.	.	.	.	.	.	.
221755	720	.	.	.	.	.	.	.	.	.	.	.	.
221800	730	.	.	.	.	.	.	.	.	.	.	.	.
221805	740	.	.	.	.	.	.	.	.	.	.	.	.
221810	750	.	.	.	.	.	.	.	.	.	.	.	.
221815	760	.	.	.	.	.	.	.	.	.	.	.	.

RI-17

221825 780  
221830 790  
221835 800  
221840 810  
221845 820  
221850 830  
221855 840  
221900 850  
221905 860  
221910 870  
221915 880  
221920 890  
221925 900  
221930 910  
221935 920  
221940 930  
221945 940  
221950 950  
221955 960  
222000 970  
222005 980  
222010 990  
222015 1000  
222020 1010  
222025 1020  
222030 1030  
222035 1040  
222040 1050  
222045 1060  
222050 1070  
222055 1080  
222100 1090  
222105 1100  
222110 1110  
222115 1120  
222120 1130  
222125 1140  
222130 1150  
222135 1160  
222140 1170  
222145 1180  
222150 1190  
222155 1200  
222200 1210  
222205 1220  
222210 1230  
222215 1240  
222220 1250  
222225 1260  
222230 1270  
222235 1280  
222240 1290  
222245 1300  
222250 1310  
222255 1320  
222300 1330  
222305 1340  
222310 1350  
222315 1360  
222320 1370  
222325 1380  
222330 1390  
222335 1400  
222340 1410  
222345 142.0  
222350 143.0  
222355 144.0  
230000 145.0  
230005 146.0  
230010 147.0  
230015 148.0  
230020 149.0  
230025 150.0  
230030 151.0  
230035 152.0  
230040 153.0  
230045 154.0  
230050 155.0  
230055 156.0  
230100 157.0  
230105 158.0  
230110 159.0  
230115 160.0  
230120 161.0  
230125 162.0  
230130 163.0  
230135 164.0

230145 166.  
230150 167.  
230155 168.  
230200 169.  
230205 170.  
230210 171.  
230215 172.  
230220 173.  
230225 174.  
230230 175.  
230235 176.  
230240 177.  
230245 178.  
230250 179.  
230255 180.  
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230305 182.  
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230325 186.  
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230400 193.  
230405 194.  
230410 195.  
230415 196.  
230420 197.  
230425 198.  
230430 199.  
230435 200.  
230440 201.  
230445 202.  
230450 203.  
230455 204.  
230500 205.  
230505 206.  
230510 207.  
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230520 209.  
230525 210.  
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230535 212.  
230540 213.  
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230600 217.  
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230610 219.  
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230620 221.  
230625 222.  
230630 223.  
230635 224.  
230640 225.  
230645 226.  
230650 227.  
230655 228.  
230700 229.  
230705 230.  
230710 231.  
230715 232.  
230720 233.  
230725 234.  
230730 235.  
230735 236.  
230740 237.  
230745 238.  
230750 239.  
230755 240.  
230800 241.  
230805 242.  
230810 243.  
230815 244.  
230820 245.  
230825 246.  
230830 247.  
230835 248.  
230840 249.  
230845 250.  
230850 251.  
230855 252.



R1-19



230905 254. 0  
230910 255. 0  
230915 256. 0  
230920 257. 0  
230925 258. 0  
230930 259. 0  
230935 260. 0  
230940 261. 0  
230945 262. 0  
230950 263. 0  
230955 264. 0  
231000 265. 0  
231005 266. 0  
231010 267. 0  
231015 268. 0  
231020 269. 0  
231025 270. 0  
231030 271. 0  
231035 272. 0  
231040 273. 0  
231045 274. 0  
231050 275. 0  
231055 276. 0  
231100 277. 0  
231105 278. 0  
231110 279. 0  
231115 280. 0  
231120 281. 0  
231125 282. 0  
231130 283. 0  
231135 284. 0  
231140 285. 0  
231145 286. 0  
231150 287. 0  
231155 288. 0  
231200 289. 0  
231205 290. 0  
231210 291. 0  
231215 292. 0  
231220 293. 0  
231225 294. 0  
231230 295. 0  
231235 296. 0  
231240 297. 0  
231245 298. 0  
231250 299. 0  
231255 300. -0

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	OF1	1.	12.58	0.	0.	0.	.02		
ROUTED TO	DITCH2	1.	12.75	0.	0.	0.	.02		
HYDROGRAPH AT	A	2.	13.33	1.	0.	0.	.04		
2 COMBINED AT	A	3.	13.08	1.	0.	0.	.06		
HYDROGRAPH AT	OF2	1.	13.00	0.	0.	0.	.02		
ROUTED TO	DITCH1	1.	13.08	0.	0.	0.	.02		
HYDROGRAPH AT	OF3	0.	13.08	0.	0.	0.	.00		
2 COMBINED AT	OF3	1.	13.08	0.	0.	0.	.02		
2 COMBINED AT	OF3	4.	13.08	1.	0.	0.	.08		
ROUTED TO	DITCH3	4.	13.17	1.	0.	0.	.08		
HYDROGRAPH AT	B	2.	13.25	1.	0.	0.	.04		
2 COMBINED AT	B	6.	13.25	2.	1.	1.	.12		
HYDROGRAPH AT	C	2.	13.25	1.	0.	0.	.02		
2 COMBINED AT	C	8.	13.25	3.	1.	1.	.14		

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
 (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO		VOLUME
							COMPUTATION PEAK	INTERVAL TIME TO PEAK	
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
DITCH2	MANE	3.00	1.10	765.00	.17	5.00	1.10	765.00	.17

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1577E+00 EXCESS= .0000E+00 OUTFLOW= .1577E+00 BASIN STORAGE= .5110E-03 PERCENT ERROR= -.3

DITCH1	MANE	4.25	.86	786.25	.17	5.00	.86	785.00	.17
--------	------	------	-----	--------	-----	------	-----	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1681E+00 EXCESS= .0000E+00 OUTFLOW= .1679E+00 BASIN STORAGE= .2035E-03 PERCENT ERROR= .0

DITCH3	MANE	5.00	3.88	790.00	.20	5.00	3.88	790.00	.20
--------	------	------	------	--------	-----	------	------	--------	-----

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8523E+00 EXCESS= .0000E+00 OUTFLOW= .8502E+00 BASIN STORAGE= .3213E-02 PERCENT ERROR= -.1

\*\*\* NORMAL END OF HEC-1 \*\*\*

R1-22

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
* RUN DATE 06/27/1994 TIME 16:01:49 *
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

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X   X  XXXXXXX  XXXXX   X
X   X  X       X   X   XX
X   X  X       X       X
XXXXXXX XXXX   X       XXXXX X
X   X  X       X       X
X   X  X       X   X   X
X   X  XXXXXXX  XXXXX   XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY.  
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HISTORIC 100 YEAR STORM  
 RUN#2

R2-1

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1         ID  GRAND VIEW SUBDIVISION
2         ID  HISTORIC CONDITION
3         ID  100 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)
4         IT   5 22JUN94   1200   300
5         IO   5       2       0
          * *****

6         KK   OF1
7         KM   Basin runoff calculation for   OF1
8         KO   3       1       0       1       21       110       300
9         BA   0.0175
10        PH           0       0.39   0.76   1.34   1.40   1.44   1.56   1.69   2.01
11        LS           91
12        UD   0.304
          * *****

13        KK   DITCH2
14        KM   Muskingum-Cunge channel routing from   CP1 to   CP3
15        KO   3       1       0       1       21       110       300
16        RD
17        RC   0.035   0.025   0.035   1770   0.010
18        RX   100     110     111     113     113.5   115.5   116.5   126.5
19        RY   4742   4742   4741.5   4739.5   4739.5   4741.5   4742   4742
          * *****

20        KK   A
21        KM   Basin runoff calculation for   A
22        KO   3       1       0       1       21       110       300
23        BA   0.0407
24        PH           0       0.39   0.76   1.34   1.40   1.44   1.56   1.69   2.01
25        LS           93
26        UD   0.673
          * *****

27        KK   A
28        KM   Combining two hydrographs at control point   CP3
29        KO   3       1       0       1       21       110       300
30        HC   2
          * *****

31        KK   OF2
32        KM   Basin runoff calculation for   OF2
33        KO   3       1       0       1       21       110       300
34        BA   0.0187
35        PH           0       0.39   0.76   1.34   1.40   1.44   1.56   1.69   2.01
36        LS           91
37        UD   0.478
          * *****

38        KK   DITCH1
39        KM   Muskingum-Cunge channel routing from   CP2 to   CP3
40        KO   3       1       0       1       21       110       300
41        RD
42        RC   0.035   0.025   0.035   700   0.012
43        RX   100     110     111     113     113.5   115.5   116.5   126.5
    
```

RZ-2

HEC-1 INPUT

LINE	ID.....	1.....	2.....	3.....	4.....	5.....	6.....	7.....	8.....	9.....	10
44	RY	4734	4734	4733.5	4731.5	4731.5	4733.5	4734	4734		
	* *****										
45	KK	OF3									
46	KM	Basin runoff calculation for			OF3						
47	KO	3	1	0	1	21	110	300			
48	BA	0.0048									
49	PH		0	0.39	0.76	1.34	1.40	1.44	1.56	1.69	2.01
50	LS		88								
51	UD	0.478									
	* *****										
52	KK	OF3									
53	KM	Combining two hydrographs at control point					CP3				
54	KO	3	1	0	1	21	110	300			
55	HC	2									
	* *****										
56	KK	OF3									
57	KM	Combining two hydrographs at control point					CP3				
58	KO	3	1	0	1	21	110	300			
59	HC	2									
	* *****										
60	KK	DITCH3									
61	KM	Muskingum-Cunge channel routing from					CP3 to	CP4			
62	KO	3	1	0	1	21	110	300			
63	RD										
64	RC	0.035	0.025	0.035	1480	0.007					
65	RX	100	105	110	113	114	117	122	127		
66	RY	4726	4726	4725.5	4723.5	4723.5	4725.5	4726	4726		
	* *****										
67	KK	B									
68	KM	Basin runoff calculation for			B						
69	KO	3	1	0	1	21	110	300			
70	BA	0.0363									
71	PH		0	0.39	0.76	1.34	1.40	1.44	1.56	1.69	2.01
72	LS		94								
73	UD	0.682									
	* *****										
74	KK	B									
75	KM	Combining two hydrographs at control point					CP4				
76	KO	3	1	0	1	21	110	300			
77	HC	2									
	* *****										
78	KK	C									
79	KM	Basin runoff calculation for			C						
80	KO	3	1	0	1	21	110	300			
81	BA	0.0245									
82	PH		0	0.39	0.76	1.34	1.40	1.44	1.56	1.69	2.01
83	LS		94								
84	UD	0.696									
	* *****										

RZ-3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

85 KK C  
86 KM Combining two hydrographs at control point CP4  
87 KO 1 2 0 1 21 110 300  
88 HC 2  
\* \*\*\*\*\*  
89 ZZ

R2-4

\*\*\*\*\*  
 \* FLOOD HYDROGRAPH PACKAGE (HEC-1) \*  
 \* SEPTEMBER 1990 \*  
 \* VERSION 4.0 \*  
 \* RUN DATE 06/27/1994 TIME 16:01:49 \*  
 \*\*\*\*\*

\*\*\*\*\*  
 \* U.S. ARMY CORPS OF ENGINEERS \*  
 \* HYDROLOGIC ENGINEERING CENTER \*  
 \* 609 SECOND STREET \*  
 \* DAVIS, CALIFORNIA 95616 \*  
 \* (916) 756-1104 \*  
 \*\*\*\*\*

GRAND VIEW SUBDIVISION  
 HISTORIC CONDITION  
 100 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)

5 IO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 2 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA  
 NMIN 5 MINUTES IN COMPUTATION INTERVAL  
 IDATE 22JUN94 STARTING DATE  
 ITIME 1200 STARTING TIME  
 NO 300 NUMBER OF HYDROGRAPH ORDINATES  
 NDDATE 23JUN94 ENDING DATE  
 NDTIME 1255 ENDING TIME  
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .08 HOURS  
 TOTAL TIME BASE 24.92 HOURS

ENGLISH UNITS  
 DRAINAGE AREA SQUARE MILES  
 PRECIPITATION DEPTH INCHES  
 LENGTH, ELEVATION FEET  
 FLOW CUBIC FEET PER SECOND  
 STORAGE VOLUME ACRE-FEET  
 SURFACE AREA ACRES  
 TEMPERATURE DEGREES FAHRENHEIT

\*\*\* \*\*

\*\*\*\*\*  
 \* OF1 \*  
 \*\*\*\*\*

8 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .083 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

9 BA SUBBASIN CHARACTERISTICS  
 TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.39	.76	1.34	1.40	1.44	1.56	1.69	2.01	.00	.00	.00	.00

STORM AREA = .02

11 LS SCS LOSS RATE

R2-S



CRVNBK 91.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

12 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .30 LAG

\*\*\*

UNIT HYDROGRAPH  
 20 END-OF-PERIOD ORDINATES

3. 11. 21. 24. 23. 18. 12. 8. 5. 4.  
 2. 2. 1. 1. 1. 0. 0. 0. 0. 0.

\*\*\* \*\*\* \*\*\* \*\*\* \*\*\*

HYDROGRAPH AT STATION OF1

TOTAL RAINFALL = 2.01, TOTAL LOSS = .84, TOTAL EXCESS = 1.17

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR (INCHES)	72-HR (INCHES)	24.92-HR (INCHES)
15.	12.42	2.	1.009	1.172	1.172
		(AC-FT)	1.	1.	1.

CUMULATIVE AREA = .02 SQ MI

*JOB-BASIN*  
*"OF1 INTO PROJECT"*

\*\*\* \*\*

\*\*\*\*\*  
 \* \*  
 \* DITCH2 \*  
 \* \*  
 \*\*\*\*\*

13 KK

15 KO

OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	110	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.083	TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

16 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

17 RC

NORMAL DEPTH CHANNEL

ANL	.035	LEFT OVERBANK N-VALUE
ANCH	.025	MAIN CHANNEL N-VALUE
ANR	.035	RIGHT OVERBANK N-VALUE
RLNTH	1770.	REACH LENGTH
SEL	.0100	ENERGY SLOPE
ELMAX	.0	MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVERBANK ---	+	----- MAIN CHANNEL -----	+	--- RIGHT OVERBANK ---
19 RY ELEVATION	4742.00		4741.50		4742.00
18 RX DISTANCE	100.00		111.00		126.50

\*\*\*

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.00	.01	.01	.02	.03	.04	.05	.07	.08
OUTFLOW	.00	.10	.35	.76	1.35	2.13	3.13	4.37	5.87	7.64
ELEVATION	4739.50	4739.63	4739.76	4739.89	4740.03	4740.16	4740.29	4740.42	4740.55	4740.68
STORAGE	.10	.11	.13	.15	.17	.20	.22	.25	.28	.31
OUTFLOW	9.70	12.08	14.78	17.83	21.24	25.03	30.02	35.72	41.95	48.73
ELEVATION	4740.81	4740.94	4741.08	4741.21	4741.34	4741.47	4741.60	4741.73	4741.86	4742.00

COMPUTED MUSKINGUM-CUNGE PARAMETERS

ELEMENT	ALPHA	M	COMPUTATION TIME STEP		PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
			DT (MIN)	DX (FT)				
MAIN			5.00	885.00	14.95	745.00	1.17	4.91

*R2-6*

MAIN 5.00 14.95 745.00 1.17

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1094E+01 EXCESS= .0000E+00 OUTFLOW= .1094E+01 BASIN STORAGE= .6151E-03 PERCENT ERROR= -.1

\*\*\* \*\*

HYDROGRAPH AT STATION DITCH2

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR (INCHES)	72-HR (INCHES)	24.92-HR (AC-FT)
15.	12.42	2.	1.009	1.173	1.173
		1.	1.	1.	1.

CUMULATIVE AREA = .02 SQ MI

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
\* A \*  
\* \*  
\*\*\*\*\*

20 KK

22 KO

OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	110	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.083	TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

23 BA

SUBBASIN CHARACTERISTICS

TAREA .04 SUBBASIN AREA

PRECIPITATION DATA

24 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.39	.76	1.34	1.40	1.44	1.56	1.69	2.01	.00	.00	.00	.00

STORM AREA = .04

25 LS

SCS LOSS RATE

STRTL	.15	INITIAL ABSTRACTION
CRVNBR	93.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

26 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .67 LAG

\*\*\*

UNIT HYDROGRAPH  
42 END-OF-PERIOD ORDINATES

1.	4.	7.	11.	17.	23.	26.	27.	27.	26.
24.	21.	18.	15.	12.	10.	8.	7.	6.	5.
4.	3.	3.	2.	2.	1.	1.	1.	1.	1.
1.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.								

\*\*\* \*\*

HYDROGRAPH AT STATION A

TOTAL RAINFALL = 2.01, TOTAL LOSS = .69, TOTAL EXCESS = 1.32

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR (INCHES)	72-HR (INCHES)	24.92-HR (AC-FT)
24.	12.75	5.	1.133	1.320	1.320
		1.	1.	1.	1.

R2-7

CUMULATIVE AREA = .04 SQ MI

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
\* A \*  
\* \*  
\*\*\*\*\*

27 KK

29 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .083 TIME INTERVAL IN HOURS

30 HC

HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

\*\*\*

\*\*\* \*\*

HYDROGRAPH AT STATION A

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR (INCHES)	72-HR (AC-FT)	24.92-HR (INCHES)
36.	12.58	7.	1.276	4.	2.
<i>Sum of</i>		1.096	1.276	4.	1.276

*Sum of "OF1" and "A"*

CUMULATIVE AREA = .06 SQ MI

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
\* OF2 \*  
\* \*  
\*\*\*\*\*

31 KK

33 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .083 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

34 BA

SUBBASIN CHARACTERISTICS

TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

35 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.39	.76	1.34	1.40	1.44	1.56	1.69	2.01	.00	.00	.00	.00

STORM AREA = .02

36 LS

SCS LOSS RATE

STRTL .20 INITIAL ABSTRACTION  
CRVNR 91.00 CURVE NUMBER  
RTIMP .00 PERCENT IMPERVIOUS AREA

37 UD

SCS DIMENSIONLESS UNITGRAPH

TI AG 48 I AG

R2-8

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UNIT HYDROGRAPH  
31 END-OF-PERIOD ORDINATES

1.	4.	8.	13.	16.	17.	17.	15.	13.	10.
7.	6.	4.	3.	3.	2.	2.	1.	1.	1.
1.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.									

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HYDROGRAPH AT STATION      OF2

TOTAL RAINFALL = 2.01. TOTAL LOSS = .84. TOTAL EXCESS = 1.17

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	24.92-HR	
12.	12.58	2.	1.	1.	1.	
		(INCHES)	1.007	1.171	1.171	1.171
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = .02 SQ MI

↑  
"OF2" INTO  
PROJECT

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\*                      \*  
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38 KK

OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	110	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.083	TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

41 RD      MUSKINGUM-CUNGE CHANNEL ROUTING

42 RC      NORMAL DEPTH CHANNEL

ANL	.035	LEFT OVERBANK N-VALUE
ANCH	.025	MAIN CHANNEL N-VALUE
ANR	.035	RIGHT OVERBANK N-VALUE
RLNTH	700.	REACH LENGTH
SEL	.0120	ENERGY SLOPE
ELMAX	.0	MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

		--- LEFT OVERBANK ---		+ ----- MAIN CHANNEL -----			+ --- RIGHT OVERBANK ---		
44 RY	ELEVATION	4734.00	4734.00	4733.50	4731.50	4731.50	4733.50	4734.00	4734.00
43 RX	DISTANCE	100.00	110.00	111.00	113.00	113.50	115.50	116.50	126.50

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.00	.00	.01	.01	.01	.02	.02	.03	.03
OUTFLOW	.00	.11	.39	.83	1.47	2.33	3.43	4.79	6.43	8.37
ELEVATION	4731.50	4731.63	4731.76	4731.89	4732.03	4732.16	4732.29	4732.42	4732.55	4732.68
STORAGE	.04	.05	.05	.06	.07	.08	.09	.10	.11	.12
OUTFLOW	10.63	13.23	16.20	19.54	23.27	27.42	32.89	39.13	45.95	53.38
ELEVATION	4732.81	4732.94	4733.08	4733.21	4733.34	4733.47	4733.60	4733.73	4733.86	4734.00

COMPUTED MUSKINGUM-CUNGE PARAMETERS

		COMPUTATION TIME STEP						
ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)
MAIN			2.33	350.00	12.36	755.35	1.17	5.00

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

R2-9

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HYDROGRAPH AT STATION DITCH1

PEAK FLOW (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
12.	12.58	(CFS)	6-HR	24-HR	72-HR	24.92-HR
		(INCHES)	2.	1.	1.	1.
		(AC-FT)	1.007	1.171	1.171	1.171
			1.	1.	1.	1.

CUMULATIVE AREA = .02 SQ MI

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\* \*  
\* OF3 \*  
\* \*  
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45 KK

47 KO

OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLST	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	110	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.083	TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

48 BA

SUBBASIN CHARACTERISTICS

TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

49 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.39	.76	1.34	1.40	1.44	1.56	1.69	2.01	.00	.00	.00	.00

STORM AREA = .00

50 LS

SCS LOSS RATE

STRTL	.27	INITIAL ABSTRACTION
CRVNB	88.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

51 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .48 LAG

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UNIT HYDROGRAPH  
31 END-OF-PERIOD ORDINATES

0.	1.	2.	3.	4.	4.	4.	4.	3.	2.
2.	1.	1.	1.	1.	1.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.									

\*\*\* \*\*

HYDROGRAPH AT STATION OF3

TOTAL RAINFALL = 2.01. TOTAL LOSS = 1.04. TOTAL EXCESS = .97

PEAK FLOW (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
3.	12.58	(CFS)	6-HR	24-HR	72-HR	24.92-HR
		(INCHES)	0.	0.	0.	0.
		(AC-FT)	.832	.972	.972	.972
			0.	0.	0.	0.

CUMULATIVE AREA = .00 SQ MI

"OF3" INTO PROJECT SITE

R2-10

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\* \*  
\* OF3 \*  
\* \*  
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54 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .083 TIME INTERVAL IN HOURS

55 HC HYDROGRAPH COMBINATION  
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION OF3

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.92-HR
15.	12.58	(CFS) 2.	1.	1.	1.
		(INCHES) .971	1.130	1.130	1.130
		(AC-FT) 1.	1.	1.	1.

Sum of "OF2"  
AND "OF3" INTO PROJECT SITE  
CUMULATIVE AREA = .02 SQ MI

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\* \*  
\* OF3 \*  
\* \*  
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58 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .083 TIME INTERVAL IN HOURS

59 HC HYDROGRAPH COMBINATION  
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION OF3

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.92-HR
51.	12.58	(CFS) 9.	3.	3.	3.
		(INCHES) 1.060	1.234	1.234	1.234
		(AC-FT) 5.	5.	5.	5.

Sum of "OF1, OF2, OF3 AND A"  
CUMULATIVE AREA = .08 SQ MI

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\* \*  
\* DITCH3 \*  
\* \*  
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62 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLST	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	110	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.083	TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

63 RD MUSKINGUM-CUNGE CHANNEL ROUTING

64 RC NORMAL DEPTH CHANNEL

ANL	.035	LEFT OVERBANK N-VALUE
ANCH	.025	MAIN CHANNEL N-VALUE
ANR	.035	RIGHT OVERBANK N-VALUE
RLNTH	1480.	REACH LENGTH
SEL	.0070	ENERGY SLOPE
ELMAX	.0	MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	---	LEFT OVERBANK	---	+	-----	MAIN CHANNEL	-----	+	---	RIGHT OVERBANK	---
66 RY	ELEVATION	4726.00	4726.00	4725.50	4723.50	4723.50	4725.50	4726.00	4726.00		
65 RX	DISTANCE	100.00	105.00	110.00	113.00	114.00	117.00	122.00	127.00		

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.01	.01	.02	.03	.04	.06	.07	.09	.11
OUTFLOW	.00	.18	.60	1.27	2.21	3.45	5.02	6.93	9.23	11.92
ELEVATION	4723.50	4723.63	4723.76	4723.89	4724.03	4724.16	4724.29	4724.42	4724.55	4724.68
STORAGE	.13	.16	.18	.21	.23	.26	.30	.35	.40	.47
OUTFLOW	15.05	18.62	22.66	27.20	32.26	37.86	45.23	53.87	63.69	74.90
ELEVATION	4724.81	4724.94	4725.08	4725.21	4725.34	4725.47	4725.60	4725.73	4725.86	4726.00

COMPUTED MUSKINGUM-CUNGE PARAMETERS

		COMPUTATION TIME STEP							
ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY	
			(MIN)	(FT)	(CFS)	(MIN)	(IN)	(FPS)	
MAIN			4.31	740.00	50.34	759.08	1.23	5.72	

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN			5.00		50.18	760.00	1.23	
------	--	--	------	--	-------	--------	------	--

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5376E+01 EXCESS= .0000E+00 OUTFLOW= .5371E+01 BASIN STORAGE= .9706E-02 PERCENT ERROR= -.1

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HYDROGRAPH AT STATION DITCH3

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR (INCHES)	72-HR (INCHES)	24.92-HR (INCHES)
50.	12.67	9.	1.061	1.233	1.233
		5.	5.	5.	5.

CUMULATIVE AREA = .08 SQ MI

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\* B \*  
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67 KK

69 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
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R2-12

OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .083 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

70 BA SUBBASIN CHARACTERISTICS  
 TAREA .04 SUBBASIN AREA

PRECIPITATION DATA

71 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
 HYDRO-35 TP-40 TP-49  
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
 .39 .76 1.34 1.40 1.44 1.56 1.69 2.01 .00 .00 .00 .00  
 STORM AREA = .04

72 LS SCS LOSS RATE  
 STRL .13 INITIAL ABSTRACTION  
 CRVNB 94.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

73 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .68 LAG

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UNIT HYDROGRAPH  
43 END-OF-PERIOD ORDINATES

1.	3.	6.	10.	15.	20.	23.	24.	24.	23.
21.	19.	17.	13.	11.	9.	7.	6.	5.	4.
3.	3.	2.	2.	2.	1.	1.	1.	1.	1.
1.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.							

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HYDROGRAPH AT STATION B

TOTAL RAINFALL = 2.01, TOTAL LOSS = .60, TOTAL EXCESS = 1.41

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR (INCHES)	72-HR (AC-FT)	24.92-HR (INCHES)
23.	12.75	5.	1.200	1.402	1.402
		2.	2.	3.	3.

CUMULATIVE AREA = .04 SQ MI

↑  
SUB-BASIN  
"B"

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 \* B \*  
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74 KK

76 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .083 TIME INTERVAL IN HOURS

77 HC

HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION B

PEAK FLOW TIME MAXIMUM AVERAGE FLOW

R2-13



72.

12.67

(CFS)  
(INCHES)  
(AC-FT)

14.  
1.103  
7.

4.  
1.285  
8.

4.  
1.285  
8.

4.  
1.285  
8.

CUMULATIVE AREA = .12 SQ MI

Sum of Sub-Basins "OF1, OF2, OF3, A and B"

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\* \*  
\* C \*  
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78 KK

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 110 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .083 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

81 BA

SUBBASIN CHARACTERISTICS

TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

82 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35 TP-40 TP-49  
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
.39 .76 1.34 1.40 1.44 1.56 1.69 2.01 .00 .00 .00 .00

STORM AREA = .02

83 LS

SCS LOSS RATE

STRTL .13 INITIAL ABSTRACTION  
CRVNR 94.00 CURVE NUMBER  
RTIMP .00 PERCENT IMPERVIOUS AREA

84 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .70 LAG

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UNIT HYDROGRAPH  
44 END-OF-PERIOD ORDINATES

1. 2. 4. 6. 10. 13. 15. 16. 16. 16.  
14. 13. 11. 9. 7. 6. 5. 4. 4. 3.  
2. 2. 2. 1. 1. 1. 1. 1. 1. 0.  
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.  
0. 0. 0. 0. 0. 0. 0. 0. 0. 0.

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HYDROGRAPH AT STATION C

TOTAL RAINFALL = 2.01, TOTAL LOSS = .60, TOTAL EXCESS = 1.41

PEAK FLOW TIME MAXIMUM AVERAGE FLOW  
(CFS) (HR) (CFS) 6-HR 24-HR 72-HR 24.92-HR  
15. 12.75 3. 1. 1. 1.  
(INCHES) 1.200 1.402 1.402 1.402  
(AC-FT) 2. 2. 2. 2.

CUMULATIVE AREA = .02 SQ MI

Sub-Basin "C"

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\* C \*  
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85 KK

R2-14

1 PKINI CONTROL  
 2 PLOT CONTROL  
 0. HYDROGRAPH PLOT SCALE  
 1 PUNCH COMPUTED HYDROGRAPH  
 21 SAVE HYDROGRAPH ON THIS UNIT  
 110 FIRST ORDINATE PUNCHED OR SAVED  
 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .083 TIME INTERVAL IN HOURS

88 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION C  
 SUM OF 2 HYDROGRAPHS

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DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW
22	JUN	1200	1	0.	*	22	JUN	1815	76	0.	*	23	JUN	0030	151	75.	*	23	JUN	0645	226	2.
22	JUN	1205	2	0.	*	22	JUN	1820	77	0.	*	23	JUN	0035	152	83.	*	23	JUN	0650	227	2.
22	JUN	1210	3	0.	*	22	JUN	1825	78	0.	*	23	JUN	0040	153	87.	*	23	JUN	0655	228	2.
22	JUN	1215	4	0.	*	22	JUN	1830	79	0.	*	23	JUN	0045	154	86.	*	23	JUN	0700	229	2.
22	JUN	1220	5	0.	*	22	JUN	1835	80	0.	*	23	JUN	0050	155	83.	*	23	JUN	0705	230	2.
22	JUN	1225	6	0.	*	22	JUN	1840	81	0.	*	23	JUN	0055	156	77.	*	23	JUN	0710	231	2.
22	JUN	1230	7	0.	*	22	JUN	1845	82	0.	*	23	JUN	0100	157	70.	*	23	JUN	0715	232	3.
22	JUN	1235	8	0.	*	22	JUN	1850	83	0.	*	23	JUN	0105	158	61.	*	23	JUN	0720	233	3.
22	JUN	1240	9	0.	*	22	JUN	1855	84	0.	*	23	JUN	0110	159	53.	*	23	JUN	0725	234	3.
22	JUN	1245	10	0.	*	22	JUN	1900	85	0.	*	23	JUN	0115	160	45.	*	23	JUN	0730	235	3.
22	JUN	1250	11	0.	*	22	JUN	1905	86	0.	*	23	JUN	0120	161	38.	*	23	JUN	0735	236	3.
22	JUN	1255	12	0.	*	22	JUN	1910	87	0.	*	23	JUN	0125	162	32.	*	23	JUN	0740	237	3.
22	JUN	1300	13	0.	*	22	JUN	1915	88	0.	*	23	JUN	0130	163	27.	*	23	JUN	0745	238	3.
22	JUN	1305	14	0.	*	22	JUN	1920	89	0.	*	23	JUN	0135	164	23.	*	23	JUN	0750	239	3.
22	JUN	1310	15	0.	*	22	JUN	1925	90	0.	*	23	JUN	0140	165	20.	*	23	JUN	0755	240	3.
22	JUN	1315	16	0.	*	22	JUN	1930	91	0.	*	23	JUN	0145	166	17.	*	23	JUN	0800	241	3.
22	JUN	1320	17	0.	*	22	JUN	1935	92	0.	*	23	JUN	0150	167	14.	*	23	JUN	0805	242	3.
22	JUN	1325	18	0.	*	22	JUN	1940	93	0.	*	23	JUN	0155	168	13.	*	23	JUN	0810	243	3.
22	JUN	1330	19	0.	*	22	JUN	1945	94	0.	*	23	JUN	0200	169	11.	*	23	JUN	0815	244	3.
22	JUN	1335	20	0.	*	22	JUN	1950	95	0.	*	23	JUN	0205	170	10.	*	23	JUN	0820	245	2.
22	JUN	1340	21	0.	*	22	JUN	1955	96	0.	*	23	JUN	0210	171	9.	*	23	JUN	0825	246	2.
22	JUN	1345	22	0.	*	22	JUN	2000	97	0.	*	23	JUN	0215	172	8.	*	23	JUN	0830	247	2.
22	JUN	1350	23	0.	*	22	JUN	2005	98	0.	*	23	JUN	0220	173	7.	*	23	JUN	0835	248	2.
22	JUN	1355	24	0.	*	22	JUN	2010	99	0.	*	23	JUN	0225	174	7.	*	23	JUN	0840	249	2.
22	JUN	1400	25	0.	*	22	JUN	2015	100	0.	*	23	JUN	0230	175	6.	*	23	JUN	0845	250	2.
22	JUN	1405	26	0.	*	22	JUN	2020	101	0.	*	23	JUN	0235	176	6.	*	23	JUN	0850	251	2.
22	JUN	1410	27	0.	*	22	JUN	2025	102	0.	*	23	JUN	0240	177	5.	*	23	JUN	0855	252	2.
22	JUN	1415	28	0.	*	22	JUN	2030	103	0.	*	23	JUN	0245	178	5.	*	23	JUN	0900	253	2.
22	JUN	1420	29	0.	*	22	JUN	2035	104	0.	*	23	JUN	0250	179	5.	*	23	JUN	0905	254	2.
22	JUN	1425	30	0.	*	22	JUN	2040	105	0.	*	23	JUN	0255	180	4.	*	23	JUN	0910	255	2.
22	JUN	1430	31	0.	*	22	JUN	2045	106	0.	*	23	JUN	0300	181	4.	*	23	JUN	0915	256	2.
22	JUN	1435	32	0.	*	22	JUN	2050	107	0.	*	23	JUN	0305	182	4.	*	23	JUN	0920	257	2.
22	JUN	1440	33	0.	*	22	JUN	2055	108	0.	*	23	JUN	0310	183	4.	*	23	JUN	0925	258	2.
22	JUN	1445	34	0.	*	22	JUN	2100	109	0.	*	23	JUN	0315	184	4.	*	23	JUN	0930	259	2.
22	JUN	1450	35	0.	*	22	JUN	2105	110	0.	*	23	JUN	0320	185	3.	*	23	JUN	0935	260	2.
22	JUN	1455	36	0.	*	22	JUN	2110	111	0.	*	23	JUN	0325	186	3.	*	23	JUN	0940	261	2.
22	JUN	1500	37	0.	*	22	JUN	2115	112	0.	*	23	JUN	0330	187	3.	*	23	JUN	0945	262	2.
22	JUN	1505	38	0.	*	22	JUN	2120	113	0.	*	23	JUN	0335	188	3.	*	23	JUN	0950	263	2.
22	JUN	1510	39	0.	*	22	JUN	2125	114	0.	*	23	JUN	0340	189	3.	*	23	JUN	0955	264	2.
22	JUN	1515	40	0.	*	22	JUN	2130	115	0.	*	23	JUN	0345	190	3.	*	23	JUN	1000	265	2.
22	JUN	1520	41	0.	*	22	JUN	2135	116	0.	*	23	JUN	0350	191	3.	*	23	JUN	1005	266	2.
22	JUN	1525	42	0.	*	22	JUN	2140	117	0.	*	23	JUN	0355	192	3.	*	23	JUN	1010	267	2.
22	JUN	1530	43	0.	*	22	JUN	2145	118	0.	*	23	JUN	0400	193	2.	*	23	JUN	1015	268	2.
22	JUN	1535	44	0.	*	22	JUN	2150	119	0.	*	23	JUN	0405	194	2.	*	23	JUN	1020	269	2.
22	JUN	1540	45	0.	*	22	JUN	2155	120	0.	*	23	JUN	0410	195	2.	*	23	JUN	1025	270	2.
22	JUN	1545	46	0.	*	22	JUN	2200	121	0.	*	23	JUN	0415	196	2.	*	23	JUN	1030	271	2.
22	JUN	1550	47	0.	*	22	JUN	2205	122	1.	*	23	JUN	0420	197	2.	*	23	JUN	1035	272	2.
22	JUN	1555	48	0.	*	22	JUN	2210	123	1.	*	23	JUN	0425	198	2.	*	23	JUN	1040	273	2.
22	JUN	1600	49	0.	*	22	JUN	2215	124	1.	*	23	JUN	0430	199	2.	*	23	JUN	1045	274	2.
22	JUN	1605	50	0.	*	22	JUN	2220	125	1.	*	23	JUN	0435	200	2.	*	23	JUN	1050	275	2.
22	JUN	1610	51	0.	*	22	JUN	2225	126	1.	*	23	JUN	0440	201	2.	*	23	JUN	1055	276	2.
22	JUN	1615	52	0.	*	22	JUN	2230	127	1.	*	23	JUN	0445	202	2.	*	23	JUN	1100	277	2.
22	JUN	1620	53	0.	*	22	JUN	2235	128	1.	*	23	JUN	0450	203	2.	*	23	JUN	1105	278	2.
22	JUN	1625	54	0.	*	22	JUN	2240	129	1.	*	23	JUN	0455	204	2.	*	23	JUN	1110	279	2.
22	JUN	1630	55	0.	*	22	JUN	2245	130	1.	*	23	JUN	0500	205	2.	*	23	JUN	1115	280	2.
22	JUN	1635	56	0.	*	22	JUN	2250	131	1.	*	23	JUN	0505	206	2.	*	23	JUN	1120	281	2.
22	JUN	1640	57	0.	*	22	JUN	2255	132	1.	*	23	JUN	0510	207	2.	*	23	JUN	1125	282	2.
22	JUN	1645	58	0.	*	22	JUN	2300	133	1.	*	23	JUN	0515	208	2.	*	23	JUN	1130	283	2.
22	JUN	1650	59	0.	*	22	JUN	2305	134	1.	*	23	JUN	0520	209	2.	*	23	JUN	1135	284	2.
22	JUN	1655	60	0.	*	22	JUN	2310	135	1.	*	23	JUN	0525	210	2.	*	23	JUN	1140	285	2.
22	JUN	1700	61	0.	*	22	JUN	2315	136	1.	*	23	JUN	0530	211	2.	*	23	JUN	1145	286	2.
22	JUN	1705	62	0.	*	22	JUN	2320	137	1.	*	23	JUN	0535	212	2.	*	23	JUN	1150	287	2.
22	JUN	1710	63	0.	*	22	JUN	2325	138	1.	*	23	JUN	0540	213	2.	*	23	JUN	1155	288	2.

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22 JUN 1720	65	0.	*	22 JUN 2335	140	1.	*	23 JUN 0550	215	2.	*	23 JUN 1205	290	2.
22 JUN 1725	66	0.	*	22 JUN 2340	141	1.	*	23 JUN 0555	216	2.	*	23 JUN 1210	291	2.
22 JUN 1730	67	0.	*	22 JUN 2345	142	2.	*	23 JUN 0600	217	2.	*	23 JUN 1215	292	2.
22 JUN 1735	68	0.	*	22 JUN 2350	143	2.	*	23 JUN 0605	218	2.	*	23 JUN 1220	293	2.
22 JUN 1740	69	0.	*	22 JUN 2355	144	3.	*	23 JUN 0610	219	2.	*	23 JUN 1225	294	2.
22 JUN 1745	70	0.	*	23 JUN 0000	145	5.	*	23 JUN 0615	220	2.	*	23 JUN 1230	295	2.
22 JUN 1750	71	0.	*	23 JUN 0005	146	9.	*	23 JUN 0620	221	2.	*	23 JUN 1235	296	1.
22 JUN 1755	72	0.	*	23 JUN 0010	147	16.	*	23 JUN 0625	222	2.	*	23 JUN 1240	297	1.
22 JUN 1800	73	0.	*	23 JUN 0015	148	28.	*	23 JUN 0630	223	2.	*	23 JUN 1245	298	1.
22 JUN 1805	74	0.	*	23 JUN 0020	149	43.	*	23 JUN 0635	224	2.	*	23 JUN 1250	299	1.
22 JUN 1810	75	0.	*	23 JUN 0025	150	60.	*	23 JUN 0640	225	2.	*	23 JUN 1255	300	1.

\*\*\*\*\*

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	24.92-HR
87.	12.67	(CFS) 17.	5.	5.	5.
		(INCHES) 1.120	1.305	1.305	1.305
		(AC-FT) 9.	10.	10.	10.

CUMULATIVE AREA = .14 SQ MI



SUM OF SUB-BASINS "OF 1, OF 2, OF 3, A, B AND C"  
 FLOW DISCHARGED AT SOUTHWEST CORNER OF  
 PROJECT SITE INTO DRAINAGE AND IRRIGATION  
 DITCH ALONG 28 ROAD.

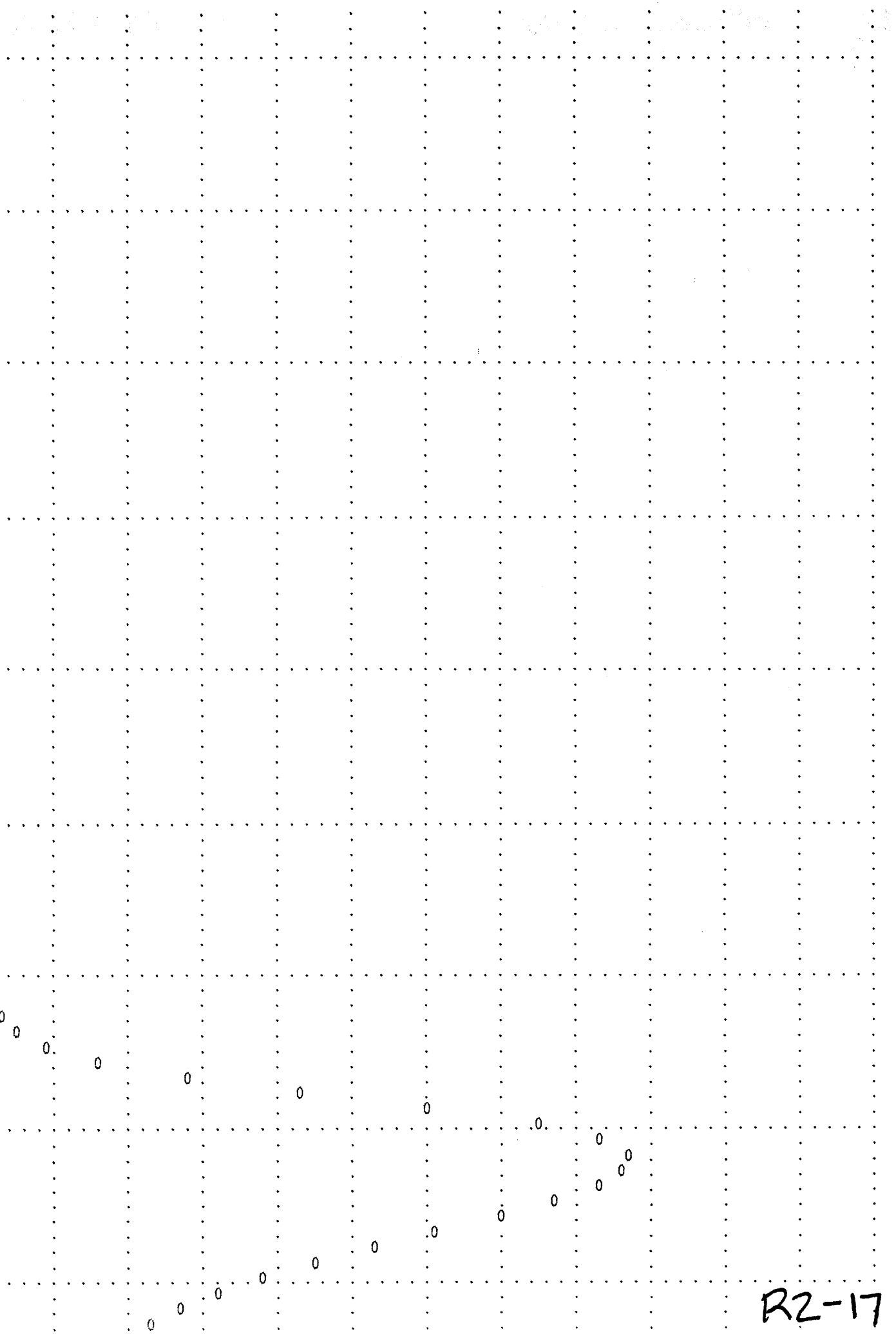
STATION C

(O) OUTFLOW

DAHRMN PER	0.	10.	20.	30.	40.	50.	60.	70.	80.	90.	0.	0.	0.
221200	10	.	.	.	.	.	.	.	.	.	.	.	.
221205	20	.	.	.	.	.	.	.	.	.	.	.	.
221210	30	.	.	.	.	.	.	.	.	.	.	.	.
221215	40	.	.	.	.	.	.	.	.	.	.	.	.
221220	50	.	.	.	.	.	.	.	.	.	.	.	.
221225	60	.	.	.	.	.	.	.	.	.	.	.	.
221230	70	.	.	.	.	.	.	.	.	.	.	.	.
221235	80	.	.	.	.	.	.	.	.	.	.	.	.
221240	90	.	.	.	.	.	.	.	.	.	.	.	.
221245	100	.	.	.	.	.	.	.	.	.	.	.	.
221250	110	.	.	.	.	.	.	.	.	.	.	.	.
221255	120	.	.	.	.	.	.	.	.	.	.	.	.
221300	130	.	.	.	.	.	.	.	.	.	.	.	.
221305	140	.	.	.	.	.	.	.	.	.	.	.	.
221310	150	.	.	.	.	.	.	.	.	.	.	.	.
221315	160	.	.	.	.	.	.	.	.	.	.	.	.
221320	170	.	.	.	.	.	.	.	.	.	.	.	.
221325	180	.	.	.	.	.	.	.	.	.	.	.	.
221330	190	.	.	.	.	.	.	.	.	.	.	.	.
221335	200	.	.	.	.	.	.	.	.	.	.	.	.
221340	210	.	.	.	.	.	.	.	.	.	.	.	.
221345	220	.	.	.	.	.	.	.	.	.	.	.	.
221350	230	.	.	.	.	.	.	.	.	.	.	.	.
221355	240	.	.	.	.	.	.	.	.	.	.	.	.
221400	250	.	.	.	.	.	.	.	.	.	.	.	.
221405	260	.	.	.	.	.	.	.	.	.	.	.	.
221410	270	.	.	.	.	.	.	.	.	.	.	.	.
221415	280	.	.	.	.	.	.	.	.	.	.	.	.
221420	290	.	.	.	.	.	.	.	.	.	.	.	.
221425	300	.	.	.	.	.	.	.	.	.	.	.	.
221430	310	.	.	.	.	.	.	.	.	.	.	.	.
221435	320	.	.	.	.	.	.	.	.	.	.	.	.
221440	330	.	.	.	.	.	.	.	.	.	.	.	.
221445	340	.	.	.	.	.	.	.	.	.	.	.	.
221450	350	.	.	.	.	.	.	.	.	.	.	.	.
221455	360	.	.	.	.	.	.	.	.	.	.	.	.
221500	370	.	.	.	.	.	.	.	.	.	.	.	.
221505	380	.	.	.	.	.	.	.	.	.	.	.	.
221510	390	.	.	.	.	.	.	.	.	.	.	.	.
221515	400	.	.	.	.	.	.	.	.	.	.	.	.
221520	410	.	.	.	.	.	.	.	.	.	.	.	.
221525	420	.	.	.	.	.	.	.	.	.	.	.	.
221530	430	.	.	.	.	.	.	.	.	.	.	.	.
221535	440	.	.	.	.	.	.	.	.	.	.	.	.
221540	450	.	.	.	.	.	.	.	.	.	.	.	.
221545	460	.	.	.	.	.	.	.	.	.	.	.	.
221550	470	.	.	.	.	.	.	.	.	.	.	.	.
221555	480	.	.	.	.	.	.	.	.	.	.	.	.
221600	490	.	.	.	.	.	.	.	.	.	.	.	.
221605	500	.	.	.	.	.	.	.	.	.	.	.	.
221610	510	.	.	.	.	.	.	.	.	.	.	.	.
221615	520	.	.	.	.	.	.	.	.	.	.	.	.
221620	530	.	.	.	.	.	.	.	.	.	.	.	.
221625	540	.	.	.	.	.	.	.	.	.	.	.	.
221630	550	.	.	.	.	.	.	.	.	.	.	.	.
221635	560	.	.	.	.	.	.	.	.	.	.	.	.
221640	570	.	.	.	.	.	.	.	.	.	.	.	.
221645	580	.	.	.	.	.	.	.	.	.	.	.	.
221650	590	.	.	.	.	.	.	.	.	.	.	.	.
221655	600	.	.	.	.	.	.	.	.	.	.	.	.
221700	610	.	.	.	.	.	.	.	.	.	.	.	.
221705	620	.	.	.	.	.	.	.	.	.	.	.	.
221710	630	.	.	.	.	.	.	.	.	.	.	.	.
221715	640	.	.	.	.	.	.	.	.	.	.	.	.
221720	650	.	.	.	.	.	.	.	.	.	.	.	.
221725	660	.	.	.	.	.	.	.	.	.	.	.	.
221730	670	.	.	.	.	.	.	.	.	.	.	.	.
221735	680	.	.	.	.	.	.	.	.	.	.	.	.
221740	690	.	.	.	.	.	.	.	.	.	.	.	.
221745	700	.	.	.	.	.	.	.	.	.	.	.	.
221750	710	.	.	.	.	.	.	.	.	.	.	.	.
221755	720	.	.	.	.	.	.	.	.	.	.	.	.
221800	730	.	.	.	.	.	.	.	.	.	.	.	.
221805	740	.	.	.	.	.	.	.	.	.	.	.	.
221810	750	.	.	.	.	.	.	.	.	.	.	.	.
221815	760	.	.	.	.	.	.	.	.	.	.	.	.

RZ-16.1

221825 780  
221830 790  
221835 800  
221840 810  
221845 820  
221850 830  
221855 840  
221900 850  
221905 860  
221910 870  
221915 880  
221920 890  
221925 900  
221930 910  
221935 920  
221940 930  
221945 940  
221950 950  
221955 960  
222000 970  
222005 980  
222010 990  
222015 1000  
222020 1010  
222025 1020  
222030 1030  
222035 1040  
222040 1050  
222045 1060  
222050 1070  
222055 1080  
222100 1090  
222105 1100  
222110 1110  
222115 1120  
222120 1130  
222125 1140  
222130 1150  
222135 1160  
222140 1170  
222145 1180  
222150 1190  
222155 1200  
222200 1210  
222205 122.0  
222210 123.0  
222215 124.0  
222220 125.0  
222225 126.0  
222230 127.0  
222235 128.0  
222240 129.0  
222245 130.0  
222250 131.0  
222255 132.0  
222300 133.0  
222305 134.0  
222310 135.0  
222315 136.0  
222320 137.0  
222325 138.0  
222330 139.0  
222335 140.0  
222340 141.0  
222345 142.0  
222350 143.0  
222355 144.0  
230000 145.0  
230005 146.0  
230010 147.0  
230015 148.0  
230020 149.0  
230025 150.0  
230030 151.0  
230035 152.0  
230040 153.0  
230045 154.0  
230050 155.0  
230055 156.0  
230100 157.0  
230105 158.0  
230110 159.0  
230115 160.0  
230120 161.0  
230125 162.0  
230130 163.0  
230135 164.0



R2-17

230145 166.  
230150 167.  
230155 168.  
230200 169.  
230205 170.  
230210 171.  
230215 172.  
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230225 174.  
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230800 241.  
230805 242.  
230810 243.  
230815 244.  
230820 245.  
230825 246.  
230830 247.  
230835 248.  
230840 249.  
230845 250.  
230850 251.  
230855 252.

R2-18

230905	254.	0
230910	255.	0
230915	256.	0
230920	257.	0
230925	258.	0
230930	259.	0
230935	260.	0
230940	261.	0
230945	262.	0
230950	263.	0
230955	264.	0
231000	265.	0
231005	266.	0
231010	267.	0
231015	268.	0
231020	269.	0
231025	270.	0
231030	271.	0
231035	272.	0
231040	273.	0
231045	274.	0
231050	275.	0
231055	276.	0
231100	277.	0
231105	278.	0
231110	279.	0
231115	280.	0
231120	281.	0
231125	282.	0
231130	283.	0
231135	284.	0
231140	285.	0
231145	286.	0
231150	287.	0
231155	288.	0
231200	289.	0
231205	290.	0
231210	291.	0
231215	292.	0
231220	293.	0
231225	294.	0
231230	295.	0
231235	296.	0
231240	297.	0
231245	298.	0
231250	299.	0
231255	300.	0

R2-19

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	OF1	15.	12.42	2.	1.	1.	.02		
ROUTED TO	DITCH2	15.	12.42	2.	1.	1.	.02		
HYDROGRAPH AT	A	24.	12.75	5.	1.	1.	.04		
2 COMBINED AT	A	36.	12.58	7.	2.	2.	.06		
HYDROGRAPH AT	OF2	12.	12.58	2.	1.	1.	.02		
ROUTED TO	DITCH1	12.	12.58	2.	1.	1.	.02		
HYDROGRAPH AT	OF3	3.	12.58	0.	0.	0.	.00		
2 COMBINED AT	OF3	15.	12.58	2.	1.	1.	.02		
2 COMBINED AT	OF3	51.	12.58	9.	3.	3.	.08		
ROUTED TO	DITCH3	50.	12.67	9.	3.	3.	.08		
HYDROGRAPH AT	B	23.	12.75	5.	1.	1.	.04		
2 COMBINED AT	B	72.	12.67	14.	4.	4.	.12		
HYDROGRAPH AT	C	15.	12.75	3.	1.	1.	.02		
2 COMBINED AT	C	87.	12.67	17.	5.	5.	.14		

R2-20



SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
 (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO		VOLUME
							COMPUTATION PEAK	INTERVAL TIME TO PEAK	
		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
DITCH2	MANE	5.00	14.95	745.00	1.17	5.00	14.95	745.00	1.17

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1094E+01 EXCESS= .0000E+00 OUTFLOW= .1094E+01 BASIN STORAGE= .6151E-03 PERCENT ERROR= -.1

DITCH1	MANE	2.33	12.36	755.35	1.17	5.00	12.34	755.00	1.17
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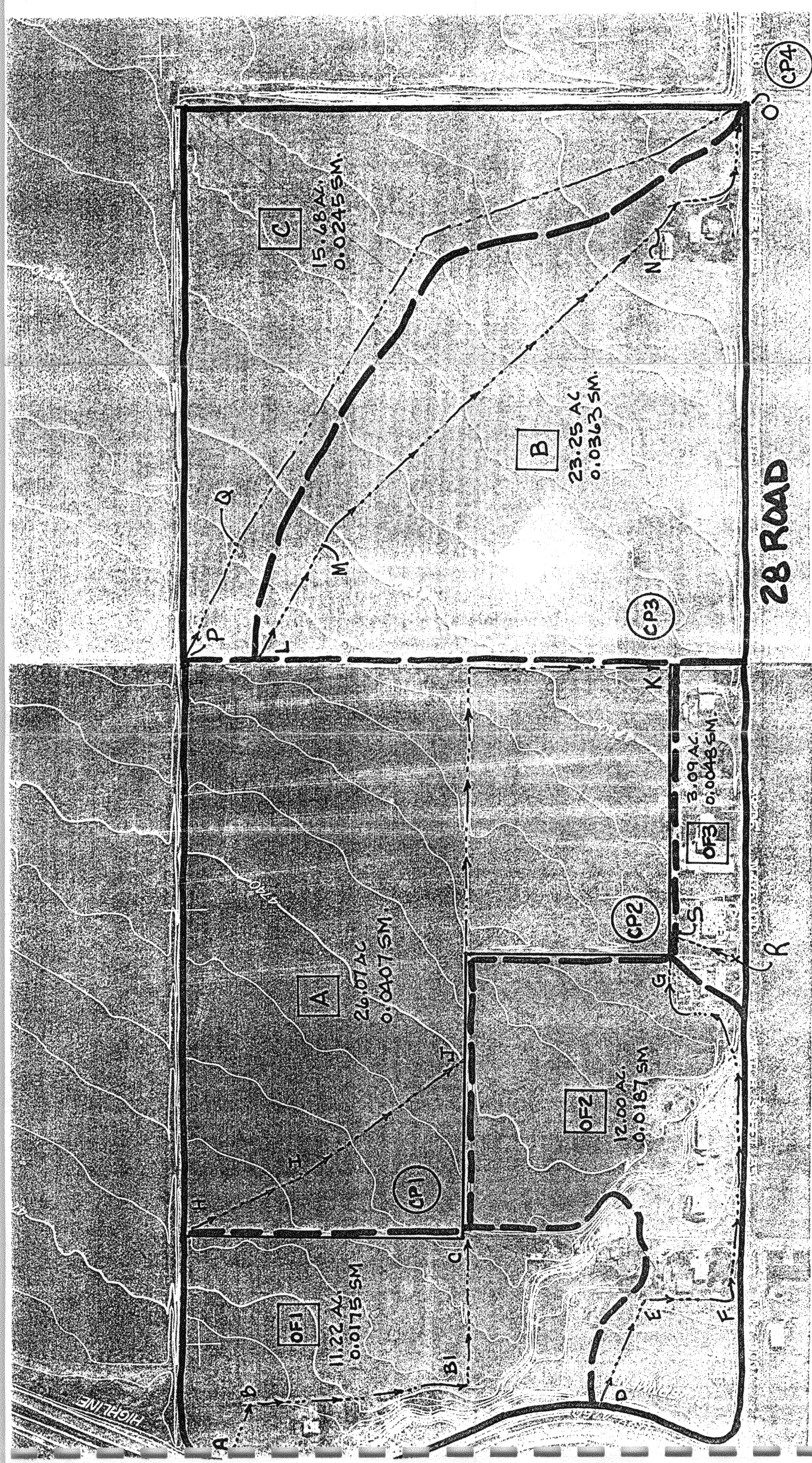
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1168E+01 EXCESS= .0000E+00 OUTFLOW= .1168E+01 BASIN STORAGE= .5788E-03 PERCENT ERROR= .0

DITCH3	MANE	4.31	50.34	759.08	1.23	5.00	50.18	760.00	1.23
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CONTINUITY SUMMARY (AC-FT) - INFLOW= .5376E+01 EXCESS= .0000E+00 OUTFLOW= .5371E+01 BASIN STORAGE= .9706E-02 PERCENT ERROR= -.1

\*\*\* NORMAL END OF HEC-1 \*\*\*





# HISTORIC WATERSHED & ROUTING MAP

**1" = 200'**



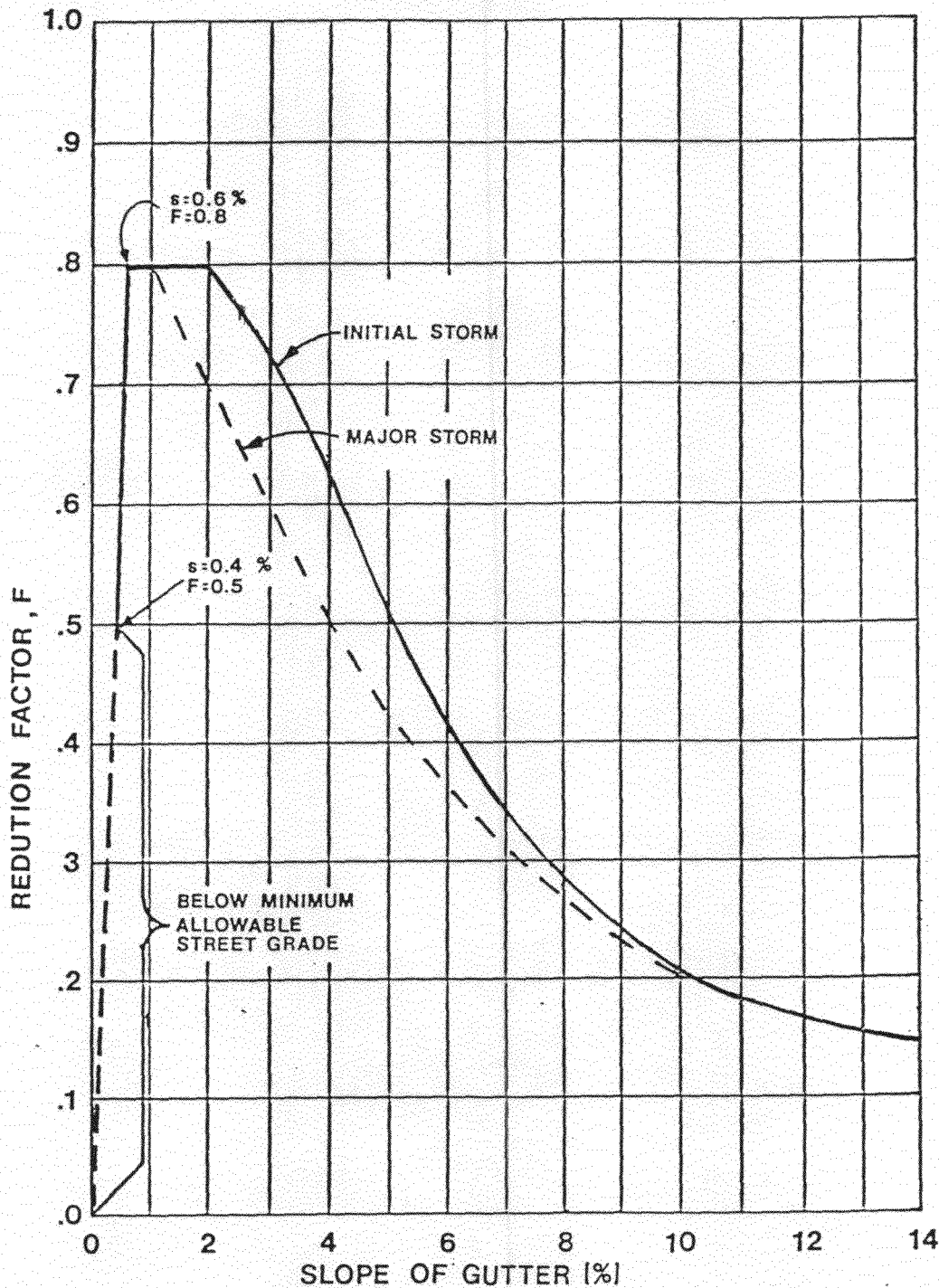


FIGURE 6-2 REDUCTION FACTOR FOR ALLOWABLE GUTTER CAPACITY LOCAL AND COLLECTOR STREETS

APPLY REDUCTION FACTOR FOR APPLICABLE SLOPE TO THE THEORETICAL GUTTER CAPACITY TO OBTAIN ALLOWABLE GUTTER CAPACITY APPROACHING ARTERIAL STREET

ALLOWABLE USE OF ROADS AND OF CROSS ROAD FLOW AS PART OF DRAINAGE SYSTEM DURING MINOR AND MAJOR STORM RUNOFF

STREET CLASSIFICATION	MINOR STORM (maximum roadway encroachment)	MAJOR STORM (allowable depth and inundation)
Local, Lane and Place (Residential or Subcollector)	Flow may spread to crown of street.	Residential dwellings, public, commercial, and industrial buildings. Access shall not be inundated at ground line, unless buildings are flood-proofed. Depth of water over gutter flow line shall not exceed 18".
URBAN SECTION	No curb overtopping.	
RURAL SECTION	Encroachment shall not extend over property line.	
Collector (Residential Collector, and Collector)	One traffic lane must remain free of inundation for both URBAN and RURAL	(same as above)

SECTIONS

Arterial	One traffic lane in each direction must remain free of inundation for both URBAN and RURAL	(same as above) Depth of water at street crown shall not exceed 6", to allow for operation of emergency vehicles.
----------	--	---

SECTIONS

ALLOWABLE CROSS STREET FLOW

STREET CLASSIFICATION	MINOR STORM	MAJOR STORM
Local, Lane, Place and Collector (Residential Access, Subcollector, Residential Collector and Collector)	Where cross pan exists allowed depth of flow shall not exceed 6"	Depth of water over gutter flow line shall not exceed 18"
Arterial	None	Depth of water at crown shall not exceed 6"

STREET CARRING CAPACITY (2 YEAR)

PROJECT: GRAND VIEW SUBDIVISION  
 LOCATION: CITY OF GRAND JUNCTION, COLORADO  
 DATE: Jun-94

Street Information: R.O.W. Width = 44.00 FT. Flow Area = 2.77 SF.  
 Flowline Width = 31.00 FT.  
 Classification = URBAN RES.  
 Mannings = 0.015  
 Max. Depth = 0.41 FT. To Top Back Of Curb  
 Str/ X-Slope = 2.00 %  
 Gutter Slope = 8.33 % Drive Over Curb, Gutter and  
 Sidewalk Slope = 2.08 % 1/4" / FT.  
 Roadside Slope = 2.08 % 1/4" / FT.

SLOPE OF STREET %	REDUCTION FACTOR FOR SLOPE	ALLOWABLE CAPACITY C.F.S.	VELOCITY F.P.S.
0.60	0.80	5.15	1.86
0.65	0.80	5.36	1.94
0.70	0.80	5.56	2.01
0.75	0.80	5.76	2.08
0.77	0.80	5.84	2.11
0.80	0.80	5.95	2.15
0.81	0.80	5.99	2.16
0.83	0.80	6.06	2.19
0.86	0.80	6.17	2.23
0.94	0.80	6.45	2.33
1.00	0.80	6.65	2.40
1.05	0.80	6.81	2.46
1.09	0.80	6.94	2.51
1.18	0.80	7.22	2.61
1.24	0.80	7.41	2.67
1.65	0.80	8.54	3.08
2.50	0.76	9.99	3.61

Formula:  $Q_a = F \times (1.49/N) \times R^{2/3} \times S^{1/2} \times A$   
 F = Reduction Factor For Slope  
 N = Mannings Coefficient = 0.0150  
 R = Hydraulic Radius = A/WP = 0.1661  
 A = Cross Sectional Area Sq.Ft. = 2.770  
 WP = Wetted Perimeter Ft. = 16.679  
 S = Street Slope FT./FT.

**NOTE: USE THIS SHEET TO CALLULATE TRAVEL TIME  $T_t$  FOR INPUT INTO WORKSHEET 3. DEVELOPED CONDITION.**

STREET CARRING CAPACITY (100 YEAR)

PROJECT: GRAND VIEW SUBDIVISION  
 LOCATION: CITY OF GRAND JUNCTION, COLORADO  
 DATE: Jun-94

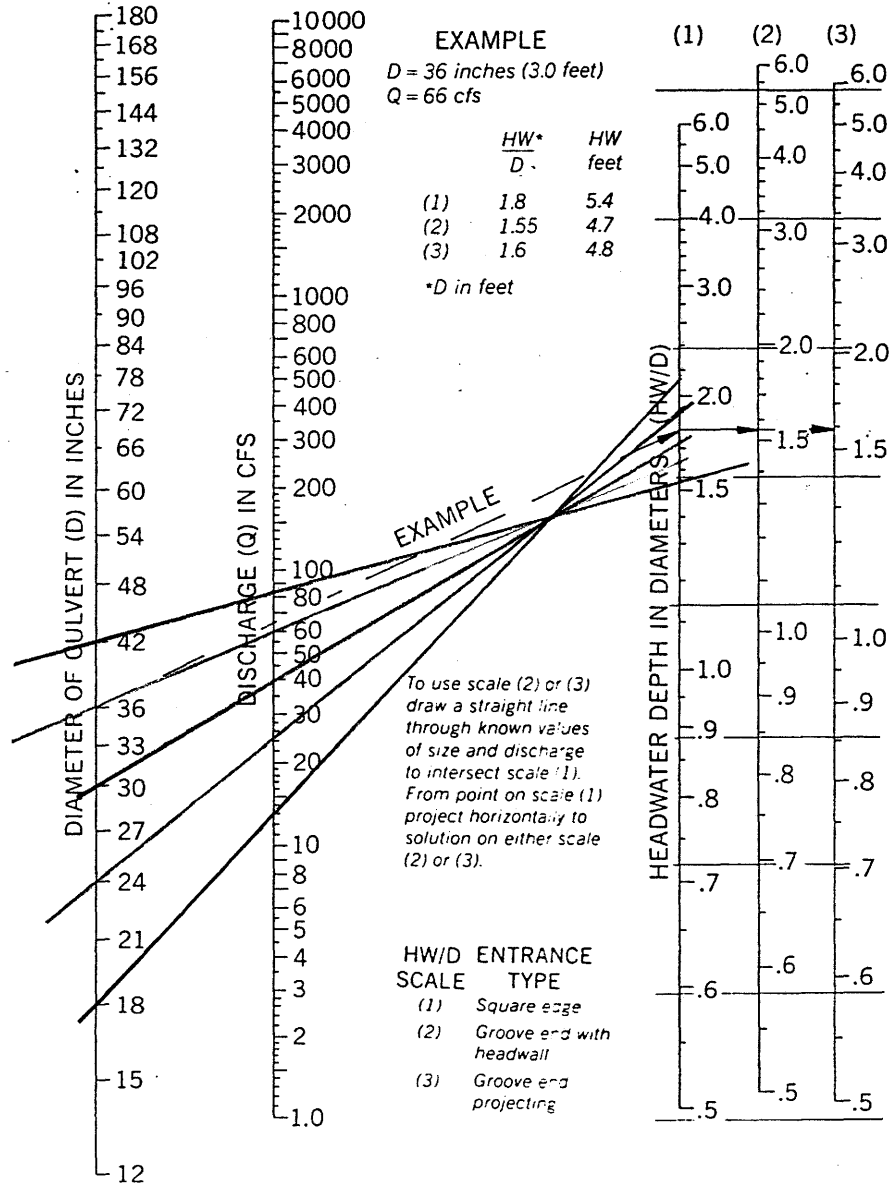
Street Information: R.O.W. Width = 44.00 FT. Flow Area = 25.45 SF.  
 Flowline Width = 31.00 FT.  
 Classification = URBAN  
 Mannings = 0.015  
 Max. Depth = 1.5 FT. Above Flowline  
 Str/ X-Slope = 2.00 %  
 Gutter Slope = 8.33 % Drive Over Curb, Gutter and  
 Sidewalk Slope = 2.08 % 1/4" / FT.  
 Roadside Slope = 2.08 % 1/4" / FT.

SLOPE OF STREET %	REDUCTION FACTOR FOR SLOPE	ALLOWABLE CAPACITY C.F.S.	VELOCITY F.P.S.
0.60	0.80	161.82	6.36
0.65	0.80	168.43	6.62
0.70	0.80	174.79	6.87
0.75	0.80	180.92	7.11
0.77	0.80	183.32	7.20
0.80	0.80	186.86	7.34
0.81	0.80	188.02	7.39
0.83	0.80	190.33	7.48
0.86	0.80	193.74	7.61
0.94	0.80	202.55	7.96
1.00	0.80	208.91	8.21
1.05	0.80	214.07	8.41
1.09	0.80	218.11	8.57
1.18	0.80	226.94	8.92
1.24	0.80	232.64	9.14
1.65	0.80	268.35	10.54
2.50	0.76	313.81	12.33

Formula:  $Q_a = F \times (1.49/N) \times R^{2/3} \times S^{1/2} \times A$   
 F = Reduction Factor For Slope  
 N = Mannings Coefficient = 0.0150  
 R = Hydraulic Radius = A/WP = 1.0497  
 A = Cross Sectional Area Sq.Ft. = 25.453  
 WP = Wetted Perimeter Ft. = 24.248  
 S = Street Slope FT./FT.

FIGURE 33

### HEADWATER DEPTH FOR CIRCULAR CONCRETE PIPE CULVERTS WITH INLET CONTROL



BUREAU OF PUBLIC ROADS JAN. 1963

HEADWATER SCALES 2&3  
REVISED MAY 1964

SIZE	HW/D =	Q CFS
24"	$3.81 / 2.0 = 1.91$	24.5 CFS
30"	$4.35 / 2.5 = 1.74$	39.5 CFS
36"	$4.90 / 3.0 = 1.63$	60.0 CFS
42"	$5.44 / 3.5 = 1.55$	84.0 CFS
18"	$3.25 / 1.5 = 2.17$	13.0 CFS

Circular Channel Analysis & Design  
Solved with Manning's Equation

Open Channel - Uniform flow

Worksheet Name: STORM SEWER B1

Comment: INLET NO.1 TO INLET NO. 2

Solve For Full Flow Capacity

Given Input Data:

Diameter.....	1.50 ft	<b>PRELIMINARY SIZE</b>
Slope.....	0.0100 ft/ft	
Manning's n.....	0.013	
Discharge.....	10.50 cfs	

Computed Results:

Full Flow Capacity.....	10.50 cfs	<b>INLET CONTROL = 13.0 CFS</b>
Full Flow Depth.....	1.50 ft	
Velocity.....	5.94 fps	
Flow Area.....	1.77 sf	
Critical Depth....	1.25 ft	
Critical Slope....	0.0098 ft/ft	
Percent Full.....	100.00 %	
Full Capacity.....	10.50 cfs	
QMAX @.94D.....	11.30 cfs	
Froude Number.....	FULL	



Circular Channel Analysis & Design  
Solved with Manning's Equation

Open Channel - Uniform flow

Worksheet Name: STORM SEWER A-1

Comment: INLET NO. 2 TO NO. 3 AND OUTFALL

Solve For Full Flow Capacity

Given Input Data:

Diameter.....	3.00 ft	<i>PRELIMINARY SIZE</i>
Slope.....	0.0060 ft/ft	
Manning's n.....	0.013	
Discharge.....	51.66 cfs	

Computed Results:

Full Flow Capacity.....	51.66 cfs	<i>INLET CONTROL = 60.0 CFS</i>
Full Flow Depth.....	3.00 ft	
Velocity.....	7.31 fps	
Flow Area.....	7.07 sf	
Critical Depth....	2.34 ft	
Critical Slope....	0.0066 ft/ft	
Percent Full.....	100.00 %	
Full Capacity.....	51.66 cfs	
QMAX @.94D.....	55.58 cfs	
Froude Number.....	FULL	

Worksheet 2: Runoff curve number and runoff

Project GRAND VIEW SUB By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed SUB-BASINS A1, A2 AND A3

1. Runoff curve number (CN)

Soil name and hydrologic group (appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN 1/			Area <input type="checkbox"/> acres <input checked="" type="checkbox"/> mi <sup>2</sup>	Product of CN x area
		Table 2-1	Fig. 2-3	Fig. 2-4		
(RF) RAVOLA VERY FINE SANDY LOAM "B"	RES. LOTS, ROADWAYS	94			33	3102
(PC) BILLINGS SILTY CLAY LOAM "C"	RES. LOTS, ROADWAYS	96			67	6432

1/ Use only one CN source per line. Totals = 100 9534

CN (weighted) =  $\frac{\text{total product}}{\text{total area}} = \frac{9534}{100} = 95.34$  Use CN = 95

2. Runoff

Frequency ..... yr  
 Rainfall, P (24-hour) ..... in  
 Runoff, Q ..... in  
 (Use P and CN with table 2-1, fig. 2-1, or eqs. 2-3 and 2-4.)

Storm #1	Storm #2	Storm #3
N/A		
N/A		
N/A		

D E V E L O P E D  
S U B - B A S I N S U M M A R Y

SUB-BASIN HISTORIC	AREA AC.	AREA SM.	HYDRO. SOIL GROUP	SCS CN
OF1	11.22	0.0175	B	91
OF2	12.00	0.0187	B	91
OF3	3.09	0.0048	B	88
A1	18.62	0.0291	B & C	95
A2	3.27	0.0051	B & C	95
A3	3.38	0.0053	B & C	95
B1	0.31	0.0005	C	96
B2	0.29	0.0005	C	96
B3	4.36	0.0068	C	96
B4	3.73	0.0058	C	96
B5	0.40	0.0006	C	96
C1	11.99	0.0187	C	96
C2	3.15	0.0049	C	96
C3	1.24	0.0019	C	96
D1	1.58	0.0025	C	96
D2	9.60	0.0150	C	96
D3	0.75	0.0012	C	96
D4	2.33	0.0036	C	96
E1	0.55	0.0009	C	91
TOTAL	91.86	0.1435		

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 2 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN A1

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

<u>Sheet flow</u> (Applicable to $T_c$ only)	Segment ID	
1. Surface description (table 3-1) .....	A-B	RESLOT
2. Manning's roughness coeff., n (table 3-1) ..	0.35	
3. Flow length, L (total L $\leq$ 300 ft) .....	165	ft
4. Two-yr 24-hr rainfall, $P_2$ .....	0.70	in
5. Land slope, s .....	0.02	ft/ft
6. $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ .....	1.027	+ [ ] = 1.027

<u>Shallow concentrated flow</u>	Segment ID	
7. Surface description (paved or unpaved) .....	B-C	PAVED ROAD
8. Flow length, L .....	689	687 ft
9. Watercourse slope, s .....	0.01	0.007 ft/ft
10. Average velocity, V (figure <del>20</del> <u>402</u> ) .....	2.40	2.01 ft/s
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	0.080	+ 0.095 = 0.175

<u>Channel flow</u>	Segment ID	
12. Cross sectional flow area, a .....	D-E	SEE
13. Wetted perimeter, $p_w$ .....	E-F	CALL.
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	SHEET FOR STREETS	
15. Channel slope, s .....	0.012	0.006 ft/ft
16. Manning's roughness coeff., n .....	0.015	0.015
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	2.67	1.86 ft/s
18. Flow length, L .....	376	202 ft
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	0.039	+ 0.030 = 0.069
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....	1.271	

$T_{LAG} = 0.60 (1.271) = 0.763 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN A1

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

- Segment ID
1. Surface description (table 3-1) .....
  2. Manning's roughness coeff., n (table 3-1) ..
  3. Flow length, L (total L  $\leq$  300 ft) ..... ft
  4. ~~24-hr~~ 100 24-hr rainfall, P ~~100~~ ..... in
  5. Land slope, s ..... ft/ft
  6.  $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

<u>A-B</u>	
<u>RES. LOT</u>	
<u>0.35</u>	
<u>165</u>	
<u>2.01</u>	
<u>0.02</u>	
<u>0.606</u> +	<u>0.606</u>

Shallow concentrated flow

- Segment ID
7. Surface description (paved or unpaved) .....
  8. Flow length, L ..... ft
  9. Watercourse slope, s ..... ft/ft
  10. Average velocity, V (figure ~~30~~ 402) ..... ft/s
  11.  $T_t = \frac{L}{3600 V}$  Compute  $T_t$  ..... hr

<u>B-C</u>	<u>C-D</u>
<u>PAVED</u>	<u>ROAD</u>
<u>689</u>	<u>687</u>
<u>0.01</u>	<u>0.007</u>
<u>2.40</u>	<u>2.01</u>
<u>0.080</u> +	<u>0.095</u> = <u>0.175</u>

Channel flow

- Segment ID
12. Cross sectional flow area, a ..... ft<sup>2</sup>
  13. Wetted perimeter,  $p_w$  ..... ft
  14. Hydraulic radius,  $r = \frac{a}{p_w}$  Compute r ..... ft
  15. Channel slope, s ..... ft/ft
  16. Manning's roughness coeff., n .....
  17.  $v = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
  18. Flow length, L ..... ft
  19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
  20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

<u>D-E</u>	<u>E-F</u>
<u>SEE</u>	
<u>CALL.</u>	
<u>SHEETS</u>	<u>FOR STREETS</u>
<u>0.012</u>	<u>0.006</u>
<u>0.015</u>	<u>0.015</u>
<u>2.67</u>	<u>1.86</u>
<u>376</u>	<u>202</u>
<u>0.039</u> +	<u>0.030</u> = <u>0.069</u>
	<u>0.850</u>

$T_{LAG} = 0.60 (0.850) = 0.510 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

2 YEAR STORM  
SUB-BASIN A2

Circle one:  $T_c$   $T_c$  through subarea

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

Segment ID

1. Surface description (table 3-1) .....
2. Manning's roughness coeff., n (table 3-1) ..
3. Flow length, L (total L  $\leq$  300 ft) ..... ft
4. Two-yr 24-hr rainfall,  $P_2$  ..... in
5. Land slope, s ..... ft/ft
6.  $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

<u>6-H</u>	
<u>RES. LOT</u>	
<u>0.35</u>	
<u>115</u>	
<u>0.70</u>	
<u>0.030</u>	
<u>0.654</u> +	

0.654

Shallow concentrated flow

Segment ID

7. Surface description (paved or unpaved) .....
8. Flow length, L ..... ft
9. Watercourse slope, s ..... ft/ft
10. Average velocity, V (figure ~~20~~) ..... ft/s
11.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr

<u>H-I</u>	<u>I-II</u>
<u>PAVED ROAD</u>	
<u>514</u>	<u>244</u>
<u>0.010</u>	<u>0.0075</u>
<u>2.40</u>	<u>2.08</u>
<u>0.059</u> +	<u>0.033</u> -

0.092

Channel flow

Segment ID

12. Cross sectional flow area, a ..... ft<sup>2</sup>
13. Wetted perimeter,  $p_w$  ..... ft
14. Hydraulic radius,  $r = \frac{a}{p_w}$  Compute r ..... ft
15. Channel slope, s ..... ft/ft
16. Manning's roughness coeff., n .....
17.  $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
18. Flow length, L ..... ft
19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

<u>I-II</u>	
<u>SEE</u>	
<u>CALL.</u>	
<u>SHEET FOR STREETS</u>	
<u>0.018</u>	
<u>0.015</u>	
<u>2.61</u>	
<u>90</u>	
<u>0.010</u> +	

0.010  
0.756

$T_{LAG} = 0.60 (0.756) = 0.454 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

Circle one:  $T_c$   $T_c$  through subarea

100 YEAR STORM  
SUB-BASIN A2

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

- Segment ID
1. Surface description (table 3-1) .....
  2. Manning's roughness coeff., n (table 3-1) ..
  3. Flow length, L (total L  $\leq$  300 ft) ..... ft
  4. ~~100~~ yr 24-hr rainfall, P ..... in  
100
  5. Land slope, s ..... ft/ft
  6.  $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

<u>G-H</u>	
<u>RES LOT</u>	
<u>0.35</u>	
<u>115</u>	
<u>2.01</u>	
<u>0.030</u>	
<u>0.386</u> +	

0.386

Shallow concentrated flow

- Segment ID
7. Surface description (paved or unpaved) .....
  8. Flow length, L ..... ft
  9. Watercourse slope, s ..... ft/ft
  10. Average velocity, V (figure ~~20~~) ..... ft/s  
402
  11.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr

<u>H-I</u>	<u>I-J</u>
<u>PAVED ROAD</u>	
<u>514</u>	<u>244</u>
<u>0.010</u>	<u>0.0075</u>
<u>2.40</u>	<u>2.08</u>
<u>0.059</u> +	<u>0.033</u> -

0.092

Channel flow

- Segment ID
12. Cross sectional flow area, a ..... ft<sup>2</sup>
  13. Wetted perimeter,  $P_w$  ..... ft
  14. Hydraulic radius,  $r = \frac{a}{P_w}$  Compute r ..... ft
  15. Channel slope, s ..... ft/ft
  16. Manning's roughness coeff., n .....
  17.  $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
  18. Flow length, L ..... ft
  19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
  20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

<u>I-J2</u>	
<u>SEE</u>	
<u>CALL</u>	
<u>SHEETS FOR STREETS</u>	
<u>0.0118</u>	
<u>0.015</u>	
<u>2.61</u>	
<u>90</u>	
<u>0.010</u> +	

0.010

0.488

$T_{LAG} = 0.60 (0.488) = 0.293 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 2 YEAR STORM  
 Circle one:  $T_c$   $T_t$  through subarea SUB-BASIN A3

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to $T_c$ only)	Segment ID
1. Surface description (table 3-1) .....	J1-J2 RES. Lot
2. Manning's roughness coeff., n (table 3-1) ..	0.35
3. Flow length, L (total L $\leq$ 300 ft) ..... ft	100
4. Two-yr 24-hr rainfall, $P_2$ ..... in	0.70
5. Land slope, s ..... ft/ft	0.02
6. $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ ..... hr	0.688 + [ ] = 0.688

Shallow concentrated flow	Segment ID
7. Surface description (paved or unpaved) .....	J2-K K-L PAVED Road
8. Flow length, L ..... ft	352 439
9. Watercourse slope, s ..... ft/ft	0.0118 0.0077
10. Average velocity, V (figure <del>20</del> <u>402</u> ) ..... ft/s	2.61 2.11
11. $T_t = \frac{L}{3600 V}$ Compute $T_t$ ..... hr	0.037 + 0.058 = 0.095

Channel flow	Segment ID
12. Cross sectional flow area, a ..... ft <sup>2</sup>	L-L1 SEE
13. Wetted perimeter, $P_w$ ..... ft	CALL
14. Hydraulic radius, $r = \frac{a}{P_w}$ Compute r ..... ft	SHEET FOR STREETS
15. Channel slope, s ..... ft/ft	0.0060
16. Manning's roughness coeff., n .....	0.015
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V ..... ft/s	1.86
18. Flow length, L ..... ft	175
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ ..... hr	0.026 + [ ] = 0.026
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) ..... hr	0.809

$T_{LAG} = 0.60 (0.809) = 0.485 \text{ HOURS}$



Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_t$  through subarea SUB-BASIN A3

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

- Segment ID
1. Surface description (table 3-1) .....
  2. Manning's roughness coeff., n (table 3-1) ..
  3. Flow length, L (total L  $\leq$  300 ft) ..... ft
  4. ~~Two~~ 100 yr 24-hr rainfall, P<sub>2</sub> ..... in
  5. Land slope, s ..... ft/ft
  6.  $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

J1-J2	
Res Lot	
0.35	
100	
2.01	
0.02	
0.406	+
- 0.406	

Shallow concentrated flow

- Segment ID
7. Surface description (paved or unpaved) .....
  8. Flow length, L ..... ft
  9. Watercourse slope, s ..... ft/ft
  10. Average velocity, V (figure ~~20~~ 402) ..... ft/s
  11.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr

J2-K	K-L
PAVED ROAD	
352	439
0.0118	0.0077
2.61	2.11
0.037	+
0.058	-
0.095	

Channel flow

- Segment ID
12. Cross sectional flow area, a ..... ft<sup>2</sup>
  13. Wetted perimeter, p<sub>w</sub> ..... ft
  14. Hydraulic radius, r =  $\frac{a}{p_w}$  Compute r ..... ft
  15. Channel slope, s ..... ft/ft
  16. Manning's roughness coeff., n .....
  17.  $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
  18. Flow length, L ..... ft
  19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
  20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

L-L1	
SEEL	
BALL	
SHEETS FOR STREETS	
0.0060	
0.015	
1.86	
175	
0.024	+
- 0.026	
0.527	

$T_{LAG} = 0.60 (0.527) = 0.316 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

2 YEAR STORM  
SUB-BASIN B1, B2 & B5

Circle one:  $T_c$   $T_c$  through subarea

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

<u>Sheet flow</u> (Applicable to $T_c$ only)	Segment ID	
1. Surface description (table 3-1) .....		M-N
2. Manning's roughness coeff., n (table 3-1) ..		TURF AREA
3. Flow length, L (total L $\leq$ 300 ft) ..... ft		0.35
4. Two-yr 24-hr rainfall, $P_2$ ..... in		20
5. Land slope, s ..... ft/ft		0.70
6. $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ ..... hr		0.33
		0.062 + _____ = 0.062

<u>Shallow concentrated flow</u>	Segment ID	
7. Surface description (paved or unpaved) .....		N/A
8. Flow length, L ..... ft		
9. Watercourse slope, s ..... ft/ft		
10. Average velocity, V (figure <del>30</del> ) ..... ft/s		
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ ..... hr		402
		_____ + _____ = _____

<u>Channel flow</u>	Segment ID	
12. Cross sectional flow area, a ..... ft <sup>2</sup>		N-O
13. Wetted perimeter, $p_w$ ..... ft		SEE
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r ..... ft		CALL
15. Channel slope, s ..... ft/ft		SHEET
16. Manning's roughness coeff., n .....		0.0051
17. $v = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V ..... ft/s		0.030
18. Flow length, L ..... ft		2.39
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ ..... hr		305
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) ..... hr		0.036 + _____ = 0.036
		0.098

$T_{LAG} = 0.60 (0.098) = 0.059 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

100 YEAR STORM  
SUB-BASIN B1, B2 & B5

Circle one:  $T_c$   $T_c$  through subarea

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

- Segment ID
1. Surface description (table 3-1) .....
  2. Manning's roughness coeff., n (table 3-1) ..
  3. Flow length, L (total L  $\leq$  300 ft) ..... ft
  4. ~~100~~ yr 24-hr rainfall, P / 100 ..... in
  5. Land slope, s ..... ft/ft
  6.  $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

<u>M-N</u>	
<u>TURF AREA</u>	
<u>0.35</u>	
<u>20</u>	
<u>2.01</u>	
<u>0.33</u>	
<u>0.037</u> +	<u>0.037</u>

Shallow concentrated flow

- Segment ID
7. Surface description (paved or unpaved) .....
  8. Flow length, L ..... ft
  9. Watercourse slope, s ..... ft/ft
  10. Average velocity, V (figure ~~30~~ 402) ..... ft/s
  11.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr

<u>N/A</u>	

Channel flow

- Segment ID
12. Cross sectional flow area, a ..... ft<sup>2</sup>
  13. Wetted perimeter,  $p_w$  ..... ft
  14. Hydraulic radius,  $r = \frac{a}{p_w}$  Compute r ..... ft
  15. Channel slope, s ..... ft/ft
  16. Manning's roughness coeff., n .....
  17.  $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
  18. Flow length, L ..... ft
  19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
  20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

<u>N-0</u>	
<u>SEE</u>	
<u>CALL</u>	
<u>SHEET</u>	
<u>0.0051</u>	
<u>0.030</u>	
<u>3.43</u>	
<u>305</u>	
<u>0.025</u> +	<u>0.025</u>
	<u>0.062</u>

$T_{LAG} = 0.60 (0.062) = 0.037 \text{ HOURS}$

Trapezoidal Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: 28 ROAD

Comment: DETENTION CHANNEL ALONG 28 ROAD - 2 YEAR

Solve For Depth

Given Input Data:

Bottom Width.....	2.00 ft
Left Side Slope..	3.00:1 (H:V)
Right Side Slope.	3.00:1 (H:V)
Manning's n.....	0.030
Channel Slope....	0.0051 ft/ft
Discharge.....	10.32 cfs <i>2 0.20 CFS/AC</i>

Computed Results:

Depth.....	0.91 ft
Velocity.....	2.39 fps - <i>INITIAL ESTIMATE</i>
Flow Area.....	4.32 sf
Flow Top Width...	7.47 ft
Wetted Perimeter.	7.77 ft
Critical Depth...	0.67 ft
Critical Slope...	0.0180 ft/ft
Froude Number....	0.55 (flow is Subcritical)

Trapezoidal Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: 28 ROAD

Comment: DETENTION CHANNEL ALONG 28 ROAD - 100 YEAR

Solve For Depth

Given Input Data:

Bottom Width.....	2.00 ft
Left Side Slope..	3.00:1 (H:V)
Right Side Slope.	3.00:1 (H:V)
Manning's n.....	0.030
Channel Slope....	0.0051 ft/ft
Discharge.....	42.00 cfs <i>20.80 cfs/ft.</i>

Computed Results:

Depth.....	1.71 ft
Velocity.....	<u>3.43 fps</u> <i>INITIAL EST.</i>
Flow Area.....	12.25 sf
Flow Top Width...	12.29 ft
Wetted Perimeter.	12.85 ft
Critical Depth...	1.35 ft
Critical Slope...	0.0149 ft/ft
Froude Number....	0.60 (flow is Subcritical)

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 2 YEAR STORM  
 Circle one:  $T_c$   $T_t$  through subarea SUB-BASIN B3

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

Segment ID

1. Surface description (table 3-1) .....
2. Manning's roughness coeff., n (table 3-1) ...
3. Flow length, L (total L  $\leq$  300 ft) ..... ft
4. Two-yr 24-hr rainfall,  $P_2$  ..... in
5. Land slope, s ..... ft/ft
6.  $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

<u>S-T</u>	
<u>RES. LOT</u>	
<u>0.35</u>	
<u>55</u>	
<u>0.70</u>	
<u>0.020</u>	
<u>0.426</u> +	

0.426

Shallow concentrated flow

Segment ID

7. Surface description (paved or unpaved) .....
8. Flow length, L ..... ft
9. Watercourse slope, s ..... ft/ft
10. Average velocity, V (figure 20) ..... ft/s
11.  $T_t = \frac{L}{3600 V}$  Compute  $T_t$  ..... hr

<u>T-U</u>	
<u>PAVED ROAD</u>	
<u>600</u>	
<u>0.0081</u>	
<u>2.16</u>	
<u>0.077</u> +	

0.077

Channel flow

Segment ID

12. Cross sectional flow area, a ..... ft<sup>2</sup>
13. Wetted perimeter,  $P_w$  ..... ft
14. Hydraulic radius,  $r = \frac{a}{P_w}$  Compute r ..... ft
15. Channel slope, s ..... ft/ft
16. Manning's roughness coeff., n .....
17.  $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
18. Flow length, L ..... ft
19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

<u>U-V</u>	
<u>SEE</u>	
<u>CALL</u>	
<u>SHEET FOR STREETS</u>	
<u>0.0065</u>	
<u>0.015</u>	
<u>1.94</u>	
<u>264</u>	
<u>0.038</u> +	

0.038

0.541

$T_{LAG} = 0.60 (0.541) = 0.325 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN B3

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

- Segment ID
1. Surface description (table 3-1) .....
  2. Manning's roughness coeff., n (table 3-1) ..
  3. Flow length, L (total L  $\leq$  300 ft) ..... ft
  4. ~~100~~ 24-hr rainfall,  $P_x$  ..... in  
100
  5. Land slope, s ..... ft/ft
  6.  $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

S-T	
RES. LOT	
0.35	
55	
2.01	
0.020	
0.252 <sup>+</sup>	0.252

Shallow concentrated flow

- Segment ID
7. Surface description (paved or unpaved) .....
  8. Flow length, L ..... ft
  9. Watercourse slope, s ..... ft/ft
  10. Average velocity, V (figure ~~20~~ 402) ..... ft/s
  11.  $T_t = \frac{L}{3600 V}$  Compute  $T_t$  ..... hr

T-U	
PAVED ROAD	
600	
0.0081	
2.16	
0.077 <sup>+</sup>	0.077

Channel flow

- Segment ID
12. Cross sectional flow area, a ..... ft<sup>2</sup>
  13. Wetted perimeter,  $p_w$  ..... ft
  14. Hydraulic radius,  $r = \frac{a}{p_w}$  Compute r ..... ft
  15. Channel slope, s ..... ft/ft
  16. Manning's roughness coeff., n .....
  17.  $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
  18. Flow length, L ..... ft
  19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
  20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

L-V	
SEE	
CALL.	
SHULT FOR STREETS	
0.0065	
0.015	
1.94	
264	
0.038 <sup>+</sup>	0.038

0.367

$T_{LAG} = 0.60 (0.367) = 0.220 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed  
 Circle one:  $T_c$   $T_t$  through subarea

2 YEAR STORM  
SUB-BASIN B4

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

<u>Sheet flow</u> (Applicable to $T_c$ only)	Segment ID	
1. Surface description (table 3-1) .....		P-Q
2. Manning's roughness coeff., n (table 3-1) ..		RES. LOT
3. Flow length, L (total L $\leq$ 300 ft) ..... ft		0.35
4. Two-yr 24-hr rainfall, $P_2$ ..... in		110
5. Land slope, s ..... ft/ft		0.70
6. $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ ..... hr		0.020
		0.742 + <span style="border: 1px solid black; padding: 2px;">0.742</span>
<u>Shallow concentrated flow</u>	Segment ID	
7. Surface description (paved or unpaved) .....		N/A
8. Flow length, L ..... ft		
9. Watercourse slope, s ..... ft/ft		
10. Average velocity, V (figure <del>30</del> ) ..... ft/s		
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ ..... hr		
<u>Channel flow</u>	Segment ID	
12. Cross sectional flow area, a ..... ft <sup>2</sup>		Q-R
13. Wetted perimeter, $P_w$ ..... ft		SEE
14. Hydraulic radius, $r = \frac{a}{P_w}$ Compute r ..... ft		CALL.
15. Channel slope, s ..... ft/ft		SHEET FOIL STREETS
16. Manning's roughness coeff., n .....		0.0086
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V ..... ft/s		0.015
18. Flow length, L ..... ft		2.23
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ ..... hr		801
		0.100 + <span style="border: 1px solid black; padding: 2px;">0.100</span>
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) ..... hr		0.842

$T_{LAG} = 0.60 (0.842) = 0.505 \text{ HOURS}$



Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN BA

NOTES: Space for as many as two segments per flow type can be used for each worksheet.  
 Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to $T_c$ only)	Segment ID	
1. Surface description (table 3-1) .....	P-Q	
2. Manning's roughness coeff., n (table 3-1) ..	Res. Lot	
3. Flow length, L (total L $\leq$ 300 ft) .....	0.35	
4. <del>100</del> 24-hr rainfall, P <del>/</del> <u>100</u> .....	110	ft
5. Land slope, s .....	2.01	in
6. $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$ Compute $T_c$ .....	0.020	ft/ft
	0.438 +	<b>0.438</b>

Shallow concentrated flow	Segment ID	
7. Surface description (paved or unpaved) .....	N/A	
8. Flow length, L .....		ft
9. Watercourse slope, s .....		ft/ft
10. Average velocity, V (figure <del>20</del> <u>402</u> ) .....		ft/s
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....		hr

Channel flow	Segment ID	
12. Cross sectional flow area, a .....	Q-R	
13. Wetted perimeter, $p_w$ .....	SEE	
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	CALL.	
15. Channel slope, s .....	SHEETS FOR STREETS	
16. Manning's roughness coeff., n .....	0.0086	ft/ft
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	0.015	ft/s
18. Flow length, L .....	2.23	ft
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	801	ft
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....	0.100 +	<b>0.100</b>
		<b>0.538</b>

$TLAG = 0.60 (0.538) = 0.323 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 2 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN C1

NOTES: Space for as many as two segments per flow type can be used for each worksheet.  
 Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to $T_c$ only)	Segment ID	
1. Surface description (table 3-1) .....	W-X	
2. Manning's roughness coeff., n (table 3-1) ..	Res. Lot	
3. Flow length, L (total L $\leq$ 300 ft) .....	0.35	
4. Two-yr 24-hr rainfall, $P_2$ .....	160	ft
5. Land slope, s .....	0.70	in
6. $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ .....	0.020	ft/ft
	1.002 +	1.002

Shallow concentrated flow	Segment ID	
7. Surface description (paved or unpaved) .....	X-Y	Y-Z
8. Flow length, L .....	Paved Road	
9. Watercourse slope, s .....	375	786
10. Average velocity, V (figure <del>20</del> <u>402</u> ) .....	0.0105	0.0070
11. $T_t = \frac{L}{3600 V}$ Compute $T_c$ .....	2.46	2.01
	0.042 +	0.109 = 0.151

Channel flow	Segment ID	
12. Cross sectional flow area, a .....	Z-AA	
13. Wetted perimeter, $p_w$ .....	SEE	
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	CALL	
15. Channel slope, s .....	SHEETS FOR STREETS	
16. Manning's roughness coeff., n .....	0.0063	ft/ft
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	0.015	
18. Flow length, L .....	1.94	ft/s
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	312	ft
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....	0.045 +	0.045
		1.198

$T_{LAG} = 0.60 (1.198) = 0.719 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN C1

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

- Segment ID
1. Surface description (table 3-1) .....
  2. Manning's roughness coeff., n (table 3-1) ..
  3. Flow length, L (total L  $\leq$  300 ft) ..... ft
  4. ~~The~~ 100 yr 24-hr rainfall, P<sub>2</sub> ..... in  
~~100~~
  5. Land slope, s ..... ft/ft
  6.  $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr  
~~100~~

W-X	
Res. Lot	
0.35	
160	
2.01	
0.020	
0.591	+ [ ] - 0.591

Shallow concentrated flow

- Segment ID
7. Surface description (paved or unpaved) .....
  8. Flow length, L ..... ft
  9. Watercourse slope, s ..... ft/ft
  10. Average velocity, V (figure ~~X~~) ..... ft/s  
~~402~~
  11.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr

X-Y	Y-Z
Paved Road	
375	786
0.0105	0.0070
2.46	2.01
0.042	+ 0.109 - 0.151

Channel flow

- Segment ID
12. Cross sectional flow area, a ..... ft<sup>2</sup>
  13. Wetted perimeter, p<sub>w</sub> ..... ft
  14. Hydraulic radius,  $r = \frac{a}{p_w}$  Compute r ..... ft
  15. Channel slope, s ..... ft/ft
  16. Manning's roughness coeff., n .....
  17.  $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
  18. Flow length, L ..... ft
  19.  $T_c = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
  20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

Z-AA	
SEE	
CALL	
SHEETS FOR STREETS	
0.0063	
0.015	
1.94	
312	
0.045	+ [ ] - 0.045
0.787	

$T_{LAG} = 0.60 (0.787) = 0.472 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

2 YEAR STORM  
SUB-BASIN C2

Circle one:  $T_c$   $T_c$  through subarea

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

<u>Sheet flow</u> (Applicable to $T_c$ only)	Segment ID	
1. Surface description (table 3-1) .....	AB-AC	RES. LOT
2. Manning's roughness coeff., n (table 3-1) ..		0.25
3. Flow length, L (total L $\leq$ 300 ft) .....		230 ft
4. Two-yr 24-hr rainfall, $P_2$ .....		0.70 in
5. Land slope, s .....		0.0128 ft/ft
6. $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ .....		1.600 + _____ = 1.600 hr

<u>Shallow concentrated flow</u>	Segment ID	
7. Surface description (paved or unpaved) .....	AC-AD	Paved Road
8. Flow length, L .....		110 ft
9. Watercourse slope, s .....		0.025 ft/ft
10. Average velocity, V (figure <del>20</del> ) .....		3.61 ft/s
11. $T_t = \frac{L}{3600 V}$ Compute $T_t$ .....		0.009 + _____ = 0.009 hr

<u>Channel flow</u>	Segment ID	
12. Cross sectional flow area, a .....	AD-AE	SEE
13. Wetted perimeter, $p_w$ .....		CALL.
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....		SHEETS FOR STREETS
15. Channel slope, s .....		0.0061 ft/ft
16. Manning's roughness coeff., n .....		0.015
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....		1.94 ft/s
18. Flow length, L .....		410 ft
19. $T_t = \frac{L}{3600 V}$ Compute $T_t$ .....		0.059 + _____ = 0.059 hr
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....		1.668 hr

$T_{LAG} = 0.60 (1.668) = 1.000 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN C2

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

	Segment ID	
1. Surface description (table 3-1) .....	AB-AC	RES LOT
2. Manning's roughness coeff., n (table 3-1) ..		0.35
3. Flow length, L (total L $\leq$ 300 ft) .....		230
4. <del>Two</del> <sup>100</sup> yr 24-hr rainfall, $P_2$ .....		2.01
5. Land slope, s .....		0.0128
6. $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$ Compute $T_c$ .....		0.945 + <span style="border: 1px solid black; padding: 2px;">0.945</span>

Shallow concentrated flow

	Segment ID	
7. Surface description (paved or unpaved) .....	AC-AD	PAVED ROAD
8. Flow length, L .....		110
9. Watercourse slope, s .....		0.025
10. Average velocity, V (figure <del>20</del> <sup>402</sup> ) .....		3.61
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....		0.009 + <span style="border: 1px solid black; padding: 2px;">0.009</span>

Channel flow

	Segment ID	
12. Cross sectional flow area, a .....	AD-AE	SEE
13. Wetted perimeter, $p_w$ .....		CALL.
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....		SHEETS FOR STREETS
15. Channel slope, s .....		0.0064
16. Manning's roughness coeff., n .....		0.015
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....		1.94
18. Flow length, L .....		410
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....		0.059 + <span style="border: 1px solid black; padding: 2px;">0.059</span>
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....		<span style="border: 1px solid black; padding: 2px;">1.013</span>

$T_{LAG} = 0.60 (1.013) = 0.608 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 2 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN C3

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

	Segment ID		
1. Surface description (table 3-1) .....		AI-AJ	
2. Manning's roughness coeff., n (table 3-1) ..		Res. Lot	
3. Flow length, L (total L $\leq$ 300 ft) .....	ft	0.35	
4. Two-yr 24-hr rainfall, $P_2$ .....	in	50	
5. Land slope, s .....	ft/ft	0.70	
6. $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ .....	hr	0.020	
		0.395 +	0.395

Shallow concentrated flow

	Segment ID		
7. Surface description (paved or unpaved) .....		N/A	
8. Flow length, L .....	ft		
9. Watercourse slope, s .....	ft/ft		
10. Average velocity, V (figure <del>30</del> <u>402</u> ) .....	ft/s		
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	hr		

Channel flow

	Segment ID		
12. Cross sectional flow area, a .....	ft <sup>2</sup>	AI-AK	
13. Wetted perimeter, $P_w$ .....	ft	SEE	
14. Hydraulic radius, $r = \frac{a}{P_w}$ Compute r .....	ft	CALL	
15. Channel slope, s .....	ft/ft	SHEETS FOR STREETS	
16. Manning's roughness coeff., n .....		0.007	
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	ft/s	0.015	
18. Flow length, L .....	ft	2.01	
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	hr	180	
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....	hr	0.025 +	0.025
			0.420

$T_{LAG} = 0.60 (0.420) = 0.252 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94  
 Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN C3

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

<u>Sheet flow</u> (Applicable to $T_c$ only)	Segment ID	AI-AJ
1. Surface description (table 3-1) .....		Res. Lot
2. Manning's roughness coeff., n (table 3-1) ..		0.35
3. Flow length, L (total L < 300 ft) .....	ft	50
4. <del>100</del> yr 24-hr rainfall, P/100 .....	in	2.01
5. Land slope, s .....	ft/ft	0.020
6. $T_t = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$ Compute $T_t$ .....	hr	0.233 + <span style="border: 1px solid black; display: inline-block; width: 40px; height: 15px;"></span> = <span style="border: 1px solid black; padding: 2px;">0.233</span>

<u>Shallow concentrated flow</u>	Segment ID	N/A
7. Surface description (paved or unpaved) .....		
8. Flow length, L .....	ft	
9. Watercourse slope, s .....	ft/ft	
10. Average velocity, V (figure <del>402</del> ) .....	ft/s	
11. $T_t = \frac{L}{3600 V}$ Compute $T_t$ .....	hr	<span style="border: 1px solid black; display: inline-block; width: 40px; height: 15px;"></span> + <span style="border: 1px solid black; display: inline-block; width: 40px; height: 15px;"></span> = <span style="border: 1px solid black; display: inline-block; width: 40px; height: 15px;"></span>

<u>Channel flow</u>	Segment ID	AI-AK
12. Cross sectional flow area, a .....	ft <sup>2</sup>	SEE
13. Wetted perimeter, $p_w$ .....	ft	CALL
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	ft	SHEETS FOR STREETS
15. Channel slope, s .....	ft/ft	0.007
16. Manning's roughness coeff., n .....		0.015
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	ft/s	2.01
18. Flow length, L .....	ft	180
19. $T_t = \frac{L}{3600 V}$ Compute $T_t$ .....	hr	0.025 + <span style="border: 1px solid black; display: inline-block; width: 40px; height: 15px;"></span> = <span style="border: 1px solid black; padding: 2px;">0.025</span>
20. Watershed or subarea $T_c$ or $T_t$ (add $T_t$ in steps 6, 11, and 19) .....	hr	<span style="border: 1px solid black; display: inline-block; width: 40px; height: 15px;"></span> = <span style="border: 1px solid black; padding: 2px;">0.258</span>

$T_{LAG} = 0.60 (0.258) = 0.155 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 2 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN D1

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

- |   |            |  |
|---|------------|--|
| 1. Surface description (table 3-1) .....                    | Segment ID |  |
| 2. Manning's roughness coeff., n (table 3-1) ..             |            |  |
| 3. Flow length, L (total L $\leq$ 300 ft) .....             | ft         |  |
| 4. Two-yr 24-hr rainfall, $P_2$ .....                       | in         |  |
| 5. Land slope, s .....                                      | ft/ft      |  |
| 6. $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ ..... | hr         |  |

<u>AF-AG</u>	
<u>Res. Lot</u>	
<u>0.35</u>	
<u>50</u>	
<u>0.70</u>	
<u>0.020</u>	
<u>0.395</u> +	<u>0.395</u>

Shallow concentrated flow

- |  |            |  |
|--|------------|--|
| 7. Surface description (paved or unpaved) .....                  | Segment ID |  |
| 8. Flow length, L .....  | ft         |  |
| 9. Watercourse slope, s .....                                    | ft/ft      |  |
| 10. Average velocity, V (figure <del>20</del> <u>402</u> ) ..... | ft/s       |  |
| 11. $T_t = \frac{L}{3600 V}$ .....                               | hr         |  |

<u>N/A</u>	

Channel flow

- |  |            |  |
|--|------------|--|
| 12. Cross sectional flow area, a .....   | Segment ID |  |
| 13. Wetted perimeter, $p_w$ .....  | ft         |  |
| 14. Hydraulic radius, $r = \frac{a}{p_w}$ .....                                  | ft         |  |
| 15. Channel slope, s .....   | ft/ft      |  |
| 16. Manning's roughness coeff., n .....  |            |  |
| 17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ .....                                   | ft/s       |  |
| 18. Flow length, L .....   | ft         |  |
| 19. $T_t = \frac{L}{3600 V}$ .....   | hr         |  |
| 20. Watershed or subarea $T_c$ or $T_t$ (add $T_t$ in steps 6, 11, and 19) ..... | hr         |  |

<u>AG-AH</u>	
<u>SEE</u>	
<u>CALL</u>	
<u>SHEET FOR STREETS</u>	
<u>0.0094</u>	
<u>0.015</u>	
<u>2.33</u>	
<u>687</u>	
<u>0.082</u> +	<u>0.082</u>
	<u>0.477</u>

$T_{LAG} = 0.60 (0.477) = 0.286 \text{ HOURS}$



Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN D1

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

<u>Sheet flow</u> (Applicable to $T_c$ only)	Segment ID		
1. Surface description (table 3-1) .....		AF-AG	
2. Manning's roughness coeff., n (table 3-1) ..		RES. LOT	
3. Flow length, L (total L $\leq$ 300 ft) .....	ft	0.35	
4. <del>100</del> 24-hr rainfall, P, <del>100</del> .....	in	50	
5. Land slope, s .....	ft/ft	2.01	
6. $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$ Compute $T_c$ .....	hr	0.020	
		0.233 +	0.233

<u>Shallow concentrated flow</u>	Segment ID		
7. Surface description (paved or unpaved) .....		N/A	
8. Flow length, L .....	ft		
9. Watercourse slope, s .....	ft/ft		
10. Average velocity, V (figure <del>30</del> <u>402</u> ) .....	ft/s		
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	hr		

<u>Channel flow</u>	Segment ID		
12. Cross sectional flow area, a .....	ft <sup>2</sup>	AG-AH	
13. Wetted perimeter, $p_w$ .....	ft	SEE	
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	ft	CALL.	
15. Channel slope, s .....	ft/ft	SHEETS FOR STREETS	
16. Manning's roughness coeff., n .....		0.0004	
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	ft/s	0.015	
18. Flow length, L .....	ft	2.33	
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	hr	687	
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....	hr	0.082 +	0.082
			0.315

$T_{LAG} = 0.60 (0.315) = 0.189 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 2 YEAR STORM  
 Circle one:  $T_c$   $T_t$  through subarea SUB-BASIN DZ

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

Segment ID

1. Surface description (table 3-1) .....
2. Manning's roughness coeff., n (table 3-1) ..
3. Flow length, L (total L  $\leq$  300 ft) ..... ft
4. Two-yr 24-hr rainfall,  $P_2$  ..... in
5. Land slope, s ..... ft/ft
6.  $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$  Compute  $T_c$  ..... hr

<u>AL-AM</u>	
<u>RES. LOT</u>	
<u>0.35</u>	
<u>150</u>	
<u>0.70</u>	
<u>0.0167</u>	
<u>1.022</u>	+ [ ] - <u>1.022</u>

Shallow concentrated flow

Segment ID

7. Surface description (paved or unpaved) .....
8. Flow length, L ..... ft
9. Watercourse slope, s ..... ft/ft
10. Average velocity, V (figure ~~20~~ 402) ..... ft/s
11.  $T_t = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr

<u>AM-AN AN-AD</u>	
<u>PAVED ROAD</u>	
<u>189</u>	<u>869</u>
<u>0.0165</u>	<u>0.0078</u>
<u>3.08</u>	<u>2.11</u>
<u>0.017</u>	+ <u>0.114</u> - <u>0.131</u>

Channel flow

Segment ID

12. Cross sectional flow area, a ..... ft<sup>2</sup>
13. Wetted perimeter,  $p_w$  ..... ft
14. Hydraulic radius,  $r = \frac{a}{p_w}$  Compute r ..... ft
15. Channel slope, s ..... ft/ft
16. Manning's roughness coeff., n .....
17.  $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$  Compute V ..... ft/s
18. Flow length, L ..... ft
19.  $T_t = \frac{L}{3600 V}$  Compute  $T_c$  ..... hr
20. Watershed or subarea  $T_c$  or  $T_t$  (add  $T_c$  in steps 6, 11, and 19) ..... hr

<u>AD-AP</u>	
<u>SEE</u>	
<u>CALL</u>	
<u>SHEETS FOR STREETS</u>	
<u>0.0083</u>	
<u>0.015</u>	
<u>2.19</u>	
<u>220</u>	
<u>0.028</u>	+ [ ] - <u>0.028</u>
<u>1.181</u>	

$T_{LAG} = 0.60 (1.181) = 0.709 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN D2

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to $T_c$ only)	Segment ID
1. Surface description (table 3-1) .....	AL-AM
2. Manning's roughness coeff., n (table 3-1) ..	RES. LOT
3. Flow length, L (total L $\leq$ 300 ft) ..... ft	0.35
4. <del>Two</del> yr 24-hr rainfall, $P_z$ ..... in	150
5. Land slope; s ..... ft/ft	2.01
6. $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$ Compute $T_c$ ..... hr	0.0167
	0.603 + [ ] = 0.603

Shallow concentrated flow	Segment ID
7. Surface description (paved or unpaved) .....	AM-AN AN-AO
8. Flow length, L ..... ft	PAVED ROAD
9. Watercourse slope, s ..... ft/ft	189
10. Average velocity, V (figure <del>20</del> <u>402</u> ) ..... ft/s	0.0165
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ ..... hr	3.08
	0.017 + 0.114 = 0.131

Channel flow	Segment ID
12. Cross sectional flow area, a ..... ft <sup>2</sup>	AO-AP
13. Wetted perimeter, $p_w$ ..... ft	SEE
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r ..... ft	CALL.
15. Channel slope, s ..... ft/ft	SHEET FOR STREETS
16. Manning's roughness coeff., n .....	0.0083
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V ..... ft/s	0.015
18. Flow length, L ..... ft	2.19
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ ..... hr	220
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) ..... hr	0.028 + [ ] = 0.028
	0.762

$T_{LAG} = 0.60 (0.762) = 0.457 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 2 YEAR STORM  
 Circle one:  $T_c$   $T_t$  through subarea SUB-BASIN D3

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to $T_c$ only)	Segment ID	
1. Surface description (table 3-1) .....	AQ-AR	
2. Manning's roughness coeff., n (table 3-1) ..	RES LOT	
3. Flow length, L (total L $\leq$ 300 ft) .....	0.35	
4. Two-yr 24-hr rainfall, $P_2$ .....	50	ft
5. Land slope, s .....	2.01	in
6. $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ .....	0.020	ft/ft
	0.395 +	hr
		0.395

Shallow concentrated flow	Segment ID	
7. Surface description (paved or unpaved) .....	N/A	
8. Flow length, L .....		ft
9. Watercourse slope, s .....		ft/ft
10. Average velocity, V (figure <del>30</del> ) .....		ft/s
11. $T_t = \frac{L}{3600 V}$ Compute $T_t$ .....	402	hr
		0

Channel flow	Segment ID	
12. Cross sectional flow area, a .....	AR-AS	ft <sup>2</sup>
13. Wetted perimeter, $p_w$ .....	SEE	ft
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	CALL.	ft
15. Channel slope, s .....	SHEET FOR STREETS	ft/ft
16. Manning's roughness coeff., n .....	0.0083	ft/ft
17. $v = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	0.015	ft/s
18. Flow length, L .....	2.19	ft
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	330	hr
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....	0.042 +	hr
		0.042
		0.437

$T_{LAG} = 0.60 (0.437) = 0.262 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN D3

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

	Segment ID			
1. Surface description (table 3-1) .....	AQ-AR	RES LOT		
2. Manning's roughness coeff., n (table 3-1) ..		0.35		
3. Flow length, L (total L $\leq$ 300 ft) .....		50	ft	
4. <del>100</del> yr 24-hr rainfall, P <del>100</del> .....		2.01	in	
5. Land slope, s .....		0.020	ft/ft	
6. $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$ Compute $T_c$ .....		0.233	hr	0.233

Shallow concentrated flow

	Segment ID			
7. Surface description (paved or unpaved) .....	N/A			
8. Flow length, L .....			ft	
9. Watercourse slope, s .....			ft/ft	
10. Average velocity, V (figure <del>402</del> ) .....			ft/s	
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....			hr	<del>0</del>

Channel flow

	Segment ID			
12. Cross sectional flow area, a .....	AR-A5	SEE	ft <sup>2</sup>	
13. Wetted perimeter, $p_w$ .....		CALL.	ft	
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....		SHEETS FOR STREETS	ft	
15. Channel slope, s .....		0.0083	ft/ft	
16. Manning's roughness coeff., n .....		0.015		
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....		2.19	ft/s	
18. Flow length, L .....		330	ft	
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....		0.042	hr	0.042
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....			hr	0.275

$T_{LAG} = 0.60 (0.275) = 0.165 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed

2 YEAR STORM  
SUB-BASIN D4

Circle one:  $T_c$   $T_c$  through subarea

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to  $T_c$  only)

	Segment ID	
1. Surface description (table 3-1) .....	AT-AU	
2. Manning's roughness coeff., n (table 3-1) ..	TURF AREA	
3. Flow length, L (total L $\leq$ 300 ft) .....	0.35	
4. Two-yr 24-hr rainfall, $P_2$ .....	300	ft
5. Land slope, s .....	0.70	in
6. $T_t = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ .....	0.0323	ft/ft
	+ [ ]	= 1.367

Shallow concentrated flow

	Segment ID	
7. Surface description (paved or unpaved) .....	N/A	
8. Flow length, L .....		ft
9. Watercourse slope, s .....		ft/ft
10. Average velocity, V (figure <del>3-1</del> ) .....		ft/s
11. $T_t = \frac{L}{3600 V}$ Compute $T_c$ .....	402	hr
	+ [ ]	= <del>0</del>

Channel flow

	Segment ID	
12. Cross sectional flow area, a .....	N/A	ft <sup>2</sup>
13. Wetted perimeter, $p_w$ .....		ft
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....		ft
15. Channel slope, s .....		ft/ft
16. Manning's roughness coeff., n .....		
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....		ft/s
18. Flow length, L .....		ft
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....		hr
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....		hr
	+ [ ]	= 1.367

$T_{LAG} = 0.60 (1.367) = 0.820 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN D4

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

<u>Sheet flow</u> (Applicable to $T_c$ only)	Segment ID	
1. Surface description (table 3-1) .....		AT-A1
2. Manning's roughness coeff., n (table 3-1) ..		TURF AREA
3. Flow length, L (total L < 300 ft) .....	ft	0.35
4. <del>100</del> yr 24-hr rainfall, P <sub>2</sub> .....	in	300
5. Land slope, s .....	ft/ft	2.01
6. $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$ Compute $T_c$ .....	hr	0.0323
		0.807 + _____ = 0.807

<u>Shallow concentrated flow</u>	Segment ID	
7. Surface description (paved or unpaved) .....		
8. Flow length, L .....	ft	
9. Watercourse slope, s .....	ft/ft	
10. Average velocity, V (figure <del>30</del> <u>402</u> ) .....	ft/s	
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	hr	+ _____ = 0

<u>Channel flow</u>	Segment ID	
12. Cross sectional flow area, a .....	ft <sup>2</sup>	
13. Wetted perimeter, p <sub>w</sub> .....	ft	
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	ft	
15. Channel slope, s .....	ft/ft	
16. Manning's roughness coeff., n .....		
17. $v = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	ft/s	
18. Flow length, L .....	ft	
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	hr	+ _____ = 0
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....	hr	0.807

$T_{LAG} = 0.60 (0.807) = 0.484 \text{ HOURS}$

Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 2 YEAR STORM  
 Circle one:  $T_c$   $T_t$  through subarea SUB-BASIN E1

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

<u>Sheet flow</u> (Applicable to $T_c$ only)	Segment ID	<u>AV-AW</u>	
1. Surface description (table 3-1) .....		GRASS	SLOPE
2. Manning's roughness coeff., n (table 3-1) ..		0.35	
3. Flow length, L (total L < 300 ft) ..... ft		20	
4. Two-yr 24-hr rainfall, $P_2$ ..... in		0.70	
5. Land slope, s ..... ft/ft		0.25	
6. $T_c = \frac{0.007 (nL)^{0.8}}{P_2^{0.5} s^{0.4}}$ Compute $T_c$ ..... hr		0.069	- 0.069

<u>Shallow concentrated flow</u>	Segment ID	<u>N/A</u>	
7. Surface description (paved or unpaved) .....			
8. Flow length, L ..... ft			
9. Watercourse slope, s ..... ft/ft			
10. Average velocity, V (figure <del>3-D</del> ) ..... ft/s			
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ ..... hr		402	- 0

<u>Channel flow</u>	Segment ID	<u>AW-AX</u>	
12. Cross sectional flow area, a ..... ft <sup>2</sup>		SEE	
13. Wetted perimeter, $P_w$ ..... ft		CALL	
14. Hydraulic radius, $r = \frac{a}{P_w}$ Compute r ..... ft		SHEET	
15. Channel slope, s ..... ft/ft		0.01	
16. Manning's roughness coeff., n .....		0.025	
17. $V = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V ..... ft/s		1.88	
18. Flow length, L ..... ft		1.180	
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ ..... hr		0.174	- 0.174
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) ..... hr			0.243

$T_{LAG} = 0.60 (0.243) = 0.146$  HOURS  
 CHECK COMPUTATION TIME INTERVAL  $0.29 * T_{LAG} = 2.54$  MINUTES  
 (210-VI-TR-55, Second Ed., June 1986) USE 2 MINUTES



Worksheet 3: Time of concentration ( $T_c$ ) or travel time ( $T_t$ )

Project GRAND VIEW SUB. By MS Date 06/94

Location GRAND JUNCTION Checked \_\_\_\_\_ Date \_\_\_\_\_

Circle one: Present Developed 100 YEAR STORM  
 Circle one:  $T_c$   $T_c$  through subarea SUB-BASIN E1

NOTES: Space for as many as two segments per flow type can be used for each worksheet.

Include a map, schematic, or description of flow segments.

Sheet flow (Applicable to $T_c$ only)	Segment ID	
1. Surface description (table 3-1) .....	AV-AW	
2. Manning's roughness coeff., n (table 3-1) ..	GRASS SLOPE	
3. Flow length, L (total L $\leq$ 300 ft) .....	0.35	
4. <del>100</del> yr 24-hr rainfall, P <sub>2</sub> <del>100</del> .....	20	
5. Land slope, s .....	2.01	
6. $T_c = \frac{0.007 (nL)^{0.8}}{P^{0.5} s^{0.4}}$ Compute $T_c$ .....	0.25	
	0.041 +	0.041

Shallow concentrated flow	Segment ID	
7. Surface description (paved or unpaved) .....	N/A	
8. Flow length, L .....		
9. Watercourse slope, s .....		
10. Average velocity, V (figure <del>30</del> <u>402</u> ) .....		
11. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....		
		0

Channel flow	Segment ID	
12. Cross sectional flow area, a .....	AW-AX	
13. Wetted perimeter, p <sub>w</sub> .....	SEE	
14. Hydraulic radius, $r = \frac{a}{p_w}$ Compute r .....	CALL.	
15. Channel slope, s .....	SHEET	
16. Manning's roughness coeff., n .....	0.01	
17. $v = \frac{1.49 r^{2/3} s^{1/2}}{n}$ Compute V .....	0.025	
18. Flow length, L .....	1.88	
19. $T_c = \frac{L}{3600 V}$ Compute $T_c$ .....	1.180	
20. Watershed or subarea $T_c$ or $T_t$ (add $T_c$ in steps 6, 11, and 19) .....	0.174 +	0.174
		0.215

$T_{LAG} = 0.60 (0.215) = 0.129 \text{ HOURS}$   
 CHECK COMPUTATION TIME INTERVAL  $0.29 * T_{LAG} = 2.25 \text{ MINUTES}$   
 (210-VI-TR-55, Second Ed., June 1986) USE 2 MINUTES

Trapezoidal Channel Analysis & Design  
Open Channel - Uniform flow

Worksheet Name: EXISITING DITCH

Comment: EXISTING DITCH ALONG SOUTH BOUNDARY LINE

Solve For Depth

Given Input Data:

Bottom Width.....	2.00 ft
Left Side Slope..	2.00:1 (H:V)
Right Side Slope.	2.00:1 (H:V)
Manning's n.....	0.025
Channel Slope....	0.0100 ft/ft
Discharge.....	1.00 cfs

Computed Results:

Depth.....	0.22 ft
Velocity.....	1.88 fps
Flow Area.....	0.53 sf
Flow Top Width...	2.87 ft
Wetted Perimeter.	2.97 ft
Critical Depth...	0.19 ft
Critical Slope...	0.0175 ft/ft
Froude Number....	0.77 (flow is Subcritical)

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
* RUN DATE 07/01/1994 TIME 16:35:56 *
*
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*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,  
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

DEVELOPED 2 YEAR STORM  
 SUB-BASINS "OF1, OF2, OF3, A and B"  
 RUN #3

R3-1

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1         ID  GRAND VIEW SUBDIVISION
2         ID  DEVELOPED CONDITION
3         ID  2 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)
4         IT   2 29JUN94   1200   300
5         IO   5   2     0
          * *****

6         KK   OF1
7         KM   Basin runoff calculation for   OF1
8         KO   3     1     0     1     21
9         BA   0.0175
10        PH           0   0.10   0.19   0.34   0.42   0.47   0.55   0.64   0.70
11        LS           91
12        UD   0.466
          * *****

13        KK   CH1
14        KM   Muskingum-Cunge channel routing from   CP1 to   CP3
15        KO   3     1     0     1     21
16        RD   760 0.0118  0.025           TRAP   2     3
          * *****

17        KK   A2
18        KM   Basin runoff calculation for   A2
19        KO   3     1     0     1     21
20        BA   0.0051
21        PH           0   0.10   0.19   0.34   0.42   0.47   0.55   0.64   0.70
22        LS           95
23        UD   0.454
          * *****

24        KK   A2
25        KM   Combining two hydrographs at control point   CP3
26        KO   3     1     0     1     21
27        HC   2
          * *****

28        KK   ST1
29        KM   Muskingum-Cunge channel routing from   CP3 to   CP4
30        KO   3     1     0     1     21
31        RD
32        RC   0.020  0.020  0.020   966  0.0084
33        RX   100   104   105   105  106.5  112.5  118.5  120.5
34        RY   34.86 34.78 34.53 34.40 34.53 34.65 34.77 34.81
          * *****

35        KK   A3
36        KM   Basin runoff calculation for   A3
37        KO   3     1     0     1     21
38        BA   0.0053
39        PH           0   0.10   0.19   0.34   0.42   0.47   0.55   0.64   0.70
40        LS           95
41        UD   0.485
          * *****
    
```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

42 KK A3
43 KM Combining two hydrographs at control point CP4
44 KO 3 1 0 1 21
45 HC 2
* *****

46 KK OF2
47 KM Basin runoff calculation for OF2
48 KO 3 1 0 1 21
49 BA 0.0187
50 PH 0 0.10 0.19 0.34 0.42 0.47 0.55 0.64 0.70
51 LS 91
52 UD 0.781
* *****

53 KK CH2
54 KM Muskingum-Cunge channel routing from CP2 to CP4
55 KO 3 1 0 1 21
56 RD 630 0.013 0.025 TRAP 2 3
* *****

57 KK OF3
58 KM Basin runoff calculation for OF3
59 KO 3 1 0 1 21
60 BA 0.0048
61 PH 0 0.10 0.19 0.34 0.42 0.47 0.55 0.64 0.70
62 LS 88
63 UD 0.799
* *****

64 KK OF3
65 KM Combining two hydrographs at control point CP4
66 KO 3 1 0 1 21
67 HC 2
* *****

68 KK OF3
69 KM Combining two hydrographs at control point CP4
70 KO 3 1 0 1 21
71 HC 2
* *****

72 KK A1
73 KM Basin runoff calculation for A1
74 KO 3 1 0 1 21
75 BA 0.0291
76 PH 0 0.10 0.19 0.34 0.42 0.47 0.55 0.64 0.70
77 LS 95
78 UD 0.763
* *****
    
```

R3-3

LINE	ID	1	2	3	4	5	6	7	8	9	10	
79	KK	A1										
80	KM	Combining two hydrographs at control point						CP4				
81	KO	3	1	0	1	21						
82	HC	2										
	* *****											
83	KK	P1										
84	KM	Muskingum-Cunge channel routing from						CP4 to	CP5			
85	KO	3	1	0	1	21						
86	RD	525	0.0060	0.015		CIRC	3.0					
	* *****											
87	KK	B1										
88	KM	Basin runoff calculation for						B1				
89	KO	3	1	0	1	21						
90	BA	0.0005										
91	PH		0	0.10	0.19	0.34	0.42	0.47	0.55	0.64	0.70	
92	LS		96									
93	UD	0.059										
	* *****											
94	KK	B1										
95	KM	Combining two hydrographs at control point						CP5				
96	KO	3	1	0	1	21						
97	HC	2										
	* *****											
98	KK	RES1										
99	KM	Reservoir routing operation										
100	KO	1	2	0	1	21						
101	RS	1	ELEV	18.6								
102	SV	0.0092	0.0613	0.1637	0.3107	0.4578						
103	SE	19.6	20.6	21.6	22.6	23.6						
104	SL	18.6	0.79	0.6	0.5							
105	SS	22.6	19.0	2.7	1.5							
	* *****											
106	KK	CH3										
107	KM	Muskingum-Cunge channel routing from						CP5 to	CP6			
108	KO	3	1	0	1	21						
109	RD	310	0.0052	0.025		TRAP	2	3				
	* *****											
110	KK	B2										
111	KM	Basin runoff calculation for						B2				
112	KO	3	1	0	1	21						
113	BA	0.0005										
114	PH		0	0.10	0.19	0.34	0.42	0.47	0.55	0.64	0.70	
115	LS		96									
116	UD	0.059										
	* *****											

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

117 KK B2  
 118 KM Combining two hydrographs at control point CP6  
 119 KO 3 1 0 1 21  
 120 HC 2  
 \* \*\*\*\*\*

121 KK RES2  
 122 KM Reservoir routing operation  
 123 KO 1 2 0 1 21  
 124 RS 1 ELEV 17.0  
 125 SV 0.0150 0.0768 0.184 0.2916  
 126 SE 18.0 19.0 20.0 21.0  
 127 SL 17.0 0.79 0.6 0.5  
 128 SS 20.0 19.0 2.7 1.5  
 \* \*\*\*\*\*

129 KK CH4  
 130 KM Muskingum-Cunge channel routing from CP6 to CP7  
 131 KO 3 1 0 1 21  
 132 RD 400 0.0079 0.025 TRAP 2 3  
 \* \*\*\*\*\*

133 KK B345  
 134 KM Basin runoff calculation for B345  
 135 KO 3 1 0 1 21  
 136 BA 0.0132  
 137 PH 0 0.10 0.19 0.34 0.42 0.47 0.55 0.64 0.70  
 138 LS 96  
 139 UD 0.505  
 \* \*\*\*\*\*

140 KK B345  
 141 KM Combining two hydrographs at control point CP7  
 142 KO 3 1 0 1 21  
 143 HC 2  
 \* \*\*\*\*\*

144 KK RES3  
 145 KM Reservoir routing operation  
 146 KO 1 2 0 1 21  
 147 RS 1 ELEV 12.83  
 148 SV 0.0817 0.2218 0.4160 0.6632  
 149 SE 14.2 15.2 16.2 17.2  
 150 SL 13.83 0.0625 0.6 0.5  
 151 SS 15.75 12.3 2.7 1.5  
 \* \*\*\*\*\*

152 ZZ

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* SEPTEMBER 1990
* VERSION 4.0
*
* RUN DATE 07/01/1994 TIME 16:35:56
*
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* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

```

GRAND VIEW SUBDIVISION  
DEVELOPED CONDITION  
2 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)

```

5 IO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      2 PLOT CONTROL
      OSCAL      0. HYDROGRAPH PLOT SCALE

```

```

IT HYDROGRAPH TIME DATA
      NMIN      2 MINUTES IN COMPUTATION INTERVAL
      IDATE     29JUN94 STARTING DATE
      ITIME     1200 STARTING TIME
      NO        300 NUMBER OF HYDROGRAPH ORDINATES
      NDDATE    29JUN94 ENDING DATE
      NDTIME    2158 ENDING TIME
      ICENT     19 CENTURY MARK

```

```

      COMPUTATION INTERVAL .03 HOURS
      TOTAL TIME BASE      9.97 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE       DEGREES FAHRENHEIT

```

\*\*\* \*\*

```

*****
*
* OF1
*
*****

```

```

8 KO OUTPUT CONTROL VARIABLES
      IPRNT      3 PRINT CONTROL
      IPLOT      1 PLOT CONTROL
      OSCAL      0. HYDROGRAPH PLOT SCALE
      IPNCH      1 PUNCH COMPUTED HYDROGRAPH
      IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2      300 LAST ORDINATE PUNCHED OR SAVED
      TIMINT     .033 TIME INTERVAL IN HOURS

```

SUBBASIN RUNOFF DATA

```

9 BA SUBBASIN CHARACTERISTICS
      TAREA      .02 SUBBASIN AREA

```

PRECIPITATION DATA

```

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
      HYDRO-35 TP-40 TP-49
      5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
      .10 .19 .34 .42 .47 .55 .64 .70 .00 .00 .00 .00

```

STORM AREA = .02

```

11 LS SCS LOSS RATE

```

R3-6



CRVNBRTIMP 91.00 CURVE NUMBER  
 .00 PERCENT IMPERVIOUS AREA

12 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .47 LAG

\*\*\*

UNIT HYDROGRAPH  
 72 END-OF-PERIOD ORDINATES

0.	1.	2.	3.	4.	6.	8.	10.	12.	14.
16.	17.	17.	17.	17.	17.	17.	16.	15.	14.
13.	12.	10.	9.	8.	7.	6.	5.	5.	4.
4.	4.	3.	3.	3.	2.	2.	2.	2.	1.
1.	1.	1.	1.	1.	1.	1.	1.	1.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.								

\*\*\* \*\*

HYDROGRAPH AT STATION OF1

TOTAL RAINFALL = .61, TOTAL LOSS = .49, TOTAL EXCESS = .12

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR (INCHES)	72-HR (AC-FT)	9.97-HR (CFS)
1.	5.60	0.	.119	.119	0.
		0.	0.	0.	.119
		0.	0.	0.	0.

CUMULATIVE AREA = .02 SQ MI

\*\*\* \*\*

\*\*\*\*\*  
 \* \*  
 \* CH1 \*  
 \* \*  
 \*\*\*\*\*

15 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

16 RD MUSKINGUM-CUNGE CHANNEL ROUTING  
 L 760. CHANNEL LENGTH  
 S .0118 SLOPE  
 N .025 CHANNEL ROUGHNESS COEFFICIENT  
 CA .00 CONTRIBUTING AREA  
 SHAPE TRAP CHANNEL SHAPE  
 WD 2.00 BOTTOM WIDTH OR DIAMETER  
 Z 3.00 SIDE SLOPE

\*\*\*

COMPUTED MUSKINGUM-CUNGE PARAMETERS

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	2.60	1.34	2.00	126.67	.90	342.00	.12	2.23

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	2.60	1.34	2.00		.90	342.00	.12	
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CONTINUITY SUMMARY (AC-FT) - INFLOW= .1113E+00 EXCESS= .0000E+00 OUTFLOW= .1101E+00 BASIN STORAGE= .1379E-02 PERCENT ERROR= -.1

R3-7

HYDROGRAPH AT STATION CH1

PEAK FLOW (CFS) 1. TIME (HR) 5.67  
 MAXIMUM AVERAGE FLOW  
 6-HR 0. 24-HR .118 72-HR .118 9.97-HR .118  
 (INCHES) .118 .118 .118 .118  
 (AC-FT) 0. 0. 0. 0.  
 CUMULATIVE AREA = .02 SQ MI

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 17 KK \* A2 \*  
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19 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

20 BA SUBBASIN CHARACTERISTICS  
 TAREA .01 SUBBASIN AREA

PRECIPITATION DATA

21 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
 HYDRO-35 TP-40 TP-49  
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
 .10 .19 .34 .42 .47 .55 .64 .70 .00 .00 .00 .00  
 STORM AREA = .01

22 LS SCS LOSS RATE  
 STRTL .11 INITIAL ABSTRACTION  
 CRVNR 95.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

23 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .45 LAG

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UNIT HYDROGRAPH  
 70 END-OF-PERIOD ORDINATES

0.	0.	1.	1.	1.	2.	2.	3.	4.	4.
5.	5.	5.	5.	5.	5.	5.	5.	4.	4.
4.	3.	3.	2.	2.	2.	2.	2.	1.	1.
1.	1.	1.	1.	1.	1.	1.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

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HYDROGRAPH AT STATION A2

TOTAL RAINFALL = .61, TOTAL LOSS = .36, TOTAL EXCESS = .25

PEAK FLOW (CFS) 1. TIME (HR) 5.50  
 MAXIMUM AVERAGE FLOW  
 6-HR 0. 24-HR .244 72-HR .244 9.97-HR .244  
 (INCHES) .244 .244 .244 .244  
 (AC-FT) 0. 0. 0. 0.  
 CUMULATIVE AREA = .01 SQ MI

R3-8

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 \* A2 \*  
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24 KK

26 KO

OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

27 HC

HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION A2

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	9.97-HR
1.	5.63	0.	0.	0.	0.
	(INCHES)	.146	.146	.146	.146
	(AC-FT)	0.	0.	0.	0.

CUMULATIVE AREA = .02 SQ MI

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 \* ST1 \*  
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28 KK

30 KO

OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

31 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

32 RC

NORMAL DEPTH CHANNEL  
 ANL .020 LEFT OVERBANK N-VALUE  
 ANCH .020 MAIN CHANNEL N-VALUE  
 ANR .020 RIGHT OVERBANK N-VALUE  
 RLNTH 966. REACH LENGTH  
 SEL .0084 ENERGY SLOPE  
 ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

	--- LEFT OVERBANK ---	+ ----- MAIN CHANNEL -----	+ --- RIGHT OVERBANK ---
34 RY ELEVATION	34.86	34.78 34.53 34.40 34.53 34.65	34.77 34.81
33 RX DISTANCE	100.00	104.00 105.00 105.00 106.50 112.50	118.50 120.50

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.00	.00	.00	.00	.00	.00	.01	.01
OUTFLOW	.00	.00	.01	.02	.05	.08	.12	.19	.32
ELEVATION	34.40	34.42	34.45	34.47	34.50	34.52	34.55	34.57	34.59
STORAGE	.01	.02	.02	.03	.04	.04	.05	.06	.07
OUTFLOW	.76	1.16	1.68	2.30	3.04	3.90	4.86	5.95	7.32
ELEVATION	34.64	34.67	34.69	34.71	34.74	34.76	34.79	34.81	34.84

COMPUTED MUSKINGUM-CUNGE PARAMETERS

R3-9

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN			2.00	96.60	1.48	346.00	.14	1.53

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN			2.00		1.48	346.00	.14	
------	--	--	------	--	------	--------	-----	--

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1766E+00 EXCESS= .0000E+00 OUTFLOW= .1737E+00 BASIN STORAGE= .2918E-02 PERCENT ERROR= .0

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HYDROGRAPH AT STATION ST1

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW
1.	5.77	
		6-HR
		24-HR
		72-HR
		9.97-HR
		(CFS)
		0.
		(INCHES)
		.144
		(AC-FT)
		0.

CUMULATIVE AREA = .02 SQ MI

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35 KK \* A3 \*  
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37 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

38 BA SUBBASIN CHARACTERISTICS

TAREA	.01	SUBBASIN AREA
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PRECIPITATION DATA

39 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.10	.19	.34	.42	.47	.55	.64	.70	.00	.00	.00	.00

STORM AREA = .01

40 LS SCS LOSS RATE

STRTL	.11	INITIAL ABSTRACTION
CRVNB	95.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

41 UD SCS DIMENSIONLESS UNITGRAPH

TLAG	.49	LAG
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UNIT HYDROGRAPH  
75 END-OF-PERIOD ORDINATES

0.	0.	1.	1.	1.	2.	2.	3.	3.	4.
4.	5.	5.	5.	5.	5.	5.	5.	5.	4.
4.	4.	3.	3.	3.	2.	2.	2.	2.	1.
1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

R3-10

HYDROGRAPH AT STATION A3

TOTAL RAINFALL = .61, TOTAL LOSS = .36, TOTAL EXCESS = .25

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	9.97-HR
1.	5.53	(CFS) 0.	0.	0.	0.
		(INCHES) .244	.244	.244	.244
		(AC-FT) 0.	0.	0.	0.

CUMULATIVE AREA = .01 SQ MI

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42 KK \* A3 \*  
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44 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

45 HC HYDROGRAPH COMBINATION  
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION A3

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	9.97-HR
2.	5.70	(CFS) 0.	0.	0.	0.
		(INCHES) .163	.163	.163	.163
		(AC-FT) 0.	0.	0.	0.

CUMULATIVE AREA = .03 SQ MI

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46 KK \* OF2 \*  
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48 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

49 BA SUBBASIN CHARACTERISTICS  
TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

50 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
10	19	34	12	17	55	64	70	00	00	00	00

R3-11

51 LS SCS LOSS RATE  
 STRTL .20 INITIAL ABSTRACTION  
 CRVNB 91.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

52 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .78 LAG

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UNIT HYDROGRAPH  
 119 END-OF-PERIOD ORDINATES

0.	0.	1.	1.	1.	2.	2.	3.	3.	4.
5.	5.	6.	7.	8.	9.	9.	10.	10.	11.
11.	11.	11.	11.	11.	11.	11.	11.	10.	10.
10.	9.	9.	9.	8.	8.	7.	7.	6.	6.
5.	5.	4.	4.	4.	4.	3.	3.	3.	3.
3.	2.	2.	2.	2.	2.	2.	2.	2.	1.
1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1.	1.	1.	1.	1.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

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HYDROGRAPH AT STATION OF2

TOTAL RAINFALL = .61, TOTAL LOSS = .49, TOTAL EXCESS = .12

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR (INCHES)	72-HR (INCHES)	9.97-HR (AC-FT)
1.	6.00	0.	.116	.116	.116
		0.	0.	0.	0.

CUMULATIVE AREA = .02 SQ MI

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53 KK \* CH2 \*

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55 KO OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

56 RD MUSKINGUM-CUNGE CHANNEL ROUTING  
 L 630. CHANNEL LENGTH  
 S .0130 SLOPE  
 N .025 CHANNEL ROUGHNESS COEFFICIENT  
 CA .00 CONTRIBUTING AREA  
 SHAPE TRAP CHANNEL SHAPE  
 WD 2.00 BOTTOM WIDTH OR DIAMETER  
 Z 3.00 SIDE SLOPE

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COMPUTED MUSKINGUM-CUNGE PARAMETERS

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	2.72	1.34	2.00	126.00	.70	364.00	.11	2.17

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

R3-12

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1157E+00 EXCESS= .0000E+00 OUTFLOW= .1146E+00 BASIN STORAGE= .1229E-02 PERCENT ERROR= -.1

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HYDROGRAPH AT STATION CH2

PEAK FLOW (CFS)	TIME (HR)		6-HR	24-HR	72-HR	9.97-HR
1.	6.07	(CFS)	0.	0.	0.	0.
		(INCHES)	.115	.115	.115	.115
		(AC-FT)	0.	0.	0.	0.

CUMULATIVE AREA = .02 SQ MI

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\* OF3 \*  
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57 KK

59 KO

OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	1	PLOT CONTROL
OSCAL	0.	HYDROGRAPH PLOT SCALE
IPNCH	1	PUNCH COMPUTED HYDROGRAPH
IOUT	21	SAVE HYDROGRAPH ON THIS UNIT
ISAV1	1	FIRST ORDINATE PUNCHED OR SAVED
ISAV2	300	LAST ORDINATE PUNCHED OR SAVED
TIMINT	.033	TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

60 BA

SUBBASIN CHARACTERISTICS

TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

61 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40					TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
.10	.19	.34	.42	.47	.55	.64	.70	.00	.00	.00	.00

STORM AREA = .00

62 LS

SCS LOSS RATE

STRTL	.27	INITIAL ABSTRACTION
CRVNB	88.00	CURVE NUMBER
RTIMP	.00	PERCENT IMPERVIOUS AREA

63 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .80 LAG

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UNIT HYDROGRAPH  
122 END-OF-PERIOD ORDINATES

0.	0.	0.	0.	0.	0.	1.	1.	1.	1.
1.	1.	2.	2.	2.	2.	2.	2.	3.	3.
3.	3.	3.	3.	3.	3.	3.	3.	3.	3.
3.	2.	2.	2.	2.	2.	2.	2.	2.	1.
1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
1.	1.	1.	1.	1.	1.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

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HYDROGRAPH AT STATION OF3

TOTAL DRAINAGE - 61 TOTAL LOSS - 55 TOTAL EXCESS - 07

R3-13

PEAK FLOW (CFS) 0.	TIME (HR) 6.10		6-HR 0.	24-HR 0.	72-HR 0.	9.97-HR 0.
		(CFS) 0.				
		(INCHES) .063		.063	.063	.063
		(AC-FT) 0.		0.	0.	0.

CUMULATIVE AREA = .00 SQ MI

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64 KK * OF3 *
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66 KO OUTPUT CONTROL VARIABLES
      IPRNT      3 PRINT CONTROL
      IPLOT      1 PLOT CONTROL
      OSCAL      0. HYDROGRAPH PLOT SCALE
      IPNCH      1 PUNCH COMPUTED HYDROGRAPH
      IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2     300 LAST ORDINATE PUNCHED OR SAVED
      TIMINT     .033 TIME INTERVAL IN HOURS

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67 HC HYDROGRAPH COMBINATION
      ICOMP      2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION OF3

PEAK FLOW (CFS) 1.	TIME (HR) 6.07		6-HR 0.	24-HR 0.	72-HR 0.	9.97-HR 0.
		(CFS) .104		.104	.104	.104
		(INCHES) 0.		0.	0.	0.
		(AC-FT) 0.		0.	0.	0.

CUMULATIVE AREA = .02 SQ MI

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68 KK * OF3 *
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70 KO OUTPUT CONTROL VARIABLES
      IPRNT      3 PRINT CONTROL
      IPLOT      1 PLOT CONTROL
      OSCAL      0. HYDROGRAPH PLOT SCALE
      IPNCH      1 PUNCH COMPUTED HYDROGRAPH
      IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2     300 LAST ORDINATE PUNCHED OR SAVED
      TIMINT     .033 TIME INTERVAL IN HOURS

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71 HC HYDROGRAPH COMBINATION
      ICOMP      2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION OF3

PEAK FLOW (CFS) 3.	TIME (HR) 5.77		6-HR 1.	24-HR 0.	72-HR 0.	9.97-HR 0.
		(CFS) .136		.136	.136	.136
		(INCHES) 0.		0.	0.	0.
		(AC-FT) 0.		0.	0.	0.

CUMULATIVE AREA = .05 SQ MI

R3-14



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72 KK \* A1 \*  
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74 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

75 BA SUBBASIN CHARACTERISTICS  
TAREA .03 SUBBASIN AREA

PRECIPITATION DATA

76 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
HYDRO-35 TP-40 TP-49  
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
.10 .19 .34 .42 .47 .55 .64 .70 .00 .00 .00 .00  
STORM AREA = .03

77 LS SCS LOSS RATE  
STRTL .11 INITIAL ABSTRACTION  
CRVNBR 95.00 CURVE NUMBER  
RTIMP .00 PERCENT IMPERVIOUS AREA

78 UD SCS DIMENSIONLESS UNITGRAPH  
TLAG .76 LAG

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UNIT HYDROGRAPH  
116 END-OF-PERIOD ORDINATES

0.	0.	1.	1.	2.	3.	3.	4.	5.	6.
8.	9.	10.	12.	13.	14.	15.	16.	17.	17.
18.	18.	18.	18.	18.	18.	17.	17.	16.	16.
15.	15.	14.	13.	12.	11.	10.	10.	9.	8.
8.	7.	7.	6.	6.	5.	5.	5.	4.	4.
4.	4.	3.	3.	3.	3.	3.	2.	2.	2.
2.	2.	2.	2.	1.	1.	1.	1.	1.	1.
1.	1.	1.	1.	1.	1.	1.	1.	1.	1.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
0.	0.	0.	0.	0.	0.	0.	0.	0.	0.

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HYDROGRAPH AT STATION A1

TOTAL RAINFALL = .61, TOTAL LOSS = .36, TOTAL EXCESS = .25

PEAK FLOW TIME MAXIMUM AVERAGE FLOW  
(CFS) (HR) (CFS) 6-HR 24-HR 72-HR 9.97-HR  
3. 5.87 (CFS) 1. 0. 0. 0.  
(INCHES) .239 .239 .239 .239  
(AC-FT) 0. 0. 0. 0.

CUMULATIVE AREA = .03 SQ MI

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79 KK \* A1 \*  
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R3-15

81 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

82 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION A1

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	9.97-HR
5.	5.80	(CFS) 2.	1.	1.	1.
		(INCHES) .173	.173	.173	.173
		(AC-FT) 1.	1.	1.	1.

CUMULATIVE AREA = .08 SQ MI

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83 KK \* \*  
 \* P1 \*  
 \* \*  
 \*\*\*\*\*

85 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

86 RD MUSKINGUM-CUNGE CHANNEL ROUTING  
 L 525. CHANNEL LENGTH  
 S .0060 SLOPE  
 N .015 CHANNEL ROUGHNESS COEFFICIENT  
 CA .00 CONTRIBUTING AREA  
 SHAPE CIRC CHANNEL SHAPE  
 WD 3.00 BOTTOM WIDTH OR DIAMETER  
 Z .00 SIDE SLOPE

\*\*\*

COMPUTED MUSKINGUM-CUNGE PARAMETERS

ELEMENT	ALPHA	COMPUTATION TIME STEP			PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
		M	DT	DX				
MAIN	4.99	1.25	1.60	262.50	5.18	350.47	.17	5.47

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	4.99	1.25	2.00		5.18	350.00	.17	
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CONTINUITY SUMMARY (AC-FT) - INFLOW= .7442E+00 EXCESS= .0000E+00 OUTFLOW= .7423E+00 BASIN STORAGE= .1955E-02 PERCENT ERROR= .0

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HYDROGRAPH AT STATION P1

PEAK FLOW TIME MAXIMUM AVERAGE FLOW

R3-16

5. 5.83 (CFS) 1. .173 1. .173 1. .173 1. .173  
 (INCHES) .173 .173 .173 .173  
 (AC-FT) 1. 1. 1. 1.  
 CUMULATIVE AREA = .08 SQ MI

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 \* \*  
 \* B1 \*  
 \* \*  
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89 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

90 BA SUBBASIN CHARACTERISTICS  
 TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

91 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
 ..... HYDRO-35 ..... TP-40 ..... TP-49 .....  
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
 .10 .19 .34 .42 .47 .55 .64 .70 .00 .00 .00 .00  
 STORM AREA = .00

92 LS SCS LOSS RATE  
 STRTL .08 INITIAL ABSTRACTION  
 CRVNB 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

93 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .06 LAG

\*\*\*

UNIT HYDROGRAPH  
 11 END-OF-PERIOD ORDINATES

1. 3. 3. 1. 1. 0. 0. 0. 0. 0. 0.  
 0.

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HYDROGRAPH AT STATION B1

TOTAL RAINFALL = .61, TOTAL LOSS = .32, TOTAL EXCESS = .30

| PEAK FLOW (CFS) | TIME (HR) | 6-HR (CFS)    | 24-HR (CFS)   | 72-HR (CFS)   | 9.97-HR (CFS) |
|-----------------|-----------|---------------|---------------|---------------|---------------|
| 0.              | 5.07      | 0.            | 0.            | 0.            | 0.            |
|                 |           | (INCHES) .297 | (INCHES) .297 | (INCHES) .297 | (INCHES) .297 |
|                 |           | (AC-FT) 0.    | (AC-FT) 0.    | (AC-FT) 0.    | (AC-FT) 0.    |

CUMULATIVE AREA = .00 SQ MI

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 \* \*  
 \* B1 \*  
 \* \*  
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96 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL

R3-17

IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

97 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION 81

| PEAK FLOW (CFS) | TIME (HR) | MAXIMUM AVERAGE FLOW     |
|-----------------|-----------|--------------------------|
| 5.              | 5.83      |                          |
|                 | (CFS)     | 6-HR 24-HR 72-HR 9.97-HR |
|                 | (INCHES)  | 2. 1. 1. 1.              |
|                 | (AC-FT)   | .174 .174 .174 .174      |
|                 |           | 1. 1. 1. 1.              |

CUMULATIVE AREA = .08 SQ MI

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 \* RES1 \*  
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98 KK

100 KO

OUTPUT CONTROL VARIABLES

IPRNT 1 PRINT CONTROL  
 IPLOT 2 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

101 RS

STORAGE ROUTING

NSTPS 1 NUMBER OF SUBREACHES  
 ITYP ELEV TYPE OF INITIAL CONDITION  
 RSVRIC 18.60 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

102 SV

STORAGE .0 .1 .2 .3 .5

103 SE

ELEVATION 19.60 20.60 21.60 22.60 23.60

104 SL

LOW-LEVEL OUTLET

ELEVL 18.60 ELEVATION AT CENTER OF OUTLET  
 CAREA .79 CROSS-SECTIONAL AREA  
 COOL .60 COEFFICIENT  
 EXPL .50 EXPONENT OF HEAD

105 SS

SPILLWAY

CREL 22.60 SPILLWAY CREST ELEVATION  
 SPWID 19.00 SPILLWAY WIDTH  
 COOW 2.70 WEIR COEFFICIENT  
 EXPW 1.50 EXPONENT OF HEAD

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COMPUTED OUTFLOW-ELEVATION DATA

|                   |       |       |       |       |       |       |       |       |       |       |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| OUTFLOW ELEVATION | .00   | 4.03  | 4.28  | 4.56  | 4.89  | 5.26  | 5.70  | 6.22  | 6.84  | 7.60  |
|                   | 19.60 | 19.72 | 19.87 | 20.04 | 20.25 | 20.52 | 20.85 | 21.28 | 21.84 | 22.60 |
| OUTFLOW ELEVATION | 7.69  | 8.12  | 9.20  | 11.23 | 14.50 | 19.30 | 25.94 | 34.70 | 45.89 | 59.80 |
|                   | 22.61 | 22.64 | 22.70 | 22.77 | 22.86 | 22.97 | 23.10 | 23.24 | 23.41 | 23.60 |

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

|                   |       |       |       |       |       |       |       |       |       |       |
|-------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| STORAGE           | .01   | .02   | .02   | .03   | .04   | .06   | .06   | .09   | .13   | .16   |
| OUTFLOW ELEVATION | 3.80  | 4.03  | 4.28  | 4.56  | 4.89  | 5.26  | 5.38  | 5.70  | 6.22  | 6.58  |
|                   | 19.60 | 19.72 | 19.87 | 20.04 | 20.25 | 20.52 | 20.60 | 20.85 | 21.28 | 21.60 |

R3-18

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| STORAGE   | .20   | .31   | .31   | .32   | .32   | .34   | .35   | .36   | .38   | .41   |
| OUTFLOW   | 6.84  | 7.60  | 7.69  | 8.12  | 9.20  | 11.23 | 14.50 | 19.30 | 25.94 | 34.70 |
| ELEVATION | 21.84 | 22.60 | 22.61 | 22.64 | 22.70 | 22.77 | 22.86 | 22.97 | 23.10 | 23.24 |

|           |       |       |
|-----------|-------|-------|
| STORAGE   | .43   | .46   |
| OUTFLOW   | 45.89 | 59.80 |
| ELEVATION | 23.41 | 23.60 |

HYDROGRAPH AT STATION RES1

| DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | * | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | * | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE |
|----|-----|------|-----|---------|---------|-------|---|----|-----|------|-----|---------|---------|-------|---|----|-----|------|-----|---------|---------|-------|
| 29 | JUN | 1200 | 1   | 0.      | .0      | 19.6  | * | 29 | JUN | 1520 | 101 | 4.      | .0      | 19.6  | * | 29 | JUN | 1840 | 201 | 4.      | .0      | 19.6  |
| 29 | JUN | 1202 | 2   | 4.      | .0      | 19.6  | * | 29 | JUN | 1522 | 102 | 4.      | .0      | 19.6  | * | 29 | JUN | 1842 | 202 | 4.      | .0      | 19.6  |
| 29 | JUN | 1204 | 3   | 4.      | .0      | 19.6  | * | 29 | JUN | 1524 | 103 | 4.      | .0      | 19.6  | * | 29 | JUN | 1844 | 203 | 4.      | .0      | 19.6  |
| 29 | JUN | 1206 | 4   | 4.      | .0      | 19.6  | * | 29 | JUN | 1526 | 104 | 4.      | .0      | 19.6  | * | 29 | JUN | 1846 | 204 | 4.      | .0      | 19.6  |
| 29 | JUN | 1208 | 5   | 4.      | .0      | 19.6  | * | 29 | JUN | 1528 | 105 | 4.      | .0      | 19.6  | * | 29 | JUN | 1848 | 205 | 4.      | .0      | 19.6  |
| 29 | JUN | 1210 | 6   | 4.      | .0      | 19.6  | * | 29 | JUN | 1530 | 106 | 4.      | .0      | 19.6  | * | 29 | JUN | 1850 | 206 | 4.      | .0      | 19.6  |
| 29 | JUN | 1212 | 7   | 4.      | .0      | 19.6  | * | 29 | JUN | 1532 | 107 | 4.      | .0      | 19.6  | * | 29 | JUN | 1852 | 207 | 4.      | .0      | 19.6  |
| 29 | JUN | 1214 | 8   | 4.      | .0      | 19.6  | * | 29 | JUN | 1534 | 108 | 4.      | .0      | 19.6  | * | 29 | JUN | 1854 | 208 | 4.      | .0      | 19.6  |
| 29 | JUN | 1216 | 9   | 4.      | .0      | 19.6  | * | 29 | JUN | 1536 | 109 | 4.      | .0      | 19.6  | * | 29 | JUN | 1856 | 209 | 4.      | .0      | 19.6  |
| 29 | JUN | 1218 | 10  | 4.      | .0      | 19.6  | * | 29 | JUN | 1538 | 110 | 4.      | .0      | 19.6  | * | 29 | JUN | 1858 | 210 | 4.      | .0      | 19.6  |
| 29 | JUN | 1220 | 11  | 4.      | .0      | 19.6  | * | 29 | JUN | 1540 | 111 | 4.      | .0      | 19.6  | * | 29 | JUN | 1900 | 211 | 4.      | .0      | 19.6  |
| 29 | JUN | 1222 | 12  | 4.      | .0      | 19.6  | * | 29 | JUN | 1542 | 112 | 4.      | .0      | 19.6  | * | 29 | JUN | 1902 | 212 | 4.      | .0      | 19.6  |
| 29 | JUN | 1224 | 13  | 4.      | .0      | 19.6  | * | 29 | JUN | 1544 | 113 | 4.      | .0      | 19.6  | * | 29 | JUN | 1904 | 213 | 4.      | .0      | 19.6  |
| 29 | JUN | 1226 | 14  | 4.      | .0      | 19.6  | * | 29 | JUN | 1546 | 114 | 4.      | .0      | 19.6  | * | 29 | JUN | 1906 | 214 | 4.      | .0      | 19.6  |
| 29 | JUN | 1228 | 15  | 4.      | .0      | 19.6  | * | 29 | JUN | 1548 | 115 | 4.      | .0      | 19.6  | * | 29 | JUN | 1908 | 215 | 4.      | .0      | 19.6  |
| 29 | JUN | 1230 | 16  | 4.      | .0      | 19.6  | * | 29 | JUN | 1550 | 116 | 4.      | .0      | 19.6  | * | 29 | JUN | 1910 | 216 | 4.      | .0      | 19.6  |
| 29 | JUN | 1232 | 17  | 4.      | .0      | 19.6  | * | 29 | JUN | 1552 | 117 | 4.      | .0      | 19.6  | * | 29 | JUN | 1912 | 217 | 4.      | .0      | 19.6  |
| 29 | JUN | 1234 | 18  | 4.      | .0      | 19.6  | * | 29 | JUN | 1554 | 118 | 4.      | .0      | 19.6  | * | 29 | JUN | 1914 | 218 | 4.      | .0      | 19.6  |
| 29 | JUN | 1236 | 19  | 4.      | .0      | 19.6  | * | 29 | JUN | 1556 | 119 | 4.      | .0      | 19.6  | * | 29 | JUN | 1916 | 219 | 4.      | .0      | 19.6  |
| 29 | JUN | 1238 | 20  | 4.      | .0      | 19.6  | * | 29 | JUN | 1558 | 120 | 4.      | .0      | 19.6  | * | 29 | JUN | 1918 | 220 | 4.      | .0      | 19.6  |
| 29 | JUN | 1240 | 21  | 4.      | .0      | 19.6  | * | 29 | JUN | 1600 | 121 | 4.      | .0      | 19.6  | * | 29 | JUN | 1920 | 221 | 4.      | .0      | 19.6  |
| 29 | JUN | 1242 | 22  | 4.      | .0      | 19.6  | * | 29 | JUN | 1602 | 122 | 4.      | .0      | 19.6  | * | 29 | JUN | 1922 | 222 | 4.      | .0      | 19.6  |
| 29 | JUN | 1244 | 23  | 4.      | .0      | 19.6  | * | 29 | JUN | 1604 | 123 | 4.      | .0      | 19.6  | * | 29 | JUN | 1924 | 223 | 4.      | .0      | 19.6  |
| 29 | JUN | 1246 | 24  | 4.      | .0      | 19.6  | * | 29 | JUN | 1606 | 124 | 4.      | .0      | 19.6  | * | 29 | JUN | 1926 | 224 | 4.      | .0      | 19.6  |
| 29 | JUN | 1248 | 25  | 4.      | .0      | 19.6  | * | 29 | JUN | 1608 | 125 | 4.      | .0      | 19.6  | * | 29 | JUN | 1928 | 225 | 4.      | .0      | 19.6  |
| 29 | JUN | 1250 | 26  | 4.      | .0      | 19.6  | * | 29 | JUN | 1610 | 126 | 4.      | .0      | 19.6  | * | 29 | JUN | 1930 | 226 | 4.      | .0      | 19.6  |
| 29 | JUN | 1252 | 27  | 4.      | .0      | 19.6  | * | 29 | JUN | 1612 | 127 | 4.      | .0      | 19.6  | * | 29 | JUN | 1932 | 227 | 4.      | .0      | 19.6  |
| 29 | JUN | 1254 | 28  | 4.      | .0      | 19.6  | * | 29 | JUN | 1614 | 128 | 4.      | .0      | 19.6  | * | 29 | JUN | 1934 | 228 | 4.      | .0      | 19.6  |
| 29 | JUN | 1256 | 29  | 4.      | .0      | 19.6  | * | 29 | JUN | 1616 | 129 | 4.      | .0      | 19.6  | * | 29 | JUN | 1936 | 229 | 4.      | .0      | 19.6  |
| 29 | JUN | 1258 | 30  | 4.      | .0      | 19.6  | * | 29 | JUN | 1618 | 130 | 4.      | .0      | 19.6  | * | 29 | JUN | 1938 | 230 | 4.      | .0      | 19.6  |
| 29 | JUN | 1300 | 31  | 4.      | .0      | 19.6  | * | 29 | JUN | 1620 | 131 | 4.      | .0      | 19.6  | * | 29 | JUN | 1940 | 231 | 4.      | .0      | 19.6  |
| 29 | JUN | 1302 | 32  | 4.      | .0      | 19.6  | * | 29 | JUN | 1622 | 132 | 4.      | .0      | 19.6  | * | 29 | JUN | 1942 | 232 | 4.      | .0      | 19.6  |
| 29 | JUN | 1304 | 33  | 4.      | .0      | 19.6  | * | 29 | JUN | 1624 | 133 | 4.      | .0      | 19.6  | * | 29 | JUN | 1944 | 233 | 4.      | .0      | 19.6  |
| 29 | JUN | 1306 | 34  | 4.      | .0      | 19.6  | * | 29 | JUN | 1626 | 134 | 4.      | .0      | 19.6  | * | 29 | JUN | 1946 | 234 | 4.      | .0      | 19.6  |
| 29 | JUN | 1308 | 35  | 4.      | .0      | 19.6  | * | 29 | JUN | 1628 | 135 | 4.      | .0      | 19.6  | * | 29 | JUN | 1948 | 235 | 4.      | .0      | 19.6  |
| 29 | JUN | 1310 | 36  | 4.      | .0      | 19.6  | * | 29 | JUN | 1630 | 136 | 4.      | .0      | 19.6  | * | 29 | JUN | 1950 | 236 | 4.      | .0      | 19.6  |
| 29 | JUN | 1312 | 37  | 4.      | .0      | 19.6  | * | 29 | JUN | 1632 | 137 | 4.      | .0      | 19.6  | * | 29 | JUN | 1952 | 237 | 4.      | .0      | 19.6  |
| 29 | JUN | 1314 | 38  | 4.      | .0      | 19.6  | * | 29 | JUN | 1634 | 138 | 4.      | .0      | 19.6  | * | 29 | JUN | 1954 | 238 | 4.      | .0      | 19.6  |
| 29 | JUN | 1316 | 39  | 4.      | .0      | 19.6  | * | 29 | JUN | 1636 | 139 | 4.      | .0      | 19.6  | * | 29 | JUN | 1956 | 239 | 4.      | .0      | 19.6  |
| 29 | JUN | 1318 | 40  | 4.      | .0      | 19.6  | * | 29 | JUN | 1638 | 140 | 4.      | .0      | 19.6  | * | 29 | JUN | 1958 | 240 | 4.      | .0      | 19.6  |
| 29 | JUN | 1320 | 41  | 4.      | .0      | 19.6  | * | 29 | JUN | 1640 | 141 | 4.      | .0      | 19.6  | * | 29 | JUN | 2000 | 241 | 4.      | .0      | 19.6  |
| 29 | JUN | 1322 | 42  | 4.      | .0      | 19.6  | * | 29 | JUN | 1642 | 142 | 4.      | .0      | 19.6  | * | 29 | JUN | 2002 | 242 | 4.      | .0      | 19.6  |
| 29 | JUN | 1324 | 43  | 4.      | .0      | 19.6  | * | 29 | JUN | 1644 | 143 | 4.      | .0      | 19.6  | * | 29 | JUN | 2004 | 243 | 4.      | .0      | 19.6  |
| 29 | JUN | 1326 | 44  | 4.      | .0      | 19.6  | * | 29 | JUN | 1646 | 144 | 4.      | .0      | 19.6  | * | 29 | JUN | 2006 | 244 | 4.      | .0      | 19.6  |
| 29 | JUN | 1328 | 45  | 4.      | .0      | 19.6  | * | 29 | JUN | 1648 | 145 | 4.      | .0      | 19.6  | * | 29 | JUN | 2008 | 245 | 4.      | .0      | 19.6  |
| 29 | JUN | 1330 | 46  | 4.      | .0      | 19.6  | * | 29 | JUN | 1650 | 146 | 4.      | .0      | 19.6  | * | 29 | JUN | 2010 | 246 | 4.      | .0      | 19.6  |
| 29 | JUN | 1332 | 47  | 4.      | .0      | 19.6  | * | 29 | JUN | 1652 | 147 | 4.      | .0      | 19.6  | * | 29 | JUN | 2012 | 247 | 4.      | .0      | 19.6  |
| 29 | JUN | 1334 | 48  | 4.      | .0      | 19.6  | * | 29 | JUN | 1654 | 148 | 4.      | .0      | 19.6  | * | 29 | JUN | 2014 | 248 | 4.      | .0      | 19.6  |
| 29 | JUN | 1336 | 49  | 4.      | .0      | 19.6  | * | 29 | JUN | 1656 | 149 | 4.      | .0      | 19.6  | * | 29 | JUN | 2016 | 249 | 4.      | .0      | 19.6  |
| 29 | JUN | 1338 | 50  | 4.      | .0      | 19.6  | * | 29 | JUN | 1658 | 150 | 4.      | .0      | 19.6  | * | 29 | JUN | 2018 | 250 | 4.      | .0      | 19.6  |
| 29 | JUN | 1340 | 51  | 4.      | .0      | 19.6  | * | 29 | JUN | 1700 | 151 | 4.      | .0      | 19.6  | * | 29 | JUN | 2020 | 251 | 4.      | .0      | 19.6  |
| 29 | JUN | 1342 | 52  | 4.      | .0      | 19.6  | * | 29 | JUN | 1702 | 152 | 4.      | .0      | 19.6  | * | 29 | JUN | 2022 | 252 | 4.      | .0      | 19.6  |
| 29 | JUN | 1344 | 53  | 4.      | .0      | 19.6  | * | 29 | JUN | 1704 | 153 | 4.      | .0      | 19.6  | * | 29 | JUN | 2024 | 253 | 4.      | .0      | 19.6  |
| 29 | JUN | 1346 | 54  | 4.      | .0      | 19.6  | * | 29 | JUN | 1706 | 154 | 4.      | .0      | 19.6  | * | 29 | JUN | 2026 | 254 | 4.      | .0      | 19.6  |
| 29 | JUN | 1348 | 55  | 4.      | .0      | 19.6  | * | 29 | JUN | 1708 | 155 | 4.      | .0      | 19.6  | * | 29 | JUN | 2028 | 255 | 4.      | .0      | 19.6  |
| 29 | JUN | 1350 | 56  | 4.      | .0      | 19.6  | * | 29 | JUN | 1710 | 156 | 4.      | .0      | 19.6  | * | 29 | JUN | 2030 | 256 | 4.      | .0      | 19.6  |
| 29 | JUN | 1352 | 57  | 4.      | .0      | 19.6  | * | 29 | JUN | 1712 | 157 | 4.      | .0      | 19.6  | * | 29 | JUN | 2032 | 257 | 4.      | .0      | 19.6  |
| 29 | JUN | 1354 | 58  | 4.      | .0      | 19.6  | * | 29 | JUN | 1714 | 158 | 4.      | .0      | 19.6  | * | 29 | JUN | 2034 | 258 | 4.      | .0      | 19.6  |
| 29 | JUN | 1356 | 59  | 4.      | .0      | 19.6  | * | 29 | JUN | 1716 | 159 | 4.      | .0      | 19.6  | * | 29 | JUN | 2036 | 259 | 4.      | .0      | 19.6  |
| 29 | JUN | 1358 | 60  | 4.      | .0      | 19.6  | * | 29 | JUN | 1718 | 160 | 4.      | .0      | 19.6  | * | 29 | JUN | 2038 | 260 | 4.      | .0      | 19.6  |
| 29 | JUN | 1400 | 61  | 4.      | .0      | 19.6  | * | 29 | JUN | 1720 | 161 | 4.      | .0      | 19.6  | * | 29 | JUN | 2040 | 261 | 4.      | .0      | 19.6  |
| 29 | JUN | 1402 | 62  | 4.      | .0      | 19.6  | * | 29 | JUN | 1722 | 162 | 4.      | .0      | 19.6  | * | 29 | JUN | 2042 | 262 | 4.      | .0      | 19.6  |
| 29 | JUN | 1404 | 63  | 4.      | .0      | 19.6  | * | 29 | JUN | 1724 | 163 | 4.      | .0      | 19.6  | * | 29 | JUN | 2044 | 263 | 4.      | .0      | 19.6  |
| 29 | JUN | 1406 | 64  | 4.      | .0      | 19.6  | * | 29 | JUN | 1726 | 164 | 4.      | .0      | 19.6  | * | 29 | JUN | 2046 | 264 | 4.      | .0      | 19.6  |
| 29 | JUN | 1408 | 65  | 4.      | .0      | 19.6  | * | 29 | JUN | 1728 | 165 | 4.      | .0      | 19.6  | * | 29 | JUN | 2048 | 265 | 4.      | .0      | 19.6  |
| 29 | JUN | 1410 | 66  | 4.      | .0      | 19.6  | * | 29 | JUN | 1730 | 166 | 4.      | .0      | 19.6  | * | 29 | JUN | 2050 | 266 | 4.      | .0      | 19.6  |
| 29 | JUN | 1412 | 67  | 4.      | .0      | 19.6  | * | 29 | JUN | 1732 | 167 | 4.      | .0      | 19.6  | * | 29 | JUN | 2052 | 267 | 4.      | .0      | 19.6  |
| 29 | JUN | 1414 | 68  | 4.      | .0      | 19.6  | * | 29 | JUN | 1734 | 168 | 4.      | .0      | 19.6  | * | 29 | JUN | 2054 | 268 | 4.      | .0      | 19.6  |
| 29 | JUN | 1416 | 69  | 4.      | .0      | 19.6  | * | 29 | JUN | 1736 | 169 | 4.      | .0      | 19.6  | * | 29 | JUN | 2056 | 269 | 4.      | .0      | 19.6  |
| 29 | JUN | 1418 | 70  | 4.      | .0      | 19.6  | * | 29 | JUN | 1738 | 170 | 4.      | .0      | 19.6  | * | 29 | JUN | 2058 | 270 | 4.      | .0      | 19.6  |
| 29 | JUN | 1420 | 71  | 4.      | .0      | 19.6  |   |    |     |      |     |         |         |       |   |    |     |      |     |         |         |       |

|             |     |    |    |      |   |             |     |    |    |      |   |             |     |    |    |      |
|-------------|-----|----|----|------|---|-------------|-----|----|----|------|---|-------------|-----|----|----|------|
| 29 JUN 1424 | 73  | 4. | .0 | 19.6 | * | 29 JUN 1744 | 173 | 4. | .0 | 19.6 | * | 29 JUN 2104 | 273 | 4. | .0 | 19.6 |
| 29 JUN 1426 | 74  | 4. | .0 | 19.6 | * | 29 JUN 1746 | 174 | 4. | .0 | 19.6 | * | 29 JUN 2106 | 274 | 4. | .0 | 19.6 |
| 29 JUN 1428 | 75  | 4. | .0 | 19.6 | * | 29 JUN 1748 | 175 | 4. | .0 | 19.7 | * | 29 JUN 2108 | 275 | 4. | .0 | 19.6 |
| 29 JUN 1430 | 76  | 4. | .0 | 19.6 | * | 29 JUN 1750 | 176 | 4. | .0 | 19.7 | * | 29 JUN 2110 | 276 | 4. | .0 | 19.6 |
| 29 JUN 1432 | 77  | 4. | .0 | 19.6 | * | 29 JUN 1752 | 177 | 4. | .0 | 19.8 | * | 29 JUN 2112 | 277 | 4. | .0 | 19.6 |
| 29 JUN 1434 | 78  | 4. | .0 | 19.6 | * | 29 JUN 1754 | 178 | 4. | .0 | 19.9 | * | 29 JUN 2114 | 278 | 4. | .0 | 19.6 |
| 29 JUN 1436 | 79  | 4. | .0 | 19.6 | * | 29 JUN 1756 | 179 | 4. | .0 | 19.9 | * | 29 JUN 2116 | 279 | 4. | .0 | 19.6 |
| 29 JUN 1438 | 80  | 4. | .0 | 19.6 | * | 29 JUN 1758 | 180 | 4. | .0 | 19.9 | * | 29 JUN 2118 | 280 | 4. | .0 | 19.6 |
| 29 JUN 1440 | 81  | 4. | .0 | 19.6 | * | 29 JUN 1800 | 181 | 4. | .0 | 20.0 | * | 29 JUN 2120 | 281 | 4. | .0 | 19.6 |
| 29 JUN 1442 | 82  | 4. | .0 | 19.6 | * | 29 JUN 1802 | 182 | 4. | .0 | 20.0 | * | 29 JUN 2122 | 282 | 4. | .0 | 19.6 |
| 29 JUN 1444 | 83  | 4. | .0 | 19.6 | * | 29 JUN 1804 | 183 | 5. | .0 | 20.0 | * | 29 JUN 2124 | 283 | 4. | .0 | 19.6 |
| 29 JUN 1446 | 84  | 4. | .0 | 19.6 | * | 29 JUN 1806 | 184 | 5. | .0 | 20.0 | * | 29 JUN 2126 | 284 | 4. | .0 | 19.6 |
| 29 JUN 1448 | 85  | 4. | .0 | 19.6 | * | 29 JUN 1808 | 185 | 5. | .0 | 20.0 | * | 29 JUN 2128 | 285 | 4. | .0 | 19.6 |
| 29 JUN 1450 | 86  | 4. | .0 | 19.6 | * | 29 JUN 1810 | 186 | 5. | .0 | 20.0 | * | 29 JUN 2130 | 286 | 4. | .0 | 19.6 |
| 29 JUN 1452 | 87  | 4. | .0 | 19.6 | * | 29 JUN 1812 | 187 | 5. | .0 | 20.0 | * | 29 JUN 2132 | 287 | 4. | .0 | 19.6 |
| 29 JUN 1454 | 88  | 4. | .0 | 19.6 | * | 29 JUN 1814 | 188 | 4. | .0 | 20.0 | * | 29 JUN 2134 | 288 | 4. | .0 | 19.6 |
| 29 JUN 1456 | 89  | 4. | .0 | 19.6 | * | 29 JUN 1816 | 189 | 4. | .0 | 20.0 | * | 29 JUN 2136 | 289 | 4. | .0 | 19.6 |
| 29 JUN 1458 | 90  | 4. | .0 | 19.6 | * | 29 JUN 1818 | 190 | 4. | .0 | 19.9 | * | 29 JUN 2138 | 290 | 4. | .0 | 19.6 |
| 29 JUN 1500 | 91  | 4. | .0 | 19.6 | * | 29 JUN 1820 | 191 | 4. | .0 | 19.9 | * | 29 JUN 2140 | 291 | 4. | .0 | 19.6 |
| 29 JUN 1502 | 92  | 4. | .0 | 19.6 | * | 29 JUN 1822 | 192 | 4. | .0 | 19.9 | * | 29 JUN 2142 | 292 | 4. | .0 | 19.6 |
| 29 JUN 1504 | 93  | 4. | .0 | 19.6 | * | 29 JUN 1824 | 193 | 4. | .0 | 19.8 | * | 29 JUN 2144 | 293 | 4. | .0 | 19.6 |
| 29 JUN 1506 | 94  | 4. | .0 | 19.6 | * | 29 JUN 1826 | 194 | 4. | .0 | 19.8 | * | 29 JUN 2146 | 294 | 4. | .0 | 19.6 |
| 29 JUN 1508 | 95  | 4. | .0 | 19.6 | * | 29 JUN 1828 | 195 | 4. | .0 | 19.8 | * | 29 JUN 2148 | 295 | 4. | .0 | 19.6 |
| 29 JUN 1510 | 96  | 4. | .0 | 19.6 | * | 29 JUN 1830 | 196 | 4. | .0 | 19.7 | * | 29 JUN 2150 | 296 | 4. | .0 | 19.6 |
| 29 JUN 1512 | 97  | 4. | .0 | 19.6 | * | 29 JUN 1832 | 197 | 4. | .0 | 19.7 | * | 29 JUN 2152 | 297 | 4. | .0 | 19.6 |
| 29 JUN 1514 | 98  | 4. | .0 | 19.6 | * | 29 JUN 1834 | 198 | 4. | .0 | 19.6 | * | 29 JUN 2154 | 298 | 4. | .0 | 19.6 |
| 29 JUN 1516 | 99  | 4. | .0 | 19.6 | * | 29 JUN 1836 | 199 | 4. | .0 | 19.6 | * | 29 JUN 2156 | 299 | 4. | .0 | 19.6 |
| 29 JUN 1518 | 100 | 4. | .0 | 19.6 | * | 29 JUN 1838 | 200 | 4. | .0 | 19.6 | * | 29 JUN 2158 | 300 | 4. | .0 | 19.6 |

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|           |      |          |                      |       |       |         |
|-----------|------|----------|----------------------|-------|-------|---------|
| PEAK FLOW | TIME |          | MAXIMUM AVERAGE FLOW |       |       |         |
| (CFS)     | (HR) |          | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 5.        | 6.13 | (CFS)    | 4.                   | 4.    | 4.    | 4.      |
|           |      | (INCHES) | .444                 | .731  | .731  | .731    |
|           |      | (AC-FT)  | 2.                   | 3.    | 3.    | 3.      |

|              |      |  |                         |       |       |         |
|--------------|------|--|-------------------------|-------|-------|---------|
| PEAK STORAGE | TIME |  | MAXIMUM AVERAGE STORAGE |       |       |         |
| (AC-FT)      | (HR) |  | 6-HR                    | 24-HR | 72-HR | 9.97-HR |
| 0.           | 6.10 |  | 0.                      | 0.    | 0.    | 0.      |

|            |      |  |                       |       |       |         |
|------------|------|--|-----------------------|-------|-------|---------|
| PEAK STAGE | TIME |  | MAXIMUM AVERAGE STAGE |       |       |         |
| (FEET)     | (HR) |  | 6-HR                  | 24-HR | 72-HR | 9.97-HR |
| 20.02      | 6.13 |  | 19.64                 | 19.62 | 19.62 | 19.62   |

CUMULATIVE AREA = .08 SQ MI

R3-20

STATION RES1

(I) INFLOW, (O) OUTFLOW

0. 1. 2. 3. 4. 5. 6. 0. 0. 0. 0. 0. 0.  
 .00 .00 .00 .00 .00 .00 -.01 .00 .01 .02 .03 .04 .00  
 (S) STORAGE

DAHRMN PER

| STATION | RES1 | (I) INFLOW | (O) OUTFLOW | STORAGE |
|---------|------|------------|-------------|---------|
| 291200  | 1I   |            |             | S       |
| 291202  | 2I   |            | 0           | S       |
| 291204  | 3I   |            | 0           | S       |
| 291206  | 4I   |            | 0           |         |
| 291208  | 5I   |            | 0           |         |
| 291210  | 6I   |            | 0           |         |
| 291212  | 7I   |            | 0           |         |
| 291214  | 8I   |            | 0           |         |
| 291216  | 9I   |            | 0           |         |
| 291218  | 10I  |            | 0           |         |
| 291220  | 11I  |            | 0           |         |
| 291222  | 12I  |            | 0           |         |
| 291224  | 13I  |            | 0           |         |
| 291226  | 14I  |            | 0           |         |
| 291228  | 15I  |            | 0           |         |
| 291230  | 16I  |            | 0           |         |
| 291232  | 17I  |            | 0           |         |
| 291234  | 18I  |            | 0           |         |
| 291236  | 19I  |            | 0           |         |
| 291238  | 20I  |            | 0           |         |
| 291240  | 21I  |            | 0           |         |
| 291242  | 22I  |            | 0           |         |
| 291244  | 23I  |            | 0           |         |
| 291246  | 24I  |            | 0           |         |
| 291248  | 25I  |            | 0           |         |
| 291250  | 26I  |            | 0           |         |
| 291252  | 27I  |            | 0           |         |
| 291254  | 28I  |            | 0           |         |
| 291256  | 29I  |            | 0           |         |
| 291258  | 30I  |            | 0           |         |
| 291300  | 31I  |            | 0           |         |
| 291302  | 32I  |            | 0           |         |
| 291304  | 33I  |            | 0           |         |
| 291306  | 34I  |            | 0           |         |
| 291308  | 35I  |            | 0           |         |
| 291310  | 36I  |            | 0           |         |
| 291312  | 37I  |            | 0           |         |
| 291314  | 38I  |            | 0           |         |
| 291316  | 39I  |            | 0           |         |
| 291318  | 40I  |            | 0           |         |
| 291320  | 41I  |            | 0           |         |
| 291322  | 42I  |            | 0           |         |
| 291324  | 43I  |            | 0           |         |
| 291326  | 44I  |            | 0           |         |
| 291328  | 45I  |            | 0           |         |
| 291330  | 46I  |            | 0           |         |
| 291332  | 47I  |            | 0           |         |
| 291334  | 48I  |            | 0           |         |
| 291336  | 49I  |            | 0           |         |
| 291338  | 50I  |            | 0           |         |
| 291340  | 51I  |            | 0           |         |
| 291342  | 52I  |            | 0           |         |
| 291344  | 53I  |            | 0           |         |
| 291346  | 54I  |            | 0           |         |
| 291348  | 55I  |            | 0           |         |
| 291350  | 56I  |            | 0           |         |
| 291352  | 57I  |            | 0           |         |
| 291354  | 58I  |            | 0           |         |
| 291356  | 59I  |            | 0           |         |
| 291358  | 60I  |            | 0           |         |
| 291400  | 61I  |            | 0           |         |
| 291402  | 62I  |            | 0           |         |
| 291404  | 63I  |            | 0           |         |
| 291406  | 64I  |            | 0           |         |
| 291408  | 65I  |            | 0           |         |
| 291410  | 66I  |            | 0           |         |
| 291412  | 67I  |            | 0           |         |
| 291414  | 68I  |            | 0           |         |
| 291416  | 69I  |            | 0           |         |
| 291418  | 70I  |            | 0           |         |
| 291420  | 71I  |            | 0           |         |
| 291422  | 72I  |            | 0           |         |
| 291424  | 73I  |            | 0           |         |
| 291426  | 74I  |            | 0           |         |

R3-21









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\* CH3 \*  
\* \*  
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106 KK

108 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

109 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

L 310. CHANNEL LENGTH  
S .0052 SLOPE  
N .025 CHANNEL ROUGHNESS COEFFICIENT  
CA .00 CONTRIBUTING AREA  
SHAPE TRAP CHANNEL SHAPE  
WD 2.00 BOTTOM WIDTH OR DIAMETER  
Z 3.00 SIDE SLOPE

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COMPUTED MUSKINGUM-CUNGE PARAMETERS

| ELEMENT | ALPHA | COMPUTATION TIME STEP |             | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|-----------------------|-------------|---------------|--------------------------|----------------|------------------------------|
|         |       | M                     | DT<br>(MIN) |               |                          |                |                              |
| MAIN    | 1.72  | 1.34                  | 1.30        | 103.33        | 4.52                     | 369.20         | .73                          |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |      |        |     |
|------|------|------|------|------|--------|-----|
| MAIN | 1.72 | 1.34 | 2.00 | 4.52 | 370.00 | .73 |
|------|------|------|------|------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3161E+01 EXCESS= .0000E+00 OUTFLOW= .3149E+01 BASIN STORAGE= .1282E-01 PERCENT ERROR= .0

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HYDROGRAPH AT STATION CH3

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 5.                 | 6.17         | (CFS) 4.             | 4.    | 4.    | 4.      |
|                    |              | (INCHES) .444        | .728  | .728  | .728    |
|                    |              | (AC-FT) 2.           | 3.    | 3.    | 3.      |

CUMULATIVE AREA = .08 SQ MI

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\* \*  
\* B2 \*  
\* \*  
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110 KK

112 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL

R3-25

OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

113 BA SUBBASIN CHARACTERISTICS  
 TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

114 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
 ..... HYDRO-35 ..... TP-40 ..... TP-49 .....  
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
 .10 .19 .34 .42 .47 .55 .64 .70 .00 .00 .00 .00  
 STORM AREA = .00

115 LS SCS LOSS RATE  
 STRL .08 INITIAL ABSTRACTION  
 CRVNR 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

116 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .06 LAG

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UNIT HYDROGRAPH  
 11 END-OF-PERIOD ORDINATES

1. 3. 3. 1. 1. 0. 0. 0. 0. 0.

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HYDROGRAPH AT STATION B2

TOTAL RAINFALL = .61, TOTAL LOSS = .32, TOTAL EXCESS = .30

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |      |
|--------------------|--------------|----------------------|-------|-------|---------|------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |      |
| 0.                 | 5.07         | 0.                   | 0.    | 0.    | 0.      |      |
|                    |              | (INCHES)             | .297  | .297  | .297    | .297 |
|                    |              | (AC-FT)              | 0.    | 0.    | 0.      | 0.   |

CUMULATIVE AREA = .00 SQ MI

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117 KK \* \*  
 \* B2 \*  
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119 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

120 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION B2

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |      |
|--------------------|--------------|----------------------|-------|-------|---------|------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |      |
| 5.                 | 6.17         | 4.                   | 4.    | 4.    | 4.      |      |
|                    |              | (INCHES)             | .443  | .725  | .725    | .725 |

R3-26

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 \* \*  
 \* RES2 \*  
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123 KO OUTPUT CONTROL VARIABLES  
 IPRNT 1 PRINT CONTROL  
 IPLOT 2 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

124 RS STORAGE ROUTING  
 NSTPS 1 NUMBER OF SUBREACHES  
 ITYP ELEV TYPE OF INITIAL CONDITION  
 RSVRIC 17.00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

125 SV STORAGE .0 .1 .2 .3

126 SE ELEVATION 18.00 19.00 20.00 21.00

127 SL LOW-LEVEL OUTLET  
 ELEV 17.00 ELEVATION AT CENTER OF OUTLET  
 CAREA .79 CROSS-SECTIONAL AREA  
 COOL .60 COEFFICIENT  
 EXPL .50 EXPONENT OF HEAD

128 SS SPILLWAY  
 CREL 20.00 SPILLWAY CREST ELEVATION  
 SPWID 19.00 SPILLWAY WIDTH  
 COOW 2.70 WEIR COEFFICIENT  
 EXPW 1.50 EXPONENT OF HEAD

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COMPUTED OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| OUTFLOW   | .00   | 3.99  | 4.20  | 4.42  | 4.68  | 4.97  | 5.29  | 5.66  | 6.09  | 6.58  |
| ELEVATION | 18.00 | 18.10 | 18.22 | 18.35 | 18.52 | 18.71 | 18.94 | 19.22 | 19.57 | 20.00 |
| OUTFLOW   | 6.67  | 7.12  | 8.22  | 10.26 | 13.55 | 18.38 | 25.03 | 33.80 | 45.00 | 58.90 |
| ELEVATION | 20.01 | 20.04 | 20.10 | 20.17 | 20.26 | 20.37 | 20.50 | 20.64 | 20.81 | 21.00 |

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| STORAGE   | .01   | .02   | .03   | .04   | .05   | .06   | .07   | .08   | .10   | .14   |
| OUTFLOW   | 3.80  | 3.99  | 4.20  | 4.42  | 4.68  | 4.97  | 5.29  | 5.38  | 5.66  | 6.09  |
| ELEVATION | 18.00 | 18.10 | 18.22 | 18.35 | 18.52 | 18.71 | 18.94 | 19.00 | 19.22 | 19.57 |
| STORAGE   | .18   | .19   | .19   | .19   | .20   | .21   | .22   | .24   | .25   | .27   |
| OUTFLOW   | 6.58  | 6.67  | 7.12  | 8.22  | 10.26 | 13.55 | 18.38 | 25.03 | 33.80 | 45.00 |
| ELEVATION | 20.00 | 20.01 | 20.04 | 20.10 | 20.17 | 20.26 | 20.37 | 20.50 | 20.64 | 20.81 |
| STORAGE   | .29   |       |       |       |       |       |       |       |       |       |
| OUTFLOW   | 58.90 |       |       |       |       |       |       |       |       |       |
| ELEVATION | 21.00 |       |       |       |       |       |       |       |       |       |

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HYDROGRAPH AT STATION RES2

R3-27

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| DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | * | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | * | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE |
|----|-----|------|-----|---------|---------|-------|---|----|-----|------|-----|---------|---------|-------|---|----|-----|------|-----|---------|---------|-------|
| 29 | JUN | 1200 | 1   | 0.      | .0      | 18.0  | * | 29 | JUN | 1520 | 101 | 4.      | .0      | 18.0  | * | 29 | JUN | 1840 | 201 | 4.      | .0      | 18.2  |
| 29 | JUN | 1202 | 2   | 4.      | .0      | 18.0  | * | 29 | JUN | 1522 | 102 | 4.      | .0      | 18.0  | * | 29 | JUN | 1842 | 202 | 4.      | .0      | 18.2  |
| 29 | JUN | 1204 | 3   | 4.      | .0      | 18.0  | * | 29 | JUN | 1524 | 103 | 4.      | .0      | 18.0  | * | 29 | JUN | 1844 | 203 | 4.      | .0      | 18.2  |
| 29 | JUN | 1206 | 4   | 4.      | .0      | 18.0  | * | 29 | JUN | 1526 | 104 | 4.      | .0      | 18.0  | * | 29 | JUN | 1846 | 204 | 4.      | .0      | 18.1  |



|                 |    |    |      |   |                 |    |    |      |   |                 |    |    |      |
|-----------------|----|----|------|---|-----------------|----|----|------|---|-----------------|----|----|------|
| 29 JUN 1506 94  | 4. | .0 | 18.0 | * | 29 JUN 1826 194 | 4. | .0 | 18.2 | * | 29 JUN 2146 294 | 4. | .0 | 18.0 |
| 29 JUN 1508 95  | 4. | .0 | 18.0 | * | 29 JUN 1828 195 | 4. | .0 | 18.2 | * | 29 JUN 2148 295 | 4. | .0 | 18.0 |
| 29 JUN 1510 96  | 4. | .0 | 18.0 | * | 29 JUN 1830 196 | 4. | .0 | 18.2 | * | 29 JUN 2150 296 | 4. | .0 | 18.0 |
| 29 JUN 1512 97  | 4. | .0 | 18.0 | * | 29 JUN 1832 197 | 4. | .0 | 18.2 | * | 29 JUN 2152 297 | 4. | .0 | 18.0 |
| 29 JUN 1514 98  | 4. | .0 | 18.0 | * | 29 JUN 1834 198 | 4. | .0 | 18.2 | * | 29 JUN 2154 298 | 4. | .0 | 18.0 |
| 29 JUN 1516 99  | 4. | .0 | 18.0 | * | 29 JUN 1836 199 | 4. | .0 | 18.2 | * | 29 JUN 2156 299 | 4. | .0 | 18.0 |
| 29 JUN 1518 100 | 4. | .0 | 18.0 | * | 29 JUN 1838 200 | 4. | .0 | 18.2 | * | 29 JUN 2158 300 | 4. | .0 | 18.0 |

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| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 4.                 | 6.43         | (CFS) 4.             | 4.    | 4.    | 4.      |
|                    |              | (INCHES) .441        | .727  | .727  | .727    |
|                    |              | (AC-FT) 2.           | 3.    | 3.    | 3.      |

| PEAK STORAGE<br>(AC-FT) | TIME<br>(HR) | MAXIMUM AVERAGE STORAGE |       |       |         |
|-------------------------|--------------|-------------------------|-------|-------|---------|
|                         |              | 6-HR                    | 24-HR | 72-HR | 9.97-HR |
| 0.                      | 6.40         | 0.                      | 0.    | 0.    | 0.      |

| PEAK STAGE<br>(FEET) | TIME<br>(HR) | MAXIMUM AVERAGE STAGE |       |       |         |
|----------------------|--------------|-----------------------|-------|-------|---------|
|                      |              | 6-HR                  | 24-HR | 72-HR | 9.97-HR |
| 18.25                | 6.43         | 18.04                 | 18.02 | 18.02 | 18.02   |

CUMULATIVE AREA = .08 SQ MI

R3-29











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129 KK \*\*\*\*\*  
\* \*  
\* CH4 \*  
\* \*  
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131 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

132 RD MUSKINGUM-CUNGE CHANNEL ROUTING  
L 400. CHANNEL LENGTH  
S .0079 SLOPE  
N .025 CHANNEL ROUGHNESS COEFFICIENT  
CA .00 CONTRIBUTING AREA  
SHAPE TRAP CHANNEL SHAPE  
WD 2.00 BOTTOM WIDTH OR DIAMETER  
Z 3.00 SIDE SLOPE

\*\*\*  
COMPUTED MUSKINGUM-CUNGE PARAMETERS

| ELEMENT | ALPHA | M    | DT<br>(MIN) | DX<br>(FT) | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|------|-------------|------------|---------------|--------------------------|----------------|------------------------------|
| MAIN    | 2.12  | 1.34 | 1.70        | 133.33     | 4.25          | 387.60                   | .72            | 2.86                         |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |  |      |        |     |  |
|------|------|------|------|--|------|--------|-----|--|
| MAIN | 2.12 | 1.34 | 2.00 |  | 4.25 | 388.00 | .72 |  |
|------|------|------|------|--|------|--------|-----|--|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3155E+01 EXCESS= .0000E+00 OUTFLOW= .3142E+01 BASIN STORAGE= .1417E-01 PERCENT ERROR= .0

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HYDROGRAPH AT STATION CH4

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 4.                 | 6.47         | 4.                   | 4.    | 4.    | 4.      |
| (INCHES)           |              | .441                 | .723  | .723  | .723    |
| (AC-FT)            |              | 2.                   | 3.    | 3.    | 3.      |

CUMULATIVE AREA = .08 SQ MI

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133 KK \*\*\*\*\*  
\* \*  
\* B345 \*  
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135 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL

R3-34

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OSCAL      0.  HYDROGRAPH PLOT SCALE
IPNCH      1  PUNCH COMPUTED HYDROGRAPH
IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     300 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS

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SUBBASIN RUNOFF DATA

136 BA SUBBASIN CHARACTERISTICS  
TAREA .01 SUBBASIN AREA

PRECIPITATION DATA

137 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

| HYDRO-35 |        |        | TP-40 |      |      |       | TP-49 |       |       |       |        |
|----------|--------|--------|-------|------|------|-------|-------|-------|-------|-------|--------|
| 5-MIN    | 15-MIN | 60-MIN | 2-HR  | 3-HR | 6-HR | 12-HR | 24-HR | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
| .10      | .19    | .34    | .42   | .47  | .55  | .64   | .70   | .00   | .00   | .00   | .00    |

STORM AREA = .01

138 LS SCS LOSS RATE  
STRTL .08 INITIAL ABSTRACTION  
CRVNB 96.00 CURVE NUMBER  
RTIMP .00 PERCENT IMPERVIOUS AREA

139 UD SCS DIMENSIONLESS UNITGRAPH  
TLAG .50 LAG

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UNIT HYDROGRAPH  
78 END-OF-PERIOD ORDINATES

|     |     |     |     |     |     |     |     |     |     |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| 0.  | 1.  | 1.  | 2.  | 3.  | 4.  | 5.  | 6.  | 7.  | 9.  |
| 10. | 11. | 12. | 12. | 12. | 12. | 12. | 12. | 11. | 11. |
| 10. | 9.  | 9.  | 8.  | 7.  | 6.  | 5.  | 5.  | 4.  | 4.  |
| 4.  | 3.  | 3.  | 3.  | 2.  | 2.  | 2.  | 2.  | 2.  | 1.  |
| 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |

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HYDROGRAPH AT STATION B345

TOTAL RAINFALL = .61, TOTAL LOSS = .32, TOTAL EXCESS = .30

| PEAK FLOW (CFS) | TIME (HR) | MAXIMUM AVERAGE FLOW (CFS) | 6-HR (INCHES) | 24-HR (AC-FT) | 72-HR (INCHES) | 9.97-HR (AC-FT) |
|-----------------|-----------|----------------------------|---------------|---------------|----------------|-----------------|
| 2.              | 5.57      | 0.                         | .290          | 0.            | .290           | 0.              |
|                 |           | 0.                         | 0.            | 0.            | 0.             | 0.              |

CUMULATIVE AREA = .01 SQ MI

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\* \*  
\* B345 \*  
\* \*  
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142 KO OUTPUT CONTROL VARIABLES

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IPRNT      3  PRINT CONTROL
IPLOT      1  PLOT CONTROL
OSCAL      0.  HYDROGRAPH PLOT SCALE
IPNCH      1  PUNCH COMPUTED HYDROGRAPH
IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
ISAV2     300 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS

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143 HC HYDROGRAPH COMBINATION  
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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R3-35

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 6.                 | 5.57         | 4.                   | 4.    | 4.    | 4.      |
| (CFS)              |              | .420                 | .663  | .663  | .663    |
| (INCHES)           |              | 2.                   | 3.    | 3.    | 3.      |
| (AC-FT)            |              |                      |       |       |         |

CUMULATIVE AREA = .09 SQ MI

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 \* \*  
 144 KK \* RES3 \*  
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146 KO OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 1    | PRINT CONTROL                   |
| IPLOT  | 2    | PLOT CONTROL                    |
| OSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

HYDROGRAPH ROUTING DATA

147 RS STORAGE ROUTING

|        |       |                             |
|--------|-------|-----------------------------|
| NSTPS  | 1     | NUMBER OF SUBREACHES        |
| ITYP   | ELEV  | TYPE OF INITIAL CONDITION   |
| RSVRIC | 12.83 | INITIAL CONDITION           |
| X      | .00   | WORKING R AND D COEFFICIENT |

148 SV STORAGE .1 .2 .4 .7

149 SE ELEVATION 14.20 15.20 16.20 17.20

150 SL LOW-LEVEL OUTLET

|       |       |                               |
|-------|-------|-------------------------------|
| ELEVL | 13.83 | ELEVATION AT CENTER OF OUTLET |
| CAREA | .06   | CROSS-SECTIONAL AREA          |
| COOL  | .60   | COEFFICIENT                   |
| EXPL  | .50   | EXPONENT OF HEAD              |

151 SS SPILLWAY

|       |       |                          |
|-------|-------|--------------------------|
| CREL  | 15.75 | SPILLWAY CREST ELEVATION |
| SPWID | 12.30 | SPILLWAY WIDTH           |
| COOW  | 2.70  | WEIR COEFFICIENT         |
| EXPW  | 1.50  | EXPONENT OF HEAD         |

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COMPUTED OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| OUTFLOW   | .00   | .20   | .21   | .23   | .24   | .27   | .29   | .32   | .36   | .42   |
| ELEVATION | 14.20 | 14.25 | 14.31 | 14.39 | 14.49 | 14.61 | 14.77 | 14.99 | 15.30 | 15.75 |
| OUTFLOW   | .48   | .90   | 2.02  | 4.18  | 7.74  | 13.04 | 20.42 | 30.24 | 42.83 | 58.54 |
| ELEVATION | 15.76 | 15.81 | 15.88 | 15.98 | 16.11 | 16.27 | 16.46 | 16.68 | 16.92 | 17.20 |

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| STORAGE   | .08   | .09   | .10   | .11   | .12   | .14   | .16   | .19   | .22   | .24   |
| OUTFLOW   | .18   | .20   | .21   | .23   | .24   | .27   | .29   | .32   | .35   | .36   |
| ELEVATION | 14.20 | 14.25 | 14.31 | 14.39 | 14.49 | 14.61 | 14.77 | 14.99 | 15.20 | 15.30 |
| STORAGE   | .33   | .33   | .34   | .35   | .37   | .40   | .42   | .43   | .48   | .53   |
| OUTFLOW   | .42   | .48   | .90   | 2.02  | 4.18  | 7.74  | 10.49 | 13.04 | 20.42 | 30.24 |
| ELEVATION | 15.75 | 15.76 | 15.81 | 15.88 | 15.98 | 16.11 | 16.20 | 16.27 | 16.46 | 16.68 |
| STORAGE   | .60   | .66   |       |       |       |       |       |       |       |       |
| OUTFLOW   | 42.83 | 58.54 |       |       |       |       |       |       |       |       |
| ELEVATION | 16.92 | 17.20 |       |       |       |       |       |       |       |       |

HYDROGRAPH AT STATION RES3

R3-36

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|             |     |    |    |      |   |             |     |    |    |      |   |             |     |    |    |      |
|-------------|-----|----|----|------|---|-------------|-----|----|----|------|---|-------------|-----|----|----|------|
| 29 JUN 1454 | 88  | 4. | .4 | 16.0 | * | 29 JUN 1814 | 188 | 5. | .4 | 16.0 | * | 29 JUN 2134 | 288 | 4. | .4 | 16.0 |
| 29 JUN 1456 | 89  | 4. | .4 | 16.0 | * | 29 JUN 1816 | 189 | 5. | .4 | 16.0 | * | 29 JUN 2136 | 289 | 4. | .4 | 16.0 |
| 29 JUN 1458 | 90  | 4. | .4 | 16.0 | * | 29 JUN 1818 | 190 | 5. | .4 | 16.0 | * | 29 JUN 2138 | 290 | 4. | .4 | 16.0 |
| 29 JUN 1500 | 91  | 4. | .4 | 16.0 | * | 29 JUN 1820 | 191 | 5. | .4 | 16.0 | * | 29 JUN 2140 | 291 | 4. | .4 | 16.0 |
| 29 JUN 1502 | 92  | 4. | .4 | 16.0 | * | 29 JUN 1822 | 192 | 5. | .4 | 16.0 | * | 29 JUN 2142 | 292 | 4. | .4 | 16.0 |
| 29 JUN 1504 | 93  | 4. | .4 | 16.0 | * | 29 JUN 1824 | 193 | 5. | .4 | 16.0 | * | 29 JUN 2144 | 293 | 4. | .4 | 16.0 |
| 29 JUN 1506 | 94  | 4. | .4 | 16.0 | * | 29 JUN 1826 | 194 | 5. | .4 | 16.0 | * | 29 JUN 2146 | 294 | 4. | .4 | 16.0 |
| 29 JUN 1508 | 95  | 4. | .4 | 16.0 | * | 29 JUN 1828 | 195 | 5. | .4 | 16.0 | * | 29 JUN 2148 | 295 | 4. | .4 | 16.0 |
| 29 JUN 1510 | 96  | 4. | .4 | 16.0 | * | 29 JUN 1830 | 196 | 5. | .4 | 16.0 | * | 29 JUN 2150 | 296 | 4. | .4 | 16.0 |
| 29 JUN 1512 | 97  | 4. | .4 | 16.0 | * | 29 JUN 1832 | 197 | 5. | .4 | 16.0 | * | 29 JUN 2152 | 297 | 4. | .4 | 16.0 |
| 29 JUN 1514 | 98  | 4. | .4 | 16.0 | * | 29 JUN 1834 | 198 | 5. | .4 | 16.0 | * | 29 JUN 2154 | 298 | 4. | .4 | 16.0 |
| 29 JUN 1516 | 99  | 4. | .4 | 16.0 | * | 29 JUN 1836 | 199 | 5. | .4 | 16.0 | * | 29 JUN 2156 | 299 | 4. | .4 | 16.0 |
| 29 JUN 1518 | 100 | 4. | .4 | 16.0 | * | 29 JUN 1838 | 200 | 5. | .4 | 16.0 | * | 29 JUN 2158 | 300 | 4. | .4 | 16.0 |

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|           |      |          |      |                      |       |         |
|-----------|------|----------|------|----------------------|-------|---------|
| PEAK FLOW | TIME |          |      | MAXIMUM AVERAGE FLOW |       |         |
| (CFS)     | (HR) |          | 6-HR | 24-HR                | 72-HR | 9.97-HR |
| 6.        | 5.63 | (CFS)    | 4.   | 4.                   | 4.    | 4.      |
|           |      | (INCHES) | .420 | .606                 | .606  | .606    |
|           |      | (AC-FT)  | 2.   | 3.                   | 3.    | 3.      |

|              |      |  |      |                         |       |         |
|--------------|------|--|------|-------------------------|-------|---------|
| PEAK STORAGE | TIME |  |      | MAXIMUM AVERAGE STORAGE |       |         |
| (AC-FT)      | (HR) |  | 6-HR | 24-HR                   | 72-HR | 9.97-HR |
| 0.           | 5.60 |  | 0.   | 0.                      | 0.    | 0.      |

|            |      |  |       |                       |       |         |
|------------|------|--|-------|-----------------------|-------|---------|
| PEAK STAGE | TIME |  |       | MAXIMUM AVERAGE STAGE |       |         |
| (FEET)     | (HR) |  | 6-HR  | 24-HR                 | 72-HR | 9.97-HR |
| 16.04      | 5.63 |  | 15.99 | 15.88                 | 15.88 | 15.88   |

CUMULATIVE AREA = .09 SQ MI

R3-38



STATION RES3

(I) INFLOW, (O) OUTFLOW

0. 1. 2. 3. 4. 5. 6. 0. 0. 0. 0. 0. 0.  
 0. 0. 0. 0. 0. 0. 0. .1 .2 .3 .4 .0 .0  
 (S) STORAGE

DAHRMN PER

| STATION | RES3 | (I) INFLOW | (O) OUTFLOW | (S) STORAGE |
|---------|------|------------|-------------|-------------|
| 291200  | 11   |            |             | S           |
| 291202  | 2    | I          |             | S           |
| 291204  | 3    | O          | I           | S           |
| 291206  | 4    | O          | I           | S           |
| 291208  | 5    | O          | I           | S           |
| 291210  | 6    | O          | I           | S           |
| 291212  | 7    | O          | I           | S           |
| 291214  | 8    | O          | I           | S           |
| 291216  | 9    | O          | I           | S           |
| 291218  | 10   | O          | I           | S           |
| 291220  | 11   | O          | I           | S           |
| 291222  | 12   | O          | I           | S           |
| 291224  | 13   | O          | I           | S           |
| 291226  | 14   | O          | I           | S           |
| 291228  | 15   | O          | I           | S           |
| 291230  | 16   | O          | I           | S           |
| 291232  | 17   | O          | I           | S           |
| 291234  | 18   | O          | I           | S           |
| 291236  | 19   | O          | I           | S           |
| 291238  | 20   | O          | I           | S           |
| 291240  | 21   | O          | I           | S           |
| 291242  | 22   | O          | I           | S           |
| 291244  | 23   | O          | I           | S           |
| 291246  | 24   | O          | I           | S           |
| 291248  | 25   | O          | I           | S           |
| 291250  | 26   | O          | I           | S           |
| 291252  | 27   | O          | I           | S           |
| 291254  | 28   | O          | I           | S           |
| 291256  | 29   | O          | I           | S           |
| 291258  | 30   | O          | I           | S           |
| 291300  | 31   | O          | I           | S           |
| 291302  | 32   | O          | I           | S           |
| 291304  | 33   | O          | I           | S           |
| 291306  | 34   | O          | I           | S           |
| 291308  | 35   | O          | I           | S           |
| 291310  | 36   | O          | I           | S           |
| 291312  | 37   | O          | I           | S           |
| 291314  | 38   | O          | I           | S           |
| 291316  | 39   | O          | I           | S           |
| 291318  | 40   | O          | I           | S           |
| 291320  | 41   | O          | I           | S           |
| 291322  | 42   | O          | I           | S           |
| 291324  | 43   | O          | I           | S           |
| 291326  | 44   | O          | I           | S           |
| 291328  | 45   | O          | I           | S           |
| 291330  | 46   | O          | I           | S           |
| 291332  | 47   | O          | I           | S           |
| 291334  | 48   | O          | I           | S           |
| 291336  | 49   | O          | I           | S           |
| 291338  | 50   | O          | I           | S           |
| 291340  | 51   | O          | I           | S           |
| 291342  | 52   | O          | I           | S           |
| 291344  | 53   | O          | I           | S           |
| 291346  | 54   | O          | I           | S           |
| 291348  | 55   | O          | I           | S           |
| 291350  | 56   | O          | I           | S           |
| 291352  | 57   | O          | I           | S           |
| 291354  | 58   | O          | I           | S           |
| 291356  | 59   | O          | I           | S           |
| 291358  | 60   | O          | I           | S           |
| 291400  | 61   | O          | I           | S           |
| 291402  | 62   | O          | I           | S           |
| 291404  | 63   | O          | I           | S           |
| 291406  | 64   | O          | I           | S           |
| 291408  | 65   | O          | I           | S           |
| 291410  | 66   | O          | I           | S           |
| 291412  | 67   | O          | I           | S           |
| 291414  | 68   | O          | I           | S           |
| 291416  | 69   | O          | I           | S           |
| 291418  | 70   | O          | I           | S           |
| 291420  | 71   | O          | I           | S           |
| 291422  | 72   | O          | I           | S           |
| 291424  | 73   | O          | I           | S           |
| 291426  | 74   | O          | I           | S           |

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RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

| OPERATION     | STATION | PEAK FLOW | TIME OF PEAK | AVERAGE FLOW FOR MAXIMUM PERIOD |         |         | BASIN AREA | MAXIMUM STAGE | TIME OF MAX STAGE |
|---------------|---------|-----------|--------------|---------------------------------|---------|---------|------------|---------------|-------------------|
|               |         |           |              | 6-HOUR                          | 24-HOUR | 72-HOUR |            |               |                   |
| HYDROGRAPH AT | OF1     | 1.        | 5.60         | 0.                              | 0.      | 0.      | .02        |               |                   |
| ROUTED TO     | CH1     | 1.        | 5.67         | 0.                              | 0.      | 0.      | .02        |               |                   |
| HYDROGRAPH AT | A2      | 1.        | 5.50         | 0.                              | 0.      | 0.      | .01        |               |                   |
| 2 COMBINED AT | A2      | 1.        | 5.63         | 0.                              | 0.      | 0.      | .02        |               |                   |
| ROUTED TO     | ST1     | 1.        | 5.77         | 0.                              | 0.      | 0.      | .02        |               |                   |
| HYDROGRAPH AT | A3      | 1.        | 5.53         | 0.                              | 0.      | 0.      | .01        |               |                   |
| 2 COMBINED AT | A3      | 2.        | 5.70         | 0.                              | 0.      | 0.      | .03        |               |                   |
| HYDROGRAPH AT | OF2     | 1.        | 6.00         | 0.                              | 0.      | 0.      | .02        |               |                   |
| ROUTED TO     | CH2     | 1.        | 6.07         | 0.                              | 0.      | 0.      | .02        |               |                   |
| HYDROGRAPH AT | OF3     | 0.        | 6.10         | 0.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | OF3     | 1.        | 6.07         | 0.                              | 0.      | 0.      | .02        |               |                   |
| 2 COMBINED AT | OF3     | 3.        | 5.77         | 1.                              | 0.      | 0.      | .05        |               |                   |
| HYDROGRAPH AT | A1      | 3.        | 5.87         | 1.                              | 0.      | 0.      | .03        |               |                   |
| 2 COMBINED AT | A1      | 5.        | 5.80         | 2.                              | 1.      | 1.      | .08        |               |                   |
| ROUTED TO     | P1      | 5.        | 5.83         | 1.                              | 1.      | 1.      | .08        |               |                   |
| HYDROGRAPH AT | B1      | 0.        | 5.07         | 0.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | B1      | 5.        | 5.83         | 2.                              | 1.      | 1.      | .08        |               |                   |
| ROUTED TO     | RES1    | 5.        | 6.13         | 4.                              | 4.      | 4.      | .08        | 20.02         | 6.13              |
| ROUTED TO     | CH3     | 5.        | 6.17         | 4.                              | 4.      | 4.      | .08        |               |                   |
| HYDROGRAPH AT | B2      | 0.        | 5.07         | 0.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | B2      | 5.        | 6.17         | 4.                              | 4.      | 4.      | .08        |               |                   |
| ROUTED TO     | RES2    | 4.        | 6.43         | 4.                              | 4.      | 4.      | .08        | 18.25         | 6.43              |
| ROUTED TO     | CH4     | 4.        | 6.47         | 4.                              | 4.      | 4.      | .08        |               |                   |
| HYDROGRAPH AT | B345    | 2.        | 5.57         | 0.                              | 0.      | 0.      | .01        |               |                   |
| 2 COMBINED AT | B345    | 6.        | 5.57         | 4.                              | 4.      | 4.      | .09        |               |                   |
| ROUTED TO     | RES3    | 6.        | 5.63         | 4.                              | 4.      | 4.      | .09        | 16.04         | 5.63              |

R3-43

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
 (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

| ISTAO | ELEMENT | DT<br>(MIN) | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | DT<br>(MIN) | INTERPOLATED TO              |                                      | VOLUME<br>(IN) |
|-------|---------|-------------|---------------|--------------------------|----------------|-------------|------------------------------|--------------------------------------|----------------|
|       |         |             |               |                          |                |             | COMPUTATION<br>PEAK<br>(CFS) | INTERVAL<br>TIME TO<br>PEAK<br>(MIN) |                |
| CH1   | MANE    | 2.00        | .90           | 342.00                   | .12            | 2.00        | .90                          | 342.00                               | .12            |

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1113E+00 EXCESS= .0000E+00 OUTFLOW= .1101E+00 BASIN STORAGE= .1379E-02 PERCENT ERROR= -.1

|     |      |      |      |        |     |      |      |        |     |
|-----|------|------|------|--------|-----|------|------|--------|-----|
| ST1 | MANE | 2.00 | 1.48 | 346.00 | .14 | 2.00 | 1.48 | 346.00 | .14 |
|-----|------|------|------|--------|-----|------|------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1766E+00 EXCESS= .0000E+00 OUTFLOW= .1737E+00 BASIN STORAGE= .2918E-02 PERCENT ERROR= .0

|     |      |      |     |        |     |      |     |        |     |
|-----|------|------|-----|--------|-----|------|-----|--------|-----|
| CH2 | MANE | 2.00 | .70 | 364.00 | .11 | 2.00 | .70 | 364.00 | .11 |
|-----|------|------|-----|--------|-----|------|-----|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1157E+00 EXCESS= .0000E+00 OUTFLOW= .1146E+00 BASIN STORAGE= .1229E-02 PERCENT ERROR= -.1

|    |      |      |      |        |     |      |      |        |     |
|----|------|------|------|--------|-----|------|------|--------|-----|
| P1 | MANE | 1.60 | 5.18 | 350.47 | .17 | 2.00 | 5.18 | 350.00 | .17 |
|----|------|------|------|--------|-----|------|------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7442E+00 EXCESS= .0000E+00 OUTFLOW= .7423E+00 BASIN STORAGE= .1955E-02 PERCENT ERROR= .0

|     |      |      |      |        |     |      |      |        |     |
|-----|------|------|------|--------|-----|------|------|--------|-----|
| CH3 | MANE | 1.30 | 4.52 | 369.20 | .73 | 2.00 | 4.52 | 370.00 | .73 |
|-----|------|------|------|--------|-----|------|------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3161E+01 EXCESS= .0000E+00 OUTFLOW= .3149E+01 BASIN STORAGE= .1282E-01 PERCENT ERROR= .0

|     |      |      |      |        |     |      |      |        |     |
|-----|------|------|------|--------|-----|------|------|--------|-----|
| CH4 | MANE | 1.70 | 4.25 | 387.60 | .72 | 2.00 | 4.25 | 388.00 | .72 |
|-----|------|------|------|--------|-----|------|------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3155E+01 EXCESS= .0000E+00 OUTFLOW= .3142E+01 BASIN STORAGE= .1417E-01 PERCENT ERROR= .0

\*\*\* NORMAL END OF HEC-1 \*\*\*

R3-44

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
* RUN DATE 07/01/1994 TIME 20:14:45 *
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,  
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

DEVELOPED 100 YEAR STORM  
 BASINS "OF1, OF2, OF3, A AND B"  
 RUN #4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

1 ID GRAND VIEW SUBDIVISION
2 ID DEVELOPED CONDITION
3 ID 100 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)
4 IT 2 29JUN94 1200 300
5 IO 5 2 0
  * *****

6 KK OF1
7 KM Basin runoff calculation for OF1
8 KO 3 1 0 1 21
9 BA 0.0175
10 PH 0 0.39 0.76 1.34 1.40 1.44 1.56 1.69 2.01
11 LS 91
12 UD 0.304
  * *****

13 KK CH1
14 KM Muskingum-Cunge channel routing from CP1 to CP3
15 KO 3 1 0 1 21
16 RD 760 0.0118 0.025 TRAP 2 3
  * *****

17 KK A2
18 KM Basin runoff calculation for A2
19 KO 3 1 0 1 21
20 BA 0.0051
21 PH 0 0.39 0.76 1.34 1.40 1.44 1.56 1.69 2.01
22 LS 95
23 UD 0.293
  * *****

24 KK A2
25 KM Combining two hydrographs at control point CP3
26 KO 3 1 0 1 21
27 HC 2
  * *****

28 KK ST1
29 KM Muskingum-Cunge channel routing from CP3 to CP4
30 KO 3 1 0 1 21
31 RD
32 RC 0.020 0.020 0.020 966 0.0084
33 RX 100 104 105 105 106.5 112.5 118.5 120.5
34 RY 34.86 34.78 34.53 34.40 34.53 34.65 34.77 34.81
  * *****

35 KK A3
36 KM Basin runoff calculation for A3
37 KO 3 1 0 1 21
38 BA 0.0053
39 PH 0 0.39 0.76 1.34 1.40 1.44 1.56 1.69 2.01
40 LS 95
41 UD 0.316
  * *****
    
```

R4-2



HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

42 KK A3
43 KM Combining two hydrographs at control point CP4
44 KO 3 1 0 1 21
45 HC 2
* *****

46 KK OF2
47 KM Basin runoff calculation for OF2
48 KO 3 1 0 1 21
49 BA 0.0187
50 PH 0 0.39 0.76 1.34 1.40 1.44 1.56 1.69 2.01
51 LS 91
52 UD 0.478
* *****

53 KK CH2
54 KM Muskingum-Cunge channel routing from CP2 to CP4
55 KO 3 1 0 1 21
56 RD 630 0.013 0.025 TRAP 2 3
* *****

57 KK OF3
58 KM Basin runoff calculation for OF3
59 KO 3 1 0 1 21
60 BA 0.0048
61 PH 0 0.39 0.76 1.34 1.40 1.44 1.56 1.69 2.01
62 LS 88
63 UD 0.478
* *****

64 KK OF3
65 KM Combining two hydrographs at control point CP4
66 KO 3 1 0 1 21
67 HC 2
* *****

68 KK OF3
69 KM Combining two hydrographs at control point CP4
70 KO 3 1 0 1 21
71 HC 2
* *****

72 KK A1
73 KM Basin runoff calculation for A1
74 KO 3 1 0 1 21
75 BA 0.0291
76 PH 0 0.39 0.76 1.34 1.40 1.44 1.56 1.69 2.01
77 LS 95
78 UD 0.510
* *****
    
```

R4-3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

79 KK A1
80 KM Combining two hydrographs at control point CP4
81 KO 3 1 0 1 21
82 HC 2
* *****

83 KK P1
84 KM Muskingum-Cunge channel routing from CP4 to CP5
85 KO 3 1 0 1 21
86 RD 525 0.0060 0.015 CIRC 3.0
* *****

87 KK B1
88 KM Basin runoff calculation for B1
89 KO 3 1 0 1 21
90 BA 0.0005
91 PH 0 0.39 0.76 1.34 1.40 1.44 1.56 1.69 2.01
92 LS 96
93 UD 0.037
* *****

94 KK B1
95 KM Combining two hydrographs at control point CP5
96 KO 3 1 0 1 21
97 HC 2
* *****

98 KK RES1
99 KM Reservoir routing operation
100 KO 1 2 0 1 21
101 RS 1 ELEV 18.6
102 SV 0.0092 0.0613 0.1637 0.3107 0.4578
103 SE 19.6 20.6 21.6 22.6 23.6
104 SL 18.6 0.79 0.6 0.5
105 SS 22.6 19.0 2.7 1.5
* *****

106 KK CH3
107 KM Muskingum-Cunge channel routing from CP5 to CP6
108 KO 3 1 0 1 21
109 RD 310 0.0052 0.025 TRAP 2 3
* *****

110 KK B2
111 KM Basin runoff calculation for B2
112 KO 3 1 0 1 21
113 BA 0.0005
114 PH 0 0.39 0.76 1.34 1.40 1.44 1.56 1.69 2.01
115 LS 96
116 UD 0.037
* *****
    
```

R4-4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

117 KK B2  
 118 KM Combining two hydrographs at control point CP6  
 119 KO 3 1 0 1 21  
 120 HC 2  
 \* \*\*\*\*\*

121 KK RES2  
 122 KM Reservoir routing operation  
 123 KO 1 2 0 1 21  
 124 RS 1 ELEV 17.0  
 125 SV 0.0150 0.0768 0.184 0.2916  
 126 SE 18.0 19.0 20.0 21.0  
 127 SL 17.0 0.79 0.6 0.5  
 128 SS 20.0 19.0 2.7 1.5  
 \* \*\*\*\*\*

129 KK CH4  
 130 KM Muskingum-Cunge channel routing from CP6 to CP7  
 131 KO 3 1 0 1 21  
 132 RD 400 0.0079 0.025 TRAP 2 3  
 \* \*\*\*\*\*

133 KK 8345  
 134 KM Basin runoff calculation for 8345  
 135 KO 3 1 0 1 21  
 136 BA 0.0132  
 137 PH 0 0.39 0.76 1.34 1.40 1.44 1.56 1.69 2.01  
 138 LS 96  
 139 UD 0.323  
 \* \*\*\*\*\*

140 KK 8345  
 141 KM Combining two hydrographs at control point CP7  
 142 KO 3 1 0 1 21  
 143 HC 2  
 \* \*\*\*\*\*

144 KK RES3  
 145 KM Reservoir routing operation  
 146 KO 1 2 0 1 21  
 147 RS 1 ELEV 12.83  
 148 SV 0.0817 0.2218 0.4160 0.6632  
 149 SE 14.2 15.2 16.2 17.2  
 150 SL 13.83 0.0625 0.6 0.5  
 151 SS 15.75 14.0 2.7 1.5  
 \* \*\*\*\*\*

152 ZZ

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
* RUN DATE 07/01/1994 TIME 20:14:45 *
*
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*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

GRAND VIEW SUBDIVISION  
DEVELOPED CONDITION  
100 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)

5 IO OUTPUT CONTROL VARIABLES

IPRNT 5 PRINT CONTROL  
IPLOT 2 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA

NMIN 2 MINUTES IN COMPUTATION INTERVAL  
IDATE 29JUN94 STARTING DATE  
ITIME 1200 STARTING TIME  
NO 300 NUMBER OF HYDROGRAPH ORDINATES  
NDDATE 29JUN94 ENDING DATE  
NDTIME 2158 ENDING TIME  
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .03 HOURS  
TOTAL TIME BASE 9.97 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES  
PRECIPITATION DEPTH INCHES  
LENGTH, ELEVATION FEET  
FLOW CUBIC FEET PER SECOND  
STORAGE VOLUME ACRE-FEET  
SURFACE AREA ACRES  
TEMPERATURE DEGREES FAHRENHEIT

\*\*\* \*\*

```

*****
*
* OF1 *
*
*****

```

8 KO OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

9 BA SUBBASIN CHARACTERISTICS  
TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

| HYDRO-35 |        |        | TP-40 |      |      |       | TP-49 |       |       |       |        |
|----------|--------|--------|-------|------|------|-------|-------|-------|-------|-------|--------|
| 5-MIN    | 15-MIN | 60-MIN | 2-HR  | 3-HR | 6-HR | 12-HR | 24-HR | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
| .39      | .76    | 1.34   | 1.40  | 1.44 | 1.56 | 1.69  | 2.01  | .00   | .00   | .00   | .00    |

STORM AREA = .02

11 IS SCS LOSS RATE

R4-6

CRVNR 91.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

12 UD

SCS DIMENSIONLESS UNITGRAPH  
 TLAG .30 LAG

\*\*\*

UNIT HYDROGRAPH  
 48 END-OF-PERIOD ORDINATES

|     |     |     |     |     |     |     |     |     |     |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| 1.  | 3.  | 5.  | 9.  | 13. | 18. | 22. | 25. | 26. | 26. |
| 25. | 24. | 22. | 19. | 16. | 13. | 11. | 9.  | 8.  | 7.  |
| 6.  | 5.  | 4.  | 3.  | 3.  | 2.  | 2.  | 2.  | 1.  | 1.  |
| 1.  | 1.  | 1.  | 1.  | 1.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |

\*\*\* \*\*

HYDROGRAPH AT STATION OF1

TOTAL RAINFALL = 1.65, TOTAL LOSS = .79, TOTAL EXCESS = .87

| PEAK FLOW (CFS) | TIME (HR) | MAXIMUM AVERAGE FLOW 6-HR | 24-HR | 72-HR | 9.97-HR |
|-----------------|-----------|---------------------------|-------|-------|---------|
| 14.             | 5.33      | (CFS) 2.                  | 1.    | 1.    | 1.      |
|                 |           | (INCHES) .861             | .861  | .861  | .861    |
|                 |           | (AC-FT) 1.                | 1.    | 1.    | 1.      |

CUMULATIVE AREA = .02 SQ MI

\*\*\* \*\*

\*\*\*\*\*  
 \* \*  
 \* CH1 \*  
 \* \*  
 \*\*\*\*\*

13 KK

15 KO

OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 3    | PRINT CONTROL                   |
| IPLOT  | 1    | PLOT CONTROL                    |
| OSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

HYDROGRAPH ROUTING DATA

16 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

|       |       |                               |
|-------|-------|-------------------------------|
| L     | 760.  | CHANNEL LENGTH                |
| S     | .0118 | SLOPE                         |
| N     | .025  | CHANNEL ROUGHNESS COEFFICIENT |
| CA    | .00   | CONTRIBUTING AREA             |
| SHAPE | TRAP  | CHANNEL SHAPE                 |
| WD    | 2.00  | BOTTOM WIDTH OR DIAMETER      |
| Z     | 3.00  | SIDE SLOPE                    |

\*\*\*

COMPUTED MUSKINGUM-CUNGE PARAMETERS

| ELEMENT | ALPHA | M    | DT (MIN) | DX (FT) | PEAK (CFS) | TIME TO PEAK (MIN) | VOLUME (IN) | MAXIMUM CELERITY (FPS) |
|---------|-------|------|----------|---------|------------|--------------------|-------------|------------------------|
| MAIN    | 2.60  | 1.34 | 2.00     | 253.33  | 13.80      | 322.00             | .86         | 4.49                   |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |  |       |        |     |  |
|------|------|------|------|--|-------|--------|-----|--|
| MAIN | 2.60 | 1.34 | 2.00 |  | 13.80 | 322.00 | .86 |  |
|------|------|------|------|--|-------|--------|-----|--|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8035E+00 EXCESS= .0000E+00 OUTFLOW= .8017E+00 BASIN STORAGE= .2569E-02 PERCENT ERROR= -.1

\*\*\* \*\*

HYDROGRAPH AT STATION CH1

R4-7

|                    |              |          |      |       |       |         |
|--------------------|--------------|----------|------|-------|-------|---------|
| PEAK FLOW<br>(CFS) | TIME<br>(HR) |          | 6-HR | 24-HR | 72-HR | 9.97-HR |
| 14.                | 5.37         | (CFS)    | 2.   | 1.    | 1.    | 1.      |
|                    |              | (INCHES) | .859 | .859  | .859  | .859    |
|                    |              | (AC-FT)  | 1.   | 1.    | 1.    | 1.      |

CUMULATIVE AREA = .02 SQ MI

\*\*\* \*\*

```

*****
*
*
17 KK *      A2 *
*      *
*****

```

```

19 KO      OUTPUT CONTROL VARIABLES
          IPRNT      3  PRINT CONTROL
          IPLOT      1  PLOT CONTROL
          OSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      1  PUNCH COMPUTED HYDROGRAPH
          IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
          ISAV2     300 LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

SUBBASIN RUNOFF DATA

```

20 BA      SUBBASIN CHARACTERISTICS
          TAREA      .01 SUBBASIN AREA

```

PRECIPITATION DATA

```

21 PH      DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
          HYDRO-35      TP-40      TP-49
          5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
          .39   .76   1.34   1.40  1.44  1.56  1.69  2.01   .00   .00   .00   .00
          STORM AREA = .01

```

```

22 LS      SCS LOSS RATE
          STRTL      .11 INITIAL ABSTRACTION
          CRVNB      95.00 CURVE NUMBER
          RTIMP      .00 PERCENT IMPERVIOUS AREA

```

```

23 UD      SCS DIMENSIONLESS UNITGRAPH
          TLAG      .29 LAG

```

\*\*\*

UNIT HYDROGRAPH  
46 END-OF-PERIOD ORDINATES

|    |    |    |    |    |    |    |    |    |    |
|----|----|----|----|----|----|----|----|----|----|
| 0. | 1. | 2. | 3. | 4. | 6. | 7. | 8. | 8. | 8. |
| 7. | 7. | 6. | 5. | 4. | 4. | 3. | 2. | 2. | 2. |
| 2. | 1. | 1. | 1. | 1. | 1. | 1. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |

\*\*\* \*\*

HYDROGRAPH AT STATION A2

TOTAL RAINFALL = 1.65, TOTAL LOSS = .50, TOTAL EXCESS = 1.16

|                    |              |          |       |       |       |         |
|--------------------|--------------|----------|-------|-------|-------|---------|
| PEAK FLOW<br>(CFS) | TIME<br>(HR) |          | 6-HR  | 24-HR | 72-HR | 9.97-HR |
| 5.                 | 5.30         | (CFS)    | 1.    | 0.    | 0.    | 0.      |
|                    |              | (INCHES) | 1.149 | 1.149 | 1.149 | 1.149   |
|                    |              | (AC-FT)  | 0.    | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .01 SQ MI

\*\*\* \*\*

```

*****
*
*
24 KK *      A2 *
*      *

```

24-8

26 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

27 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

\*\*\*

\*\*\* \*\*

HYDROGRAPH AT STATION A2

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 19.                | 5.37         | 2.                   | 1.    | 1.    | 1.      |
|                    |              | (INCHES)             | .924  | .924  | .924    |
|                    |              | (AC-FT)              | 1.    | 1.    | 1.      |

CUMULATIVE AREA = .02 SQ MI

\*\*\* \*\*

\*\*\*\*\*  
 \* \*  
 \* ST1 \*  
 \* \*  
 \*\*\*\*\*

30 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

31 RD MUSKINGUM-CUNGE CHANNEL ROUTING

32 RC NORMAL DEPTH CHANNEL  
 ANL .020 LEFT OVERBANK N-VALUE  
 ANCH .020 MAIN CHANNEL N-VALUE  
 ANR .020 RIGHT OVERBANK N-VALUE  
 RLNTH 966. REACH LENGTH  
 SEL .0084 ENERGY SLOPE  
 ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

CROSS-SECTION DATA

|                 | --- LEFT OVERBANK --- | + | ----- MAIN CHANNEL ----- | + | --- RIGHT OVERBANK --- |
|-----------------|-----------------------|---|--------------------------|---|------------------------|
| 34 RY ELEVATION | 34.86                 |   | 34.53                    |   | 34.77                  |
| 33 RX DISTANCE  | 100.00                |   | 105.00                   |   | 120.50                 |

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| STORAGE   | .00   | .00   | .00   | .00   | .00   | .00   | .00   | .01   | .01   |
| OUTFLOW   | .00   | .00   | .01   | .02   | .05   | .08   | .12   | .19   | .32   |
| ELEVATION | 34.40 | 34.42 | 34.45 | 34.47 | 34.50 | 34.52 | 34.55 | 34.57 | 34.62 |
| STORAGE   | .01   | .02   | .02   | .03   | .04   | .04   | .05   | .06   | .08   |
| OUTFLOW   | .76   | 1.16  | 1.68  | 2.30  | 3.04  | 3.90  | 4.86  | 5.95  | 7.32  |
| ELEVATION | 34.64 | 34.67 | 34.69 | 34.71 | 34.74 | 34.76 | 34.79 | 34.81 | 34.86 |

\*\*\*\*\* WARNING \*\*\*\*\* THE FLOW RATE THAT YOU ARE ROUTING IS GREATER THAN WHAT CAN BE CALCULATE FROM THE 8 POINT CROSS SECTION YOU ENTERED ON RC, RX, AND RY RECORDS. THE PROGRAM HAD TO EXTRAPOLATE BEYOND THE MAXIMUM STORAGE-DISCHARGE VALUE CALCULATED. INCREASE ELMAX ON THE RC RECORD OR MAKE THE CROSS SECTION LARGER

RA-9

COMPUTED MUSKINGUM-CUNGE PARAMETERS

| ELEMENT | ALPHA | COMPUTATION TIME STEP |             | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|-----------------------|-------------|---------------|--------------------------|----------------|------------------------------|
|         |       | M                     | DT<br>(MIN) |               |                          |                |                              |
| MAIN    |       |                       | 2.00        | 193.20        | 19.07                    | 326.00         | 3.30                         |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |  |  |      |  |       |        |     |
|------|--|--|------|--|-------|--------|-----|
| MAIN |  |  | 2.00 |  | 19.07 | 326.00 | .92 |
|------|--|--|------|--|-------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1114E+01 EXCESS= .0000E+00 OUTFLOW= .1110E+01 BASIN STORAGE= .5794E-02 PERCENT ERROR= -.2

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HYDROGRAPH AT STATION ST1

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 19.                | 5.43         | (CFS) 2.             | 1.    | 1.    | 1.      |
|                    |              | (INCHES) .921        | .921  | .921  | .921    |
|                    |              | (AC-FT) 1.           | 1.    | 1.    | 1.      |
| CUMULATIVE AREA =  |              | .02 SQ MI            |       |       |         |

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\* \*  
\* A3 \*  
\* \*  
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37 KO OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 3    | PRINT CONTROL                   |
| IPLOT  | 1    | PLOT CONTROL                    |
| OSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

SUBBASIN RUNOFF DATA

38 BA SUBBASIN CHARACTERISTICS

|       |     |               |
|-------|-----|---------------|
| TAREA | .01 | SUBBASIN AREA |
|-------|-----|---------------|

PRECIPITATION DATA

39 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

| HYDRO-35         |        |        | TP-40 |      |      |       |       | TP-49 |       |       |        |
|------------------|--------|--------|-------|------|------|-------|-------|-------|-------|-------|--------|
| 5-MIN            | 15-MIN | 60-MIN | 2-HR  | 3-HR | 6-HR | 12-HR | 24-HR | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
| .39              | .76    | 1.34   | 1.40  | 1.44 | 1.56 | 1.69  | 2.01  | .00   | .00   | .00   | .00    |
| STORM AREA = .01 |        |        |       |      |      |       |       |       |       |       |        |

40 LS SCS LOSS RATE

|       |       |                         |
|-------|-------|-------------------------|
| STRTL | .11   | INITIAL ABSTRACTION     |
| CRVNB | 95.00 | CURVE NUMBER            |
| RTIMP | .00   | PERCENT IMPERVIOUS AREA |

41 UD SCS DIMENSIONLESS UNITGRAPH

|      |     |     |
|------|-----|-----|
| TLAG | .32 | LAG |
|------|-----|-----|

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UNIT HYDROGRAPH  
49 END-OF-PERIOD ORDINATES

|    |    |    |    |    |    |    |    |    |    |
|----|----|----|----|----|----|----|----|----|----|
| 0. | 1. | 1. | 2. | 4. | 5. | 6. | 7. | 8. | 8. |
| 8. | 7. | 7. | 6. | 5. | 4. | 4. | 3. | 3. | 2. |
| 2. | 2. | 1. | 1. | 1. | 1. | 1. | 1. | 1. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |

R4-10

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HYDROGRAPH AT STATION A3

TOTAL RAINFALL = 1.65, TOTAL LOSS = .50, TOTAL EXCESS = 1.16

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 5.                 | 5.33         | (CFS) 1.             | 0.    | 0.    | 0.      |
|                    |              | (INCHES) 1.148       | 1.148 | 1.148 | 1.148   |
|                    |              | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .01 SQ MI

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\* \*  
\* A3 \*  
\* \*  
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42 KK

44 KO

OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 3    | PRINT CONTROL                   |
| IPLOT  | 1    | PLOT CONTROL                    |
| OSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

45 HC

HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION A3

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 24.                | 5.40         | (CFS) 3.             | 2.    | 2.    | 2.      |
|                    |              | (INCHES) .964        | .964  | .964  | .964    |
|                    |              | (AC-FT) 1.           | 1.    | 1.    | 1.      |

CUMULATIVE AREA = .03 SQ MI

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\* \*  
\* OF2 \*  
\* \*  
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46 KK

48 KO

OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 3    | PRINT CONTROL                   |
| IPLOT  | 1    | PLOT CONTROL                    |
| OSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

SUBBASIN RUNOFF DATA

49 BA

SUBBASIN CHARACTERISTICS

TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

50 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

| HYDRO-35 |        | TP-40  |      |      |      | TP-49 |       |       |       |       |        |
|----------|--------|--------|------|------|------|-------|-------|-------|-------|-------|--------|
| 5-MIN    | 15-MIN | 60-MIN | 2-HR | 3-HR | 6-HR | 12-HR | 24-HR | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
| .39      | .76    | 1.34   | 1.40 | 1.44 | 1.56 | 1.69  | 2.01  | .00   | .00   | .00   | .00    |

RA-11

51 LS SCS LOSS RATE  
 STRTL .20 INITIAL ABSTRACTION  
 CRVNR 91.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

52 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .48 LAG

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UNIT HYDROGRAPH  
 74 END-OF-PERIOD ORDINATES

|     |     |     |     |     |     |     |     |     |     |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| 0.  | 1.  | 2.  | 3.  | 4.  | 6.  | 8.  | 10. | 12. | 14. |
| 16. | 17. | 18. | 18. | 18. | 18. | 18. | 17. | 16. | 15. |
| 14. | 13. | 11. | 10. | 9.  | 8.  | 7.  | 6.  | 6.  | 5.  |
| 5.  | 4.  | 4.  | 3.  | 3.  | 3.  | 2.  | 2.  | 2.  | 2.  |
| 2.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  |
| 1.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |

\*\*\* \*\*

HYDROGRAPH AT STATION OF2

TOTAL RAINFALL = 1.65, TOTAL LOSS = .79, TOTAL EXCESS = .87

|                 |           |            |                |                |                  |
|-----------------|-----------|------------|----------------|----------------|------------------|
| PEAK FLOW (CFS) | TIME (HR) | 6-HR (CFS) | 24-HR (INCHES) | 72-HR (INCHES) | 9.97-HR (INCHES) |
| 11.             | 5.53      | 2.         | .857           | .857           | .857             |
|                 |           | (AC-FT)    | 1.             | 1.             | 1.               |

CUMULATIVE AREA = .02 SQ MI

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 \* \*  
 53 KK \* CH2 \*  
 \* \*  
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55 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

56 RD MUSKINGUM-CUNGE CHANNEL ROUTING  
 L 630. CHANNEL LENGTH  
 S .0130 SLOPE  
 N .025 CHANNEL ROUGHNESS COEFFICIENT  
 CA .00 CONTRIBUTING AREA  
 SHAPE TRAP CHANNEL SHAPE  
 WD 2.00 BOTTOM WIDTH OR DIAMETER  
 Z 3.00 SIDE SLOPE

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COMPUTED MUSKINGUM-CUNGE PARAMETERS

| ELEMENT | ALPHA | M    | DT (MIN) | DX (FT) | PEAK (CFS) | TIME TO PEAK (MIN) | VOLUME (IN) | MAXIMUM CELERITY (FPS) |
|---------|-------|------|----------|---------|------------|--------------------|-------------|------------------------|
| MAIN    | 2.72  | 1.34 | 2.00     | 315.00  | 11.16      | 334.00             | .86         | 4.40                   |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |  |       |        |     |  |
|------|------|------|------|--|-------|--------|-----|--|
| MAIN | 2.72 | 1.34 | 2.00 |  | 11.16 | 334.00 | .86 |  |
|------|------|------|------|--|-------|--------|-----|--|

RA-12

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HYDROGRAPH AT STATION CH2

|                           |                      |                              |                          |   |                           |                             |
|---------------------------|----------------------|------------------------------|--------------------------|---|---------------------------|-----------------------------|
| PEAK FLOW<br>(CFS)<br>11. | TIME<br>(HR)<br>5.57 | (CFS)<br>(INCHES)<br>(AC-FT) | 6-HR<br>2.<br>.855<br>1. | MAXIMUM AVERAGE FLOW<br>24-HR<br>1.<br>.855<br>1. | 72-HR<br>1.<br>.855<br>1. | 9.97-HR<br>1.<br>.855<br>1. |
| CUMULATIVE AREA =         |                      |                              | .02 SQ MI                |   |                           |                             |

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\* OF3 \*  
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59 KO OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 3    | PRINT CONTROL                   |
| IPLT   | 1    | PLOT CONTROL                    |
| OSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

SUBBASIN RUNOFF DATA

60 BA SUBBASIN CHARACTERISTICS

|       |     |               |
|-------|-----|---------------|
| TAREA | .00 | SUBBASIN AREA |
|-------|-----|---------------|

PRECIPITATION DATA

61 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

|          |        |        |       |      |      |       |       |       |       |       |        |
|----------|--------|--------|-------|------|------|-------|-------|-------|-------|-------|--------|
| HYDRO-35 |        |        | TP-40 |      |      |       | TP-49 |       |       |       |        |
| 5-MIN    | 15-MIN | 60-MIN | 2-HR  | 3-HR | 6-HR | 12-HR | 24-HR | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
| .39      | .76    | 1.34   | 1.40  | 1.44 | 1.56 | 1.69  | 2.01  | .00   | .00   | .00   | .00    |

STORM AREA = .00

62 LS SCS LOSS RATE

|       |       |                         |
|-------|-------|-------------------------|
| STRTL | .27   | INITIAL ABSTRACTION     |
| CRVNR | 88.00 | CURVE NUMBER            |
| RTIMP | .00   | PERCENT IMPERVIOUS AREA |

63 UD SCS DIMENSIONLESS UNITGRAPH

|      |     |     |
|------|-----|-----|
| TLAG | .48 | LAG |
|------|-----|-----|

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UNIT HYDROGRAPH  
74 END-OF-PERIOD ORDINATES

|    |    |    |    |    |    |    |    |    |    |
|----|----|----|----|----|----|----|----|----|----|
| 0. | 0. | 0. | 1. | 1. | 1. | 2. | 3. | 3. | 4. |
| 4. | 4. | 5. | 5. | 5. | 5. | 5. | 4. | 4. | 4. |
| 4. | 3. | 3. | 3. | 2. | 2. | 2. | 2. | 1. | 1. |
| 1. | 1. | 1. | 1. | 1. | 1. | 1. | 1. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |

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HYDROGRAPH AT STATION OF3

TOTAL RAINFALL = 1.65, TOTAL LOSS = .96, TOTAL EXCESS = .70

|                          |                      |                              |                          |   |                           |                             |
|--------------------------|----------------------|------------------------------|--------------------------|---|---------------------------|-----------------------------|
| PEAK FLOW<br>(CFS)<br>2. | TIME<br>(HR)<br>5.57 | (CFS)<br>(INCHES)<br>(AC-FT) | 6-HR<br>0.<br>.685<br>0. | MAXIMUM AVERAGE FLOW<br>24-HR<br>0.<br>.685<br>0. | 72-HR<br>0.<br>.685<br>0. | 9.97-HR<br>0.<br>.685<br>0. |
| CUMULATIVE AREA =        |                      |                              | .00 SQ MI                |   |                           |                             |

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\* OF3 \*  
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66 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

67 HC HYDROGRAPH COMBINATION  
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION OF3

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 13.                | 5.57         | (CFS) 2.             | 1.    | 1.    | 1.      |
|                    |              | (INCHES) .821        | .821  | .821  | .821    |
|                    |              | (AC-FT) 1.           | 1.    | 1.    | 1.      |

CUMULATIVE AREA = .02 SQ MI

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\* \*  
\* OF3 \*  
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70 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

71 HC HYDROGRAPH COMBINATION  
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

\*\*\*

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HYDROGRAPH AT STATION OF3

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 36.                | 5.43         | (CFS) 5.             | 3.    | 3.    | 3.      |
|                    |              | (INCHES) .898        | .898  | .898  | .898    |
|                    |              | (AC-FT) 2.           | 2.    | 2.    | 2.      |

CUMULATIVE AREA = .05 SQ MI

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\* \*  
\* A1 \*  
\* \*  
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72 KK

R4-14

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74 KO  OUTPUT CONTROL VARIABLES
        IPRNT      3  PRINT CONTROL
        IPLOT      1  PLOT CONTROL
        OSCAL      0.  HYDROGRAPH PLOT SCALE
        IPNCH      1  PUNCH COMPUTED HYDROGRAPH
        IOUT       21  SAVE HYDROGRAPH ON THIS UNIT
        ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2     300  LAST ORDINATE PUNCHED OR SAVED
        TIMINT     .033  TIME INTERVAL IN HOURS

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SUBBASIN RUNOFF DATA

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75 BA  SUBBASIN CHARACTERISTICS
        TAREA     .03  SUBBASIN AREA

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PRECIPITATION DATA

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76 PH  DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
        HYDRO-35  TP-40  TP-49
        5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
        .39 .76 1.34 1.40 1.44 1.56 1.69 2.01 .00 .00 .00 .00
        STORM AREA = .03

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77 LS  SCS LOSS RATE
        STRTL     .11  INITIAL ABSTRACTION
        CRVNBR    95.00  CURVE NUMBER
        RTIMP     .00  PERCENT IMPERVIOUS AREA

```

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78 UD  SCS DIMENSIONLESS UNITGRAPH
        TLAG     .51  LAG

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UNIT HYDROGRAPH  
78 END-OF-PERIOD ORDINATES

|     |     |     |     |     |     |     |     |     |     |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| 1.  | 1.  | 2.  | 4.  | 6.  | 8.  | 10. | 13. | 16. | 19. |
| 22. | 24. | 25. | 26. | 27. | 27. | 26. | 26. | 25. | 24. |
| 22. | 21. | 19. | 18. | 16. | 14. | 12. | 11. | 10. | 9.  |
| 8.  | 7.  | 7.  | 6.  | 5.  | 5.  | 4.  | 4.  | 4.  | 3.  |
| 3.  | 3.  | 2.  | 2.  | 2.  | 2.  | 2.  | 1.  | 1.  | 1.  |
| 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |

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HYDROGRAPH AT STATION A1

TOTAL RAINFALL = 1.65, TOTAL LOSS = .50, TOTAL EXCESS = 1.16

```

PEAK FLOW  TIME  MAXIMUM AVERAGE FLOW
(CFS)      (HR)      6-HR 24-HR 72-HR 9.97-HR
22.        5.53      (CFS) 4. 2. 2. 2.
(INCHES) 1.143 1.143 1.143 1.143
(AC-FT) 2. 2. 2. 2.

```

CUMULATIVE AREA = .03 SQ MI

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* *
* A1 *
* *
*****

```

79 KK

```

81 KO  OUTPUT CONTROL VARIABLES
        IPRNT      3  PRINT CONTROL
        IPLOT      1  PLOT CONTROL
        OSCAL      0.  HYDROGRAPH PLOT SCALE
        IPNCH      1  PUNCH COMPUTED HYDROGRAPH
        IOUT       21  SAVE HYDROGRAPH ON THIS UNIT
        ISAV1      1  FIRST ORDINATE PUNCHED OR SAVED
        ISAV2     300  LAST ORDINATE PUNCHED OR SAVED
        TIMINT     .033  TIME INTERVAL IN HOURS

```

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82 HC  HYDROGRAPH COMBINATION
        ICOMP     2  NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION A1

|                           |                      |                     |                           |                        |   |
|---------------------------|----------------------|---------------------|---------------------------|------------------------|---|
| PEAK FLOW<br>(CFS)<br>58. | TIME<br>(HR)<br>5.47 | 6-HR<br>(CFS)<br>9. | 24-HR<br>(INCHES)<br>.987 | 72-HR<br>(AC-FT)<br>4. | MAXIMUM AVERAGE FLOW<br>9.97-HR<br>5.<br>.987<br>4. |
|---------------------------|----------------------|---------------------|---------------------------|------------------------|---|

CUMULATIVE AREA = .08 SQ MI

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\* \*  
\* P1 \*  
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85 KO OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 3    | PRINT CONTROL                   |
| IPLOT  | 1    | PLOT CONTROL                    |
| OSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

HYDROGRAPH ROUTING DATA

86 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

|       |       |                               |
|-------|-------|-------------------------------|
| L     | 525.  | CHANNEL LENGTH                |
| S     | .0060 | SLOPE                         |
| N     | .015  | CHANNEL ROUGHNESS COEFFICIENT |
| CA    | .00   | CONTRIBUTING AREA             |
| SHAPE | CIRC  | CHANNEL SHAPE                 |
| WD    | 3.00  | BOTTOM WIDTH OR DIAMETER      |
| Z     | .00   | SIDE SLOPE                    |

\*\*\*

COMPUTED MUSKINGUM-CUNGE PARAMETERS

| ELEMENT | ALPHA | M    | DT<br>(MIN) | DX<br>(FT) | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|------|-------------|------------|---------------|--------------------------|----------------|------------------------------|
| MAIN    | 4.99  | 1.25 | .99         | 525.00     | 57.49         | 329.26                   | .99            | 8.85                         |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |  |       |        |     |  |
|------|------|------|------|--|-------|--------|-----|--|
| MAIN | 4.99 | 1.25 | 2.00 |  | 57.48 | 330.00 | .99 |  |
|------|------|------|------|--|-------|--------|-----|--|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4237E+01 EXCESS= .0000E+00 OUTFLOW= .4234E+01 BASIN STORAGE= .3330E-02 PERCENT ERROR= .0

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HYDROGRAPH AT STATION P1

|                           |                      |                     |                           |                        |   |
|---------------------------|----------------------|---------------------|---------------------------|------------------------|---|
| PEAK FLOW<br>(CFS)<br>57. | TIME<br>(HR)<br>5.50 | 6-HR<br>(CFS)<br>9. | 24-HR<br>(INCHES)<br>.986 | 72-HR<br>(AC-FT)<br>4. | MAXIMUM AVERAGE FLOW<br>9.97-HR<br>5.<br>.986<br>4. |
|---------------------------|----------------------|---------------------|---------------------------|------------------------|---|

CUMULATIVE AREA = .08 SQ MI

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RA-16

01 AN  
\* \* \*  
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89 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

90 BA SUBBASIN CHARACTERISTICS  
TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

91 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
..... HYDRO-35 ..... TP-40 ..... TP-49 .....  
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
.39 .76 1.34 1.40 1.44 1.56 1.69 2.01 .00 .00 .00 .00  
STORM AREA = .00

92 LS SCS LOSS RATE  
STRTL .08 INITIAL ABSTRACTION  
CRVNB 96.00 CURVE NUMBER  
RTIMP .00 PERCENT IMPERVIOUS AREA

93 UD SCS DIMENSIONLESS UNITGRAPH  
TLAG .04 LAG

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UNIT HYDROGRAPH  
8 END-OF-PERIOD ORDINATES

3. 4. 2. 1. 0. 0. 0. 0.  
\*\*\* \*\*

HYDROGRAPH AT STATION B1

TOTAL RAINFALL = 1.65, TOTAL LOSS = .41, TOTAL EXCESS = 1.24

| PEAK FLOW (CFS) | TIME (HR) | 6-HR (CFS) | 24-HR (INCHES) | 72-HR (AC-FT) | 9.97-HR (INCHES) |
|-----------------|-----------|------------|----------------|---------------|------------------|
| 1.              | 5.03      | 0.         | 1.237          | 0.            | 1.241            |
|                 |           | 0.         | 1.241          | 0.            | 1.241            |
|                 |           | 0.         | 0.             | 0.            | 0.               |

CUMULATIVE AREA = .00 SQ MI

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\* \*  
\* B1 \*  
\* \*  
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94 KK

96 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

97 HC HYDROGRAPH COMBINATION  
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

\*\*\*

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R4-17

|                           |                      |                              |                          |   |                           |                             |
|---------------------------|----------------------|------------------------------|--------------------------|---|---------------------------|-----------------------------|
| PEAK FLOW<br>(CFS)<br>58. | TIME<br>(HR)<br>5.50 | (CFS)<br>(INCHES)<br>(AC-FT) | 6-HR<br>9.<br>.988<br>4. | MAXIMUM AVERAGE FLOW<br>24-HR<br>5.<br>.988<br>4. | 72-HR<br>5.<br>.988<br>4. | 9.97-HR<br>5.<br>.988<br>4. |
|---------------------------|----------------------|------------------------------|--------------------------|---|---------------------------|-----------------------------|

CUMULATIVE AREA = .08 SQ MI

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\*           \*  
98 KK    \*    RES1   \*  
\*           \*  
\*\*\*\*\*

100 KO           OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 1    | PRINT CONTROL                   |
| IPLOT  | 2    | PLOT CONTROL                    |
| OSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

HYDROGRAPH ROUTING DATA

101 RS           STORAGE ROUTING

|        |       |                             |
|--------|-------|-----------------------------|
| NSTPS  | 1     | NUMBER OF SUBREACHES        |
| ITYP   | ELEV  | TYPE OF INITIAL CONDITION   |
| RSVRIC | 18.60 | INITIAL CONDITION           |
| X      | .00   | WORKING R AND D COEFFICIENT |

102 SV           STORAGE           .0           .1           .2           .3           .5

103 SE           ELEVATION           19.60       20.60       21.60       22.60       23.60

104 SL           LOW-LEVEL OUTLET

|       |       |                               |
|-------|-------|-------------------------------|
| ELEVL | 18.60 | ELEVATION AT CENTER OF OUTLET |
| CAREA | .79   | CROSS-SECTIONAL AREA          |
| COOL  | .60   | COEFFICIENT                   |
| EXPL  | .50   | EXPONENT OF HEAD              |

105 SS           SPILLWAY

|       |       |                          |
|-------|-------|--------------------------|
| CREL  | 22.60 | SPILLWAY CREST ELEVATION |
| SPWID | 19.00 | SPILLWAY WIDTH           |
| COOW  | 2.70  | WEIR COEFFICIENT         |
| EXPW  | 1.50  | EXPONENT OF HEAD         |

\*\*\*

COMPUTED OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| OUTFLOW   | .00   | 4.03  | 4.28  | 4.56  | 4.89  | 5.26  | 5.70  | 6.22  | 6.84  | 7.60  |
| ELEVATION | 19.60 | 19.72 | 19.87 | 20.04 | 20.25 | 20.52 | 20.85 | 21.28 | 21.84 | 22.60 |
| OUTFLOW   | 7.69  | 8.12  | 9.20  | 11.23 | 14.50 | 19.30 | 25.94 | 34.70 | 45.89 | 59.80 |
| ELEVATION | 22.61 | 22.64 | 22.70 | 22.77 | 22.86 | 22.97 | 23.10 | 23.24 | 23.41 | 23.60 |

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| STORAGE   | .01   | .02   | .02   | .03   | .04   | .06   | .06   | .09   | .13   | .16   |
| OUTFLOW   | 3.80  | 4.03  | 4.28  | 4.56  | 4.89  | 5.26  | 5.38  | 5.70  | 6.22  | 6.58  |
| ELEVATION | 19.60 | 19.72 | 19.87 | 20.04 | 20.25 | 20.52 | 20.60 | 20.85 | 21.28 | 21.60 |
| STORAGE   | .20   | .31   | .31   | .32   | .32   | .34   | .35   | .36   | .38   | .41   |
| OUTFLOW   | 6.84  | 7.60  | 7.69  | 8.12  | 9.20  | 11.23 | 14.50 | 19.30 | 25.94 | 34.70 |
| ELEVATION | 21.84 | 22.60 | 22.61 | 22.64 | 22.70 | 22.77 | 22.86 | 22.97 | 23.10 | 23.24 |
| STORAGE   | .43   | .46   |       |       |       |       |       |       |       |       |
| OUTFLOW   | 45.89 | 59.80 |       |       |       |       |       |       |       |       |
| ELEVATION | 23.41 | 23.60 |       |       |       |       |       |       |       |       |

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HYDROGRAPH AT STATION   RES1

R4-18

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|             |    |    |   |      |   |             |     |     |   |      |   |             |     |    |   |      |
|-------------|----|----|---|------|---|-------------|-----|-----|---|------|---|-------------|-----|----|---|------|
| 29 JUN 1200 | 1  | 0. | 0 | 19.6 | * | 29 JUN 1520 | 101 | 4.  | 0 | 19.6 | * | 29 JUN 1840 | 201 | 7. | 3 | 22.4 |
| 29 JUN 1202 | 2  | 4. | 0 | 19.6 | * | 29 JUN 1522 | 102 | 4.  | 0 | 19.6 | * | 29 JUN 1842 | 202 | 7. | 3 | 22.4 |
| 29 JUN 1204 | 3  | 4. | 0 | 19.6 | * | 29 JUN 1524 | 103 | 4.  | 0 | 19.6 | * | 29 JUN 1844 | 203 | 7. | 3 | 22.3 |
| 29 JUN 1206 | 4  | 4. | 0 | 19.6 | * | 29 JUN 1526 | 104 | 4.  | 0 | 19.6 | * | 29 JUN 1846 | 204 | 7. | 3 | 22.2 |
| 29 JUN 1208 | 5  | 4. | 0 | 19.6 | * | 29 JUN 1528 | 105 | 4.  | 0 | 19.6 | * | 29 JUN 1848 | 205 | 7. | 2 | 22.2 |
| 29 JUN 1210 | 6  | 4. | 0 | 19.6 | * | 29 JUN 1530 | 106 | 4.  | 0 | 19.6 | * | 29 JUN 1850 | 206 | 7. | 2 | 22.1 |
| 29 JUN 1212 | 7  | 4. | 0 | 19.6 | * | 29 JUN 1532 | 107 | 4.  | 0 | 19.6 | * | 29 JUN 1852 | 207 | 7. | 2 | 22.0 |
| 29 JUN 1214 | 8  | 4. | 0 | 19.6 | * | 29 JUN 1534 | 108 | 4.  | 0 | 19.6 | * | 29 JUN 1854 | 208 | 7. | 2 | 22.0 |
| 29 JUN 1216 | 9  | 4. | 0 | 19.6 | * | 29 JUN 1536 | 109 | 4.  | 0 | 19.6 | * | 29 JUN 1856 | 209 | 7. | 2 | 21.9 |
| 29 JUN 1218 | 10 | 4. | 0 | 19.6 | * | 29 JUN 1538 | 110 | 4.  | 0 | 19.6 | * | 29 JUN 1858 | 210 | 7. | 2 | 21.8 |
| 29 JUN 1220 | 11 | 4. | 0 | 19.6 | * | 29 JUN 1540 | 111 | 4.  | 0 | 19.6 | * | 29 JUN 1900 | 211 | 7. | 2 | 21.7 |
| 29 JUN 1222 | 12 | 4. | 0 | 19.6 | * | 29 JUN 1542 | 112 | 4.  | 0 | 19.6 | * | 29 JUN 1902 | 212 | 7. | 2 | 21.7 |
| 29 JUN 1224 | 13 | 4. | 0 | 19.6 | * | 29 JUN 1544 | 113 | 4.  | 0 | 19.6 | * | 29 JUN 1904 | 213 | 7. | 2 | 21.6 |
| 29 JUN 1226 | 14 | 4. | 0 | 19.6 | * | 29 JUN 1546 | 114 | 4.  | 0 | 19.6 | * | 29 JUN 1906 | 214 | 6. | 2 | 21.5 |
| 29 JUN 1228 | 15 | 4. | 0 | 19.6 | * | 29 JUN 1548 | 115 | 4.  | 0 | 19.6 | * | 29 JUN 1908 | 215 | 6. | 1 | 21.4 |
| 29 JUN 1230 | 16 | 4. | 0 | 19.6 | * | 29 JUN 1550 | 116 | 4.  | 0 | 19.6 | * | 29 JUN 1910 | 216 | 6. | 1 | 21.3 |
| 29 JUN 1232 | 17 | 4. | 0 | 19.6 | * | 29 JUN 1552 | 117 | 4.  | 0 | 19.6 | * | 29 JUN 1912 | 217 | 6. | 1 | 21.2 |
| 29 JUN 1234 | 18 | 4. | 0 | 19.6 | * | 29 JUN 1554 | 118 | 4.  | 0 | 19.6 | * | 29 JUN 1914 | 218 | 6. | 1 | 21.1 |
| 29 JUN 1236 | 19 | 4. | 0 | 19.6 | * | 29 JUN 1556 | 119 | 4.  | 0 | 19.6 | * | 29 JUN 1916 | 219 | 6. | 1 | 21.0 |
| 29 JUN 1238 | 20 | 4. | 0 | 19.6 | * | 29 JUN 1558 | 120 | 4.  | 0 | 19.6 | * | 29 JUN 1918 | 220 | 6. | 1 | 20.9 |
| 29 JUN 1240 | 21 | 4. | 0 | 19.6 | * | 29 JUN 1600 | 121 | 4.  | 0 | 19.6 | * | 29 JUN 1920 | 221 | 6. | 1 | 20.8 |
| 29 JUN 1242 | 22 | 4. | 0 | 19.6 | * | 29 JUN 1602 | 122 | 4.  | 0 | 19.6 | * | 29 JUN 1922 | 222 | 6. | 1 | 20.7 |
| 29 JUN 1244 | 23 | 4. | 0 | 19.6 | * | 29 JUN 1604 | 123 | 4.  | 0 | 19.6 | * | 29 JUN 1924 | 223 | 5. | 1 | 20.6 |
| 29 JUN 1246 | 24 | 4. | 0 | 19.6 | * | 29 JUN 1606 | 124 | 4.  | 0 | 19.6 | * | 29 JUN 1926 | 224 | 5. | 1 | 20.5 |
| 29 JUN 1248 | 25 | 4. | 0 | 19.6 | * | 29 JUN 1608 | 125 | 4.  | 0 | 19.6 | * | 29 JUN 1928 | 225 | 5. | 0 | 20.4 |
| 29 JUN 1250 | 26 | 4. | 0 | 19.6 | * | 29 JUN 1610 | 126 | 4.  | 0 | 19.6 | * | 29 JUN 1930 | 226 | 5. | 0 | 20.2 |
| 29 JUN 1252 | 27 | 4. | 0 | 19.6 | * | 29 JUN 1612 | 127 | 4.  | 0 | 19.6 | * | 29 JUN 1932 | 227 | 5. | 0 | 20.1 |
| 29 JUN 1254 | 28 | 4. | 0 | 19.6 | * | 29 JUN 1614 | 128 | 4.  | 0 | 19.6 | * | 29 JUN 1934 | 228 | 4. | 0 | 20.0 |
| 29 JUN 1256 | 29 | 4. | 0 | 19.6 | * | 29 JUN 1616 | 129 | 4.  | 0 | 19.6 | * | 29 JUN 1936 | 229 | 4. | 0 | 19.8 |
| 29 JUN 1258 | 30 | 4. | 0 | 19.6 | * | 29 JUN 1618 | 130 | 4.  | 0 | 19.6 | * | 29 JUN 1938 | 230 | 4. | 0 | 19.7 |
| 29 JUN 1300 | 31 | 4. | 0 | 19.6 | * | 29 JUN 1620 | 131 | 4.  | 0 | 19.6 | * | 29 JUN 1940 | 231 | 4. | 0 | 19.6 |
| 29 JUN 1302 | 32 | 4. | 0 | 19.6 | * | 29 JUN 1622 | 132 | 4.  | 0 | 19.6 | * | 29 JUN 1942 | 232 | 4. | 0 | 19.6 |
| 29 JUN 1304 | 33 | 4. | 0 | 19.6 | * | 29 JUN 1624 | 133 | 4.  | 0 | 19.6 | * | 29 JUN 1944 | 233 | 4. | 0 | 19.6 |
| 29 JUN 1306 | 34 | 4. | 0 | 19.6 | * | 29 JUN 1626 | 134 | 4.  | 0 | 19.6 | * | 29 JUN 1946 | 234 | 4. | 0 | 19.6 |
| 29 JUN 1308 | 35 | 4. | 0 | 19.6 | * | 29 JUN 1628 | 135 | 4.  | 0 | 19.6 | * | 29 JUN 1948 | 235 | 4. | 0 | 19.6 |
| 29 JUN 1310 | 36 | 4. | 0 | 19.6 | * | 29 JUN 1630 | 136 | 4.  | 0 | 19.6 | * | 29 JUN 1950 | 236 | 4. | 0 | 19.6 |
| 29 JUN 1312 | 37 | 4. | 0 | 19.6 | * | 29 JUN 1632 | 137 | 4.  | 0 | 19.6 | * | 29 JUN 1952 | 237 | 4. | 0 | 19.6 |
| 29 JUN 1314 | 38 | 4. | 0 | 19.6 | * | 29 JUN 1634 | 138 | 4.  | 0 | 19.6 | * | 29 JUN 1954 | 238 | 4. | 0 | 19.6 |
| 29 JUN 1316 | 39 | 4. | 0 | 19.6 | * | 29 JUN 1636 | 139 | 4.  | 0 | 19.6 | * | 29 JUN 1956 | 239 | 4. | 0 | 19.6 |
| 29 JUN 1318 | 40 | 4. | 0 | 19.6 | * | 29 JUN 1638 | 140 | 4.  | 0 | 19.6 | * | 29 JUN 1958 | 240 | 4. | 0 | 19.6 |
| 29 JUN 1320 | 41 | 4. | 0 | 19.6 | * | 29 JUN 1640 | 141 | 4.  | 0 | 19.6 | * | 29 JUN 2000 | 241 | 4. | 0 | 19.6 |
| 29 JUN 1322 | 42 | 4. | 0 | 19.6 | * | 29 JUN 1642 | 142 | 4.  | 0 | 19.6 | * | 29 JUN 2002 | 242 | 4. | 0 | 19.6 |
| 29 JUN 1324 | 43 | 4. | 0 | 19.6 | * | 29 JUN 1644 | 143 | 4.  | 0 | 19.6 | * | 29 JUN 2004 | 243 | 4. | 0 | 19.6 |
| 29 JUN 1326 | 44 | 4. | 0 | 19.6 | * | 29 JUN 1646 | 144 | 4.  | 0 | 19.6 | * | 29 JUN 2006 | 244 | 4. | 0 | 19.6 |
| 29 JUN 1328 | 45 | 4. | 0 | 19.6 | * | 29 JUN 1648 | 145 | 4.  | 0 | 19.6 | * | 29 JUN 2008 | 245 | 4. | 0 | 19.6 |
| 29 JUN 1330 | 46 | 4. | 0 | 19.6 | * | 29 JUN 1650 | 146 | 4.  | 0 | 19.6 | * | 29 JUN 2010 | 246 | 4. | 0 | 19.6 |
| 29 JUN 1332 | 47 | 4. | 0 | 19.6 | * | 29 JUN 1652 | 147 | 4.  | 0 | 19.6 | * | 29 JUN 2012 | 247 | 4. | 0 | 19.6 |
| 29 JUN 1334 | 48 | 4. | 0 | 19.6 | * | 29 JUN 1654 | 148 | 4.  | 0 | 19.6 | * | 29 JUN 2014 | 248 | 4. | 0 | 19.6 |
| 29 JUN 1336 | 49 | 4. | 0 | 19.6 | * | 29 JUN 1656 | 149 | 4.  | 0 | 19.6 | * | 29 JUN 2016 | 249 | 4. | 0 | 19.6 |
| 29 JUN 1338 | 50 | 4. | 0 | 19.6 | * | 29 JUN 1658 | 150 | 4.  | 0 | 19.6 | * | 29 JUN 2018 | 250 | 4. | 0 | 19.6 |
| 29 JUN 1340 | 51 | 4. | 0 | 19.6 | * | 29 JUN 1700 | 151 | 4.  | 0 | 19.6 | * | 29 JUN 2020 | 251 | 4. | 0 | 19.6 |
| 29 JUN 1342 | 52 | 4. | 0 | 19.6 | * | 29 JUN 1702 | 152 | 4.  | 0 | 19.6 | * | 29 JUN 2022 | 252 | 4. | 0 | 19.6 |
| 29 JUN 1344 | 53 | 4. | 0 | 19.6 | * | 29 JUN 1704 | 153 | 4.  | 0 | 19.6 | * | 29 JUN 2024 | 253 | 4. | 0 | 19.6 |
| 29 JUN 1346 | 54 | 4. | 0 | 19.6 | * | 29 JUN 1706 | 154 | 4.  | 0 | 19.9 | * | 29 JUN 2026 | 254 | 4. | 0 | 19.6 |
| 29 JUN 1348 | 55 | 4. | 0 | 19.6 | * | 29 JUN 1708 | 155 | 5.  | 0 | 20.3 | * | 29 JUN 2028 | 255 | 4. | 0 | 19.6 |
| 29 JUN 1350 | 56 | 4. | 0 | 19.6 | * | 29 JUN 1710 | 156 | 6.  | 1 | 20.7 | * | 29 JUN 2030 | 256 | 4. | 0 | 19.6 |
| 29 JUN 1352 | 57 | 4. | 0 | 19.6 | * | 29 JUN 1712 | 157 | 6.  | 1 | 21.1 | * | 29 JUN 2032 | 257 | 4. | 0 | 19.6 |
| 29 JUN 1354 | 58 | 4. | 0 | 19.6 | * | 29 JUN 1714 | 158 | 7.  | 2 | 21.6 | * | 29 JUN 2034 | 258 | 4. | 0 | 19.6 |
| 29 JUN 1356 | 59 | 4. | 0 | 19.6 | * | 29 JUN 1716 | 159 | 7.  | 2 | 22.1 | * | 29 JUN 2036 | 259 | 4. | 0 | 19.6 |
| 29 JUN 1358 | 60 | 4. | 0 | 19.6 | * | 29 JUN 1718 | 160 | 9.  | 3 | 22.7 | * | 29 JUN 2038 | 260 | 4. | 0 | 19.6 |
| 29 JUN 1400 | 61 | 4. | 0 | 19.6 | * | 29 JUN 1720 | 161 | 29. | 4 | 23.1 | * | 29 JUN 2040 | 261 | 4. | 0 | 19.6 |
| 29 JUN 1402 | 62 | 4. | 0 | 19.6 | * | 29 JUN 1722 | 162 | 44. | 4 | 23.4 | * | 29 JUN 2042 | 262 | 4. | 0 | 19.6 |
| 29 JUN 1404 | 63 | 4. | 0 | 19.6 | * | 29 JUN 1724 | 163 | 51. | 4 | 23.5 | * | 29 JUN 2044 | 263 | 4. | 0 | 19.6 |
| 29 JUN 1406 | 64 | 4. | 0 | 19.6 | * | 29 JUN 1726 | 164 | 54. | 4 | 23.5 | * | 29 JUN 2046 | 264 | 4. | 0 | 19.6 |
| 29 JUN 1408 | 65 | 4. | 0 | 19.6 | * | 29 JUN 1728 | 165 | 56. | 5 | 23.6 | * | 29 JUN 2048 | 265 | 4. | 0 | 19.6 |
| 29 JUN 1410 | 66 | 4. | 0 | 19.6 | * | 29 JUN 1730 | 166 | 57. | 5 | 23.6 | * | 29 JUN 2050 | 266 | 4. | 0 | 19.6 |
| 29 JUN 1412 | 67 | 4. | 0 | 19.6 | * | 29 JUN 1732 | 167 | 57. | 5 | 23.6 | * | 29 JUN 2052 | 267 | 4. | 0 | 19.6 |
| 29 JUN 1414 | 68 | 4. | 0 | 19.6 | * | 29 JUN 1734 | 168 | 57. | 5 | 23.6 | * | 29 JUN 2054 | 268 | 4. | 0 | 19.6 |
| 29 JUN 1416 | 69 | 4. | 0 | 19.6 | * | 29 JUN 1736 | 169 | 56. | 4 | 23.5 | * | 29 JUN 2056 | 269 | 4. | 0 | 19.6 |
| 29 JUN 1418 | 70 | 4. | 0 | 19.6 | * | 29 JUN 1738 | 170 | 54. | 4 | 23.5 | * | 29 JUN 2058 | 270 | 4. | 0 | 19.6 |
| 29 JUN 1420 | 71 | 4. | 0 | 19.6 | * | 29 JUN 1740 | 171 | 52. | 4 | 23.5 | * | 29 JUN 2100 | 271 | 4. | 0 | 19.6 |
| 29 JUN 1422 | 72 | 4. | 0 | 19.6 | * | 29 JUN 1742 | 172 | 50. | 4 | 23.5 | * | 29 JUN 2102 | 272 | 4. | 0 | 19.6 |
| 29 JUN 1424 | 73 | 4. | 0 | 19.6 | * | 29 JUN 1744 | 173 | 48. | 4 | 23.4 | * | 29 JUN 2104 | 273 | 4. | 0 | 19.6 |
| 29 JUN 1426 | 74 | 4. | 0 | 19.6 | * | 29 JUN 1746 | 174 | 45. | 4 | 23.4 | * | 29 JUN 2106 | 274 | 4. | 0 | 19.6 |
| 29 JUN 1428 | 75 | 4. | 0 | 19.6 | * | 29 JUN 1748 | 175 | 42. | 4 | 23.4 | * | 29 JUN 2108 | 275 | 4. | 0 | 19.6 |
| 29 JUN 1430 | 76 | 4. | 0 | 19.6 | * | 29 JUN 1750 | 176 | 40. | 4 | 23.3 | * | 29 JUN 2110 | 276 | 4. | 0 | 19.6 |
| 29 JUN 1432 | 77 | 4. | 0 | 19.6 | * | 29 JUN 1752 | 177 | 37. | 4 | 23.3 | * | 29 JUN 2112 | 277 | 4. | 0 | 19.6 |
| 29 JUN 1434 | 78 | 4. | 0 | 19.6 | * | 29 JUN 1754 | 178 | 34. | 4 | 23.2 | * | 29 JUN 2114 | 278 | 4. | 0 | 19.6 |
| 29 JUN 1436 | 79 | 4. | 0 | 19.6 | * | 29 JUN 1756 | 179 | 31. | 4 | 23.2 | * | 29 JUN 2116 | 279 | 4. | 0 | 19.6 |
| 29 JUN 1438 | 80 | 4. | 0 | 19.6 | * | 29 JUN 1758 | 180 | 29. | 4 | 23.1 | * | 29 JUN 2118 | 280 | 4. | 0 | 19.6 |
| 29 JUN 1440 | 81 | 4. | 0 | 19.6 | * | 29 JUN 1800 | 181 | 26. | 4 | 23.1 | * | 29 JUN 2120 | 281 | 4. | 0 | 19.6 |
| 29 JUN 1442 | 82 | 4. | 0 | 19.6 | * | 29 JUN 1802 | 182 | 24. | 4 | 23.1 | * | 29 JUN 2122 | 282 | 4. | 0 | 19.6 |
| 29 JUN 1444 | 83 | 4. | 0 | 19.6 | * | 29 JUN 1804 | 183 | 22. | 4 | 23.0 | * | 29 JUN 2124 | 283 | 4. | 0 | 19.6 |
| 29 JUN 1446 | 84 | 4. | 0 | 19.6 | * | 29 JUN 1806 | 184 | 20. | 4 | 23.0 | * | 29 JUN 2126 | 284 | 4. | 0 | 19.6 |
| 29 JUN 1448 | 85 | 4. | 0 | 19.6 | * | 29 JUN 1808 | 185 | 19. | 4 | 23.0 | * | 29 JUN 2128 | 285 | 4. | 0 | 19.6 |
| 29 JUN 1450 | 86 | 4. | 0 | 19.6 | * | 29 JUN 1810 | 186 | 17. | 4 | 22.9 | * | 29 JUN 2130 | 286 | 4. | 0 | 19.6 |

RA-19

|                 |    |    |      |   |                 |     |    |      |   |                 |    |    |      |
|-----------------|----|----|------|---|-----------------|-----|----|------|---|-----------------|----|----|------|
| 29 JUN 1454 88  | 4. | .0 | 19.6 | * | 29 JUN 1814 188 | 14. | .3 | 22.9 | * | 29 JUN 2134 288 | 4. | .0 | 19.6 |
| 29 JUN 1456 89  | 4. | .0 | 19.6 | * | 29 JUN 1816 189 | 13. | .3 | 22.8 | * | 29 JUN 2136 289 | 4. | .0 | 19.6 |
| 29 JUN 1458 90  | 4. | .0 | 19.6 | * | 29 JUN 1818 190 | 12. | .3 | 22.8 | * | 29 JUN 2138 290 | 4. | .0 | 19.6 |
| 29 JUN 1500 91  | 4. | .0 | 19.6 | * | 29 JUN 1820 191 | 11. | .3 | 22.8 | * | 29 JUN 2140 291 | 4. | .0 | 19.6 |
| 29 JUN 1502 92  | 4. | .0 | 19.6 | * | 29 JUN 1822 192 | 10. | .3 | 22.7 | * | 29 JUN 2142 292 | 4. | .0 | 19.6 |
| 29 JUN 1504 93  | 4. | .0 | 19.6 | * | 29 JUN 1824 193 | 10. | .3 | 22.7 | * | 29 JUN 2144 293 | 4. | .0 | 19.6 |
| 29 JUN 1506 94  | 4. | .0 | 19.6 | * | 29 JUN 1826 194 | 9.  | .3 | 22.7 | * | 29 JUN 2146 294 | 4. | .0 | 19.6 |
| 29 JUN 1508 95  | 4. | .0 | 19.6 | * | 29 JUN 1828 195 | 8.  | .3 | 22.7 | * | 29 JUN 2148 295 | 4. | .0 | 19.6 |
| 29 JUN 1510 96  | 4. | .0 | 19.6 | * | 29 JUN 1830 196 | 8.  | .3 | 22.6 | * | 29 JUN 2150 296 | 4. | .0 | 19.6 |
| 29 JUN 1512 97  | 4. | .0 | 19.6 | * | 29 JUN 1832 197 | 8.  | .3 | 22.6 | * | 29 JUN 2152 297 | 4. | .0 | 19.6 |
| 29 JUN 1514 98  | 4. | .0 | 19.6 | * | 29 JUN 1834 198 | 8.  | .3 | 22.6 | * | 29 JUN 2154 298 | 4. | .0 | 19.6 |
| 29 JUN 1516 99  | 4. | .0 | 19.6 | * | 29 JUN 1836 199 | 8.  | .3 | 22.5 | * | 29 JUN 2156 299 | 4. | .0 | 19.6 |
| 29 JUN 1518 100 | 4. | .0 | 19.6 | * | 29 JUN 1838 200 | 7.  | .3 | 22.5 | * | 29 JUN 2158 300 | 4. | .0 | 19.6 |

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|           |      |          |                      |       |       |         |
|-----------|------|----------|----------------------|-------|-------|---------|
| PEAK FLOW | TIME |          | MAXIMUM AVERAGE FLOW |       |       |         |
| (CFS)     | (HR) |          | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 57.       | 5.53 | (CFS)    | 10.                  | 8.    | 8.    | 8.      |
|           |      | (INCHES) | 1.155                | 1.442 | 1.442 | 1.442   |
|           |      | (AC-FT)  | 5.                   | 6.    | 6.    | 6.      |

|              |      |  |                         |       |       |         |
|--------------|------|--|-------------------------|-------|-------|---------|
| PEAK STORAGE | TIME |  | MAXIMUM AVERAGE STORAGE |       |       |         |
| (AC-FT)      | (HR) |  | 6-HR                    | 24-HR | 72-HR | 9.97-HR |
| 0.           | 5.50 |  | 0.                      | 0.    | 0.    | 0.      |

|            |      |  |                       |       |       |         |
|------------|------|--|-----------------------|-------|-------|---------|
| PEAK STAGE | TIME |  | MAXIMUM AVERAGE STAGE |       |       |         |
| (FEET)     | (HR) |  | 6-HR                  | 24-HR | 72-HR | 9.97-HR |
| 23.57      | 5.53 |  | 20.72                 | 20.27 | 20.27 | 20.27   |

CUMULATIVE AREA = .08 SQ MI

STATION RES1

|            |     | (I) INFLOW, |     | (O) OUTFLOW |     | 50. | 60. | 0.          | 0. | 0. | 0. | 0. | 0. |
|------------|-----|-------------|-----|-------------|-----|-----|-----|-------------|----|----|----|----|----|
|            |     | 20.         | 30. | 40.         |     |     |     |             |    |    |    |    |    |
| 0.         | 10. | 20.         | 30. | 40.         | 50. | 60. | 0.  | 0.          | 0. | 0. | 0. | 0. | 0. |
| .0         | .0  | .0          | .0  | .0          | .0  | -.2 | .0  | .2          | .4 | .6 | .0 | .0 |    |
| DAHRMN PER |     |             |     |             |     |     |     | (S) STORAGE |    |    |    |    |    |
| 291200     | 1I  |             |     |             |     |     |     | S           |    |    |    |    |    |
| 291202     | 2I  | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291204     | 3I  | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291206     | 4I  | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291208     | 5I  | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291210     | 6I  | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291212     | 7I  | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291214     | 8I  | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291216     | 9I  | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291218     | 10I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291220     | 11I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291222     | 12I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291224     | 13I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291226     | 14I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291228     | 15I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291230     | 16I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291232     | 17I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291234     | 18I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291236     | 19I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291238     | 20I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291240     | 21I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291242     | 22I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291244     | 23I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291246     | 24I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291248     | 25I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291250     | 26I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291252     | 27I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291254     | 28I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291256     | 29I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291258     | 30I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291300     | 31I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291302     | 32I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291304     | 33I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291306     | 34I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291308     | 35I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291310     | 36I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291312     | 37I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291314     | 38I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291316     | 39I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291318     | 40I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291320     | 41I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291322     | 42I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291324     | 43I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291326     | 44I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291328     | 45I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291330     | 46I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291332     | 47I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291334     | 48I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291336     | 49I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291338     | 50I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291340     | 51I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291342     | 52I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291344     | 53I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291346     | 54I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291348     | 55I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291350     | 56I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291352     | 57I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291354     | 58I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291356     | 59I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291358     | 60I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291400     | 61I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291402     | 62I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291404     | 63I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291406     | 64I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291408     | 65I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291410     | 66I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291412     | 67I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291414     | 68I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291416     | 69I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291418     | 70I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291420     | 71I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291422     | 72I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291424     | 73I | 0           |     |             |     |     |     | S           |    |    |    |    |    |
| 291426     | 74I | 0           |     |             |     |     |     | S           |    |    |    |    |    |

R4-21







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106 KK \*\*\*\*\*  
\* \*  
\* CH3 \*  
\* \*  
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108 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

109 RD MUSKINGUM-CUNGE CHANNEL ROUTING  
L 310. CHANNEL LENGTH  
S .0052 SLOPE  
N .025 CHANNEL ROUGHNESS COEFFICIENT  
CA .00 CONTRIBUTING AREA  
SHAPE TRAP CHANNEL SHAPE  
WD 2.00 BOTTOM WIDTH OR DIAMETER  
Z 3.00 SIDE SLOPE

\*\*\*  
COMPUTED MUSKINGUM-CUNGE PARAMETERS

| ELEMENT | ALPHA | COMPUTATION TIME STEP |             |            | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|-----------------------|-------------|------------|---------------|--------------------------|----------------|------------------------------|
|         |       | M                     | DT<br>(MIN) | DX<br>(FT) |               |                          |                |                              |
| MAIN    | 1.72  | 1.34                  | 1.08        | 310.00     | 57.29         | 331.74                   | 1.44           | 4.77                         |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |       |        |      |
|------|------|------|------|-------|--------|------|
| MAIN | 1.72 | 1.34 | 2.00 | 57.28 | 332.00 | 1.44 |
|------|------|------|------|-------|--------|------|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6232E+01 EXCESS= .0000E+00 OUTFLOW= .6223E+01 BASIN STORAGE= .1282E-01 PERCENT ERROR= -.1

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HYDROGRAPH AT STATION CH3

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 57.                | 5.53         | (CFS) 10.            | 8.    | 8.    | 8.      |
|                    |              | (INCHES) 1.155       | 1.441 | 1.441 | 1.441   |
|                    |              | (AC-FT) 5.           | 6.    | 6.    | 6.      |

CUMULATIVE AREA = .08 SQ MI

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110 KK \*\*\*\*\*  
\* \*  
\* B2 \*  
\* \*  
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112 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL

OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

113 BA SUBBASIN CHARACTERISTICS  
 TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

114 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
 ..... HYDRO-35 ..... TP-40 ..... TP-49 .....  
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
 .39 .76 1.34 1.40 1.44 1.56 1.69 2.01 .00 .00 .00 .00  
 STORM AREA = .00

115 LS SCS LOSS RATE  
 STRTL .08 INITIAL ABSTRACTION  
 CRVNBR 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

116 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .04 LAG

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UNIT HYDROGRAPH  
8 END-OF-PERIOD ORDINATES

3. 4. 2. 1. 0. 0. 0. 0.

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HYDROGRAPH AT STATION B2

TOTAL RAINFALL = 1.65. TOTAL LOSS = .41. TOTAL EXCESS = 1.24

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 1.                 | 5.03         | (CFS) 0.             | 0.    | 0.    | 0.      |
|                    |              | (INCHES) 1.237       | 1.241 | 1.241 | 1.241   |
|                    |              | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .00 SQ MI

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 \* \*  
 \* B2 \*  
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119 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

120 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION B2

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 57.                | 5.53         | (CFS) 10.            | 8.    | 8.    | 8.      |
|                    |              | (INCHES) 1.439       | 1.439 | 1.439 | 1.439   |
|                    |              | (AC-FT) 0.           | 0.    | 0.    | 0.      |

RA-26



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 \* \*  
 121 KK \* RES2 \*  
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123 KO OUTPUT CONTROL VARIABLES  
 IPRNT 1 PRINT CONTROL  
 IPLOT 2 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

124 RS STORAGE ROUTING  
 NSTPS 1 NUMBER OF SUBREACHES  
 ITYP ELEV TYPE OF INITIAL CONDITION  
 RSVRIC 17.00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

125 SV STORAGE .0 .1 .2 .3

126 SE ELEVATION 18.00 19.00 20.00 21.00

127 SL LOW-LEVEL OUTLET  
 ELEV 17.00 ELEVATION AT CENTER OF OUTLET  
 CAREA .79 CROSS-SECTIONAL AREA  
 COOL .60 COEFFICIENT  
 EXPL .50 EXPONENT OF HEAD

128 SS SPILLWAY  
 CREL 20.00 SPILLWAY CREST ELEVATION  
 SPWID 19.00 SPILLWAY WIDTH  
 COOW 2.70 WEIR COEFFICIENT  
 EXPW 1.50 EXPONENT OF HEAD

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COMPUTED OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| OUTFLOW   | .00   | 3.99  | 4.20  | 4.42  | 4.68  | 4.97  | 5.29  | 5.66  | 6.09  | 6.58  |
| ELEVATION | 18.00 | 18.10 | 18.22 | 18.35 | 18.52 | 18.71 | 18.94 | 19.22 | 19.57 | 20.00 |
| OUTFLOW   | 6.67  | 7.12  | 8.22  | 10.26 | 13.55 | 18.38 | 25.03 | 33.80 | 45.00 | 58.90 |
| ELEVATION | 20.01 | 20.04 | 20.10 | 20.17 | 20.26 | 20.37 | 20.50 | 20.64 | 20.81 | 21.00 |

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| STORAGE   | .01   | .02   | .03   | .04   | .05   | .06   | .07   | .08   | .10   | .14   |
| OUTFLOW   | 3.80  | 3.99  | 4.20  | 4.42  | 4.68  | 4.97  | 5.29  | 5.38  | 5.66  | 6.09  |
| ELEVATION | 18.00 | 18.10 | 18.22 | 18.35 | 18.52 | 18.71 | 18.94 | 19.00 | 19.22 | 19.57 |
| STORAGE   | .18   | .19   | .19   | .19   | .20   | .21   | .22   | .24   | .25   | .27   |
| OUTFLOW   | 6.58  | 6.67  | 7.12  | 8.22  | 10.26 | 13.55 | 18.38 | 25.03 | 33.80 | 45.00 |
| ELEVATION | 20.00 | 20.01 | 20.04 | 20.10 | 20.17 | 20.26 | 20.37 | 20.50 | 20.64 | 20.81 |
| STORAGE   | .29   |       |       |       |       |       |       |       |       |       |
| OUTFLOW   | 58.90 |       |       |       |       |       |       |       |       |       |
| ELEVATION | 21.00 |       |       |       |       |       |       |       |       |       |

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HYDROGRAPH AT STATION RES2

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| DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | * | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | * | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE |
|----|-----|------|-----|---------|---------|-------|---|----|-----|------|-----|---------|---------|-------|---|----|-----|------|-----|---------|---------|-------|
| 29 | JUN | 1200 | 1   | 0.      | .0      | 18.0  | * | 29 | JUN | 1520 | 101 | 4.      | .0      | 18.0  | * | 29 | JUN | 1840 | 201 | 8.      | .2      | 20.1  |
| 29 | JUN | 1202 | 2   | 4.      | .0      | 18.0  | * | 29 | JUN | 1522 | 102 | 4.      | .0      | 18.0  | * | 29 | JUN | 1842 | 202 | 8.      | .2      | 20.1  |
| 29 | JUN | 1204 | 3   | 4.      | .0      | 18.0  | * | 29 | JUN | 1524 | 103 | 4.      | .0      | 18.0  | * | 29 | JUN | 1844 | 203 | 7.      | .2      | 20.1  |
| 29 | JUN | 1206 | 4   | 4.      | .0      | 18.0  | * | 29 | JUN | 1526 | 104 | 4.      | .0      | 18.0  | * | 29 | JUN | 1846 | 204 | 7.      | .2      | 20.1  |
| 29 | JUN | 1208 | 5   | 4.      | .0      | 18.0  | * | 29 | JUN | 1528 | 105 | 4.      | .0      | 18.0  | * | 29 | JUN | 1848 | 205 | 7.      | .2      | 20.1  |

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|             |    |    |    |      |   |             |     |     |    |      |   |             |     |    |    |      |
|-------------|----|----|----|------|---|-------------|-----|-----|----|------|---|-------------|-----|----|----|------|
| 29 JUN 1212 | 7  | 4. | .0 | 18.0 | * | 29 JUN 1532 | 107 | 4.  | .0 | 18.0 | * | 29 JUN 1852 | 207 | 7. | .2 | 20.0 |
| 29 JUN 1214 | 8  | 4. | .0 | 18.0 | * | 29 JUN 1534 | 108 | 4.  | .0 | 18.0 | * | 29 JUN 1854 | 208 | 7. | .2 | 20.0 |
| 29 JUN 1216 | 9  | 4. | .0 | 18.0 | * | 29 JUN 1536 | 109 | 4.  | .0 | 18.0 | * | 29 JUN 1856 | 209 | 7. | .2 | 20.0 |
| 29 JUN 1218 | 10 | 4. | .0 | 18.0 | * | 29 JUN 1538 | 110 | 4.  | .0 | 18.0 | * | 29 JUN 1858 | 210 | 7. | .2 | 20.0 |
| 29 JUN 1220 | 11 | 4. | .0 | 18.0 | * | 29 JUN 1540 | 111 | 4.  | .0 | 18.0 | * | 29 JUN 1900 | 211 | 7. | .2 | 20.0 |
| 29 JUN 1222 | 12 | 4. | .0 | 18.0 | * | 29 JUN 1542 | 112 | 4.  | .0 | 18.0 | * | 29 JUN 1902 | 212 | 7. | .2 | 20.0 |
| 29 JUN 1224 | 13 | 4. | .0 | 18.0 | * | 29 JUN 1544 | 113 | 4.  | .0 | 18.0 | * | 29 JUN 1904 | 213 | 7. | .2 | 20.0 |
| 29 JUN 1226 | 14 | 4. | .0 | 18.0 | * | 29 JUN 1546 | 114 | 4.  | .0 | 18.0 | * | 29 JUN 1906 | 214 | 7. | .2 | 20.0 |
| 29 JUN 1228 | 15 | 4. | .0 | 18.0 | * | 29 JUN 1548 | 115 | 4.  | .0 | 18.0 | * | 29 JUN 1908 | 215 | 7. | .2 | 20.0 |
| 29 JUN 1230 | 16 | 4. | .0 | 18.0 | * | 29 JUN 1550 | 116 | 4.  | .0 | 18.0 | * | 29 JUN 1910 | 216 | 7. | .2 | 20.0 |
| 29 JUN 1232 | 17 | 4. | .0 | 18.0 | * | 29 JUN 1552 | 117 | 4.  | .0 | 18.0 | * | 29 JUN 1912 | 217 | 7. | .2 | 20.0 |
| 29 JUN 1234 | 18 | 4. | .0 | 18.0 | * | 29 JUN 1554 | 118 | 4.  | .0 | 18.0 | * | 29 JUN 1914 | 218 | 7. | .2 | 20.0 |
| 29 JUN 1236 | 19 | 4. | .0 | 18.0 | * | 29 JUN 1556 | 119 | 4.  | .0 | 18.0 | * | 29 JUN 1916 | 219 | 7. | .2 | 20.0 |
| 29 JUN 1238 | 20 | 4. | .0 | 18.0 | * | 29 JUN 1558 | 120 | 4.  | .0 | 18.0 | * | 29 JUN 1918 | 220 | 7. | .2 | 20.0 |
| 29 JUN 1240 | 21 | 4. | .0 | 18.0 | * | 29 JUN 1600 | 121 | 4.  | .0 | 18.0 | * | 29 JUN 1920 | 221 | 7. | .2 | 19.9 |
| 29 JUN 1242 | 22 | 4. | .0 | 18.0 | * | 29 JUN 1602 | 122 | 4.  | .0 | 18.0 | * | 29 JUN 1922 | 222 | 6. | .2 | 19.9 |
| 29 JUN 1244 | 23 | 4. | .0 | 18.0 | * | 29 JUN 1604 | 123 | 4.  | .0 | 18.0 | * | 29 JUN 1924 | 223 | 6. | .2 | 19.9 |
| 29 JUN 1246 | 24 | 4. | .0 | 18.0 | * | 29 JUN 1606 | 124 | 4.  | .0 | 18.0 | * | 29 JUN 1926 | 224 | 6. | .2 | 19.9 |
| 29 JUN 1248 | 25 | 4. | .0 | 18.0 | * | 29 JUN 1608 | 125 | 4.  | .0 | 18.0 | * | 29 JUN 1928 | 225 | 6. | .2 | 19.8 |
| 29 JUN 1250 | 26 | 4. | .0 | 18.0 | * | 29 JUN 1610 | 126 | 4.  | .0 | 18.0 | * | 29 JUN 1930 | 226 | 6. | .2 | 19.8 |
| 29 JUN 1252 | 27 | 4. | .0 | 18.0 | * | 29 JUN 1612 | 127 | 4.  | .0 | 18.0 | * | 29 JUN 1932 | 227 | 6. | .2 | 19.8 |
| 29 JUN 1254 | 28 | 4. | .0 | 18.0 | * | 29 JUN 1614 | 128 | 4.  | .0 | 18.0 | * | 29 JUN 1934 | 228 | 6. | .2 | 19.7 |
| 29 JUN 1256 | 29 | 4. | .0 | 18.0 | * | 29 JUN 1616 | 129 | 4.  | .0 | 18.0 | * | 29 JUN 1936 | 229 | 6. | .2 | 19.7 |
| 29 JUN 1258 | 30 | 4. | .0 | 18.0 | * | 29 JUN 1618 | 130 | 4.  | .0 | 18.0 | * | 29 JUN 1938 | 230 | 6. | .1 | 19.6 |
| 29 JUN 1300 | 31 | 4. | .0 | 18.0 | * | 29 JUN 1620 | 131 | 4.  | .0 | 18.0 | * | 29 JUN 1940 | 231 | 6. | .1 | 19.6 |
| 29 JUN 1302 | 32 | 4. | .0 | 18.0 | * | 29 JUN 1622 | 132 | 4.  | .0 | 18.0 | * | 29 JUN 1942 | 232 | 6. | .1 | 19.5 |
| 29 JUN 1304 | 33 | 4. | .0 | 18.0 | * | 29 JUN 1624 | 133 | 4.  | .0 | 18.0 | * | 29 JUN 1944 | 233 | 6. | .1 | 19.5 |
| 29 JUN 1306 | 34 | 4. | .0 | 18.0 | * | 29 JUN 1626 | 134 | 4.  | .0 | 18.0 | * | 29 JUN 1946 | 234 | 6. | .1 | 19.4 |
| 29 JUN 1308 | 35 | 4. | .0 | 18.0 | * | 29 JUN 1628 | 135 | 4.  | .0 | 18.0 | * | 29 JUN 1948 | 235 | 6. | .1 | 19.4 |
| 29 JUN 1310 | 36 | 4. | .0 | 18.0 | * | 29 JUN 1630 | 136 | 4.  | .0 | 18.0 | * | 29 JUN 1950 | 236 | 6. | .1 | 19.3 |
| 29 JUN 1312 | 37 | 4. | .0 | 18.0 | * | 29 JUN 1632 | 137 | 4.  | .0 | 18.0 | * | 29 JUN 1952 | 237 | 6. | .1 | 19.3 |
| 29 JUN 1314 | 38 | 4. | .0 | 18.0 | * | 29 JUN 1634 | 138 | 4.  | .0 | 18.0 | * | 29 JUN 1954 | 238 | 6. | .1 | 19.2 |
| 29 JUN 1316 | 39 | 4. | .0 | 18.0 | * | 29 JUN 1636 | 139 | 4.  | .0 | 18.0 | * | 29 JUN 1956 | 239 | 6. | .1 | 19.2 |
| 29 JUN 1318 | 40 | 4. | .0 | 18.0 | * | 29 JUN 1638 | 140 | 4.  | .0 | 18.0 | * | 29 JUN 1958 | 240 | 6. | .1 | 19.1 |
| 29 JUN 1320 | 41 | 4. | .0 | 18.0 | * | 29 JUN 1640 | 141 | 4.  | .0 | 18.0 | * | 29 JUN 2000 | 241 | 5. | .1 | 19.1 |
| 29 JUN 1322 | 42 | 4. | .0 | 18.0 | * | 29 JUN 1642 | 142 | 4.  | .0 | 18.0 | * | 29 JUN 2002 | 242 | 5. | .1 | 19.0 |
| 29 JUN 1324 | 43 | 4. | .0 | 18.0 | * | 29 JUN 1644 | 143 | 4.  | .0 | 18.0 | * | 29 JUN 2004 | 243 | 5. | .1 | 19.0 |
| 29 JUN 1326 | 44 | 4. | .0 | 18.0 | * | 29 JUN 1646 | 144 | 4.  | .0 | 18.0 | * | 29 JUN 2006 | 244 | 5. | .1 | 18.9 |
| 29 JUN 1328 | 45 | 4. | .0 | 18.0 | * | 29 JUN 1648 | 145 | 4.  | .0 | 18.0 | * | 29 JUN 2008 | 245 | 5. | .1 | 18.9 |
| 29 JUN 1330 | 46 | 4. | .0 | 18.0 | * | 29 JUN 1650 | 146 | 4.  | .0 | 18.0 | * | 29 JUN 2010 | 246 | 5. | .1 | 18.8 |
| 29 JUN 1332 | 47 | 4. | .0 | 18.0 | * | 29 JUN 1652 | 147 | 4.  | .0 | 18.0 | * | 29 JUN 2012 | 247 | 5. | .1 | 18.7 |
| 29 JUN 1334 | 48 | 4. | .0 | 18.0 | * | 29 JUN 1654 | 148 | 4.  | .0 | 18.0 | * | 29 JUN 2014 | 248 | 5. | .1 | 18.7 |
| 29 JUN 1336 | 49 | 4. | .0 | 18.0 | * | 29 JUN 1656 | 149 | 4.  | .0 | 18.0 | * | 29 JUN 2016 | 249 | 5. | .1 | 18.6 |
| 29 JUN 1338 | 50 | 4. | .0 | 18.0 | * | 29 JUN 1658 | 150 | 4.  | .0 | 18.1 | * | 29 JUN 2018 | 250 | 5. | .1 | 18.6 |
| 29 JUN 1340 | 51 | 4. | .0 | 18.0 | * | 29 JUN 1700 | 151 | 4.  | .0 | 18.1 | * | 29 JUN 2020 | 251 | 5. | .0 | 18.6 |
| 29 JUN 1342 | 52 | 4. | .0 | 18.0 | * | 29 JUN 1702 | 152 | 4.  | .0 | 18.1 | * | 29 JUN 2022 | 252 | 5. | .0 | 18.5 |
| 29 JUN 1344 | 53 | 4. | .0 | 18.0 | * | 29 JUN 1704 | 153 | 4.  | .0 | 18.2 | * | 29 JUN 2024 | 253 | 5. | .0 | 18.5 |
| 29 JUN 1346 | 54 | 4. | .0 | 18.0 | * | 29 JUN 1706 | 154 | 4.  | .0 | 18.2 | * | 29 JUN 2026 | 254 | 5. | .0 | 18.4 |
| 29 JUN 1348 | 55 | 4. | .0 | 18.0 | * | 29 JUN 1708 | 155 | 4.  | .0 | 18.2 | * | 29 JUN 2028 | 255 | 5. | .0 | 18.4 |
| 29 JUN 1350 | 56 | 4. | .0 | 18.0 | * | 29 JUN 1710 | 156 | 4.  | .0 | 18.3 | * | 29 JUN 2030 | 256 | 4. | .0 | 18.4 |
| 29 JUN 1352 | 57 | 4. | .0 | 18.0 | * | 29 JUN 1712 | 157 | 4.  | .0 | 18.4 | * | 29 JUN 2032 | 257 | 4. | .0 | 18.4 |
| 29 JUN 1354 | 58 | 4. | .0 | 18.0 | * | 29 JUN 1714 | 158 | 5.  | .0 | 18.4 | * | 29 JUN 2034 | 258 | 4. | .0 | 18.3 |
| 29 JUN 1356 | 59 | 4. | .0 | 18.0 | * | 29 JUN 1716 | 159 | 5.  | .0 | 18.5 | * | 29 JUN 2036 | 259 | 4. | .0 | 18.3 |
| 29 JUN 1358 | 60 | 4. | .0 | 18.0 | * | 29 JUN 1718 | 160 | 5.  | .1 | 18.6 | * | 29 JUN 2038 | 260 | 4. | .0 | 18.3 |
| 29 JUN 1400 | 61 | 4. | .0 | 18.0 | * | 29 JUN 1720 | 161 | 5.  | .1 | 19.0 | * | 29 JUN 2040 | 261 | 4. | .0 | 18.3 |
| 29 JUN 1402 | 62 | 4. | .0 | 18.0 | * | 29 JUN 1722 | 162 | 6.  | .1 | 19.5 | * | 29 JUN 2042 | 262 | 4. | .0 | 18.2 |
| 29 JUN 1404 | 63 | 4. | .0 | 18.0 | * | 29 JUN 1724 | 163 | 16. | .2 | 20.3 | * | 29 JUN 2044 | 263 | 4. | .0 | 18.2 |
| 29 JUN 1406 | 64 | 4. | .0 | 18.0 | * | 29 JUN 1726 | 164 | 45. | .3 | 20.8 | * | 29 JUN 2046 | 264 | 4. | .0 | 18.2 |
| 29 JUN 1408 | 65 | 4. | .0 | 18.0 | * | 29 JUN 1728 | 165 | 54. | .3 | 20.9 | * | 29 JUN 2048 | 265 | 4. | .0 | 18.2 |
| 29 JUN 1410 | 66 | 4. | .0 | 18.0 | * | 29 JUN 1730 | 166 | 56. | .3 | 21.0 | * | 29 JUN 2050 | 266 | 4. | .0 | 18.2 |
| 29 JUN 1412 | 67 | 4. | .0 | 18.0 | * | 29 JUN 1732 | 167 | 57. | .3 | 21.0 | * | 29 JUN 2052 | 267 | 4. | .0 | 18.2 |
| 29 JUN 1414 | 68 | 4. | .0 | 18.0 | * | 29 JUN 1734 | 168 | 57. | .3 | 21.0 | * | 29 JUN 2054 | 268 | 4. | .0 | 18.1 |
| 29 JUN 1416 | 69 | 4. | .0 | 18.0 | * | 29 JUN 1736 | 169 | 57. | .3 | 21.0 | * | 29 JUN 2056 | 269 | 4. | .0 | 18.1 |
| 29 JUN 1418 | 70 | 4. | .0 | 18.0 | * | 29 JUN 1738 | 170 | 55. | .3 | 21.0 | * | 29 JUN 2058 | 270 | 4. | .0 | 18.1 |
| 29 JUN 1420 | 71 | 4. | .0 | 18.0 | * | 29 JUN 1740 | 171 | 54. | .3 | 20.9 | * | 29 JUN 2100 | 271 | 4. | .0 | 18.1 |
| 29 JUN 1422 | 72 | 4. | .0 | 18.0 | * | 29 JUN 1742 | 172 | 52. | .3 | 20.9 | * | 29 JUN 2102 | 272 | 4. | .0 | 18.1 |
| 29 JUN 1424 | 73 | 4. | .0 | 18.0 | * | 29 JUN 1744 | 173 | 50. | .3 | 20.9 | * | 29 JUN 2104 | 273 | 4. | .0 | 18.1 |
| 29 JUN 1426 | 74 | 4. | .0 | 18.0 | * | 29 JUN 1746 | 174 | 48. | .3 | 20.8 | * | 29 JUN 2106 | 274 | 4. | .0 | 18.1 |
| 29 JUN 1428 | 75 | 4. | .0 | 18.0 | * | 29 JUN 1748 | 175 | 45. | .3 | 20.8 | * | 29 JUN 2108 | 275 | 4. | .0 | 18.1 |
| 29 JUN 1430 | 76 | 4. | .0 | 18.0 | * | 29 JUN 1750 | 176 | 43. | .3 | 20.8 | * | 29 JUN 2110 | 276 | 4. | .0 | 18.1 |
| 29 JUN 1432 | 77 | 4. | .0 | 18.0 | * | 29 JUN 1752 | 177 | 40. | .3 | 20.7 | * | 29 JUN 2112 | 277 | 4. | .0 | 18.1 |
| 29 JUN 1434 | 78 | 4. | .0 | 18.0 | * | 29 JUN 1754 | 178 | 37. | .3 | 20.7 | * | 29 JUN 2114 | 278 | 4. | .0 | 18.1 |
| 29 JUN 1436 | 79 | 4. | .0 | 18.0 | * | 29 JUN 1756 | 179 | 34. | .3 | 20.7 | * | 29 JUN 2116 | 279 | 4. | .0 | 18.1 |
| 29 JUN 1438 | 80 | 4. | .0 | 18.0 | * | 29 JUN 1758 | 180 | 32. | .2 | 20.6 | * | 29 JUN 2118 | 280 | 4. | .0 | 18.1 |
| 29 JUN 1440 | 81 | 4. | .0 | 18.0 | * | 29 JUN 1800 | 181 | 29. | .2 | 20.6 | * | 29 JUN 2120 | 281 | 4. | .0 | 18.1 |
| 29 JUN 1442 | 82 | 4. | .0 | 18.0 | * | 29 JUN 1802 | 182 | 27. | .2 | 20.5 | * | 29 JUN 2122 | 282 | 4. | .0 | 18.0 |
| 29 JUN 1444 | 83 | 4. | .0 | 18.0 | * | 29 JUN 1804 | 183 | 25. | .2 | 20.5 | * | 29 JUN 2124 | 283 | 4. | .0 | 18.0 |
| 29 JUN 1446 | 84 | 4. | .0 | 18.0 | * | 29 JUN 1806 | 184 | 23. | .2 | 20.5 | * | 29 JUN 2126 | 284 | 4. | .0 | 18.0 |
| 29 JUN 1448 | 85 | 4. | .0 | 18.0 | * | 29 JUN 1808 | 185 | 21. | .2 | 20.4 | * | 29 JUN 2128 | 285 | 4. | .0 | 18.0 |
| 29 JUN 1450 | 86 | 4. | .0 | 18.0 | * | 29 JUN 1810 | 186 | 19. | .2 | 20.4 | * | 29 JUN 2130 | 286 | 4. | .0 | 18.0 |
| 29 JUN 1452 | 87 | 4. | .0 | 18.0 | * | 29 JUN 1812 | 187 | 18. | .2 | 20.4 | * | 29 JUN 2132 | 287 | 4. | .0 | 18.0 |
| 29 JUN 1454 | 88 | 4. | .0 | 18.0 | * | 29 JUN 1814 | 188 | 16. | .2 | 20.3 | * | 29 JUN 2134 | 288 | 4. | .0 | 18.0 |
| 29 JUN 1456 | 89 | 4. | .0 | 18.0 | * | 29 JUN 1816 | 189 | 15. | .2 | 20.3 | * | 29 JUN 2136 | 289 | 4. | .0 | 18.0 |
| 29 JUN 1458 | 90 | 4. | .0 | 18.0 | * | 29 JUN 1818 | 190 | 14. | .2 | 20.3 | * | 29 JUN 2138 | 290 | 4. | .0 | 18.0 |
| 29 JUN 1500 | 91 | 4. | .0 | 18.0 | * | 29 JUN 1820 | 191 | 13. | .2 | 20.2 | * | 29 JUN 2140 | 291 | 4. | .0 | 18.0 |
| 29 JUN 1502 | 92 | 4. | .0 | 18.0 | * | 29 JUN 1822 | 192 | 12. | .2 | 20.2 | * | 29 JUN 2142 | 292 | 4. | .0 | 18.0 |
| 29 JUN 1504 | 93 | 4. | .0 | 18.0 | * | 29 JUN 1824 | 193 | 11. | .2 | 20.2 | * | 29 JUN 2144 | 293 | 4. | .0 | 18.0 |

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|                 |    |    |      |   |                 |     |    |      |   |                 |    |    |      |
|-----------------|----|----|------|---|-----------------|-----|----|------|---|-----------------|----|----|------|
| 29 JUN 1508 95  | 4. | .0 | 18.0 | * | 29 JUN 1828 195 | 10. | .2 | 20.1 | * | 29 JUN 2148 295 | 4. | .0 | 18.0 |
| 29 JUN 1510 96  | 4. | .0 | 18.0 | * | 29 JUN 1830 196 | 9.  | .2 | 20.1 | * | 29 JUN 2150 296 | 4. | .0 | 18.0 |
| 29 JUN 1512 97  | 4. | .0 | 18.0 | * | 29 JUN 1832 197 | 8.  | .2 | 20.1 | * | 29 JUN 2152 297 | 4. | .0 | 18.0 |
| 29 JUN 1514 98  | 4. | .0 | 18.0 | * | 29 JUN 1834 198 | 8.  | .2 | 20.1 | * | 29 JUN 2154 298 | 4. | .0 | 18.0 |
| 29 JUN 1516 99  | 4. | .0 | 18.0 | * | 29 JUN 1836 199 | 8.  | .2 | 20.1 | * | 29 JUN 2156 299 | 4. | .0 | 18.0 |
| 29 JUN 1518 100 | 4. | .0 | 18.0 | * | 29 JUN 1838 200 | 8.  | .2 | 20.1 | * | 29 JUN 2158 300 | 4. | .0 | 18.0 |

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|                    |              |          |                      |       |       |         |
|--------------------|--------------|----------|----------------------|-------|-------|---------|
| PEAK FLOW<br>(CFS) | TIME<br>(HR) |          | MAXIMUM AVERAGE FLOW |       |       |         |
| 57.                | 5.57         | (CFS)    | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
|                    |              | (INCHES) | 10.                  | 8.    | 8.    | 8.      |
|                    |              | (AC-FT)  | 1.155                | 1.441 | 1.441 | 1.441   |
|                    |              |          | 5.                   | 6.    | 6.    | 6.      |

|                         |              |  |                         |       |       |         |
|-------------------------|--------------|--|-------------------------|-------|-------|---------|
| PEAK STORAGE<br>(AC-FT) | TIME<br>(HR) |  | MAXIMUM AVERAGE STORAGE |       |       |         |
| 0.                      | 5.53         |  | 6-HR                    | 24-HR | 72-HR | 9.97-HR |
|                         |              |  | 0.                      | 0.    | 0.    | 0.      |

|                      |              |  |                       |       |       |         |
|----------------------|--------------|--|-----------------------|-------|-------|---------|
| PEAK STAGE<br>(FEET) | TIME<br>(HR) |  | MAXIMUM AVERAGE STAGE |       |       |         |
| 20.98                | 5.57         |  | 6-HR                  | 24-HR | 72-HR | 9.97-HR |
|                      |              |  | 19.06                 | 18.64 | 18.64 | 18.64   |

CUMULATIVE AREA = .08 SQ MI

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| DAHRMN PER |     |     | (I) INFLOW, |     | (O) OUTFLOW |     | STATION |    | RES2 |    | (S) STORAGE |    |
|------------|-----|-----|-------------|-----|-------------|-----|---------|----|------|----|-------------|----|
|            | 0.  | 10. | 20.         | 30. | 40.         | 50. | 60.     | 0. | 0.   | 0. | 0.          | 0. |
| 291200     | 0.0 | .0  | .0          | .0  | .0          | .0  | .0      | .1 | .2   | .3 | .0          | .0 |
| 291202     | 2.  | I   | O           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291204     | 3.  | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291206     | 4.  | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291208     | 5.  | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291210     | 6.  | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291212     | 7.  | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291214     | 8.  | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291216     | 9.  | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291218     | 10. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291220     | 11. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291222     | 12. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291224     | 13. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291226     | 14. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291228     | 15. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291230     | 16. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291232     | 17. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291234     | 18. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291236     | 19. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291238     | 20. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291240     | 21. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291242     | 22. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291244     | 23. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291246     | 24. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291248     | 25. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291250     | 26. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291252     | 27. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291254     | 28. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291256     | 29. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291258     | 30. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291300     | 31. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291302     | 32. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291304     | 33. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291306     | 34. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291308     | 35. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291310     | 36. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291312     | 37. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291314     | 38. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291316     | 39. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291318     | 40. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291320     | 41. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291322     | 42. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291324     | 43. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291326     | 44. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291328     | 45. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291330     | 46. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291332     | 47. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291334     | 48. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291336     | 49. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291338     | 50. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291340     | 51. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291342     | 52. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291344     | 53. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291346     | 54. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291348     | 55. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291350     | 56. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291352     | 57. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291354     | 58. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291356     | 59. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291358     | 60. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291400     | 61. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291402     | 62. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291404     | 63. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291406     | 64. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291408     | 65. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291410     | 66. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291412     | 67. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291414     | 68. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291416     | 69. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291418     | 70. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291420     | 71. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291422     | 72. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291424     | 73. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |
| 291426     | 74. | I   | .           | .   | .           | .   | .       | .  | .    | .  | .           | .  |

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129 KK \*\*\*\*\*  
\* \*  
\* CH4 \*  
\* \*  
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131 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

132 RD MUSKINGUM-CUNGE CHANNEL ROUTING  
L 400. CHANNEL LENGTH  
S .0079 SLOPE  
N .025 CHANNEL ROUGHNESS COEFFICIENT  
CA .00 CONTRIBUTING AREA  
SHAPE TRAP CHANNEL SHAPE  
WD 2.00 BOTTOM WIDTH OR DIAMETER  
Z 3.00 SIDE SLOPE

\*\*\*  
COMPUTED MUSKINGUM-CUNGE PARAMETERS  
COMPUTATION TIME STEP

| ELEMENT | ALPHA | M    | DT<br>(MIN) | DX<br>(FT) | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|------|-------------|------------|---------------|--------------------------|----------------|------------------------------|
| MAIN    | 2.12  | 1.34 | 1.20        | 200.00     | 57.15         | 334.32                   | 1.44           | 5.56                         |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |  |       |        |      |  |
|------|------|------|------|--|-------|--------|------|--|
| MAIN | 2.12 | 1.34 | 2.00 |  | 57.14 | 334.00 | 1.44 |  |
|------|------|------|------|--|-------|--------|------|--|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6266E+01 EXCESS= .0000E+00 OUTFLOW= .6253E+01 BASIN STORAGE= .1423E-01 PERCENT ERROR= .0

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HYDROGRAPH AT STATION CH4

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 57.                | 5.57         | (CFS)<br>10.         | 8.    | 8.    | 8.      |
|                    |              | (INCHES)<br>1.155    | 1.438 | 1.438 | 1.438   |
|                    |              | (AC-FT)<br>5.        | 6.    | 6.    | 6.      |

CUMULATIVE AREA = .08 SQ MI

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133 KK \*\*\*\*\*  
\* \*  
\* B345 \*  
\* \*  
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135 KO OUTPUT CONTROL VARIABLES  
IDDMT 2 PRINT CONTROL

R4-34



OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

136 BA SUBBASIN CHARACTERISTICS  
 TAREA .01 SUBBASIN AREA

PRECIPITATION DATA

137 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
 ..... HYDRO-35 ..... TP-40 ..... TP-49 .....  
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
 .39 .76 1.34 1.40 1.44 1.56 1.69 2.01 .00 .00 .00 .00

STORM AREA = .01

138 LS SCS LOSS RATE  
 STRL .08 INITIAL ABSTRACTION  
 CRVNBR 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

139 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .32 LAG

\*\*\*

UNIT HYDROGRAPH  
50 END-OF-PERIOD ORDINATES

|     |     |     |     |     |     |     |     |     |     |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| 1.  | 2.  | 3.  | 6.  | 9.  | 12. | 15. | 17. | 18. | 19. |
| 19. | 18. | 16. | 15. | 13. | 11. | 9.  | 8.  | 7.  | 6.  |
| 5.  | 4.  | 4.  | 3.  | 3.  | 2.  | 2.  | 2.  | 1.  | 1.  |
| 1.  | 1.  | 1.  | 1.  | 1.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |

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HYDROGRAPH AT STATION B345

TOTAL RAINFALL = 1.65. TOTAL LOSS = .41, TOTAL EXCESS = 1.24

|                 |           |                |       |       |         |
|-----------------|-----------|----------------|-------|-------|---------|
| PEAK FLOW (CFS) | TIME (HR) | 6-HR           | 24-HR | 72-HR | 9.97-HR |
| 14.             | 5.33      | (CFS) 2.       | 1.    | 1.    | 1.      |
|                 |           | (INCHES) 1.232 | 1.233 | 1.233 | 1.233   |
|                 |           | (AC-FT) 1.     | 1.    | 1.    | 1.      |

CUMULATIVE AREA = .01 SQ MI

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 \* \*  
 \* B345 \*  
 \* \*  
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140 KK

142 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 OSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

143 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION B345

PEAK FLOW TIME MAXIMUM AVERAGE FLOW

R4-35

67.

5.53

(CFS)  
(INCHES)  
(AC-FT)

12.  
1.166  
6.

9.  
1.409  
7.

9.  
1.409  
7.

9.  
1.409  
7.

CUMULATIVE AREA = .09 SQ MI

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144 KK

\* \*  
\* RES3 \*  
\* \*  
\*\*\*\*\*

146 KO

OUTPUT CONTROL VARIABLES

IPRNT 1 PRINT CONTROL  
IPLOT 2 PLOT CONTROL  
OSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

147 RS

STORAGE ROUTING

NSTPS 1 NUMBER OF SUBREACHES  
ITYP ELEV TYPE OF INITIAL CONDITION  
RSVRIC 12.83 INITIAL CONDITION  
X .00 WORKING R AND D COEFFICIENT

148 SV

STORAGE

.1 .2 .4 .7

149 SE

ELEVATION

14.20 15.20 16.20 17.20

150 SL

LOW-LEVEL OUTLET

ELEVL 13.83 ELEVATION AT CENTER OF OUTLET  
CAREA .06 CROSS-SECTIONAL AREA  
COOL .60 COEFFICIENT  
EXPL .50 EXPONENT OF HEAD

151 SS

SPILLWAY

CREL 15.75 SPILLWAY CREST ELEVATION  
SPWID 14.00 SPILLWAY WIDTH  
COOW 2.70 WEIR COEFFICIENT  
EXPW 1.50 EXPONENT OF HEAD

\*\*\*

COMPUTED OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| OUTFLOW   | .00   | .20   | .21   | .23   | .24   | .27   | .29   | .32   | .36   | .42   |
| ELEVATION | 14.20 | 14.25 | 14.31 | 14.39 | 14.49 | 14.61 | 14.77 | 14.99 | 15.30 | 15.75 |
| OUTFLOW   | .49   | .96   | 2.23  | 4.70  | 8.74  | 14.77 | 23.17 | 34.34 | 48.67 | 66.55 |
| ELEVATION | 15.76 | 15.81 | 15.88 | 15.98 | 16.11 | 16.27 | 16.46 | 16.68 | 16.92 | 17.20 |

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| STORAGE   | .08   | .09   | .10   | .11   | .12   | .14   | .16   | .19   | .22   | .24   |
| OUTFLOW   | .18   | .20   | .21   | .23   | .24   | .27   | .29   | .32   | .35   | .36   |
| ELEVATION | 14.20 | 14.25 | 14.31 | 14.39 | 14.49 | 14.61 | 14.77 | 14.99 | 15.20 | 15.30 |
| STORAGE   | .33   | .33   | .34   | .35   | .37   | .40   | .42   | .43   | .48   | .53   |
| OUTFLOW   | .42   | .49   | .96   | 2.23  | 4.70  | 8.74  | 11.87 | 14.77 | 23.17 | 34.34 |
| ELEVATION | 15.75 | 15.76 | 15.81 | 15.88 | 15.98 | 16.11 | 16.20 | 16.27 | 16.46 | 16.68 |
| STORAGE   | .60   | .66   |       |       |       |       |       |       |       |       |
| OUTFLOW   | 48.67 | 66.55 |       |       |       |       |       |       |       |       |
| ELEVATION | 16.92 | 17.20 |       |       |       |       |       |       |       |       |

HYDROGRAPH AT STATION RES3

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DA MON HRMN ORD OUTFLOW STORAGE STAGE \* DA MON HRMN ORD OUTFLOW STORAGE STAGE \* DA MON HRMN ORD OUTFLOW STORAGE STAGE \*  
04 JUN 1960 1 0 1 14 2 \* 04 JUN 1960 101 A A 15 0 \* 04 JUN 1960 201 0 A 16 1



|             |     |    |    |      |   |             |     |     |    |      |   |             |     |    |    |      |
|-------------|-----|----|----|------|---|-------------|-----|-----|----|------|---|-------------|-----|----|----|------|
| 29 JUN 1500 | 91  | 4. | .4 | 15.9 | * | 29 JUN 1820 | 191 | 17. | .4 | 16.3 | * | 29 JUN 2140 | 291 | 4. | .4 | 16.0 |
| 29 JUN 1502 | 92  | 4. | .4 | 15.9 | * | 29 JUN 1822 | 192 | 16. | .4 | 16.3 | * | 29 JUN 2142 | 292 | 4. | .4 | 16.0 |
| 29 JUN 1504 | 93  | 4. | .4 | 15.9 | * | 29 JUN 1824 | 193 | 15. | .4 | 16.3 | * | 29 JUN 2144 | 293 | 4. | .4 | 16.0 |
| 29 JUN 1506 | 94  | 4. | .4 | 15.9 | * | 29 JUN 1826 | 194 | 14. | .4 | 16.2 | * | 29 JUN 2146 | 294 | 4. | .4 | 16.0 |
| 29 JUN 1508 | 95  | 4. | .4 | 15.9 | * | 29 JUN 1828 | 195 | 13. | .4 | 16.2 | * | 29 JUN 2148 | 295 | 4. | .4 | 16.0 |
| 29 JUN 1510 | 96  | 4. | .4 | 15.9 | * | 29 JUN 1830 | 196 | 12. | .4 | 16.2 | * | 29 JUN 2150 | 296 | 4. | .4 | 16.0 |
| 29 JUN 1512 | 97  | 4. | .4 | 15.9 | * | 29 JUN 1832 | 197 | 11. | .4 | 16.2 | * | 29 JUN 2152 | 297 | 4. | .4 | 16.0 |
| 29 JUN 1514 | 98  | 4. | .4 | 15.9 | * | 29 JUN 1834 | 198 | 10. | .4 | 16.2 | * | 29 JUN 2154 | 298 | 4. | .4 | 16.0 |
| 29 JUN 1516 | 99  | 4. | .4 | 15.9 | * | 29 JUN 1836 | 199 | 10. | .4 | 16.1 | * | 29 JUN 2156 | 299 | 4. | .4 | 16.0 |
| 29 JUN 1518 | 100 | 4. | .4 | 15.9 | * | 29 JUN 1838 | 200 | 9.  | .4 | 16.1 | * | 29 JUN 2158 | 300 | 4. | .4 | 16.0 |

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|           |      |          |                      |       |       |         |
|-----------|------|----------|----------------------|-------|-------|---------|
| PEAK FLOW | TIME |          | MAXIMUM AVERAGE FLOW |       |       |         |
| (CFS)     | (HR) |          | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 65.       | 5.63 | (CFS)    | 12.                  | 8.    | 8.    | 8.      |
|           |      | (INCHES) | 1.165                | 1.352 | 1.352 | 1.352   |
|           |      | (AC-FT)  | 6.                   | 7.    | 7.    | 7.      |

|              |      |  |                         |       |       |         |
|--------------|------|--|-------------------------|-------|-------|---------|
| PEAK STORAGE | TIME |  | MAXIMUM AVERAGE STORAGE |       |       |         |
| (AC-FT)      | (HR) |  | 6-HR                    | 24-HR | 72-HR | 9.97-HR |
| 1.           | 5.60 |  | 0.                      | 0.    | 0.    | 0.      |

|            |      |  |                       |       |       |         |
|------------|------|--|-----------------------|-------|-------|---------|
| PEAK STAGE | TIME |  | MAXIMUM AVERAGE STAGE |       |       |         |
| (FEET)     | (HR) |  | 6-HR                  | 24-HR | 72-HR | 9.97-HR |
| 17.18      | 5.63 |  | 16.15                 | 15.98 | 15.98 | 15.98   |

CUMULATIVE AREA = .09 SQ MI











RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

| OPERATION     | STATION | PEAK FLOW | TIME OF PEAK | AVERAGE FLOW FOR MAXIMUM PERIOD |         |         | BASIN AREA | MAXIMUM STAGE | TIME OF MAX STAGE |
|---------------|---------|-----------|--------------|---------------------------------|---------|---------|------------|---------------|-------------------|
|               |         |           |              | 6-HOUR                          | 24-HOUR | 72-HOUR |            |               |                   |
| HYDROGRAPH AT | OF1     | 14.       | 5.33         | 2.                              | 1.      | 1.      | .02        |               |                   |
| ROUTED TO     | CH1     | 14.       | 5.37         | 2.                              | 1.      | 1.      | .02        |               |                   |
| HYDROGRAPH AT | A2      | 5.        | 5.30         | 1.                              | 0.      | 0.      | .01        |               |                   |
| 2 COMBINED AT | A2      | 19.       | 5.37         | 2.                              | 1.      | 1.      | .02        |               |                   |
| ROUTED TO     | ST1     | 19.       | 5.43         | 2.                              | 1.      | 1.      | .02        |               |                   |
| HYDROGRAPH AT | A3      | 5.        | 5.33         | 1.                              | 0.      | 0.      | .01        |               |                   |
| 2 COMBINED AT | A3      | 24.       | 5.40         | 3.                              | 2.      | 2.      | .03        |               |                   |
| HYDROGRAPH AT | OF2     | 11.       | 5.53         | 2.                              | 1.      | 1.      | .02        |               |                   |
| ROUTED TO     | CH2     | 11.       | 5.57         | 2.                              | 1.      | 1.      | .02        |               |                   |
| HYDROGRAPH AT | OF3     | 2.        | 5.57         | 0.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | OF3     | 13.       | 5.57         | 2.                              | 1.      | 1.      | .02        |               |                   |
| 2 COMBINED AT | OF3     | 36.       | 5.43         | 5.                              | 3.      | 3.      | .05        |               |                   |
| HYDROGRAPH AT | A1      | 22.       | 5.53         | 4.                              | 2.      | 2.      | .03        |               |                   |
| 2 COMBINED AT | A1      | 58.       | 5.47         | 9.                              | 5.      | 5.      | .08        |               |                   |
| ROUTED TO     | P1      | 57.       | 5.50         | 9.                              | 5.      | 5.      | .08        |               |                   |
| HYDROGRAPH AT | B1      | 1.        | 5.03         | 0.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | B1      | 58.       | 5.50         | 9.                              | 5.      | 5.      | .08        |               |                   |
| ROUTED TO     | RES1    | 57.       | 5.53         | 10.                             | 8.      | 8.      | .08        | 23.57         | 5.53              |
| ROUTED TO     | CH3     | 57.       | 5.53         | 10.                             | 8.      | 8.      | .08        |               |                   |
| HYDROGRAPH AT | B2      | 1.        | 5.03         | 0.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | B2      | 57.       | 5.53         | 10.                             | 8.      | 8.      | .08        |               |                   |
| ROUTED TO     | RES2    | 57.       | 5.57         | 10.                             | 8.      | 8.      | .08        | 20.98         | 5.57              |
| ROUTED TO     | CH4     | 57.       | 5.57         | 10.                             | 8.      | 8.      | .08        |               |                   |
| HYDROGRAPH AT | B345    | 14.       | 5.33         | 2.                              | 1.      | 1.      | .01        |               |                   |
| 2 COMBINED AT | B345    | 67.       | 5.53         | 12.                             | 9.      | 9.      | .09        |               |                   |
| ROUTED TO     | RES3    | 65.       | 5.63         | 12.                             | 8.      | 8.      | .09        | 17.18         | 5.63              |

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SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

| ISTAO | ELEMENT | DT    | PEAK  | TIME TO<br>PEAK | VOLUME | DT    | INTERPOLATED TO     |                             | VOLUME |
|-------|---------|-------|-------|-----------------|--------|-------|---------------------|-----------------------------|--------|
|       |         |       |       |                 |        |       | COMPUTATION<br>PEAK | INTERVAL<br>TIME TO<br>PEAK |        |
|       |         | (MIN) | (CFS) | (MIN)           | (IN)   | (MIN) | (CFS)               | (MIN)                       | (IN)   |
| CH1   | MANE    | 2.00  | 13.80 | 322.00          | .86    | 2.00  | 13.80               | 322.00                      | .86    |

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8035E+00 EXCESS= .0000E+00 OUTFLOW= .8017E+00 BASIN STORAGE= .2569E-02 PERCENT ERROR= -.1

|     |      |      |       |        |     |      |       |        |     |
|-----|------|------|-------|--------|-----|------|-------|--------|-----|
| ST1 | MANE | 2.00 | 19.07 | 326.00 | .92 | 2.00 | 19.07 | 326.00 | .92 |
|-----|------|------|-------|--------|-----|------|-------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1114E+01 EXCESS= .0000E+00 OUTFLOW= .1110E+01 BASIN STORAGE= .5794E-02 PERCENT ERROR= -.2

|     |      |      |       |        |     |      |       |        |     |
|-----|------|------|-------|--------|-----|------|-------|--------|-----|
| CH2 | MANE | 2.00 | 11.16 | 334.00 | .86 | 2.00 | 11.16 | 334.00 | .86 |
|-----|------|------|-------|--------|-----|------|-------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8545E+00 EXCESS= .0000E+00 OUTFLOW= .8532E+00 BASIN STORAGE= .2230E-02 PERCENT ERROR= -.1

|    |      |     |       |        |     |      |       |        |     |
|----|------|-----|-------|--------|-----|------|-------|--------|-----|
| P1 | MANE | .99 | 57.49 | 329.26 | .99 | 2.00 | 57.48 | 330.00 | .99 |
|----|------|-----|-------|--------|-----|------|-------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4237E+01 EXCESS= .0000E+00 OUTFLOW= .4234E+01 BASIN STORAGE= .3330E-02 PERCENT ERROR= .0

|     |      |      |       |        |      |      |       |        |      |
|-----|------|------|-------|--------|------|------|-------|--------|------|
| CH3 | MANE | 1.08 | 57.29 | 331.74 | 1.44 | 2.00 | 57.28 | 332.00 | 1.44 |
|-----|------|------|-------|--------|------|------|-------|--------|------|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6232E+01 EXCESS= .0000E+00 OUTFLOW= .6223E+01 BASIN STORAGE= .1282E-01 PERCENT ERROR= -.1

|     |      |      |       |        |      |      |       |        |      |
|-----|------|------|-------|--------|------|------|-------|--------|------|
| CH4 | MANE | 1.20 | 57.15 | 334.32 | 1.44 | 2.00 | 57.14 | 334.00 | 1.44 |
|-----|------|------|-------|--------|------|------|-------|--------|------|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6266E+01 EXCESS= .0000E+00 OUTFLOW= .6253E+01 BASIN STORAGE= .1423E-01 PERCENT ERROR= .0

\*\*\* NORMAL END OF HEC-1 \*\*\*

RA-44

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   SEPTEMBER 1990                 *
*   VERSION 4.0                     *
*
* RUN DATE 07/01/1994 TIME 19:36:56 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS     *
* HYDROLOGIC ENGINEERING CENTER    *
*   609 SECOND STREET              *
* DAVIS, CALIFORNIA 95616          *
*   (916) 756-1104                 *
*
*****

```

```

X   X XXXXXXXX XXXXX           X
X   X X       X   X           XX
X   X X       X               X
XXXXXXX XXXX   X             XXXXX X
X   X X       X               X
X   X X       X   X           X
X   X XXXXXXXX XXXXX           XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,  
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

DEVELOPED 2 YEAR STORM  
 SUB-BASINS "C, D AND E  
 RUN #5

RS-1

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1         ID  GRAND VIEW SUBDIVISION
2         ID  DEVELOPED CONDITION
3         ID  2 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)
4         IT   2 30JUN94   1200   300
5         IO   5     2     0
          * *****
6         KK   D123
7         KM   Basin runoff calculation for   D123
8         KO   3     1     0     1     21
9         BA   0.0187
10        PH           0   0.10   0.19   0.34   0.42   0.47   0.55   0.64   0.70
11        LS           96
12        UD   0.709
          * *****
13        KK   CH1A
14        KM   Muskingum-Cunge channel routing from   CP9 to   CP10
15        KO   3     1     0     1     21
16        RD   315 0.0045 0.025           TRAP   2     4
          * *****
17        KK   D4
18        KM   Basin runoff calculation for   D4
19        KO   3     1     0     1     21
20        BA   0.0036
21        PH           0   0.10   0.19   0.34   0.42   0.47   0.55   0.64   0.70
22        LS           96
23        UD   0.820
          * *****
24        KK   D4
25        KM   Combining two hydrographs at control point   CP10
26        KO   3     1     0     1     21
27        HC   2
          * *****
28        KK   RES4
29        KM   Reservoir routing operation
30        KO   1     2     0     1     21
31        RS   1   ELEV  17.30
32        SV  0.0326 0.0994 0.0204 0.3618 0.5767 0.8459 1.0833
33        SE  18.3  19.3  20.3  21.3  22.3  23.3  24.0
34        SL  18.3 0.0625  0.6   0.5
35        SS  23.5 12.29  2.7   1.5
          * *****
36        KK   P1A
37        KM   Muskingum-Cunge channel routing from   CP10 to   CP8
38        KO   3     1     0     1     21
39        RD   655 0.0067 0.015           CIRC   2.0
          * *****

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

40 KK C2  
 41 KM Basin runoff calculation for C2  
 42 KO 3 1 0 1 21  
 43 BA 0.0049  
 44 PH 0 0.10 0.19 0.34 0.42 0.47 0.55 0.64 0.70  
 45 LS 96  
 46 UD 1.000  
 \* \*\*\*\*\*

47 KK C2  
 48 KM Combining two hydrographs at control point CP8  
 49 KO 3 1 0 1 21  
 50 HC 2  
 \* \*\*\*\*\*

51 KK C13  
 52 KM Basin runoff calculation for C13  
 53 KO 3 1 0 1 21  
 54 BA 0.0206  
 55 PH 0 0.10 0.19 0.34 0.42 0.47 0.55 0.64 0.70  
 56 LS 96  
 57 UD 0.719  
 \* \*\*\*\*\*

58 KK C12  
 59 KM Combining two hydrographs at control point CP8  
 60 KO 3 1 0 1 21  
 61 HC 2  
 \* \*\*\*\*\*

62 KK P2A  
 63 KM Muskingum-Cunge channel routing from CP8 to CP11  
 64 KO 3 1 0 1 21  
 65 RD 277 0.0054 0.015 CIRC 3.0  
 \* \*\*\*\*\*

66 KK E1  
 67 KM Basin runoff calculation for E1  
 68 KO 3 1 0 1 21  
 69 BA 0.0009  
 70 PH 0 0.10 0.19 0.34 0.42 0.47 0.55 0.64 0.70  
 71 LS 96  
 72 UD 0.146  
 \* \*\*\*\*\*

73 KK E1  
 74 KM Combining two hydrographs at control point CP11  
 75 KO 3 1 0 1 21  
 76 HC 2  
 \* \*\*\*\*\*

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```
77 KK RES5
78 KM Reservoir routing operation
79 KO 1 2 0 1 21
80 RS 1 ELEV 11.24
81 SV 0.0184 0.0693 0.1631 0.3126 0.5207 0.7889
82 SE 12.0 13.0 14.0 15.0 16.0 17.0
83 SL 12.24 0.0625 0.6 0.50
84 SS 16.0 12.29 2.7 1.5
* *****
85 ZZ
```

R5-4

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   SEPTEMBER 1990                *
*   VERSION 4.0                    *
*
* RUN DATE 07/01/1994 TIME 19:36:56 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
*   609 SECOND STREET          *
*   DAVIS, CALIFORNIA 95616    *
*   (916) 756-1104            *
*
*****

```

```

GRAND VIEW SUBDIVISION
DEVELOPED CONDITION
2 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)

```

```

5 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5 PRINT CONTROL
          IPLOT      2 PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

```

```

IT        HYDROGRAPH TIME DATA
          NMIN      2 MINUTES IN COMPUTATION INTERVAL
          IDATE     30JUN94 STARTING DATE
          ITIME     1200 STARTING TIME
          NQ        300 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE    30JUN94 ENDING DATE
          NDTIME    2158 ENDING TIME
          ICENT     19 CENTURY MARK

```

```

COMPUTATION INTERVAL .03 HOURS
TOTAL TIME BASE      9.97 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE       DEGREES FAHRENHEIT

```

\*\*\* \*\* \*\* \*\* \*\*

```

*****
*
* D123 *
*
*****

```

```

8 KO      OUTPUT CONTROL VARIABLES
          IPRNT      3 PRINT CONTROL
          IPLOT      1 PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE
          IPNCH      1 PUNCH COMPUTED HYDROGRAPH
          IDOUT      21 SAVE HYDROGRAPH ON THIS UNIT
          ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
          ISAV2      300 LAST ORDINATE PUNCHED OR SAVED
          TIMINT     .033 TIME INTERVAL IN HOURS

```

R5-5

SUBBASIN RUNOFF DATA

9 BA SUBBASIN CHARACTERISTICS

TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

| ..... HYDRO-35 ..... |        |        | ..... TP-40 ..... |      |      |       | ..... TP-49 ..... |       |       |       |        |
|----------------------|--------|--------|-------------------|------|------|-------|-------------------|-------|-------|-------|--------|
| 5-MIN                | 15-MIN | 60-MIN | 2-HR              | 3-HR | 6-HR | 12-HR | 24-HR             | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
| .10                  | .19    | .34    | .42               | .47  | .55  | .64   | .70               | .00   | .00   | .00   | .00    |

STORM AREA = .02

11 LS SCS LOSS RATE

STRTL .08 INITIAL ABSTRACTION  
 CRVNR 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

12 UD SCS DIMENSIONLESS UNITGRAPH

TLAG .71 LAG

\*\*\*

UNIT HYDROGRAPH  
108 END-OF-PERIOD ORDINATES

|     |     |     |     |     |     |     |     |     |     |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| 0.  | 0.  | 1.  | 1.  | 2.  | 2.  | 3.  | 3.  | 4.  | 5.  |
| 6.  | 7.  | 8.  | 9.  | 10. | 11. | 11. | 12. | 12. | 12. |
| 12. | 12. | 12. | 12. | 12. | 12. | 11. | 11. | 10. | 10. |
| 9.  | 9.  | 8.  | 8.  | 7.  | 6.  | 6.  | 5.  | 5.  | 5.  |
| 4.  | 4.  | 4.  | 3.  | 3.  | 3.  | 3.  | 3.  | 2.  | 2.  |
| 2.  | 2.  | 2.  | 2.  | 2.  | 1.  | 1.  | 1.  | 1.  | 1.  |
| 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |

\*\*\* \*\*\* \*\*\* \*\*\* \*\*\*

HYDROGRAPH AT STATION D123

TOTAL RAINFALL = .61, TOTAL LOSS = .32, TOTAL EXCESS = .30

| PEAK FLOW (CFS) | TIME (HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|-----------------|-----------|----------------------|-------|-------|---------|
|                 |           | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 2.              | 5.80      | (CFS) 1.             | 0.    | 0.    | 0.      |
|                 |           | (INCHES) .287        | .287  | .287  | .287    |
|                 |           | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .02 SQ MI

\*\*\* \*\*

\*\*\*\*\*  
 \* \*  
 \* CHIA \*  
 \* \*  
 \*\*\*\*\*

13 KK

R5-6



```

IPRNT      3 PRINT CONTROL
IPLOT      1 PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      1 PUNCH COMPUTED HYDROGRAPH
IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      300 LAST ORDINATE PUNCHED OR SAVED
TIMINT     .033 TIME INTERVAL IN HOURS

```

HYDROGRAPH ROUTING DATA

16 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

```

L      315. CHANNEL LENGTH
S      .0045 SLOPE
N      .025 CHANNEL ROUGHNESS COEFFICIENT
CA     .00 CONTRIBUTING AREA
SHAPE  TRAP CHANNEL SHAPE
WD     2.00 BOTTOM WIDTH OR DIAMETER
Z      4.00 SIDE SLOPE

```

\*\*\*

COMPUTED MUSKINGUM-CUNGE PARAMETERS

| ELEMENT | ALPHA | COMPUTATION TIME STEP |             |            | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|-----------------------|-------------|------------|---------------|--------------------------|----------------|------------------------------|
|         |       | M                     | DT<br>(MIN) | DX<br>(FT) |               |                          |                |                              |
| MAIN    | 1.50  | 1.34                  | 2.00        | 105.00     | 2.11          | 350.00                   | .29            | 1.84                         |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |  |      |        |     |  |
|------|------|------|------|--|------|--------|-----|--|
| MAIN | 1.50 | 1.34 | 2.00 |  | 2.11 | 350.00 | .29 |  |
|------|------|------|------|--|------|--------|-----|--|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2860E+00 EXCESS= .0000E+00 OUTFLOW= .2848E+00 BASIN STORAGE= .1343E-02 PERCENT ERROR= .0

\*\*\* \*\*

HYDROGRAPH AT STATION CH1A

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 2.                 | 5.83         | (CFS) 1.             | 0.    | 0.    | 0.      |
|                    |              | (INCHES) .285        | .285  | .285  | .285    |
|                    |              | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .02 SQ MI

\*\*\* \*\*

\*\*\*\*\*

\* \*

\* D4 \*

\* \*

\*\*\*\*\*

17 KK

RS-7

IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

20 BA SUBBASIN CHARACTERISTICS  
 TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

21 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

| HYDRO-35 |        |        | TP-40 |      |      |       | TP-49 |       |       |       |        |
|----------|--------|--------|-------|------|------|-------|-------|-------|-------|-------|--------|
| 5-MIN    | 15-MIN | 60-MIN | 2-HR  | 3-HR | 6-HR | 12-HR | 24-HR | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
| .10      | .19    | .34    | .42   | .47  | .55  | .64   | .70   | .00   | .00   | .00   | .00    |

STORM AREA = .00

22 LS SCS LOSS RATE  
 STRTL .08 INITIAL ABSTRACTION  
 CRVNB 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

23 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .82 LAG

\*\*\*

UNIT HYDROGRAPH  
 125 END-OF-PERIOD ORDINATES

|    |    |    |    |    |    |    |    |    |    |
|----|----|----|----|----|----|----|----|----|----|
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 1. | 1. |
| 1. | 1. | 1. | 1. | 1. | 1. | 1. | 2. | 2. | 2. |
| 2. | 2. | 2. | 2. | 2. | 2. | 2. | 2. | 2. | 2. |
| 2. | 2. | 2. | 2. | 2. | 2. | 1. | 1. | 1. | 1. |
| 1. | 1. | 1. | 1. | 1. | 1. | 1. | 1. | 1. | 1. |
| 1. | 1. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |

\*\*\*

HYDROGRAPH AT STATION D4

TOTAL RAINFALL = .61, TOTAL LOSS = .32, TOTAL EXCESS = .30

| PEAK FLOW (CFS) | TIME (HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|-----------------|-----------|----------------------|-------|-------|---------|
|                 |           | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 0.              | 5.90      | (CFS) 0.             | 0.    | 0.    | 0.      |
|                 |           | (INCHES) .285        | .285  | .285  | .285    |
|                 |           | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .00 SQ MI

R5-8

\*\*\*\*\*

24 KK \*\*\*\*\*  
\* \*  
\* D4 \*  
\* \*  
\*\*\*\*\*

26 KO OUTPUT CONTROL VARIABLES  
IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

27 HC HYDROGRAPH COMBINATION  
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

\*\*\*

\*\*\* \*\*

HYDROGRAPH AT STATION D4

| EAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|-------------------|--------------|----------------------|-------|-------|---------|
|                   |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 2.                | 5.83         | (CFS) 1.             | 0.    | 0.    | 0.      |
|                   |              | (INCHES) .285        | .285  | .285  | .285    |
|                   |              | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .02 SQ MI

\*\*\*\*\*

28 KK \*\*\*\*\*  
\* \*  
\* RES4 \*  
\* \*  
\*\*\*\*\*

30 KO OUTPUT CONTROL VARIABLES  
IPRNT 1 PRINT CONTROL  
IPLOT 2 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

31 RS STORAGE ROUTING  
NSTPS 1 NUMBER OF SUBREACHES

R5-9

RSVRIC 17.30 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

32 SV STORAGE .0 .1 .0 .4 .6 .8 1.1

33 SE ELEVATION 18.30 19.30 20.30 21.30 22.30 23.30 24.00

34 SL LOW-LEVEL OUTLET

ELEV 18.30 ELEVATION AT CENTER OF OUTLET  
 CAREA .06 CROSS-SECTIONAL AREA  
 COQL .60 COEFFICIENT  
 EXPL .50 EXPONENT OF HEAD

35 SS SPILLWAY

CREL 23.50 SPILLWAY CREST ELEVATION  
 SPWID 12.29 SPILLWAY WIDTH  
 COQW 2.70 WEIR COEFFICIENT  
 EXPW 1.50 EXPONENT OF HEAD

\*\*\*

COMPUTED OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| OUTFLOW   | .00   | .11   | .13   | .14   | .17   | .20   | .24   | .31   | .42   | .69   |
| ELEVATION | 18.30 | 18.45 | 18.48 | 18.53 | 18.61 | 18.73 | 18.93 | 19.33 | 20.28 | 23.50 |
| OUTFLOW   | .70   | .78   | 1.01  | 1.45  | 2.17  | 3.24  | 4.74  | 6.72  | 9.27  | 12.45 |
| ELEVATION | 23.51 | 23.52 | 23.55 | 23.58 | 23.63 | 23.68 | 23.75 | 23.82 | 23.91 | 24.00 |

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| STORAGE   | .03   | .04   | .04   | .05   | .05   | .06   | .07   | .10   | .10   | .02   |
| OUTFLOW   | .00   | .11   | .13   | .14   | .17   | .20   | .24   | .30   | .31   | .42   |
| ELEVATION | 18.30 | 18.45 | 18.48 | 18.53 | 18.61 | 18.73 | 18.93 | 19.30 | 19.33 | 20.28 |
| STORAGE   | .02   | .36   | .58   | .85   | .91   | .92   | .93   | .94   | .96   | .97   |
| OUTFLOW   | .43   | .52   | .60   | .67   | .69   | .78   | 1.01  | 1.45  | 2.17  | 3.24  |
| ELEVATION | 20.30 | 21.30 | 22.30 | 23.30 | 23.50 | 23.52 | 23.55 | 23.58 | 23.63 | 23.68 |
| STORAGE   | 1.00  | 1.02  | 1.05  | 1.08  |       |       |       |       |       |       |
| OUTFLOW   | 4.74  | 6.72  | 9.27  | 12.45 |       |       |       |       |       |       |
| ELEVATION | 23.75 | 23.82 | 23.91 | 24.00 |       |       |       |       |       |       |

HYDROGRAPH AT STATION RES4

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| DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE |
|----|-----|------|-----|---------|---------|-------|----|-----|------|-----|---------|---------|-------|----|-----|------|-----|---------|---------|-------|
| 30 | JUN | 1200 | 1   | 0.      | .0      | 18.3  | 30 | JUN | 1520 | 101 | 0.      | .0      | 18.3  | 30 | JUN | 1840 | 201 | 0.      | .2      | 20.9  |
| 30 | JUN | 1202 | 2   | 0.      | .0      | 18.3  | 30 | JUN | 1522 | 102 | 0.      | .0      | 18.3  | 30 | JUN | 1842 | 202 | 0.      | .2      | 20.9  |
| 30 | JUN | 1204 | 3   | 0.      | .0      | 18.3  | 30 | JUN | 1524 | 103 | 0.      | .0      | 18.3  | 30 | JUN | 1844 | 203 | 0.      | .2      | 20.9  |
| 30 | JUN | 1206 | 4   | 0.      | .0      | 18.3  | 30 | JUN | 1526 | 104 | 0.      | .0      | 18.3  | 30 | JUN | 1846 | 204 | 0.      | .2      | 20.9  |
| 30 | JUN | 1208 | 5   | 0.      | .0      | 18.3  | 30 | JUN | 1528 | 105 | 0.      | .0      | 18.3  | 30 | JUN | 1848 | 205 | 0.      | .2      | 20.9  |
| 30 | JUN | 1210 | 6   | 0.      | .0      | 18.3  | 30 | JUN | 1530 | 106 | 0.      | .0      | 18.3  | 30 | JUN | 1850 | 206 | 0.      | .2      | 20.9  |
| 30 | JUN | 1212 | 7   | 0.      | .0      | 18.3  | 30 | JUN | 1532 | 107 | 0.      | .0      | 18.3  | 30 | JUN | 1852 | 207 | 0.      | .2      | 20.9  |
| 30 | JUN | 1214 | 8   | 0.      | .0      | 18.3  | 30 | JUN | 1534 | 108 | 0.      | .0      | 18.3  | 30 | JUN | 1854 | 208 | 0.      | .2      | 20.9  |
| 30 | JUN | 1216 | 9   | 0.      | .0      | 18.3  | 30 | JUN | 1536 | 109 | 0.      | .0      | 18.3  | 30 | JUN | 1856 | 209 | 0.      | .2      | 20.9  |
| 30 | JUN | 1218 | 10  | 0.      | .0      | 18.3  | 30 | JUN | 1538 | 110 | 0.      | .0      | 18.3  | 30 | JUN | 1858 | 210 | 0.      | .2      | 20.9  |
| 30 | JUN | 1220 | 11  | 0.      | .0      | 18.3  | 30 | JUN | 1540 | 111 | 0.      | .0      | 18.3  | 30 | JUN | 1900 | 211 | 0.      | .2      | 20.9  |
| 30 | JUN | 1222 | 12  | 0.      | .0      | 18.3  | 30 | JUN | 1542 | 112 | 0.      | .0      | 18.3  | 30 | JUN | 1902 | 212 | 0.      | .2      | 20.9  |
| 30 | JUN | 1224 | 13  | 0.      | .0      | 18.3  | 30 | JUN | 1544 | 113 | 0.      | .0      | 18.3  | 30 | JUN | 1904 | 213 | 0.      | .2      | 20.9  |

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R5-10

|             |    |    |    |      |   |             |     |    |    |      |   |             |     |    |    |      |
|-------------|----|----|----|------|---|-------------|-----|----|----|------|---|-------------|-----|----|----|------|
| 0 JUN 1228  | 15 | 0. | .0 | 18.3 | * | 30 JUN 1548 | 115 | 0. | .0 | 18.3 | * | 30 JUN 1908 | 215 | 0. | .2 | 20.9 |
| 30 JUN 1230 | 16 | 0. | .0 | 18.3 | * | 30 JUN 1550 | 116 | 0. | .0 | 18.3 | * | 30 JUN 1910 | 216 | 0. | .2 | 20.9 |
| 30 JUN 1232 | 17 | 0. | .0 | 18.3 | * | 30 JUN 1552 | 117 | 0. | .0 | 18.3 | * | 30 JUN 1912 | 217 | 0. | .2 | 20.9 |
| 0 JUN 1234  | 18 | 0. | .0 | 18.3 | * | 30 JUN 1554 | 118 | 0. | .0 | 18.3 | * | 30 JUN 1914 | 218 | 0. | .2 | 20.9 |
| 30 JUN 1236 | 19 | 0. | .0 | 18.3 | * | 30 JUN 1556 | 119 | 0. | .0 | 18.3 | * | 30 JUN 1916 | 219 | 0. | .2 | 20.9 |
| 30 JUN 1238 | 20 | 0. | .0 | 18.3 | * | 30 JUN 1558 | 120 | 0. | .0 | 18.3 | * | 30 JUN 1918 | 220 | 0. | .2 | 20.9 |
| 0 JUN 1240  | 21 | 0. | .0 | 18.3 | * | 30 JUN 1600 | 121 | 0. | .0 | 18.3 | * | 30 JUN 1920 | 221 | 0. | .2 | 20.9 |
| 30 JUN 1242 | 22 | 0. | .0 | 18.3 | * | 30 JUN 1602 | 122 | 0. | .0 | 18.3 | * | 30 JUN 1922 | 222 | 0. | .2 | 20.9 |
| 30 JUN 1244 | 23 | 0. | .0 | 18.3 | * | 30 JUN 1604 | 123 | 0. | .0 | 18.3 | * | 30 JUN 1924 | 223 | 0. | .2 | 20.9 |
| 0 JUN 1246  | 24 | 0. | .0 | 18.3 | * | 30 JUN 1606 | 124 | 0. | .0 | 18.3 | * | 30 JUN 1926 | 224 | 0. | .2 | 20.9 |
| 30 JUN 1248 | 25 | 0. | .0 | 18.3 | * | 30 JUN 1608 | 125 | 0. | .0 | 18.3 | * | 30 JUN 1928 | 225 | 0. | .2 | 20.9 |
| 30 JUN 1250 | 26 | 0. | .0 | 18.3 | * | 30 JUN 1610 | 126 | 0. | .0 | 18.3 | * | 30 JUN 1930 | 226 | 0. | .2 | 20.9 |
| 0 JUN 1252  | 27 | 0. | .0 | 18.3 | * | 30 JUN 1612 | 127 | 0. | .0 | 18.3 | * | 30 JUN 1932 | 227 | 0. | .2 | 20.9 |
| 30 JUN 1254 | 28 | 0. | .0 | 18.3 | * | 30 JUN 1614 | 128 | 0. | .0 | 18.3 | * | 30 JUN 1934 | 228 | 0. | .2 | 20.9 |
| 30 JUN 1256 | 29 | 0. | .0 | 18.3 | * | 30 JUN 1616 | 129 | 0. | .0 | 18.3 | * | 30 JUN 1936 | 229 | 0. | .2 | 20.9 |
| 0 JUN 1258  | 30 | 0. | .0 | 18.3 | * | 30 JUN 1618 | 130 | 0. | .0 | 18.3 | * | 30 JUN 1938 | 230 | 0. | .2 | 20.9 |
| 30 JUN 1300 | 31 | 0. | .0 | 18.3 | * | 30 JUN 1620 | 131 | 0. | .0 | 18.3 | * | 30 JUN 1940 | 231 | 0. | .2 | 20.9 |
| 30 JUN 1302 | 32 | 0. | .0 | 18.3 | * | 30 JUN 1622 | 132 | 0. | .0 | 18.3 | * | 30 JUN 1942 | 232 | 0. | .2 | 20.9 |
| 0 JUN 1304  | 33 | 0. | .0 | 18.3 | * | 30 JUN 1624 | 133 | 0. | .0 | 18.3 | * | 30 JUN 1944 | 233 | 0. | .2 | 20.9 |
| 30 JUN 1306 | 34 | 0. | .0 | 18.3 | * | 30 JUN 1626 | 134 | 0. | .0 | 18.3 | * | 30 JUN 1946 | 234 | 0. | .2 | 20.9 |
| 30 JUN 1308 | 35 | 0. | .0 | 18.3 | * | 30 JUN 1628 | 135 | 0. | .0 | 18.3 | * | 30 JUN 1948 | 235 | 0. | .2 | 20.9 |
| 0 JUN 1310  | 36 | 0. | .0 | 18.3 | * | 30 JUN 1630 | 136 | 0. | .0 | 18.3 | * | 30 JUN 1950 | 236 | 0. | .2 | 20.9 |
| 30 JUN 1312 | 37 | 0. | .0 | 18.3 | * | 30 JUN 1632 | 137 | 0. | .0 | 18.3 | * | 30 JUN 1952 | 237 | 0. | .2 | 20.9 |
| 30 JUN 1314 | 38 | 0. | .0 | 18.3 | * | 30 JUN 1634 | 138 | 0. | .0 | 18.3 | * | 30 JUN 1954 | 238 | 0. | .2 | 20.9 |
| 0 JUN 1316  | 39 | 0. | .0 | 18.3 | * | 30 JUN 1636 | 139 | 0. | .0 | 18.3 | * | 30 JUN 1956 | 239 | 0. | .2 | 20.9 |
| 30 JUN 1318 | 40 | 0. | .0 | 18.3 | * | 30 JUN 1638 | 140 | 0. | .0 | 18.3 | * | 30 JUN 1958 | 240 | 0. | .2 | 20.9 |
| 30 JUN 1320 | 41 | 0. | .0 | 18.3 | * | 30 JUN 1640 | 141 | 0. | .0 | 18.3 | * | 30 JUN 2000 | 241 | 0. | .2 | 20.9 |
| 0 JUN 1322  | 42 | 0. | .0 | 18.3 | * | 30 JUN 1642 | 142 | 0. | .0 | 18.3 | * | 30 JUN 2002 | 242 | 0. | .2 | 20.9 |
| 30 JUN 1324 | 43 | 0. | .0 | 18.3 | * | 30 JUN 1644 | 143 | 0. | .0 | 18.3 | * | 30 JUN 2004 | 243 | 0. | .2 | 20.9 |
| 30 JUN 1326 | 44 | 0. | .0 | 18.3 | * | 30 JUN 1646 | 144 | 0. | .0 | 18.3 | * | 30 JUN 2006 | 244 | 0. | .2 | 20.9 |
| 0 JUN 1328  | 45 | 0. | .0 | 18.3 | * | 30 JUN 1648 | 145 | 0. | .0 | 18.3 | * | 30 JUN 2008 | 245 | 0. | .2 | 20.9 |
| 30 JUN 1330 | 46 | 0. | .0 | 18.3 | * | 30 JUN 1650 | 146 | 0. | .0 | 18.3 | * | 30 JUN 2010 | 246 | 0. | .2 | 20.9 |
| 30 JUN 1332 | 47 | 0. | .0 | 18.3 | * | 30 JUN 1652 | 147 | 0. | .0 | 18.3 | * | 30 JUN 2012 | 247 | 0. | .2 | 20.9 |
| 0 JUN 1334  | 48 | 0. | .0 | 18.3 | * | 30 JUN 1654 | 148 | 0. | .0 | 18.3 | * | 30 JUN 2014 | 248 | 0. | .2 | 20.9 |
| 30 JUN 1336 | 49 | 0. | .0 | 18.3 | * | 30 JUN 1656 | 149 | 0. | .0 | 18.3 | * | 30 JUN 2016 | 249 | 0. | .2 | 20.9 |
| 30 JUN 1338 | 50 | 0. | .0 | 18.3 | * | 30 JUN 1658 | 150 | 0. | .0 | 18.3 | * | 30 JUN 2018 | 250 | 0. | .2 | 20.9 |
| 0 JUN 1340  | 51 | 0. | .0 | 18.3 | * | 30 JUN 1700 | 151 | 0. | .0 | 18.3 | * | 30 JUN 2020 | 251 | 0. | .2 | 20.9 |
| 30 JUN 1342 | 52 | 0. | .0 | 18.3 | * | 30 JUN 1702 | 152 | 0. | .0 | 18.3 | * | 30 JUN 2022 | 252 | 0. | .2 | 20.9 |
| 30 JUN 1344 | 53 | 0. | .0 | 18.3 | * | 30 JUN 1704 | 153 | 0. | .0 | 18.4 | * | 30 JUN 2024 | 253 | 0. | .2 | 20.9 |
| 0 JUN 1346  | 54 | 0. | .0 | 18.3 | * | 30 JUN 1706 | 154 | 0. | .0 | 18.4 | * | 30 JUN 2026 | 254 | 0. | .2 | 20.9 |
| 30 JUN 1348 | 55 | 0. | .0 | 18.3 | * | 30 JUN 1708 | 155 | 0. | .0 | 18.4 | * | 30 JUN 2028 | 255 | 0. | .2 | 20.9 |
| 30 JUN 1350 | 56 | 0. | .0 | 18.3 | * | 30 JUN 1710 | 156 | 0. | .0 | 18.4 | * | 30 JUN 2030 | 256 | 0. | .2 | 20.9 |
| 0 JUN 1352  | 57 | 0. | .0 | 18.3 | * | 30 JUN 1712 | 157 | 0. | .0 | 18.4 | * | 30 JUN 2032 | 257 | 0. | .2 | 20.9 |
| 30 JUN 1354 | 58 | 0. | .0 | 18.3 | * | 30 JUN 1714 | 158 | 0. | .0 | 18.4 | * | 30 JUN 2034 | 258 | 0. | .2 | 20.9 |
| 30 JUN 1356 | 59 | 0. | .0 | 18.3 | * | 30 JUN 1716 | 159 | 0. | .0 | 18.4 | * | 30 JUN 2036 | 259 | 0. | .2 | 20.9 |
| 0 JUN 1358  | 60 | 0. | .0 | 18.3 | * | 30 JUN 1718 | 160 | 0. | .0 | 18.5 | * | 30 JUN 2038 | 260 | 0. | .2 | 20.9 |
| 30 JUN 1400 | 61 | 0. | .0 | 18.3 | * | 30 JUN 1720 | 161 | 0. | .0 | 18.5 | * | 30 JUN 2040 | 261 | 0. | .2 | 20.9 |
| 30 JUN 1402 | 62 | 0. | .0 | 18.3 | * | 30 JUN 1722 | 162 | 0. | .0 | 18.5 | * | 30 JUN 2042 | 262 | 0. | .2 | 20.9 |
| 0 JUN 1404  | 63 | 0. | .0 | 18.3 | * | 30 JUN 1724 | 163 | 0. | .1 | 18.6 | * | 30 JUN 2044 | 263 | 0. | .2 | 20.9 |
| 30 JUN 1406 | 64 | 0. | .0 | 18.3 | * | 30 JUN 1726 | 164 | 0. | .1 | 18.6 | * | 30 JUN 2046 | 264 | 0. | .2 | 20.9 |
| 30 JUN 1408 | 65 | 0. | .0 | 18.3 | * | 30 JUN 1728 | 165 | 0. | .1 | 18.7 | * | 30 JUN 2048 | 265 | 0. | .2 | 20.9 |
| 0 JUN 1410  | 66 | 0. | .0 | 18.3 | * | 30 JUN 1730 | 166 | 0. | .1 | 18.8 | * | 30 JUN 2050 | 266 | 0. | .2 | 20.9 |
| 30 JUN 1412 | 67 | 0. | .0 | 18.3 | * | 30 JUN 1732 | 167 | 0. | .1 | 18.8 | * | 30 JUN 2052 | 267 | 0. | .2 | 20.9 |
| 30 JUN 1414 | 68 | 0. | .0 | 18.3 | * | 30 JUN 1734 | 168 | 0. | .1 | 18.9 | * | 30 JUN 2054 | 268 | 0. | .2 | 20.9 |
| 0 JUN 1416  | 69 | 0. | .0 | 18.3 | * | 30 JUN 1736 | 169 | 0. | .1 | 19.0 | * | 30 JUN 2056 | 269 | 0. | .2 | 20.9 |
| 30 JUN 1418 | 70 | 0. | .0 | 18.3 | * | 30 JUN 1738 | 170 | 0. | .1 | 19.1 | * | 30 JUN 2058 | 270 | 0. | .2 | 20.9 |
| 30 JUN 1420 | 71 | 0. | .0 | 18.3 | * | 30 JUN 1740 | 171 | 0. | .1 | 19.1 | * | 30 JUN 2100 | 271 | 0. | .2 | 20.9 |
| 0 JUN 1422  | 72 | 0. | .0 | 18.3 | * | 30 JUN 1742 | 172 | 0. | .1 | 19.2 | * | 30 JUN 2102 | 272 | 0. | .2 | 20.9 |
| 30 JUN 1424 | 73 | 0. | .0 | 18.3 | * | 30 JUN 1744 | 173 | 0. | .1 | 20.5 | * | 30 JUN 2104 | 273 | 0. | .2 | 20.9 |
| 30 JUN 1426 | 74 | 0. | .0 | 18.3 | * | 30 JUN 1746 | 174 | 0. | .1 | 20.5 | * | 30 JUN 2106 | 274 | 0. | .2 | 20.9 |
| 0 JUN 1428  | 75 | 0. | .0 | 18.3 | * | 30 JUN 1748 | 175 | 0. | .1 | 20.6 | * | 30 JUN 2108 | 275 | 0. | .2 | 20.9 |
| 30 JUN 1430 | 76 | 0. | .0 | 18.3 | * | 30 JUN 1750 | 176 | 0. | .1 | 20.6 | * | 30 JUN 2110 | 276 | 0. | .2 | 20.9 |
| 30 JUN 1432 | 77 | 0. | .0 | 18.3 | * | 30 JUN 1752 | 177 | 0. | .1 | 20.6 | * | 30 JUN 2112 | 277 | 0. | .2 | 20.9 |
| 0 JUN 1434  | 78 | 0. | .0 | 18.3 | * | 30 JUN 1754 | 178 | 0. | .1 | 20.6 | * | 30 JUN 2114 | 278 | 0. | .2 | 20.9 |
| 30 JUN 1436 | 79 | 0. | .0 | 18.3 | * | 30 JUN 1756 | 179 | 0. | .1 | 20.6 | * | 30 JUN 2116 | 279 | 0. | .2 | 20.9 |

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|             |     |    |    |      |   |             |     |    |    |      |   |             |     |    |    |      |
|-------------|-----|----|----|------|---|-------------|-----|----|----|------|---|-------------|-----|----|----|------|
| 30 JUN 1440 | 81  | 0. | .0 | 18.3 | * | 30 JUN 1800 | 181 | 0. | .1 | 20.7 | * | 30 JUN 2120 | 281 | 0. | .2 | 20.9 |
| 30 JUN 1442 | 82  | 0. | .0 | 18.3 | * | 30 JUN 1802 | 182 | 0. | .1 | 20.7 | * | 30 JUN 2122 | 282 | 0. | .2 | 20.9 |
| 30 JUN 1444 | 83  | 0. | .0 | 18.3 | * | 30 JUN 1804 | 183 | 0. | .2 | 20.7 | * | 30 JUN 2124 | 283 | 0. | .2 | 20.8 |
| 30 JUN 1446 | 84  | 0. | .0 | 18.3 | * | 30 JUN 1806 | 184 | 0. | .2 | 20.7 | * | 30 JUN 2126 | 284 | 0. | .2 | 20.8 |
| 30 JUN 1448 | 85  | 0. | .0 | 18.3 | * | 30 JUN 1808 | 185 | 0. | .2 | 20.7 | * | 30 JUN 2128 | 285 | 0. | .2 | 20.8 |
| 30 JUN 1450 | 86  | 0. | .0 | 18.3 | * | 30 JUN 1810 | 186 | 0. | .2 | 20.7 | * | 30 JUN 2130 | 286 | 0. | .2 | 20.8 |
| 30 JUN 1452 | 87  | 0. | .0 | 18.3 | * | 30 JUN 1812 | 187 | 0. | .2 | 20.7 | * | 30 JUN 2132 | 287 | 0. | .2 | 20.8 |
| 30 JUN 1454 | 88  | 0. | .0 | 18.3 | * | 30 JUN 1814 | 188 | 0. | .2 | 20.8 | * | 30 JUN 2134 | 288 | 0. | .2 | 20.8 |
| 30 JUN 1456 | 89  | 0. | .0 | 18.3 | * | 30 JUN 1816 | 189 | 0. | .2 | 20.8 | * | 30 JUN 2136 | 289 | 0. | .2 | 20.8 |
| 30 JUN 1458 | 90  | 0. | .0 | 18.3 | * | 30 JUN 1818 | 190 | 0. | .2 | 20.8 | * | 30 JUN 2138 | 290 | 0. | .2 | 20.8 |
| 30 JUN 1500 | 91  | 0. | .0 | 18.3 | * | 30 JUN 1820 | 191 | 0. | .2 | 20.8 | * | 30 JUN 2140 | 291 | 0. | .2 | 20.8 |
| 30 JUN 1502 | 92  | 0. | .0 | 18.3 | * | 30 JUN 1822 | 192 | 0. | .2 | 20.8 | * | 30 JUN 2142 | 292 | 0. | .2 | 20.8 |
| 30 JUN 1504 | 93  | 0. | .0 | 18.3 | * | 30 JUN 1824 | 193 | 0. | .2 | 20.8 | * | 30 JUN 2144 | 293 | 0. | .2 | 20.8 |
| 30 JUN 1506 | 94  | 0. | .0 | 18.3 | * | 30 JUN 1826 | 194 | 0. | .2 | 20.8 | * | 30 JUN 2146 | 294 | 0. | .2 | 20.8 |
| 30 JUN 1508 | 95  | 0. | .0 | 18.3 | * | 30 JUN 1828 | 195 | 0. | .2 | 20.8 | * | 30 JUN 2148 | 295 | 0. | .2 | 20.8 |
| 30 JUN 1510 | 96  | 0. | .0 | 18.3 | * | 30 JUN 1830 | 196 | 0. | .2 | 20.8 | * | 30 JUN 2150 | 296 | 0. | .2 | 20.8 |
| 30 JUN 1512 | 97  | 0. | .0 | 18.3 | * | 30 JUN 1832 | 197 | 0. | .2 | 20.8 | * | 30 JUN 2152 | 297 | 0. | .2 | 20.8 |
| 30 JUN 1514 | 98  | 0. | .0 | 18.3 | * | 30 JUN 1834 | 198 | 0. | .2 | 20.8 | * | 30 JUN 2154 | 298 | 0. | .2 | 20.8 |
| 30 JUN 1516 | 99  | 0. | .0 | 18.3 | * | 30 JUN 1836 | 199 | 0. | .2 | 20.8 | * | 30 JUN 2156 | 299 | 0. | .2 | 20.8 |
| 30 JUN 1518 | 100 | 0. | .0 | 18.3 | * | 30 JUN 1838 | 200 | 0. | .2 | 20.9 | * | 30 JUN 2158 | 300 | 0. | .2 | 20.8 |

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| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 0.                 | 7.20         | 0.                   | 0.    | 0.    | 0.      |
|                    |              | (CFS)                |       |       |         |
|                    |              | (INCHES)             | .149  | .149  | .149    |
|                    |              | (AC-FT)              | 0.    | 0.    | 0.      |

| PEAK STORAGE<br>(AC-FT) | TIME<br>(HR) | MAXIMUM AVERAGE STORAGE |       |       |         |
|-------------------------|--------------|-------------------------|-------|-------|---------|
|                         |              | 6-HR                    | 24-HR | 72-HR | 9.97-HR |
| 0.                      | 7.43         | 0.                      | 0.    | 0.    | 0.      |

| PEAK STAGE<br>(FEET) | TIME<br>(HR) | MAXIMUM AVERAGE STAGE |       |       |         |
|----------------------|--------------|-----------------------|-------|-------|---------|
|                      |              | 6-HR                  | 24-HR | 72-HR | 9.97-HR |
| 20.92                | 7.60         | 20.15                 | 19.41 | 19.41 | 19.41   |

CUMULATIVE AREA = .02 SQ MI

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STATION RES4

(I) INFLOW, (O) OUTFLOW

.0 .4 .8 1.2 1.6 2.0 2.4 2.8 .0 .0 .0 .0 .0  
 (S) STORAGE  
 .0 .0 .0 .0 .0 .0 .0 .1 .2 .3 .0 .0 .0

| STATION    | RES4 | (I) INFLOW | (O) OUTFLOW | (S) STORAGE |
|------------|------|------------|-------------|-------------|
| DAHRMN PER |      |            |             |             |
| 01200 1I   | S    |            |             |             |
| 01202 2I   | S    |            |             |             |
| 01204 3I   | S    |            |             |             |
| 01206 4I   | S    |            |             |             |
| 01208 5I   | S    |            |             |             |
| 01210 6I   | S    |            |             |             |
| 01212 7I   | S    |            |             |             |
| 01214 8I   | S    |            |             |             |
| 01216 9I   | S    |            |             |             |
| 01218 10I  | S    |            |             |             |
| 01220 11I  | S    |            |             |             |
| 01222 12I  | S    |            |             |             |
| 01224 13I  | S    |            |             |             |
| 01226 14I  | S    |            |             |             |
| 01228 15I  | S    |            |             |             |
| 01230 16I  | S    |            |             |             |
| 01232 17I  | S    |            |             |             |
| 01234 18I  | S    |            |             |             |
| 01236 19I  | S    |            |             |             |
| 01238 20I  | S    |            |             |             |
| 01240 21I  | S    |            |             |             |
| 01242 22I  | S    |            |             |             |
| 01244 23I  | S    |            |             |             |
| 01246 24I  | S    |            |             |             |
| 01248 25I  | S    |            |             |             |
| 01250 26I  | S    |            |             |             |
| 01252 27I  | S    |            |             |             |
| 01254 28I  | S    |            |             |             |
| 01256 29I  | S    |            |             |             |
| 01258 30I  | S    |            |             |             |
| 01300 31I  | S    |            |             |             |
| 01302 32I  | S    |            |             |             |
| 01304 33I  | S    |            |             |             |
| 01306 34I  | S    |            |             |             |
| 01308 35I  | S    |            |             |             |
| 01310 36I  | S    |            |             |             |
| 01312 37I  | S    |            |             |             |
| 01314 38I  | S    |            |             |             |
| 01316 39I  | S    |            |             |             |
| 01318 40I  | S    |            |             |             |
| 01320 41I  | S    |            |             |             |
| 01322 42I  | S    |            |             |             |
| 01324 43I  | S    |            |             |             |
| 01326 44I  | S    |            |             |             |
| 01328 45I  | S    |            |             |             |
| 01330 46I  | S    |            |             |             |
| 01332 47I  | S    |            |             |             |
| 01334 48I  | S    |            |             |             |
| 01336 49I  | S    |            |             |             |
| 01338 50I  | S    |            |             |             |
| 01340 51I  | S    |            |             |             |
| 01342 52I  | S    |            |             |             |
| 01344 53I  | S    |            |             |             |
| 01346 54I  | S    |            |             |             |
| 01348 55I  | S    |            |             |             |
| 01350 56I  | S    |            |             |             |
| 01352 57I  | S    |            |             |             |

R5-13

|       |      |   |
|-------|------|---|
| 01356 | 59I  | S |
| 01358 | 60I  | S |
| 01400 | 61I  | S |
| 01402 | 62I  | S |
| 01404 | 63I  | S |
| 01406 | 64I  | S |
| 01408 | 65I  | S |
| 01410 | 66I  | S |
| 01412 | 67I  | S |
| 01414 | 68I  | S |
| 01416 | 69I  | S |
| 01418 | 70I  | S |
| 01420 | 71I  | S |
| 01422 | 72I  | S |
| 01424 | 73I  | S |
| 01426 | 74I  | S |
| 01428 | 75I  | S |
| 01430 | 76I  | S |
| 01432 | 77I  | S |
| 01434 | 78I  | S |
| 01436 | 79I  | S |
| 01438 | 80I  | S |
| 01440 | 81I  | S |
| 01442 | 82I  | S |
| 01444 | 83I  | S |
| 01446 | 84I  | S |
| 01448 | 85I  | S |
| 01450 | 86I  | S |
| 01452 | 87I  | S |
| 01454 | 88I  | S |
| 01456 | 89I  | S |
| 01458 | 90I  | S |
| 01500 | 91I  | S |
| 01502 | 92I  | S |
| 01504 | 93I  | S |
| 01506 | 94I  | S |
| 01508 | 95I  | S |
| 01510 | 96I  | S |
| 01512 | 97I  | S |
| 01514 | 98I  | S |
| 01516 | 99I  | S |
| 01518 | 100I | S |
| 01520 | 101I | S |
| 01522 | 102I | S |
| 01524 | 103I | S |
| 01526 | 104I | S |
| 01528 | 105I | S |
| 01530 | 106I | S |
| 01532 | 107I | S |
| 01534 | 108I | S |
| 01536 | 109I | S |
| 01538 | 110I | S |
| 01540 | 111I | S |
| 01542 | 112I | S |
| 01544 | 113I | S |
| 01546 | 114I | S |
| 01548 | 115I | S |
| 01550 | 116I | S |
| 01552 | 117I | S |
| 01554 | 118I | S |
| 01556 | 119I | S |
| 01558 | 120I | S |
| 01600 | 121I | S |
| 01602 | 122I | S |
| 01604 | 123I | S |

R5-14





|        |      |   |    |    |
|--------|------|---|----|----|
| 01820  | 191. | 0 | I  | S. |
| 301822 | 192. | 0 | I  | S. |
| 701824 | 193. | 0 | I. | S. |
| 01826  | 194. | 0 | I  | S  |
| 301828 | 195. | 0 | I  | S  |
| 301830 | 196. | 0 | I  | S  |
| 01832  | 197. | 0 | I  | S  |
| 301834 | 198. | 0 | I  | S  |
| 301836 | 199. | 0 | I  | S  |
| 01838  | 200. | 0 | I  | S  |
| 301840 | 201. | 0 | I  | S. |
| 301842 | 202. | 0 | I  | S  |
| 01844  | 203. | 0 | I  | S  |
| 301846 | 204. | 0 | I  | S  |
| 301848 | 205. | 0 | I  | S  |
| 01850  | 206. | 0 | I  | S  |
| 301852 | 207. | 0 | I  | S  |
| 301854 | 208. | 0 | I  | S  |
| 01856  | 209. | 0 | I  | S  |
| 301858 | 210. | 0 | I  | S  |
| 301900 | 211. | 0 | I. | S. |
| 01902  | 212. | 0 | I  | S  |
| 301904 | 213. | 0 | I. | S  |
| 301906 | 214. | 0 | I. | S  |
| 01908  | 215. | 0 | I  | S  |
| 301910 | 216. | 0 | I  | S  |
| 301912 | 217. | 0 | I  | S  |
| 01914  | 218. | 0 | I  | S  |
| 301916 | 219. | 0 | I  | S  |
| 301918 | 220. | 0 | I  | S  |
| 01920  | 221. | 0 | I. | S. |
| 301922 | 222. | 0 | I  | S  |
| 301924 | 223. | 0 | I  | S  |
| 01926  | 224. | 0 | I  | S  |
| 301928 | 225. | 0 | I  | S  |
| 301930 | 226. | 0 | I  | S  |
| 01932  | 227. | 0 | I  | S  |
| 301934 | 228. | 0 | I  | S  |
| 301936 | 229. | 0 | I  | S  |
| 01938  | 230. | 0 | I  | S  |
| 301940 | 231. | 0 | I  | S. |
| 301942 | 232. | 0 | I  | S  |
| 01944  | 233. | 0 | I  | S  |
| 301946 | 234. | 0 | I  | S  |
| 301948 | 235. | 0 | I  | S  |
| 01950  | 236. | 0 | I  | S  |
| 301952 | 237. | 0 | I  | S  |
| 301954 | 238. | 0 | I  | S  |
| 01956  | 239. | 0 | I  | S  |
| 301958 | 240. | 0 | I  | S  |
| 302000 | 241. | 0 | I. | S. |
| 2002   | 242. | 0 | I  | S  |
| 302004 | 243. | 0 | I  | S  |
| 302006 | 244. | 0 | I  | S  |
| 2008   | 245. | 0 | I  | S  |
| 302010 | 246. | 0 | I  | S  |
| 302012 | 247. | 0 | I  | S  |
| 2014   | 248. | 0 | I  | S  |
| 302016 | 249. | 0 | I  | S  |
| 302018 | 250. | 0 | I  | S  |
| 2020   | 251. | 0 | I  | S. |
| 302022 | 252. | 0 | I  | S  |
| 302024 | 253. | 0 | I  | S  |
| 2026   | 254. | 0 | I  | S  |
| 302028 | 255. | 0 | I  | S  |

RS-16



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 \* PIA \*  
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36 KK

38 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

39 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

L 655. CHANNEL LENGTH  
 S .0067 SLOPE  
 N .015 CHANNEL ROUGHNESS COEFFICIENT  
 CA .00 CONTRIBUTING AREA  
 SHAPE CIRC CHANNEL SHAPE  
 WD 2.00 BOTTOM WIDTH OR DIAMETER  
 Z .00 SIDE SLOPE

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COMPUTED MUSKINGUM-CUNGE PARAMETERS

| ELEMENT | ALPHA | COMPUTATION TIME STEP |             |            | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|-----------------------|-------------|------------|---------------|--------------------------|----------------|------------------------------|
|         |       | M                     | DT<br>(MIN) | DX<br>(FT) |               |                          |                |                              |
| MAIN    | 4.92  | 1.25                  | 2.00        | 218.33     | .48           | 458.00                   | .15            | 3.37                         |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |  |     |        |     |
|------|------|------|------|--|-----|--------|-----|
| MAIN | 4.92 | 1.25 | 2.00 |  | .48 | 458.00 | .15 |
|------|------|------|------|--|-----|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1779E+00 EXCESS= .0000E+00 OUTFLOW= .1756E+00 BASIN STORAGE= .2313E-02 PERCENT ERROR= .0

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HYDROGRAPH AT STATION P1A

| EAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |          |          |          |
|-------------------|--------------|----------------------|----------|----------|----------|
|                   |              | 6-HR                 | 24-HR    | 72-HR    | 9.97-HR  |
| 0.                | 7.23         | 0.                   | 0.       | 0.       | 0.       |
|                   |              | (CFS)                | (CFS)    | (CFS)    | (CFS)    |
|                   |              | .147                 | .147     | .147     | .147     |
|                   |              | (INCHES)             | (INCHES) | (INCHES) | (INCHES) |
|                   |              | .0                   | .0       | .0       | .0       |
|                   |              | (AC-FT)              | (AC-FT)  | (AC-FT)  | (AC-FT)  |

RS-18

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 \* C2 \*  
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40 KK

42 KO

OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

43 BA

SUBBASIN CHARACTERISTICS  
 TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

44 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
 ..... HYDRO-35 ..... TP-40 ..... TP-49 .....  
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
 .10 .19 .34 .42 .47 .55 .64 .70 .00 .00 .00 .00  
 STORM AREA = .00

45 LS

SCS LOSS RATE  
 STRL .08 INITIAL ABSTRACTION  
 CRVNBR 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

46 UD

SCS DIMENSIONLESS UNITGRAPH  
 TLAG 1.00 LAG

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UNIT HYDROGRAPH  
 152 END-OF-PERIOD ORDINATES

|    |    |    |    |    |    |    |    |    |    |    |
|----|----|----|----|----|----|----|----|----|----|----|
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 1. |
| 1. | 1. | 1. | 1. | 1. | 1. | 1. | 1. | 1. | 2. | 2. |
| 2. | 2. | 2. | 2. | 2. | 2. | 2. | 2. | 2. | 2. | 2. |
| 2. | 2. | 2. | 2. | 2. | 2. | 2. | 2. | 2. | 2. | 2. |
| 2. | 2. | 2. | 2. | 2. | 2. | 1. | 1. | 1. | 1. | 1. |
| 1. | 1. | 1. | 1. | 1. | 1. | 1. | 1. | 1. | 1. | 1. |
| 1. | 1. | 1. | 1. | 1. | 1. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |

R5-19

0. 0.

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HYDROGRAPH AT STATION C2

TOTAL RAINFALL = .61, TOTAL LOSS = .32, TOTAL EXCESS = .30

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 0.                 | 6.10         | (CFS) 0.             | 0.    | 0.    | 0.      |
|                    |              | (INCHES) .281        | .281  | .281  | .281    |
|                    |              | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .00 SQ MI

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 47 KK \* C2 \*  
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49 KO OUTPUT CONTROL VARIABLES

- IPRNT 3 PRINT CONTROL
- IPLOT 1 PLOT CONTROL
- QSCAL 0. HYDROGRAPH PLOT SCALE
- IPNCH 1 PUNCH COMPUTED HYDROGRAPH
- IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
- ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
- ISAV2 300 LAST ORDINATE PUNCHED OR SAVED
- TIMINT .033 TIME INTERVAL IN HOURS

50 HC HYDROGRAPH COMBINATION

- ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION C2

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 1.                 | 6.13         | (CFS) 1.             | 0.    | 0.    | 0.      |
|                    |              | (INCHES) .171        | .171  | .171  | .171    |
|                    |              | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .03 SQ MI

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 51 KK \* C13 \*

R5-20

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53 KO OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

54 BA SUBBASIN CHARACTERISTICS

TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

55 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

..... HYDRO-35 ..... TP-40 ..... TP-49 .....

| 5-MIN | 15-MIN | 60-MIN | 2-HR | 3-HR | 6-HR | 12-HR | 24-HR | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
|-------|--------|--------|------|------|------|-------|-------|-------|-------|-------|--------|
| .10   | .19    | .34    | .42  | .47  | .55  | .64   | .70   | .00   | .00   | .00   | .00    |

STORM AREA = .02

56 LS SCS LOSS RATE

STRTL .08 INITIAL ABSTRACTION  
 CRVNBR 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

57 UD SCS DIMENSIONLESS UNITGRAPH

TLAG .72 LAG

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UNIT HYDROGRAPH  
110 END-OF-PERIOD ORDINATES

|     |     |     |     |     |     |     |     |     |     |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| 0.  | 0.  | 1.  | 1.  | 2.  | 2.  | 3.  | 4.  | 4.  | 5.  |
| 6.  | 7.  | 9.  | 10. | 11. | 11. | 12. | 13. | 13. | 13. |
| 13. | 14. | 13. | 13. | 13. | 13. | 12. | 12. | 11. | 11. |
| 10. | 10. | 9.  | 9.  | 8.  | 7.  | 7.  | 6.  | 6.  | 5.  |
| 5.  | 4.  | 4.  | 4.  | 4.  | 3.  | 3.  | 3.  | 3.  | 3.  |
| 2.  | 2.  | 2.  | 2.  | 2.  | 2.  | 1.  | 1.  | 1.  | 1.  |
| 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  |
| 1.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |

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HYDROGRAPH AT STATION C13

TOTAL RAINFALL = .61, TOTAL LOSS = .32, TOTAL EXCESS = .30

| PEAK FLOW (CFS) | TIME (HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|-----------------|-----------|----------------------|-------|-------|---------|
|                 |           | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 2.              | 5.80      | (CFS) 1.             | 0.    | 0.    | 0.      |
|                 |           | (INCHES) .286        | .286  | .286  | .286    |
|                 |           | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .02 SQ MI

R5-21

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58 KK \* C12 \*

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60 KO OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL
IPLOT 1 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 1 PUNCH COMPUTED HYDROGRAPH
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

61 HC HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION C12

Table with 7 columns: PEAK FLOW (CFS), TIME (HR), MAXIMUM AVERAGE FLOW (6-HR, 24-HR, 72-HR, 9.97-HR). Values include 3, 5.83, 1, .221, 1.

CUMULATIVE AREA = .05 SQ MI

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62 KK \* P2A \*

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64 KO OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL
IPLOT 1 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 1 PUNCH COMPUTED HYDROGRAPH
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED
TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

MUSKINGUM-CUNGE CHANNEL ROUTING

R5-22



S .0054 SLOPE  
 N .015 CHANNEL ROUGHNESS COEFFICIENT  
 CA .00 CONTRIBUTING AREA  
 SHAPE CIRC CHANNEL SHAPE  
 WD 3.00 BOTTOM WIDTH OR DIAMETER  
 Z .00 SIDE SLOPE

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 COMPUTED MUSKINGUM-CUNGE PARAMETERS

| ELEMENT | ALPHA | COMPUTATION TIME STEP |             |            | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|-----------------------|-------------|------------|---------------|--------------------------|----------------|------------------------------|
|         |       | M                     | DT<br>(MIN) | DX<br>(FT) |               |                          |                |                              |
| MAIN    | 4.73  | 1.25                  | .97         | 138.50     | 3.15          | 350.03                   | .22            | 4.75                         |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |  |      |        |     |  |
|------|------|------|------|--|------|--------|-----|--|
| MAIN | 4.73 | 1.25 | 2.00 |  | 3.15 | 350.00 | .22 |  |
|------|------|------|------|--|------|--------|-----|--|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5635E+00 EXCESS= .0000E+00 OUTFLOW= .5621E+00 BASIN STORAGE= .1366E-02 PERCENT ERROR= .0

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HYDROGRAPH AT STATION P2A

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |          |          |          |
|--------------------|--------------|----------------------|----------|----------|----------|
|                    |              | 6-HR                 | 24-HR    | 72-HR    | 9.97-HR  |
| 3.                 | 5.83         | 1.                   | 1.       | 1.       | 1.       |
|                    |              | (CFS)                | (CFS)    | (CFS)    | (CFS)    |
|                    |              | (INCHES)             | (INCHES) | (INCHES) | (INCHES) |
|                    |              | (AC-FT)              | (AC-FT)  | (AC-FT)  | (AC-FT)  |

CUMULATIVE AREA = .05 SQ MI

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66 KK \* E1 \*  
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68 KO OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 3    | PRINT CONTROL                   |
| IPLOT  | 1    | PLOT CONTROL                    |
| QSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

SUBBASIN RUNOFF DATA

69 RA SUBBASIN CHARACTERISTICS

R5-23

PRECIPITATION DATA

70 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

| ..... HYDRO-35 ..... |        |        | ..... TP-40 ..... |      |      |       | ..... TP-49 ..... |       |       |       |        |
|----------------------|--------|--------|-------------------|------|------|-------|-------------------|-------|-------|-------|--------|
| 5-MIN                | 15-MIN | 60-MIN | 2-HR              | 3-HR | 6-HR | 12-HR | 24-HR             | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
| .10                  | .19    | .34    | .42               | .47  | .55  | .64   | .70               | .00   | .00   | .00   | .00    |

STORM AREA = .00

71 LS

SCS LOSS RATE

STRTL .08 INITIAL ABSTRACTION  
 CRVNB 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

72 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .15 LAG

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UNIT HYDROGRAPH  
24 END-OF-PERIOD ORDINATES

|    |    |    |    |    |    |    |    |    |    |
|----|----|----|----|----|----|----|----|----|----|
| 0. | 1. | 2. | 3. | 3. | 2. | 2. | 1. | 1. | 1. |
| 1. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. |    |    |    |    |    |    |

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HYDROGRAPH AT STATION E1

TOTAL RAINFALL = .61, TOTAL LOSS = .32, TOTAL EXCESS = .30

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 0.                 | 5.13         | (CFS) 0.             | 0.    | 0.    | 0.      |
|                    |              | (INCHES) .295        | .296  | .296  | .296    |
|                    |              | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .00 SQ MI

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73 KK

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 \* E1 \*  
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75 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

76 HC

HYDROGRAPH COMBINATION

TCOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

RS-24

\*\*\*

\*\*\*                    \*\*\*                    \*\*\*                    \*\*\*                    \*\*\*

HYDROGRAPH AT STATION        E1

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |      |
|--------------------|--------------|----------------------|-------|-------|---------|------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |      |
| 3.                 | 5.83         | (CFS) 1.             | 1.    | 1.    | 1.      | 1.   |
|                    |              | (INCHES) .222        | .222  | .222  | .222    | .222 |
|                    |              | (AC-FT) 1.           | 1.    | 1.    | 1.      | 1.   |

CUMULATIVE AREA = .05 SQ MI

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*           *
77 KK *   RES5 *
*           *
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79 KO        OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 1    | PRINT CONTROL                   |
| IPLOT  | 2    | PLOT CONTROL                    |
| QSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

HYDROGRAPH ROUTING DATA

80 RS        STORAGE ROUTING

|        |       |                             |
|--------|-------|-----------------------------|
| NSTPS  | 1     | NUMBER OF SUBREACHES        |
| ITYP   | ELEV  | TYPE OF INITIAL CONDITION   |
| RSVRIC | 11.24 | INITIAL CONDITION           |
| X      | .00   | WORKING R AND D COEFFICIENT |

81 SV        STORAGE        .0        .1        .2        .3        .5        .8

82 SE        ELEVATION     12.00    13.00    14.00    15.00    16.00    17.00

83 SL        LOW-LEVEL OUTLET

|       |       |                               |
|-------|-------|-------------------------------|
| ELEVL | 12.24 | ELEVATION AT CENTER OF OUTLET |
| CAREA | .06   | CROSS-SECTIONAL AREA          |
| COQL  | .60   | COEFFICIENT                   |
| EXPL  | .50   | EXPONENT OF HEAD              |

84 SS        SPILLWAY

|       |       |                          |
|-------|-------|--------------------------|
| CREL  | 16.00 | SPILLWAY CREST ELEVATION |
| SPWID | 12.29 | SPILLWAY WIDTH           |
| COQW  | 2.70  | WEIR COEFFICIENT         |
| EXPW  | 1.50  | EXPONENT OF HEAD         |

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COMPUTED OUTFLOW-ELEVATION DATA

R5-25

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| ELEVATION | 12.00 | 12.24 | 12.39 | 12.43 | 12.49 | 12.58 | 12.74 | 13.05 | 13.75 | 16.00 |
| OUTFLOW   | .62   | .86   | 1.50  | 2.73  | 4.77  | 7.80  | 12.02 | 17.64 | 24.85 | 33.84 |
| ELEVATION | 16.01 | 16.04 | 16.09 | 16.16 | 16.25 | 16.36 | 16.49 | 16.64 | 16.81 | 17.00 |

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| STORAGE   | .02   | .03   | .04   | .04   | .04   | .05   | .06   | .07   | .07   | .14   |
| OUTFLOW   | .00   | .00   | .12   | .13   | .15   | .18   | .21   | .26   | .27   | .37   |
| ELEVATION | 12.00 | 12.24 | 12.39 | 12.43 | 12.49 | 12.58 | 12.74 | 13.00 | 13.05 | 13.75 |
| STORAGE   | .16   | .31   | .52   | .52   | .53   | .55   | .56   | .59   | .62   | .65   |
| OUTFLOW   | .40   | .50   | .58   | .62   | .86   | 1.50  | 2.73  | 4.77  | 7.80  | 12.02 |
| ELEVATION | 14.00 | 15.00 | 16.00 | 16.01 | 16.04 | 16.09 | 16.16 | 16.25 | 16.36 | 16.49 |
| STORAGE   | .69   | .74   | .79   |       |       |       |       |       |       |       |
| OUTFLOW   | 17.64 | 24.85 | 33.84 |       |       |       |       |       |       |       |
| ELEVATION | 16.64 | 16.81 | 17.00 |       |       |       |       |       |       |       |

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HYDROGRAPH AT STATION RES5

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| * * * |     |      |     |         |         |       |    |     |      |     |         |         |       |    |     |      |     |         |         |       |
|-------|-----|------|-----|---------|---------|-------|----|-----|------|-----|---------|---------|-------|----|-----|------|-----|---------|---------|-------|
| DA    | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE |
| 30    | JUN | 1200 | 1   | 0.      | .0      | 12.0  | 30 | JUN | 1520 | 101 | 0.      | .0      | 12.0  | 30 | JUN | 1840 | 201 | 0.      | .3      | 14.8  |
| 30    | JUN | 1202 | 2   | 0.      | .0      | 12.0  | 30 | JUN | 1522 | 102 | 0.      | .0      | 12.0  | 30 | JUN | 1842 | 202 | 0.      | .3      | 14.8  |
| 30    | JUN | 1204 | 3   | 0.      | .0      | 12.0  | 30 | JUN | 1524 | 103 | 0.      | .0      | 12.0  | 30 | JUN | 1844 | 203 | 0.      | .3      | 14.8  |
| 30    | JUN | 1206 | 4   | 0.      | .0      | 12.0  | 30 | JUN | 1526 | 104 | 0.      | .0      | 12.0  | 30 | JUN | 1846 | 204 | 0.      | .3      | 14.9  |
| 30    | JUN | 1208 | 5   | 0.      | .0      | 12.0  | 30 | JUN | 1528 | 105 | 0.      | .0      | 12.0  | 30 | JUN | 1848 | 205 | 0.      | .3      | 14.9  |
| 30    | JUN | 1210 | 6   | 0.      | .0      | 12.0  | 30 | JUN | 1530 | 106 | 0.      | .0      | 12.0  | 30 | JUN | 1850 | 206 | 0.      | .3      | 14.9  |
| 30    | JUN | 1212 | 7   | 0.      | .0      | 12.0  | 30 | JUN | 1532 | 107 | 0.      | .0      | 12.0  | 30 | JUN | 1852 | 207 | 0.      | .3      | 14.9  |
| 30    | JUN | 1214 | 8   | 0.      | .0      | 12.0  | 30 | JUN | 1534 | 108 | 0.      | .0      | 12.0  | 30 | JUN | 1854 | 208 | 0.      | .3      | 15.0  |
| 30    | JUN | 1216 | 9   | 0.      | .0      | 12.0  | 30 | JUN | 1536 | 109 | 0.      | .0      | 12.0  | 30 | JUN | 1856 | 209 | 0.      | .3      | 15.0  |
| 30    | JUN | 1218 | 10  | 0.      | .0      | 12.0  | 30 | JUN | 1538 | 110 | 0.      | .0      | 12.0  | 30 | JUN | 1858 | 210 | 0.      | .3      | 15.0  |
| 30    | JUN | 1220 | 11  | 0.      | .0      | 12.0  | 30 | JUN | 1540 | 111 | 0.      | .0      | 12.0  | 30 | JUN | 1900 | 211 | 1.      | .3      | 15.0  |
| 30    | JUN | 1222 | 12  | 0.      | .0      | 12.0  | 30 | JUN | 1542 | 112 | 0.      | .0      | 12.0  | 30 | JUN | 1902 | 212 | 1.      | .3      | 15.0  |
| 30    | JUN | 1224 | 13  | 0.      | .0      | 12.0  | 30 | JUN | 1544 | 113 | 0.      | .0      | 12.0  | 30 | JUN | 1904 | 213 | 1.      | .3      | 15.0  |
| 30    | JUN | 1226 | 14  | 0.      | .0      | 12.0  | 30 | JUN | 1546 | 114 | 0.      | .0      | 12.0  | 30 | JUN | 1906 | 214 | 1.      | .3      | 15.0  |
| 30    | JUN | 1228 | 15  | 0.      | .0      | 12.0  | 30 | JUN | 1548 | 115 | 0.      | .0      | 12.0  | 30 | JUN | 1908 | 215 | 1.      | .3      | 15.1  |
| 30    | JUN | 1230 | 16  | 0.      | .0      | 12.0  | 30 | JUN | 1550 | 116 | 0.      | .0      | 12.0  | 30 | JUN | 1910 | 216 | 1.      | .3      | 15.1  |
| 30    | JUN | 1232 | 17  | 0.      | .0      | 12.0  | 30 | JUN | 1552 | 117 | 0.      | .0      | 12.0  | 30 | JUN | 1912 | 217 | 1.      | .3      | 15.1  |
| 30    | JUN | 1234 | 18  | 0.      | .0      | 12.0  | 30 | JUN | 1554 | 118 | 0.      | .0      | 12.0  | 30 | JUN | 1914 | 218 | 1.      | .3      | 15.1  |
| 30    | JUN | 1236 | 19  | 0.      | .0      | 12.0  | 30 | JUN | 1556 | 119 | 0.      | .0      | 12.0  | 30 | JUN | 1916 | 219 | 1.      | .3      | 15.1  |
| 30    | JUN | 1238 | 20  | 0.      | .0      | 12.0  | 30 | JUN | 1558 | 120 | 0.      | .0      | 12.0  | 30 | JUN | 1918 | 220 | 1.      | .3      | 15.1  |
| 30    | JUN | 1240 | 21  | 0.      | .0      | 12.0  | 30 | JUN | 1600 | 121 | 0.      | .0      | 12.0  | 30 | JUN | 1920 | 221 | 1.      | .3      | 15.1  |
| 30    | JUN | 1242 | 22  | 0.      | .0      | 12.0  | 30 | JUN | 1602 | 122 | 0.      | .0      | 12.0  | 30 | JUN | 1922 | 222 | 1.      | .3      | 15.1  |
| 30    | JUN | 1244 | 23  | 0.      | .0      | 12.0  | 30 | JUN | 1604 | 123 | 0.      | .0      | 12.0  | 30 | JUN | 1924 | 223 | 1.      | .3      | 15.1  |
| 30    | JUN | 1246 | 24  | 0.      | .0      | 12.0  | 30 | JUN | 1606 | 124 | 0.      | .0      | 12.0  | 30 | JUN | 1926 | 224 | 1.      | .3      | 15.1  |
| 30    | JUN | 1248 | 25  | 0.      | .0      | 12.0  | 30 | JUN | 1608 | 125 | 0.      | .0      | 12.0  | 30 | JUN | 1928 | 225 | 1.      | .3      | 15.2  |
| 30    | JUN | 1250 | 26  | 0.      | .0      | 12.0  | 30 | JUN | 1610 | 126 | 0.      | .0      | 12.0  | 30 | JUN | 1930 | 226 | 1.      | .3      | 15.2  |
| 30    | JUN | 1252 | 27  | 0.      | .0      | 12.0  | 30 | JUN | 1612 | 127 | 0.      | .0      | 12.0  | 30 | JUN | 1932 | 227 | 1.      | .3      | 15.2  |
| 30    | JUN | 1254 | 28  | 0.      | .0      | 12.0  | 30 | JUN | 1614 | 128 | 0.      | .0      | 12.0  | 30 | JUN | 1934 | 228 | 1.      | .4      | 15.2  |
| 30    | JUN | 1256 | 29  | 0.      | .0      | 12.0  | 30 | JUN | 1616 | 129 | 0.      | .0      | 12.0  | 30 | JUN | 1936 | 229 | 1.      | .4      | 15.2  |
| 30    | JUN | 1258 | 30  | 0.      | .0      | 12.0  | 30 | JUN | 1618 | 130 | 0.      | .0      | 12.0  | 30 | JUN | 1938 | 230 | 1.      | .4      | 15.2  |
| 30    | JUN | 1300 | 31  | 0.      | .0      | 12.0  | 30 | JUN | 1620 | 131 | 0.      | .0      | 12.0  | 30 | JUN | 1940 | 231 | 1.      | .4      | 15.2  |
| 30    | JUN | 1302 | 32  | 0.      | .0      | 12.0  | 30 | JUN | 1622 | 132 | 0.      | .0      | 12.0  | 30 | JUN | 1942 | 232 | 1.      | .4      | 15.2  |
| 30    | JUN | 1304 | 33  | 0.      | .0      | 12.0  | 30 | JUN | 1624 | 133 | 0.      | .0      | 12.0  | 30 | JUN | 1944 | 233 | 1.      | .4      | 15.2  |
| 30    | JUN | 1306 | 34  | 0.      | .0      | 12.0  | 30 | JUN | 1626 | 134 | 0.      | .0      | 12.0  | 30 | JUN | 1946 | 234 | 1.      | .4      | 15.2  |
| 30    | JUN | 1308 | 35  | 0.      | .0      | 12.0  | 30 | JUN | 1628 | 135 | 0.      | .0      | 12.0  | 30 | JUN | 1948 | 235 | 1.      | .4      | 15.2  |
| 30    | JUN | 1310 | 36  | 0.      | .0      | 12.0  | 30 | JUN | 1630 | 136 | 0.      | .0      | 12.0  | 30 | JUN | 1950 | 236 | 1.      | .4      | 15.2  |
| 30    | JUN | 1312 | 37  | 0.      | .0      | 12.0  | 30 | JUN | 1632 | 137 | 0.      | .0      | 12.0  | 30 | JUN | 1952 | 237 | 1       | .4      | 15.2  |

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|             |     |    |    |      |               |     |    |    |      |               |     |    |    |      |
|-------------|-----|----|----|------|---------------|-----|----|----|------|---------------|-----|----|----|------|
| 30 JUN 1316 | 39  | 0. | .0 | 12.0 | * 30 JUN 1636 | 139 | 0. | .0 | 12.0 | * 30 JUN 1956 | 239 | 1. | .4 | 15.3 |
| 30 JUN 1318 | 40  | 0. | .0 | 12.0 | * 30 JUN 1638 | 140 | 0. | .0 | 12.0 | * 30 JUN 1958 | 240 | 1. | .4 | 15.3 |
| 30 JUN 1320 | 41  | 0. | .0 | 12.0 | * 30 JUN 1640 | 141 | 0. | .0 | 12.0 | * 30 JUN 2000 | 241 | 1. | .4 | 15.3 |
| 30 JUN 1322 | 42  | 0. | .0 | 12.0 | * 30 JUN 1642 | 142 | 0. | .0 | 12.0 | * 30 JUN 2002 | 242 | 1. | .4 | 15.3 |
| 30 JUN 1324 | 43  | 0. | .0 | 12.0 | * 30 JUN 1644 | 143 | 0. | .0 | 12.0 | * 30 JUN 2004 | 243 | 1. | .4 | 15.3 |
| 30 JUN 1326 | 44  | 0. | .0 | 12.0 | * 30 JUN 1646 | 144 | 0. | .0 | 12.0 | * 30 JUN 2006 | 244 | 1. | .4 | 15.3 |
| 30 JUN 1328 | 45  | 0. | .0 | 12.0 | * 30 JUN 1648 | 145 | 0. | .0 | 12.0 | * 30 JUN 2008 | 245 | 1. | .4 | 15.3 |
| 30 JUN 1330 | 46  | 0. | .0 | 12.0 | * 30 JUN 1650 | 146 | 0. | .0 | 12.1 | * 30 JUN 2010 | 246 | 1. | .4 | 15.3 |
| 30 JUN 1332 | 47  | 0. | .0 | 12.0 | * 30 JUN 1652 | 147 | 0. | .0 | 12.1 | * 30 JUN 2012 | 247 | 1. | .4 | 15.3 |
| 30 JUN 1334 | 48  | 0. | .0 | 12.0 | * 30 JUN 1654 | 148 | 0. | .0 | 12.1 | * 30 JUN 2014 | 248 | 1. | .4 | 15.3 |
| 30 JUN 1336 | 49  | 0. | .0 | 12.0 | * 30 JUN 1656 | 149 | 0. | .0 | 12.1 | * 30 JUN 2016 | 249 | 1. | .4 | 15.3 |
| 30 JUN 1338 | 50  | 0. | .0 | 12.0 | * 30 JUN 1658 | 150 | 0. | .0 | 12.1 | * 30 JUN 2018 | 250 | 1. | .4 | 15.3 |
| 30 JUN 1340 | 51  | 0. | .0 | 12.0 | * 30 JUN 1700 | 151 | 0. | .0 | 12.1 | * 30 JUN 2020 | 251 | 1. | .4 | 15.3 |
| 30 JUN 1342 | 52  | 0. | .0 | 12.0 | * 30 JUN 1702 | 152 | 0. | .0 | 12.1 | * 30 JUN 2022 | 252 | 1. | .4 | 15.3 |
| 30 JUN 1344 | 53  | 0. | .0 | 12.0 | * 30 JUN 1704 | 153 | 0. | .0 | 12.2 | * 30 JUN 2024 | 253 | 1. | .4 | 15.3 |
| 30 JUN 1346 | 54  | 0. | .0 | 12.0 | * 30 JUN 1706 | 154 | 0. | .0 | 12.2 | * 30 JUN 2026 | 254 | 1. | .4 | 15.3 |
| 30 JUN 1348 | 55  | 0. | .0 | 12.0 | * 30 JUN 1708 | 155 | 0. | .0 | 12.2 | * 30 JUN 2028 | 255 | 1. | .4 | 15.3 |
| 30 JUN 1350 | 56  | 0. | .0 | 12.0 | * 30 JUN 1710 | 156 | 0. | .0 | 12.3 | * 30 JUN 2030 | 256 | 1. | .4 | 15.3 |
| 30 JUN 1352 | 57  | 0. | .0 | 12.0 | * 30 JUN 1712 | 157 | 0. | .0 | 12.3 | * 30 JUN 2032 | 257 | 1. | .4 | 15.3 |
| 30 JUN 1354 | 58  | 0. | .0 | 12.0 | * 30 JUN 1714 | 158 | 0. | .0 | 12.4 | * 30 JUN 2034 | 258 | 1. | .4 | 15.3 |
| 30 JUN 1356 | 59  | 0. | .0 | 12.0 | * 30 JUN 1716 | 159 | 0. | .0 | 12.4 | * 30 JUN 2036 | 259 | 1. | .4 | 15.3 |
| 30 JUN 1358 | 60  | 0. | .0 | 12.0 | * 30 JUN 1718 | 160 | 0. | .0 | 12.5 | * 30 JUN 2038 | 260 | 1. | .4 | 15.4 |
| 30 JUN 1400 | 61  | 0. | .0 | 12.0 | * 30 JUN 1720 | 161 | 0. | .0 | 12.5 | * 30 JUN 2040 | 261 | 1. | .4 | 15.4 |
| 30 JUN 1402 | 62  | 0. | .0 | 12.0 | * 30 JUN 1722 | 162 | 0. | .0 | 12.6 | * 30 JUN 2042 | 262 | 1. | .4 | 15.4 |
| 30 JUN 1404 | 63  | 0. | .0 | 12.0 | * 30 JUN 1724 | 163 | 0. | .1 | 12.7 | * 30 JUN 2044 | 263 | 1. | .4 | 15.4 |
| 30 JUN 1406 | 64  | 0. | .0 | 12.0 | * 30 JUN 1726 | 164 | 0. | .1 | 12.8 | * 30 JUN 2046 | 264 | 1. | .4 | 15.4 |
| 30 JUN 1408 | 65  | 0. | .0 | 12.0 | * 30 JUN 1728 | 165 | 0. | .1 | 12.9 | * 30 JUN 2048 | 265 | 1. | .4 | 15.4 |
| 30 JUN 1410 | 66  | 0. | .0 | 12.0 | * 30 JUN 1730 | 166 | 0. | .1 | 13.0 | * 30 JUN 2050 | 266 | 1. | .4 | 15.4 |
| 30 JUN 1412 | 67  | 0. | .0 | 12.0 | * 30 JUN 1732 | 167 | 0. | .1 | 13.0 | * 30 JUN 2052 | 267 | 1. | .4 | 15.4 |
| 30 JUN 1414 | 68  | 0. | .0 | 12.0 | * 30 JUN 1734 | 168 | 0. | .1 | 13.1 | * 30 JUN 2054 | 268 | 1. | .4 | 15.4 |
| 30 JUN 1416 | 69  | 0. | .0 | 12.0 | * 30 JUN 1736 | 169 | 0. | .1 | 13.2 | * 30 JUN 2056 | 269 | 1. | .4 | 15.4 |
| 30 JUN 1418 | 70  | 0. | .0 | 12.0 | * 30 JUN 1738 | 170 | 0. | .1 | 13.2 | * 30 JUN 2058 | 270 | 1. | .4 | 15.4 |
| 30 JUN 1420 | 71  | 0. | .0 | 12.0 | * 30 JUN 1740 | 171 | 0. | .1 | 13.3 | * 30 JUN 2100 | 271 | 1. | .4 | 15.4 |
| 30 JUN 1422 | 72  | 0. | .0 | 12.0 | * 30 JUN 1742 | 172 | 0. | .1 | 13.4 | * 30 JUN 2102 | 272 | 1. | .4 | 15.4 |
| 30 JUN 1424 | 73  | 0. | .0 | 12.0 | * 30 JUN 1744 | 173 | 0. | .1 | 13.5 | * 30 JUN 2104 | 273 | 1. | .4 | 15.4 |
| 30 JUN 1426 | 74  | 0. | .0 | 12.0 | * 30 JUN 1746 | 174 | 0. | .1 | 13.5 | * 30 JUN 2106 | 274 | 1. | .4 | 15.4 |
| 30 JUN 1428 | 75  | 0. | .0 | 12.0 | * 30 JUN 1748 | 175 | 0. | .1 | 13.6 | * 30 JUN 2108 | 275 | 1. | .4 | 15.4 |
| 30 JUN 1430 | 76  | 0. | .0 | 12.0 | * 30 JUN 1750 | 176 | 0. | .1 | 13.7 | * 30 JUN 2110 | 276 | 1. | .4 | 15.4 |
| 30 JUN 1432 | 77  | 0. | .0 | 12.0 | * 30 JUN 1752 | 177 | 0. | .1 | 13.8 | * 30 JUN 2112 | 277 | 1. | .4 | 15.4 |
| 30 JUN 1434 | 78  | 0. | .0 | 12.0 | * 30 JUN 1754 | 178 | 0. | .2 | 13.9 | * 30 JUN 2114 | 278 | 1. | .4 | 15.4 |
| 30 JUN 1436 | 79  | 0. | .0 | 12.0 | * 30 JUN 1756 | 179 | 0. | .2 | 13.9 | * 30 JUN 2116 | 279 | 1. | .4 | 15.4 |
| 30 JUN 1438 | 80  | 0. | .0 | 12.0 | * 30 JUN 1758 | 180 | 0. | .2 | 14.0 | * 30 JUN 2118 | 280 | 1. | .4 | 15.4 |
| 30 JUN 1440 | 81  | 0. | .0 | 12.0 | * 30 JUN 1800 | 181 | 0. | .2 | 14.1 | * 30 JUN 2120 | 281 | 1. | .4 | 15.4 |
| 30 JUN 1442 | 82  | 0. | .0 | 12.0 | * 30 JUN 1802 | 182 | 0. | .2 | 14.1 | * 30 JUN 2122 | 282 | 1. | .4 | 15.4 |
| 30 JUN 1444 | 83  | 0. | .0 | 12.0 | * 30 JUN 1804 | 183 | 0. | .2 | 14.2 | * 30 JUN 2124 | 283 | 1. | .4 | 15.4 |
| 30 JUN 1446 | 84  | 0. | .0 | 12.0 | * 30 JUN 1806 | 184 | 0. | .2 | 14.2 | * 30 JUN 2126 | 284 | 1. | .4 | 15.4 |
| 30 JUN 1448 | 85  | 0. | .0 | 12.0 | * 30 JUN 1808 | 185 | 0. | .2 | 14.3 | * 30 JUN 2128 | 285 | 1. | .4 | 15.4 |
| 30 JUN 1450 | 86  | 0. | .0 | 12.0 | * 30 JUN 1810 | 186 | 0. | .2 | 14.3 | * 30 JUN 2130 | 286 | 1. | .4 | 15.4 |
| 30 JUN 1452 | 87  | 0. | .0 | 12.0 | * 30 JUN 1812 | 187 | 0. | .2 | 14.3 | * 30 JUN 2132 | 287 | 1. | .4 | 15.4 |
| 30 JUN 1454 | 88  | 0. | .0 | 12.0 | * 30 JUN 1814 | 188 | 0. | .2 | 14.4 | * 30 JUN 2134 | 288 | 1. | .4 | 15.4 |
| 30 JUN 1456 | 89  | 0. | .0 | 12.0 | * 30 JUN 1816 | 189 | 0. | .2 | 14.4 | * 30 JUN 2136 | 289 | 1. | .4 | 15.4 |
| 30 JUN 1458 | 90  | 0. | .0 | 12.0 | * 30 JUN 1818 | 190 | 0. | .2 | 14.5 | * 30 JUN 2138 | 290 | 1. | .4 | 15.4 |
| 30 JUN 1500 | 91  | 0. | .0 | 12.0 | * 30 JUN 1820 | 191 | 0. | .2 | 14.5 | * 30 JUN 2140 | 291 | 1. | .4 | 15.4 |
| 30 JUN 1502 | 92  | 0. | .0 | 12.0 | * 30 JUN 1822 | 192 | 0. | .2 | 14.5 | * 30 JUN 2142 | 292 | 1. | .4 | 15.4 |
| 30 JUN 1504 | 93  | 0. | .0 | 12.0 | * 30 JUN 1824 | 193 | 0. | .2 | 14.6 | * 30 JUN 2144 | 293 | 1. | .4 | 15.4 |
| 30 JUN 1506 | 94  | 0. | .0 | 12.0 | * 30 JUN 1826 | 194 | 0. | .3 | 14.6 | * 30 JUN 2146 | 294 | 1. | .4 | 15.5 |
| 30 JUN 1508 | 95  | 0. | .0 | 12.0 | * 30 JUN 1828 | 195 | 0. | .3 | 14.6 | * 30 JUN 2148 | 295 | 1. | .4 | 15.5 |
| 30 JUN 1510 | 96  | 0. | .0 | 12.0 | * 30 JUN 1830 | 196 | 0. | .3 | 14.7 | * 30 JUN 2150 | 296 | 1. | .4 | 15.5 |
| 30 JUN 1512 | 97  | 0. | .0 | 12.0 | * 30 JUN 1832 | 197 | 0. | .3 | 14.7 | * 30 JUN 2152 | 297 | 1. | .4 | 15.5 |
| 30 JUN 1514 | 98  | 0. | .0 | 12.0 | * 30 JUN 1834 | 198 | 0. | .3 | 14.7 | * 30 JUN 2154 | 298 | 1. | .4 | 15.5 |
| 30 JUN 1516 | 99  | 0. | .0 | 12.0 | * 30 JUN 1836 | 199 | 0. | .3 | 14.7 | * 30 JUN 2156 | 299 | 1. | .4 | 15.5 |
| 30 JUN 1518 | 100 | 0. | .0 | 12.0 | * 30 JUN 1838 | 200 | 0. | .3 | 14.8 | * 30 JUN 2158 | 300 | 1. | .4 | 15.5 |

\*

\*

|          |      |      |       |       |         |
|----------|------|------|-------|-------|---------|
| (CFS)    | (HR) | 6-HR | 24-HR | 72-HR | 9.97-HR |
| 1.       | 9.80 | 0.   | 0.    | 0.    | 0.      |
| (INCHES) |      | .071 | .071  | .071  | .071    |
| (AC-FT)  |      | 0.   | 0.    | 0.    | 0.      |

|              |      |                         |       |       |         |
|--------------|------|-------------------------|-------|-------|---------|
| PEAK STORAGE | TIME | MAXIMUM AVERAGE STORAGE |       |       |         |
| (AC-FT)      | (HR) | 6-HR                    | 24-HR | 72-HR | 9.97-HR |
| 0.           | 9.93 | 0.                      | 0.    | 0.    | 0.      |

|            |      |                       |       |       |         |
|------------|------|-----------------------|-------|-------|---------|
| PEAK STAGE | TIME | MAXIMUM AVERAGE STAGE |       |       |         |
| (FEET)     | (HR) | 6-HR                  | 24-HR | 72-HR | 9.97-HR |
| 15.46      | 9.97 | 14.24                 | 13.35 | 13.35 | 13.35   |

CUMULATIVE AREA = .05 SQ MI

STATION RES5

(I) INFLOW, (O) OUTFLOW

|  | .0 | .4 | .8 | 1.2 | 1.6 | 2.0 | 2.4 | 2.8 | 3.2 | .0 | .0 | .0 | .0 |
|--|----|----|----|-----|-----|-----|-----|-----|-----|----|----|----|----|
|  | .0 | .0 | .0 | .0  | .0  | .0  | .0  | .1  | .2  | .3 | .4 | .5 | .0 |

(S) STORAGE

| STATION | PER | (I) INFLOW | (O) OUTFLOW | (S) STORAGE |
|---------|-----|------------|-------------|-------------|
| 301200  | 1I  |            |             |             |
| 01202   | 2I  |            |             |             |
| 301204  | 3I  |            |             |             |
| 301206  | 4I  |            |             |             |
| 01208   | 5I  |            |             |             |
| 301210  | 6I  |            |             |             |
| 301212  | 7I  |            |             |             |
| 01214   | 8I  |            |             |             |
| 301216  | 9I  |            |             |             |
| 301218  | 10I |            |             |             |
| 01220   | 11I |            |             |             |
| 301222  | 12I |            |             |             |
| 301224  | 13I |            |             |             |
| 01226   | 14I |            |             |             |
| 301228  | 15I |            |             |             |
| 301230  | 16I |            |             |             |
| 01232   | 17I |            |             |             |
| 301234  | 18I |            |             |             |
| 301236  | 19I |            |             |             |
| 01238   | 20I |            |             |             |
| 301240  | 21I |            |             |             |
| 301242  | 22I |            |             |             |
| 01244   | 23I |            |             |             |
| 301246  | 24I |            |             |             |
| 301248  | 25I |            |             |             |
| 01250   | 26I |            |             |             |
| 301252  | 27I |            |             |             |
| 301254  | 28I |            |             |             |
| 01256   | 29I |            |             |             |
| 301258  | 30I |            |             |             |
| 301300  | 31I |            |             |             |
| 01302   | 32I |            |             |             |
| 301304  | 33I |            |             |             |
| 301306  | 34I |            |             |             |
| 01308   | 35I |            |             |             |
| 301310  | 36I |            |             |             |
| 301312  | 37I |            |             |             |
| 01314   | 38I |            |             |             |
| 301316  | 39I |            |             |             |
| 301318  | 40I |            |             |             |
| 01320   | 41I |            |             |             |
| 301322  | 42I |            |             |             |
| 301324  | 43I |            |             |             |
| 1326    | 44I |            |             |             |
| 301328  | 45I |            |             |             |
| 301330  | 46I |            |             |             |
| 1332    | 47I |            |             |             |
| 301334  | 48I |            |             |             |
| 301336  | 49I |            |             |             |
| 1338    | 50I |            |             |             |
| 301340  | 51I |            |             |             |
| 301342  | 52I |            |             |             |
| 1344    | 53I |            |             |             |
| 301346  | 54I |            |             |             |
| 301348  | 55I |            |             |             |
| 1350    | 56I |            |             |             |
| 301352  | 57I |            |             |             |

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|        |           |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |   |
|--------|-----------|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 301608 | 125I      | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301610 | 126I      | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301612 | 127I      | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301614 | 128I      | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301616 | 129I      | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301618 | 130I      | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301620 | 131I      | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301622 | 132I      | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301624 | 1330I     | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301626 | 1340I     | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301628 | 1350I     | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301630 | 1360I     | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301632 | 1370I     | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301634 | 1380I     | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301636 | 1390I     | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301638 | 1400 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301640 | 1410 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301642 | 1420 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301644 | 1430 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301646 | 1440 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301648 | 1450 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301650 | 1460 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301652 | 1470 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301654 | 1480 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301656 | 1490 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301658 | 1500 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301700 | 1510 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301702 | 1520 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301704 | 1530 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301706 | 1540 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301708 | 1550 I    | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301710 | 156.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301712 | 157.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301714 | 158.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301716 | 159.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301718 | 160.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301720 | 161.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301722 | 162.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301724 | 163.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301726 | 164.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301728 | 165.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301730 | 166.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301732 | 167.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301734 | 168.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301736 | 169.0 I   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301738 | 170.0 I S | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301740 | 171.0 IS  | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301742 | 172.0 S   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301744 | 173.0 S I | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301746 | 174.0 S I | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301748 | 175.0 S I | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301750 | 176.0 S I | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301752 | 177.0 S I | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301754 | 178.0 S I | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301756 | 179.0 S I | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301758 | 180.0 S   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301800 | 181.0 S   | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301802 | 182.0 I S | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301804 | 183.0 I S | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301806 | 184.0 I S | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301808 | 185.0 I S | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301810 | 186.0 I S | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301812 | 187.0 I S | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301814 | 188.0 I S | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |
| 301816 | 189.0 I S | . | . | . | . | . | . | . | . | S | . | . | . | . | . | . | . | . | . | . |

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|             |     |     |   |   |   |   |   |   |   |     |   |   |
|-------------|-----|-----|---|---|---|---|---|---|---|-----|---|---|
| 02032 257.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302034 258. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302036 259. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02038 260.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302040 261. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302042 262. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02044 263.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302046 264. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302048 265. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02050 266.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302052 267. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302054 268. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02056 269.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302058 270. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302100 271. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02102 272.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302104 273. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302106 274. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02108 275.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302110 276. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302112 277. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02114 278.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302116 279. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302118 280. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02120 281.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302122 282. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302124 283. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02126 284.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302128 285. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302130 286. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02132 287.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302134 288. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302136 289. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02138 290.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302140 291. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302142 292. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02144 293.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302146 294. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302148 295. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02150 296.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302152 297. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302154 298. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 02156 299.  | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |
| 302158 300. | . 0 | . I | . | . | . | . | . | . | . | S . | . | . |

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

| OPERATION     | STATION | PEAK FLOW | TIME OF PEAK | AVERAGE FLOW FOR MAXIMUM PERIOD |         |         | BASIN AREA | MAXIMUM STAGE | TIME OF MAX STAGE |
|---------------|---------|-----------|--------------|---------------------------------|---------|---------|------------|---------------|-------------------|
|               |         |           |              | 6-HOUR                          | 24-HOUR | 72-HOUR |            |               |                   |
| HYDROGRAPH AT | D123    | 2.        | 5.80         | 1.                              | 0.      | 0.      | .02        |               |                   |
| ROUTED TO     | CH1A    | 2.        | 5.83         | 1.                              | 0.      | 0.      | .02        |               |                   |
| HYDROGRAPH AT | D4      | 0.        | 5.90         | 0.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | D4      | 2.        | 5.83         | 1.                              | 0.      | 0.      | .02        |               |                   |
| ROUTED TO     | RES4    | 0.        | 7.20         | 0.                              | 0.      | 0.      | .02        | 20.92         | 7.60              |
| ROUTED TO     | P1A     | 0.        | 7.23         | 0.                              | 0.      | 0.      | .02        |               |                   |
| HYDROGRAPH AT | C2      | 0.        | 6.10         | 0.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | C2      | 1.        | 6.13         | 1.                              | 0.      | 0.      | .03        |               |                   |
| HYDROGRAPH AT | C13     | 2.        | 5.80         | 1.                              | 0.      | 0.      | .02        |               |                   |
| 2 COMBINED AT | C12     | 3.        | 5.83         | 1.                              | 1.      | 1.      | .05        |               |                   |
| ROUTED TO     | P2A     | 3.        | 5.83         | 1.                              | 1.      | 1.      | .05        |               |                   |
| HYDROGRAPH AT | E1      | 0.        | 5.13         | 0.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | E1      | 3.        | 5.83         | 1.                              | 1.      | 1.      | .05        |               |                   |
| ROUTED TO     | RESS    | 1.        | 9.80         | 0.                              | 0.      | 0.      | .05        | 15.46         | 9.97              |

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SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

| ISTAQ | ELEMENT | DT    | PEAK  | TIME TO<br>PEAK | VOLUME | DT    | INTERPOLATED TO<br>COMPUTATION INTERVAL |                 | VOLUME |
|-------|---------|-------|-------|-----------------|--------|-------|---|-----------------|--------|
|       |         |       |       |                 |        |       | PEAK                                    | TIME TO<br>PEAK |        |
|       |         | (MIN) | (CFS) | (MIN)           | (IN)   | (MIN) | (CFS)                                   | (MIN)           | (IN)   |
| CH1A  | MANE    | 2.00  | 2.11  | 350.00          | .29    | 2.00  | 2.11                                    | 350.00          | .29    |

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2860E+00 EXCESS= .0000E+00 OUTFLOW= .2848E+00 BASIN STORAGE= .1343E-02 PERCENT ERROR= .0

|     |      |      |     |        |     |      |     |        |     |
|-----|------|------|-----|--------|-----|------|-----|--------|-----|
| PIA | MANE | 2.00 | .48 | 458.00 | .15 | 2.00 | .48 | 458.00 | .15 |
|-----|------|------|-----|--------|-----|------|-----|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1779E+00 EXCESS= .0000E+00 OUTFLOW= .1756E+00 BASIN STORAGE= .2313E-02 PERCENT ERROR= .0

|     |      |     |      |        |     |      |      |        |     |
|-----|------|-----|------|--------|-----|------|------|--------|-----|
| P2A | MANE | .97 | 3.15 | 350.03 | .22 | 2.00 | 3.15 | 350.00 | .22 |
|-----|------|-----|------|--------|-----|------|------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5635E+00 EXCESS= .0000E+00 OUTFLOW= .5621E+00 BASIN STORAGE= .1366E-02 PERCENT ERROR= .0

\*\*\* NORMAL END OF HEC-1 \*\*\*

R5-35

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
*
* RUN DATE 07/01/1994 TIME 19:21:01 *
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,  
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

Developed 100 YEAR STORM  
 SUB-BASINS "C, D AND E"  
 Run # 6

R6-1

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1         ID  GRAND VIEW SUBDIVISION
2         ID  DEVELOPED CONDITION
3         ID  100 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)
4         IT   2 30JUN94   1200   300
5         IO   5   2     0
          * *****

6         KK   D123
7         KM   Basin runoff calculation for   D123
8         KO   3     1     0     1     21
9         BA   0.0187
10        PH           0   0.39   0.76   1.34   1.40   1.44   1.56   1.69   2.01
11        LS           96
12        UD   0.457
          * *****

13        KK   CH1A
14        KM   Muskingum-Cunge channel routing from   CP9 to   CP10
15        KO   3     1     0     1     21
16        RD   315 0.0045  0.025           TRAP   2     4
          * *****

17        KK   D4
18        KM   Basin runoff calculation for   D4
19        KO   3     1     0     1     21
20        BA   0.0036
21        PH           0   0.39   0.76   1.34   1.40   1.44   1.56   1.69   2.01
22        LS           96
23        UD   0.484
          * *****

24        KK   D4
25        KM   Combining two hydrographs at control point   CP10
26        KO   3     1     0     1     21
27        HC   2
          * *****

28        KK   RES4
29        KM   Reservoir routing operation
30        KO   1     2     0     1     21
31        RS   1     ELEV  17.30
32        SV   0.0326 0.0994 0.0204 0.3618 0.5767 0.8459 1.0833
33        SE   18.3  19.3  20.3  21.3  22.3  23.3  24.0
34        SL   18.3 0.0625  0.6   0.5
35        SS   23.5 12.29  2.7   1.5
          * *****

36        KK   P1A
37        KM   Muskingum-Cunge channel routing from   CP10 to   CP8
38        KO   3     1     0     1     21
39        RD   655 0.0067  0.015           CIRC   2.0
          * *****
    
```

R6-2

```

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

40      KK      C2
41      KM      Basin runoff calculation for      C2
42      KO      3      1      0      1      21
43      BA      0.0049
44      PH              0      0.39      0.76      1.34      1.40      1.44      1.56      1.69      2.01
45      LS              96
46      UD      0.608
      * *****

47      KK      C2
48      KM      Combining two hydrographs at control point      CP8
49      KO      3      1      0      1      21
50      HC      2
      * *****

51      KK      C13
52      KM      Basin runoff calculation for      C13
53      KO      3      1      0      1      21
54      BA      0.0206
55      PH              0      0.39      0.76      1.34      1.40      1.44      1.56      1.69      2.01
56      LS              96
57      UD      0.472
      * *****

58      KK      C12
59      KM      Combining two hydrographs at control point      CP8
60      KO      3      1      0      1      21
61      HC      2
      * *****

62      KK      P2A
63      KM      Muskingum-Cunge channel routing from      CP8 to      CP11
64      KO      3      1      0      1      21
65      RD      277 0.0054 0.015      CIRC      3.0
      * *****

66      KK      E1
67      KM      Basin runoff calculation for      E1
68      KO      3      1      0      1      21
69      BA      0.0009
70      PH              0      0.39      0.76      1.34      1.40      1.44      1.56      1.69      2.01
71      LS              96
72      UD      0.129
      * *****

73      KK      E1
74      KM      Combining two hydrographs at control point      CP11
75      KO      3      1      0      1      21
76      HC      2
      * *****
    
```

R6-3



LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

77 KK RES5  
78 KM Reservoir routing operation  
79 KO 1 2 0 1 21  
80 RS 1 ELEV 11.24  
81 SV 0.0184 0.0693 0.1631 0.3126 0.5207 0.7889  
82 SE 12.0 13.0 14.0 15.0 16.0 17.0  
83 SL 12.24 0.0625 0.6 0.50  
84 SS 16.0 12.29 2.7 1.5  
\* \*\*\*\*\*  
85 ZZ

R6-4

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
*
* RUN DATE 07/01/1994 TIME 19:21:01 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

GRAND VIEW SUBDIVISION  
DEVELOPED CONDITION  
100 YEAR 24 HOUR STORM (GRAND JUNCTION URBANIZED AREA D-D-F DATA)

```

5 IO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      2 PLOT CONTROL
      QSCAL      0. HYDROGRAPH PLOT SCALE

```

```

IT HYDROGRAPH TIME DATA
      NMIN      2 MINUTES IN COMPUTATION INTERVAL
      IDATE     30JUN94 STARTING DATE
      ITIME     1200 STARTING TIME
      NQ        300 NUMBER OF HYDROGRAPH ORDINATES
      NDDATE    30JUN94 ENDING DATE
      NDTIME    2158 ENDING TIME
      ICENT     19 CENTURY MARK

```

COMPUTATION INTERVAL .03 HOURS  
TOTAL TIME BASE 9.97 HOURS

ENGLISH UNITS  
DRAINAGE AREA SQUARE MILES  
PRECIPITATION DEPTH INCHES  
LENGTH, ELEVATION FEET  
FLOW CUBIC FEET PER SECOND  
STORAGE VOLUME ACRE-FEET  
SURFACE AREA ACRES  
TEMPERATURE DEGREES FAHRENHEIT

\*\*\* \*\*

```

*****
*
* D123 *
*
*****

```

```

6 KK
8 KO OUTPUT CONTROL VARIABLES
      IPRNT      3 PRINT CONTROL
      IPLOT      1 PLOT CONTROL
      QSCAL      0. HYDROGRAPH PLOT SCALE
      IPNCH      1 PUNCH COMPUTED HYDROGRAPH
      IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2      300 LAST ORDINATE PUNCHED OR SAVED
      TTMTNT     0.33 TIME INTERVAL IN HOURS

```

R6-5

SUBBASIN RUNOFF DATA

9 BA

SUBBASIN CHARACTERISTICS

TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

10 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

| HYDRO-35 |        |        | TP-40 |      |      |       | TP-49 |       |       |       |        |
|----------|--------|--------|-------|------|------|-------|-------|-------|-------|-------|--------|
| 5-MIN    | 15-MIN | 60-MIN | 2-HR  | 3-HR | 6-HR | 12-HR | 24-HR | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
| .39      | .76    | 1.34   | 1.40  | 1.44 | 1.56 | 1.69  | 2.01  | .00   | .00   | .00   | .00    |

STORM AREA = .02

11 LS

SCS LOSS RATE

STRTL .08 INITIAL ABSTRACTION  
 CRVNBR 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

12 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .46 LAG

\*\*\*

UNIT HYDROGRAPH  
 71 END-OF-PERIOD ORDINATES

|     |     |     |     |     |     |     |     |     |     |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| 0.  | 1.  | 2.  | 3.  | 5.  | 7.  | 9.  | 11. | 14. | 16. |
| 17. | 18. | 19. | 19. | 19. | 19. | 18. | 17. | 16. | 15. |
| 13. | 12. | 10. | 9.  | 8.  | 7.  | 6.  | 6.  | 5.  | 5.  |
| 4.  | 4.  | 3.  | 3.  | 3.  | 2.  | 2.  | 2.  | 2.  | 1.  |
| 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  |     |     |     |     |     |     |     |     |     |

\*\*\*                    \*\*\*                    \*\*\*                    \*\*\*                    \*\*\*

HYDROGRAPH AT STATION D123

TOTAL RAINFALL = 1.65, TOTAL LOSS = .41, TOTAL EXCESS = 1.24

| EAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|-------------------|--------------|----------------------|-------|-------|---------|
|                   |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 16.               | 5.50         | (CFS) 2.             | 1.    | 1.    | 1.      |
|                   |              | (INCHES) 1.229       | 1.230 | 1.230 | 1.230   |
|                   |              | (AC-FT) 1.           | 1.    | 1.    | 1.      |

CUMULATIVE AREA = .02 SQ MI

\*\*\* \*\*

\*\*\*\*\*

\*                    \*  
 \*        CH1A       \*  
 \*                    \*  
 \*\*\*\*\*

13 KK

15 KO

OUTPUT CONTROL VARIABLES

IPRNT                    3    PRINT CONTROL  
 TPRINT                    1    PRINT CONTROL

R6-6

IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

16 RD MUSKINGUM-CUNGE CHANNEL ROUTING  
 L 315. CHANNEL LENGTH  
 S .0045 SLOPE  
 N .025 CHANNEL ROUGHNESS COEFFICIENT  
 CA .00 CONTRIBUTING AREA  
 SHAPE TRAP CHANNEL SHAPE  
 WD 2.00 BOTTOM WIDTH OR DIAMETER  
 Z 4.00 SIDE SLOPE

\*\*\*  
 COMPUTED MUSKINGUM-CUNGE PARAMETERS

| ELEMENT | ALPHA | COMPUTATION TIME STEP |             |            | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|-----------------------|-------------|------------|---------------|--------------------------|----------------|------------------------------|
|         |       | M                     | DT<br>(MIN) | DX<br>(FT) |               |                          |                |                              |
| MAIN    | 1.50  | 1.34                  | 1.70        | 157.50     | 16.37         | 330.76                   | 1.23           | 3.10                         |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |  |       |        |      |  |
|------|------|------|------|--|-------|--------|------|--|
| MAIN | 1.50 | 1.34 | 2.00 |  | 16.35 | 330.00 | 1.23 |  |
|------|------|------|------|--|-------|--------|------|--|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1226E+01 EXCESS= .0000E+00 OUTFLOW= .1225E+01 BASIN STORAGE= .1911E-02 PERCENT ERROR= .0

\*\*\* \*\*

HYDROGRAPH AT STATION CH1A

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 16.                | 5.50         | (CFS) 2.             | 1.    | 1.    | 1.      |
|                    |              | (INCHES) 1.227       | 1.228 | 1.228 | 1.228   |
|                    |              | (AC-FT) 1.           | 1.    | 1.    | 1.      |

CUMULATIVE AREA = .02 SQ MI

\*\*\* \*\*

\*\*\*\*\*  
 \* \*  
 \* D4 \*  
 \* \*  
 \*\*\*\*\*

17 KK

19 KO

OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 TPRINT 1 PRINT CONTROL

RG-7

IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

20 BA SUBBASIN CHARACTERISTICS  
 TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

21 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
 ..... HYDRO-35 ..... TP-40 ..... TP-49 .....  
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
 .39 .76 1.34 1.40 1.44 1.56 1.69 2.01 .00 .00 .00 .00

STORM AREA = .00

22 LS SCS LOSS RATE  
 STRTL .08 INITIAL ABSTRACTION  
 CRVNBR 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

23 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .48 LAG

\*\*\*

UNIT HYDROGRAPH  
75 END-OF-PERIOD ORDINATES

|    |    |    |    |    |    |    |    |    |    |
|----|----|----|----|----|----|----|----|----|----|
| 0. | 0. | 0. | 1. | 1. | 1. | 1. | 2. | 2. | 3. |
| 3. | 3. | 3. | 3. | 3. | 3. | 3. | 3. | 3. | 3. |
| 3. | 2. | 2. | 2. | 2. | 2. | 1. | 1. | 1. | 1. |
| 1. | 1. | 1. | 1. | 1. | 1. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |

\*\*\* \*\*

HYDROGRAPH AT STATION D4

TOTAL RAINFALL = 1.65, TOTAL LOSS = .41, TOTAL EXCESS = 1.24

| EAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|-------------------|--------------|----------------------|-------|-------|---------|
|                   |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 3.                | 5.50         | (CFS) 0.             | 0.    | 0.    | 0.      |
|                   |              | (INCHES) 1.228       | 1.229 | 1.229 | 1.229   |
|                   |              | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .00 SQ MI

\*\*\* \*\*

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\* \*  
\* \* \*

R6-8

\*\*\*\*\*

26 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

27 HC

HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

\*\*\*

\*\*\* \*\*

HYDROGRAPH AT STATION D4

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 19.                | 5.50         | (CFS) 3.             | 2.    | 2.    | 2.      |
|                    |              | (INCHES) 1.227       | 1.228 | 1.228 | 1.228   |
|                    |              | (AC-FT) 1.           | 1.    | 1.    | 1.      |

CUMULATIVE AREA = .02 SQ MI

\* \*\*

\*\*\*\*\*

28 KK

\* \*  
 \* RES4 \*  
 \* \*  
 \*\*\*\*\*

30 KO

OUTPUT CONTROL VARIABLES

IPRNT 1 PRINT CONTROL  
 IPLOT 2 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

31 RS

STORAGE ROUTING

NSTPS 1 NUMBER OF SUBREACHES  
 ITYP ELEV TYPE OF INITIAL CONDITION  
 RSVRIC 17.30 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

32 SV

|         |    |    |    |    |    |    |     |
|---------|----|----|----|----|----|----|-----|
| STORAGE | .0 | .1 | .0 | .4 | .6 | .8 | 1.1 |
|---------|----|----|----|----|----|----|-----|

33 SE

|           |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|
| ELEVATION | 18.30 | 19.30 | 20.30 | 21.30 | 22.30 | 23.30 | 24.00 |
|-----------|-------|-------|-------|-------|-------|-------|-------|

R6-9

ELEV 18.30 ELEVATION AT CENTER OF OUTLET  
 CAREA .06 CROSS-SECTIONAL AREA  
 COQL .60 COEFFICIENT  
 EXPL .50 EXPONENT OF HEAD

35 SS

SPILLWAY

CREL 23.50 SPILLWAY CREST ELEVATION  
 SPWID 12.29 SPILLWAY WIDTH  
 COQW 2.70 WEIR COEFFICIENT  
 EXPW 1.50 EXPONENT OF HEAD

\*\*\*

COMPUTED OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| OUTFLOW   | .00   | .11   | .13   | .14   | .17   | .20   | .24   | .31   | .42   | .69   |
| ELEVATION | 18.30 | 18.45 | 18.48 | 18.53 | 18.61 | 18.73 | 18.93 | 19.33 | 20.28 | 23.50 |
| OUTFLOW   | .70   | .78   | 1.01  | 1.45  | 2.17  | 3.24  | 4.74  | 6.72  | 9.27  | 12.45 |
| ELEVATION | 23.51 | 23.52 | 23.55 | 23.58 | 23.63 | 23.68 | 23.75 | 23.82 | 23.91 | 24.00 |

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| STORAGE   | .03   | .04   | .04   | .05   | .05   | .06   | .07   | .10   | .10   | .02   |
| OUTFLOW   | .00   | .11   | .13   | .14   | .17   | .20   | .24   | .30   | .31   | .42   |
| ELEVATION | 18.30 | 18.45 | 18.48 | 18.53 | 18.61 | 18.73 | 18.93 | 19.30 | 19.33 | 20.28 |
| STORAGE   | .02   | .36   | .58   | .85   | .91   | .92   | .93   | .94   | .96   | .97   |
| OUTFLOW   | .43   | .52   | .60   | .67   | .69   | .78   | 1.01  | 1.45  | 2.17  | 3.24  |
| ELEVATION | 20.30 | 21.30 | 22.30 | 23.30 | 23.50 | 23.52 | 23.55 | 23.58 | 23.63 | 23.68 |
| STORAGE   | 1.00  | 1.02  | 1.05  | 1.08  |       |       |       |       |       |       |
| OUTFLOW   | 4.74  | 6.72  | 9.27  | 12.45 |       |       |       |       |       |       |
| ELEVATION | 23.75 | 23.82 | 23.91 | 24.00 |       |       |       |       |       |       |

HYDROGRAPH AT STATION RES4

| *****                      |     |      |     |         |         |       |   |    |     |      |     |         |         |       |   |    |     |      |     |         |         |       |
|----------------------------|-----|------|-----|---------|---------|-------|---|----|-----|------|-----|---------|---------|-------|---|----|-----|------|-----|---------|---------|-------|
| HYDROGRAPH AT STATION RES4 |     |      |     |         |         |       |   |    |     |      |     |         |         |       |   |    |     |      |     |         |         |       |
| *****                      |     |      |     |         |         |       |   |    |     |      |     |         |         |       |   |    |     |      |     |         |         |       |
| DA                         | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | * | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | * | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE |
| *****                      |     |      |     |         |         |       |   |    |     |      |     |         |         |       |   |    |     |      |     |         |         |       |
| 30                         | JUN | 1200 | 1   | 0.      | .0      | 18.3  | * | 30 | JUN | 1520 | 101 | 0.      | .0      | 18.3  | * | 30 | JUN | 1840 | 201 | 2.      | 1.0     | 23.6  |
| 30                         | JUN | 1202 | 2   | 0.      | .0      | 18.3  | * | 30 | JUN | 1522 | 102 | 0.      | .0      | 18.3  | * | 30 | JUN | 1842 | 202 | 2.      | 1.0     | 23.6  |
| 30                         | JUN | 1204 | 3   | 0.      | .0      | 18.3  | * | 30 | JUN | 1524 | 103 | 0.      | .0      | 18.3  | * | 30 | JUN | 1844 | 203 | 2.      | 1.0     | 23.6  |
| 30                         | JUN | 1206 | 4   | 0.      | .0      | 18.3  | * | 30 | JUN | 1526 | 104 | 0.      | .0      | 18.3  | * | 30 | JUN | 1846 | 204 | 2.      | 1.0     | 23.6  |
| 30                         | JUN | 1208 | 5   | 0.      | .0      | 18.3  | * | 30 | JUN | 1528 | 105 | 0.      | .0      | 18.3  | * | 30 | JUN | 1848 | 205 | 2.      | 1.0     | 23.6  |
| 30                         | JUN | 1210 | 6   | 0.      | .0      | 18.3  | * | 30 | JUN | 1530 | 106 | 0.      | .0      | 18.3  | * | 30 | JUN | 1850 | 206 | 2.      | .9      | 23.6  |
| 30                         | JUN | 1212 | 7   | 0.      | .0      | 18.3  | * | 30 | JUN | 1532 | 107 | 0.      | .0      | 18.3  | * | 30 | JUN | 1852 | 207 | 2.      | .9      | 23.6  |
| 30                         | JUN | 1214 | 8   | 0.      | .0      | 18.3  | * | 30 | JUN | 1534 | 108 | 0.      | .0      | 18.3  | * | 30 | JUN | 1854 | 208 | 2.      | .9      | 23.6  |
| 30                         | JUN | 1216 | 9   | 0.      | .0      | 18.3  | * | 30 | JUN | 1536 | 109 | 0.      | .0      | 18.3  | * | 30 | JUN | 1856 | 209 | 2.      | .9      | 23.6  |
| 30                         | JUN | 1218 | 10  | 0.      | .0      | 18.3  | * | 30 | JUN | 1538 | 110 | 0.      | .0      | 18.3  | * | 30 | JUN | 1858 | 210 | 1.      | .9      | 23.6  |
| 30                         | JUN | 1220 | 11  | 0.      | .0      | 18.3  | * | 30 | JUN | 1540 | 111 | 0.      | .0      | 18.3  | * | 30 | JUN | 1900 | 211 | 1.      | .9      | 23.6  |
| 30                         | JUN | 1222 | 12  | 0.      | .0      | 18.3  | * | 30 | JUN | 1542 | 112 | 0.      | .0      | 18.3  | * | 30 | JUN | 1902 | 212 | 1.      | .9      | 23.6  |
| 30                         | JUN | 1224 | 13  | 0.      | .0      | 18.3  | * | 30 | JUN | 1544 | 113 | 0.      | .0      | 18.3  | * | 30 | JUN | 1904 | 213 | 1.      | .9      | 23.6  |
| 30                         | JUN | 1226 | 14  | 0.      | .0      | 18.3  | * | 30 | JUN | 1546 | 114 | 0.      | .0      | 18.3  | * | 30 | JUN | 1906 | 214 | 1.      | .9      | 23.6  |
| 30                         | JUN | 1228 | 15  | 0.      | .0      | 18.3  | * | 30 | JUN | 1548 | 115 | 0.      | .0      | 18.3  | * | 30 | JUN | 1908 | 215 | 1.      | .9      | 23.6  |
| 30                         | JUN | 1230 | 16  | 0.      | .0      | 18.3  | * | 30 | JUN | 1550 | 116 | 0.      | .0      | 18.3  | * | 30 | JUN | 1910 | 216 | 1.      | .9      | 23.6  |
| 30                         | JUN | 1232 | 17  | 0.      | .0      | 18.3  | * | 30 | JUN | 1552 | 117 | 0.      | .0      | 18.3  | * | 30 | JUN | 1912 | 217 | 1.      | .9      | 23.6  |
| 30                         | JUN | 1234 | 18  | 0.      | .0      | 18.3  | * | 30 | JUN | 1554 | 118 | 0.      | .0      | 18.3  | * | 30 | JUN | 1914 | 218 | 1.      | .9      | 23.6  |
| 30                         | JUN | 1236 | 19  | 0.      | .0      | 18.3  | * | 30 | JUN | 1556 | 119 | 0.      | .0      | 18.3  | * | 30 | JUN | 1916 | 219 | 1.      | .9      | 23.6  |
| 30                         | JUN | 1238 | 20  | 0.      | .0      | 18.3  | * | 30 | JUN | 1558 | 120 | 0.      | .0      | 18.3  | * | 30 | JUN | 1918 | 220 | 1.      | .9      | 23.5  |
| 30                         | JUN | 1240 | 21  | 0.      | .0      | 18.3  | * | 30 | JUN | 1600 | 121 | 0.      | .0      | 18.3  | * | 30 | JUN | 1920 | 221 | 1.      | .9      | 23.5  |

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|             |    |    |    |      |   |             |     |    |     |      |   |             |     |    |    |      |
|-------------|----|----|----|------|---|-------------|-----|----|-----|------|---|-------------|-----|----|----|------|
| 30 JUN 1244 | 23 | 0. | .0 | 18.3 | * | 30 JUN 1604 | 123 | 0. | .0  | 18.3 | * | 30 JUN 1924 | 223 | 1. | .9 | 23.5 |
| 30 JUN 1246 | 24 | 0. | .0 | 18.3 | * | 30 JUN 1606 | 124 | 0. | .0  | 18.3 | * | 30 JUN 1926 | 224 | 1. | .9 | 23.5 |
| 30 JUN 1248 | 25 | 0. | .0 | 18.3 | * | 30 JUN 1608 | 125 | 0. | .0  | 18.3 | * | 30 JUN 1928 | 225 | 1. | .9 | 23.5 |
| 30 JUN 1250 | 26 | 0. | .0 | 18.3 | * | 30 JUN 1610 | 126 | 0. | .0  | 18.3 | * | 30 JUN 1930 | 226 | 1. | .9 | 23.5 |
| 30 JUN 1252 | 27 | 0. | .0 | 18.3 | * | 30 JUN 1612 | 127 | 0. | .0  | 18.3 | * | 30 JUN 1932 | 227 | 1. | .9 | 23.5 |
| 30 JUN 1254 | 28 | 0. | .0 | 18.3 | * | 30 JUN 1614 | 128 | 0. | .0  | 18.3 | * | 30 JUN 1934 | 228 | 1. | .9 | 23.5 |
| 30 JUN 1256 | 29 | 0. | .0 | 18.3 | * | 30 JUN 1616 | 129 | 0. | .0  | 18.3 | * | 30 JUN 1936 | 229 | 1. | .9 | 23.5 |
| 30 JUN 1258 | 30 | 0. | .0 | 18.3 | * | 30 JUN 1618 | 130 | 0. | .0  | 18.3 | * | 30 JUN 1938 | 230 | 1. | .9 | 23.5 |
| 30 JUN 1300 | 31 | 0. | .0 | 18.3 | * | 30 JUN 1620 | 131 | 0. | .0  | 18.3 | * | 30 JUN 1940 | 231 | 1. | .9 | 23.5 |
| 30 JUN 1302 | 32 | 0. | .0 | 18.3 | * | 30 JUN 1622 | 132 | 0. | .0  | 18.3 | * | 30 JUN 1942 | 232 | 1. | .9 | 23.5 |
| 30 JUN 1304 | 33 | 0. | .0 | 18.3 | * | 30 JUN 1624 | 133 | 0. | .0  | 18.3 | * | 30 JUN 1944 | 233 | 1. | .9 | 23.5 |
| 30 JUN 1306 | 34 | 0. | .0 | 18.3 | * | 30 JUN 1626 | 134 | 0. | .0  | 18.3 | * | 30 JUN 1946 | 234 | 1. | .9 | 23.5 |
| 30 JUN 1308 | 35 | 0. | .0 | 18.3 | * | 30 JUN 1628 | 135 | 0. | .0  | 18.3 | * | 30 JUN 1948 | 235 | 1. | .9 | 23.5 |
| 30 JUN 1310 | 36 | 0. | .0 | 18.3 | * | 30 JUN 1630 | 136 | 0. | .0  | 18.3 | * | 30 JUN 1950 | 236 | 1. | .9 | 23.5 |
| 30 JUN 1312 | 37 | 0. | .0 | 18.3 | * | 30 JUN 1632 | 137 | 0. | .0  | 18.3 | * | 30 JUN 1952 | 237 | 1. | .9 | 23.5 |
| 30 JUN 1314 | 38 | 0. | .0 | 18.3 | * | 30 JUN 1634 | 138 | 0. | .0  | 18.4 | * | 30 JUN 1954 | 238 | 1. | .9 | 23.5 |
| 30 JUN 1316 | 39 | 0. | .0 | 18.3 | * | 30 JUN 1636 | 139 | 0. | .0  | 18.4 | * | 30 JUN 1956 | 239 | 1. | .9 | 23.5 |
| 30 JUN 1318 | 40 | 0. | .0 | 18.3 | * | 30 JUN 1638 | 140 | 0. | .0  | 18.4 | * | 30 JUN 1958 | 240 | 1. | .9 | 23.5 |
| 30 JUN 1320 | 41 | 0. | .0 | 18.3 | * | 30 JUN 1640 | 141 | 0. | .0  | 18.4 | * | 30 JUN 2000 | 241 | 1. | .9 | 23.5 |
| 30 JUN 1322 | 42 | 0. | .0 | 18.3 | * | 30 JUN 1642 | 142 | 0. | .0  | 18.4 | * | 30 JUN 2002 | 242 | 1. | .9 | 23.5 |
| 30 JUN 1324 | 43 | 0. | .0 | 18.3 | * | 30 JUN 1644 | 143 | 0. | .0  | 18.4 | * | 30 JUN 2004 | 243 | 1. | .9 | 23.5 |
| 30 JUN 1326 | 44 | 0. | .0 | 18.3 | * | 30 JUN 1646 | 144 | 0. | .0  | 18.4 | * | 30 JUN 2006 | 244 | 1. | .9 | 23.5 |
| 30 JUN 1328 | 45 | 0. | .0 | 18.3 | * | 30 JUN 1648 | 145 | 0. | .0  | 18.4 | * | 30 JUN 2008 | 245 | 1. | .9 | 23.5 |
| 30 JUN 1330 | 46 | 0. | .0 | 18.3 | * | 30 JUN 1650 | 146 | 0. | .0  | 18.4 | * | 30 JUN 2010 | 246 | 1. | .9 | 23.5 |
| 30 JUN 1332 | 47 | 0. | .0 | 18.3 | * | 30 JUN 1652 | 147 | 0. | .0  | 18.5 | * | 30 JUN 2012 | 247 | 1. | .9 | 23.5 |
| 30 JUN 1334 | 48 | 0. | .0 | 18.3 | * | 30 JUN 1654 | 148 | 0. | .0  | 18.5 | * | 30 JUN 2014 | 248 | 1. | .9 | 23.5 |
| 30 JUN 1336 | 49 | 0. | .0 | 18.3 | * | 30 JUN 1656 | 149 | 0. | .0  | 18.5 | * | 30 JUN 2016 | 249 | 1. | .9 | 23.5 |
| 30 JUN 1338 | 50 | 0. | .0 | 18.3 | * | 30 JUN 1658 | 150 | 0. | .1  | 18.6 | * | 30 JUN 2018 | 250 | 1. | .9 | 23.5 |
| 30 JUN 1340 | 51 | 0. | .0 | 18.3 | * | 30 JUN 1700 | 151 | 0. | .1  | 18.7 | * | 30 JUN 2020 | 251 | 1. | .9 | 23.5 |
| 30 JUN 1342 | 52 | 0. | .0 | 18.3 | * | 30 JUN 1702 | 152 | 0. | .1  | 18.8 | * | 30 JUN 2022 | 252 | 1. | .9 | 23.5 |
| 30 JUN 1344 | 53 | 0. | .0 | 18.3 | * | 30 JUN 1704 | 153 | 0. | .1  | 18.9 | * | 30 JUN 2024 | 253 | 1. | .9 | 23.5 |
| 30 JUN 1346 | 54 | 0. | .0 | 18.3 | * | 30 JUN 1706 | 154 | 0. | .1  | 19.1 | * | 30 JUN 2026 | 254 | 1. | .9 | 23.5 |
| 30 JUN 1348 | 55 | 0. | .0 | 18.3 | * | 30 JUN 1708 | 155 | 0. | .1  | 20.5 | * | 30 JUN 2028 | 255 | 1. | .9 | 23.5 |
| 30 JUN 1350 | 56 | 0. | .0 | 18.3 | * | 30 JUN 1710 | 156 | 0. | .1  | 20.6 | * | 30 JUN 2030 | 256 | 1. | .9 | 23.5 |
| 30 JUN 1352 | 57 | 0. | .0 | 18.3 | * | 30 JUN 1712 | 157 | 0. | .1  | 20.7 | * | 30 JUN 2032 | 257 | 1. | .9 | 23.5 |
| 30 JUN 1354 | 58 | 0. | .0 | 18.3 | * | 30 JUN 1714 | 158 | 0. | .2  | 20.7 | * | 30 JUN 2034 | 258 | 1. | .9 | 23.5 |
| 30 JUN 1356 | 59 | 0. | .0 | 18.3 | * | 30 JUN 1716 | 159 | 0. | .2  | 20.8 | * | 30 JUN 2036 | 259 | 1. | .9 | 23.5 |
| 30 JUN 1358 | 60 | 0. | .0 | 18.3 | * | 30 JUN 1718 | 160 | 0. | .2  | 20.9 | * | 30 JUN 2038 | 260 | 1. | .9 | 23.5 |
| 30 JUN 1400 | 61 | 0. | .0 | 18.3 | * | 30 JUN 1720 | 161 | 0. | .3  | 21.1 | * | 30 JUN 2040 | 261 | 1. | .9 | 23.5 |
| 30 JUN 1402 | 62 | 0. | .0 | 18.3 | * | 30 JUN 1722 | 162 | 1. | .3  | 21.2 | * | 30 JUN 2042 | 262 | 1. | .9 | 23.5 |
| 30 JUN 1404 | 63 | 0. | .0 | 18.3 | * | 30 JUN 1724 | 163 | 1. | .4  | 21.3 | * | 30 JUN 2044 | 263 | 1. | .9 | 23.5 |
| 30 JUN 1406 | 64 | 0. | .0 | 18.3 | * | 30 JUN 1726 | 164 | 1. | .4  | 21.6 | * | 30 JUN 2046 | 264 | 1. | .9 | 23.5 |
| 30 JUN 1408 | 65 | 0. | .0 | 18.3 | * | 30 JUN 1728 | 165 | 1. | .5  | 21.8 | * | 30 JUN 2048 | 265 | 1. | .9 | 23.5 |
| 30 JUN 1410 | 66 | 0. | .0 | 18.3 | * | 30 JUN 1730 | 166 | 1. | .5  | 22.0 | * | 30 JUN 2050 | 266 | 1. | .9 | 23.5 |
| 30 JUN 1412 | 67 | 0. | .0 | 18.3 | * | 30 JUN 1732 | 167 | 1. | .6  | 22.3 | * | 30 JUN 2052 | 267 | 1. | .9 | 23.5 |
| 30 JUN 1414 | 68 | 0. | .0 | 18.3 | * | 30 JUN 1734 | 168 | 1. | .6  | 22.5 | * | 30 JUN 2054 | 268 | 1. | .9 | 23.5 |
| 30 JUN 1416 | 69 | 0. | .0 | 18.3 | * | 30 JUN 1736 | 169 | 1. | .7  | 22.7 | * | 30 JUN 2056 | 269 | 1. | .9 | 23.4 |
| 30 JUN 1418 | 70 | 0. | .0 | 18.3 | * | 30 JUN 1738 | 170 | 1. | .7  | 22.8 | * | 30 JUN 2058 | 270 | 1. | .9 | 23.4 |
| 30 JUN 1420 | 71 | 0. | .0 | 18.3 | * | 30 JUN 1740 | 171 | 1. | .8  | 23.0 | * | 30 JUN 2100 | 271 | 1. | .9 | 23.4 |
| 30 JUN 1422 | 72 | 0. | .0 | 18.3 | * | 30 JUN 1742 | 172 | 1. | .8  | 23.2 | * | 30 JUN 2102 | 272 | 1. | .9 | 23.4 |
| 30 JUN 1424 | 73 | 0. | .0 | 18.3 | * | 30 JUN 1744 | 173 | 1. | .9  | 23.3 | * | 30 JUN 2104 | 273 | 1. | .9 | 23.4 |
| 30 JUN 1426 | 74 | 0. | .0 | 18.3 | * | 30 JUN 1746 | 174 | 1. | .9  | 23.4 | * | 30 JUN 2106 | 274 | 1. | .9 | 23.4 |
| 30 JUN 1428 | 75 | 0. | .0 | 18.3 | * | 30 JUN 1748 | 175 | 1. | .9  | 23.5 | * | 30 JUN 2108 | 275 | 1. | .9 | 23.4 |
| 30 JUN 1430 | 76 | 0. | .0 | 18.3 | * | 30 JUN 1750 | 176 | 2. | 1.0 | 23.6 | * | 30 JUN 2110 | 276 | 1. | .9 | 23.4 |
| 30 JUN 1432 | 77 | 0. | .0 | 18.3 | * | 30 JUN 1752 | 177 | 4. | 1.0 | 23.7 | * | 30 JUN 2112 | 277 | 1. | .9 | 23.4 |
| 30 JUN 1434 | 78 | 0. | .0 | 18.3 | * | 30 JUN 1754 | 178 | 5. | 1.0 | 23.8 | * | 30 JUN 2114 | 278 | 1. | .9 | 23.4 |
| 30 JUN 1436 | 79 | 0. | .0 | 18.3 | * | 30 JUN 1756 | 179 | 6. | 1.0 | 23.8 | * | 30 JUN 2116 | 279 | 1. | .9 | 23.4 |
| 30 JUN 1438 | 80 | 0. | .0 | 18.3 | * | 30 JUN 1758 | 180 | 7. | 1.0 | 23.8 | * | 30 JUN 2118 | 280 | 1. | .9 | 23.4 |
| 30 JUN 1440 | 81 | 0. | .0 | 18.3 | * | 30 JUN 1800 | 181 | 7. | 1.0 | 23.8 | * | 30 JUN 2120 | 281 | 1. | .9 | 23.4 |
| 30 JUN 1442 | 82 | 0. | .0 | 18.3 | * | 30 JUN 1802 | 182 | 7. | 1.0 | 23.8 | * | 30 JUN 2122 | 282 | 1. | .9 | 23.4 |
| 30 JUN 1444 | 83 | 0. | .0 | 18.3 | * | 30 JUN 1804 | 183 | 7. | 1.0 | 23.8 | * | 30 JUN 2124 | 283 | 1. | .9 | 23.4 |
| 30 JUN 1446 | 84 | 0. | .0 | 18.3 | * | 30 JUN 1806 | 184 | 7. | 1.0 | 23.8 | * | 30 JUN 2126 | 284 | 1. | .9 | 23.4 |
| 30 JUN 1448 | 85 | 0. | .0 | 18.3 | * | 30 JUN 1808 | 185 | 7. | 1.0 | 23.8 | * | 30 JUN 2128 | 285 | 1. | .9 | 23.4 |
| 30 JUN 1450 | 86 | 0. | .0 | 18.3 | * | 30 JUN 1810 | 186 | 6. | 1.0 | 23.8 | * | 30 JUN 2130 | 286 | 1. | .9 | 23.4 |
| 30 JUN 1452 | 87 | 0. | .0 | 18.3 | * | 30 JUN 1812 | 187 | 6. | 1.0 | 23.8 | * | 30 JUN 2132 | 287 | 1. | .9 | 23.4 |

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|             |     |    |    |      |   |             |     |    |     |      |   |             |     |    |    |      |
|-------------|-----|----|----|------|---|-------------|-----|----|-----|------|---|-------------|-----|----|----|------|
| 30 JUN 1456 | 89  | 0. | .0 | 18.3 | * | 30 JUN 1816 | 189 | 5. | 1.0 | 23.8 | * | 30 JUN 2136 | 289 | 1. | .9 | 23.4 |
| 30 JUN 1458 | 90  | 0. | .0 | 18.3 | * | 30 JUN 1818 | 190 | 5. | 1.0 | 23.8 | * | 30 JUN 2138 | 290 | 1. | .9 | 23.4 |
| 30 JUN 1500 | 91  | 0. | .0 | 18.3 | * | 30 JUN 1820 | 191 | 5. | 1.0 | 23.7 | * | 30 JUN 2140 | 291 | 1. | .9 | 23.4 |
| 30 JUN 1502 | 92  | 0. | .0 | 18.3 | * | 30 JUN 1822 | 192 | 4. | 1.0 | 23.7 | * | 30 JUN 2142 | 292 | 1. | .9 | 23.4 |
| 30 JUN 1504 | 93  | 0. | .0 | 18.3 | * | 30 JUN 1824 | 193 | 4. | 1.0 | 23.7 | * | 30 JUN 2144 | 293 | 1. | .9 | 23.4 |
| 30 JUN 1506 | 94  | 0. | .0 | 18.3 | * | 30 JUN 1826 | 194 | 4. | 1.0 | 23.7 | * | 30 JUN 2146 | 294 | 1. | .9 | 23.4 |
| 30 JUN 1508 | 95  | 0. | .0 | 18.3 | * | 30 JUN 1828 | 195 | 4. | 1.0 | 23.7 | * | 30 JUN 2148 | 295 | 1. | .9 | 23.4 |
| 30 JUN 1510 | 96  | 0. | .0 | 18.3 | * | 30 JUN 1830 | 196 | 3. | 1.0 | 23.7 | * | 30 JUN 2150 | 296 | 1. | .9 | 23.4 |
| 30 JUN 1512 | 97  | 0. | .0 | 18.3 | * | 30 JUN 1832 | 197 | 3. | 1.0 | 23.7 | * | 30 JUN 2152 | 297 | 1. | .9 | 23.4 |
| 30 JUN 1514 | 98  | 0. | .0 | 18.3 | * | 30 JUN 1834 | 198 | 3. | 1.0 | 23.7 | * | 30 JUN 2154 | 298 | 1. | .9 | 23.4 |
| 30 JUN 1516 | 99  | 0. | .0 | 18.3 | * | 30 JUN 1836 | 199 | 3. | 1.0 | 23.7 | * | 30 JUN 2156 | 299 | 1. | .9 | 23.4 |
| 30 JUN 1518 | 100 | 0. | .0 | 18.3 | * | 30 JUN 1838 | 200 | 3. | 1.0 | 23.6 | * | 30 JUN 2158 | 300 | 1. | .9 | 23.4 |

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| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 7.                 | 6.03         | (CFS) 1.             | 1.    | 1.    | 1.      |
|                    |              | (INCHES) .526        | .526  | .526  | .526    |
|                    |              | (AC-FT) 1.           | 1.    | 1.    | 1.      |

| PEAK STORAGE<br>(AC-FT) | TIME<br>(HR) | MAXIMUM AVERAGE STORAGE |       |       |         |
|-------------------------|--------------|-------------------------|-------|-------|---------|
|                         |              | 6-HR                    | 24-HR | 72-HR | 9.97-HR |
| 1.                      | 6.03         | 1.                      | 0.    | 0.    | 0.      |

| PEAK STAGE<br>(FEET) | TIME<br>(HR) | MAXIMUM AVERAGE STAGE |       |       |         |
|----------------------|--------------|-----------------------|-------|-------|---------|
|                      |              | 6-HR                  | 24-HR | 72-HR | 9.97-HR |
| 23.84                | 6.03         | 22.37                 | 20.75 | 20.75 | 20.75   |

CUMULATIVE AREA = .02 SQ MI

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STATION RES4

(I) INFLOW, (O) OUTFLOW

0. 4. 8. 12. 16. 20. 0. 0. 0. 0. 0. 0. 0.  
 (S) STORAGE  
 .0 .0 .0 .0 .0 .0 .0 .4 .8 1.2 .0 .0 .0

| STATION    | RES4 | (I) INFLOW | (O) OUTFLOW | (S) STORAGE |
|------------|------|------------|-------------|-------------|
| DAHRMN PER |      |            |             |             |
| 01200 1I   | S    |            |             |             |
| 01202 2I   | S    |            |             |             |
| 01204 3I   | S    |            |             |             |
| 01206 4I   | S    |            |             |             |
| 01208 5I   | S    |            |             |             |
| 01210 6I   | S    |            |             |             |
| 01212 7I   | S    |            |             |             |
| 01214 8I   | S    |            |             |             |
| 01216 9I   | S    |            |             |             |
| 01218 10I  | S    |            |             |             |
| 01220 11I  | S    |            |             |             |
| 01222 12I  | S    |            |             |             |
| 01224 13I  | S    |            |             |             |
| 01226 14I  | S    |            |             |             |
| 01228 15I  | S    |            |             |             |
| 01230 16I  | S    |            |             |             |
| 01232 17I  | S    |            |             |             |
| 01234 18I  | S    |            |             |             |
| 01236 19I  | S    |            |             |             |
| 01238 20I  | S    |            |             |             |
| 01240 21I  | S    |            |             |             |
| 01242 22I  | S    |            |             |             |
| 01244 23I  | S    |            |             |             |
| 01246 24I  | S    |            |             |             |
| 01248 25I  | S    |            |             |             |
| 01250 26I  | S    |            |             |             |
| 01252 27I  | S    |            |             |             |
| 01254 28I  | S    |            |             |             |
| 01256 29I  | S    |            |             |             |
| 01258 30I  | S    |            |             |             |
| 01300 31I  | S    |            |             |             |
| 01302 32I  | S    |            |             |             |
| 01304 33I  | S    |            |             |             |
| 01306 34I  | S    |            |             |             |
| 01308 35I  | S    |            |             |             |
| 01310 36I  | S    |            |             |             |
| 01312 37I  | S    |            |             |             |
| 01314 38I  | S    |            |             |             |
| 01316 39I  | S    |            |             |             |
| 01318 40I  | S    |            |             |             |
| 01320 41I  | S    |            |             |             |
| 01322 42I  | S    |            |             |             |
| 01324 43I  | S    |            |             |             |
| 01326 44I  | S    |            |             |             |
| 01328 45I  | S    |            |             |             |
| 01330 46I  | S    |            |             |             |
| 01332 47I  | S    |            |             |             |
| 01334 48I  | S    |            |             |             |
| 01336 49I  | S    |            |             |             |
| 01338 50I  | S    |            |             |             |
| 01340 51I  | S    |            |             |             |
| 01342 52I  | S    |            |             |             |
| 01344 53I  | S    |            |             |             |
| 01346 54I  | S    |            |             |             |
| 01348 55I  | S    |            |             |             |
| 01350 56I  | S    |            |             |             |
| 01352 57I  | S    |            |             |             |

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|        |      |   |
|--------|------|---|
| 01356  | 59I  | S |
| 301358 | 60I  | S |
| 701400 | 61I  | S |
| 01402  | 62I  | S |
| 301404 | 63I  | S |
| 701406 | 64I  | S |
| 01408  | 65I  | S |
| 301410 | 66I  | S |
| 701412 | 67I  | S |
| 01414  | 68I  | S |
| 301416 | 69I  | S |
| 701418 | 70I  | S |
| 01420  | 71I  | S |
| 301422 | 72I  | S |
| 701424 | 73I  | S |
| 01426  | 74I  | S |
| 301428 | 75I  | S |
| 701430 | 76I  | S |
| 01432  | 77I  | S |
| 301434 | 78I  | S |
| 701436 | 79I  | S |
| 01438  | 80I  | S |
| 301440 | 81I  | S |
| 701442 | 82I  | S |
| 01444  | 83I  | S |
| 301446 | 84I  | S |
| 701448 | 85I  | S |
| 01450  | 86I  | S |
| 301452 | 87I  | S |
| 701454 | 88I  | S |
| 01456  | 89I  | S |
| 301458 | 90I  | S |
| 701500 | 91I  | S |
| 01502  | 92I  | S |
| 301504 | 93I  | S |
| 701506 | 94I  | S |
| 01508  | 95I  | S |
| 301510 | 96I  | S |
| 701512 | 97I  | S |
| 01514  | 98I  | S |
| 301516 | 99I  | S |
| 701518 | 100I | S |
| 01520  | 101I | S |
| 301522 | 102I | S |
| 701524 | 103I | S |
| 01526  | 104I | S |
| 301528 | 105I | S |
| 701530 | 106I | S |
| 01532  | 107I | S |
| 301534 | 108I | S |
| 701536 | 109I | S |
| 01538  | 110I | S |
| 301540 | 111I | S |
| 701542 | 112I | S |
| 01544  | 113I | S |
| 301546 | 114I | S |
| 701548 | 115I | S |
| 01550  | 116I | S |
| 301552 | 117I | S |
| 701554 | 118I | S |
| 01556  | 119I | S |
| 301558 | 120I | S |
| 701600 | 121I | S |
| 01602  | 122I | S |
| 301604 | 123I | S |

R6-14



|        |     |     |   |
|--------|-----|-----|---|
| 01820  | 191 | I 0 | S |
| 301822 | 192 | I 0 | S |
| 701824 | 193 | I 0 | S |
| 01826  | 194 | I 0 | S |
| 301828 | 195 | I 0 | S |
| 701830 | 196 | I 0 | S |
| 01832  | 197 | I 0 | S |
| 301834 | 198 | I 0 | S |
| 701836 | 199 | I 0 | S |
| 01838  | 200 | I 0 | S |
| 301840 | 201 | I 0 | S |
| 701842 | 202 | I 0 | S |
| 01844  | 203 | I 0 | S |
| 301846 | 204 | I 0 | S |
| 701848 | 205 | I 0 | S |
| 01850  | 206 | I 0 | S |
| 301852 | 207 | I 0 | S |
| 701854 | 208 | I 0 | S |
| 01856  | 209 | I 0 | S |
| 301858 | 210 | I 0 | S |
| 701900 | 211 | I 0 | S |
| 01902  | 212 | I 0 | S |
| 301904 | 213 | I 0 | S |
| 701906 | 214 | I 0 | S |
| 01908  | 215 | I 0 | S |
| 301910 | 216 | I 0 | S |
| 701912 | 217 | I 0 | S |
| 01914  | 218 | I 0 | S |
| 301916 | 219 | I 0 | S |
| 701918 | 220 | I 0 | S |
| 01920  | 221 | I 0 | S |
| 301922 | 222 | I 0 | S |
| 701924 | 223 | I 0 | S |
| 01926  | 224 | I 0 | S |
| 301928 | 225 | I 0 | S |
| 701930 | 226 | I 0 | S |
| 01932  | 227 | I 0 | S |
| 301934 | 228 | I 0 | S |
| 701936 | 229 | I 0 | S |
| 01938  | 230 | I 0 | S |
| 301940 | 231 | I 0 | S |
| 701942 | 232 | I 0 | S |
| 01944  | 233 | I 0 | S |
| 301946 | 234 | I 0 | S |
| 701948 | 235 | I 0 | S |
| 01950  | 236 | I 0 | S |
| 301952 | 237 | I 0 | S |
| 701954 | 238 | I 0 | S |
| 01956  | 239 | I 0 | S |
| 301958 | 240 | I 0 | S |
| 702000 | 241 | I 0 | S |
| 02002  | 242 | I 0 | S |
| 302004 | 243 | I 0 | S |
| 702006 | 244 | I 0 | S |
| 02008  | 245 | I 0 | S |
| 302010 | 246 | I 0 | S |
| 702012 | 247 | I 0 | S |
| 02014  | 248 | I 0 | S |
| 302016 | 249 | I 0 | S |
| 702018 | 250 | I 0 | S |
| 02020  | 251 | I 0 | S |
| 302022 | 252 | I 0 | S |
| 702024 | 253 | I 0 | S |
| 02026  | 254 | I 0 | S |
| 702028 | 255 | I 0 | S |

R6-16

|       |        |   |
|-------|--------|---|
| 02032 | 257.I0 | S |
| 02034 | 258.I0 | S |
| 02036 | 259.I0 | S |
| 02038 | 260.I0 | S |
| 02040 | 261.I0 | S |
| 02042 | 262.I0 | S |
| 02044 | 263.I0 | S |
| 02046 | 264.I0 | S |
| 02048 | 265.I0 | S |
| 02050 | 266.I0 | S |
| 02052 | 267.I0 | S |
| 02054 | 268.I0 | S |
| 02056 | 269.I0 | S |
| 02058 | 270.I0 | S |
| 02100 | 271.I0 | S |
| 02102 | 272.I0 | S |
| 02104 | 273.I0 | S |
| 02106 | 274.I0 | S |
| 02108 | 275.I0 | S |
| 02110 | 276.I0 | S |
| 02112 | 277.I0 | S |
| 02114 | 278.I0 | S |
| 02116 | 279.I0 | S |
| 02118 | 280.I0 | S |
| 02120 | 281.I0 | S |
| 02122 | 282.I0 | S |
| 02124 | 283.I0 | S |
| 02126 | 284.I0 | S |
| 02128 | 285.I0 | S |
| 02130 | 286.I0 | S |
| 02132 | 287.I0 | S |
| 02134 | 288.I0 | S |
| 02136 | 289.I0 | S |
| 02138 | 290.I0 | S |
| 02140 | 291.I0 | S |
| 02142 | 292.I0 | S |
| 02144 | 293.I0 | S |
| 02146 | 294.I0 | S |
| 02148 | 295.I0 | S |
| 02150 | 296.I0 | S |
| 02152 | 297.I0 | S |
| 02154 | 298.I0 | S |
| 02156 | 299.I0 | S |
| 02158 | 300.I0 | S |

R6-17

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\* P1A \*  
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36 KK

38 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

39 RD

MUSKINGUM-CUNGE CHANNEL ROUTING

L 655. CHANNEL LENGTH  
S .0067 SLOPE  
N .015 CHANNEL ROUGHNESS COEFFICIENT  
CA .00 CONTRIBUTING AREA  
SHAPE CIRC CHANNEL SHAPE  
WD 2.00 BOTTOM WIDTH OR DIAMETER  
Z .00 SIDE SLOPE

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COMPUTED MUSKINGUM-CUNGE PARAMETERS

COMPUTATION TIME STEP

| ELEMENT | ALPHA | M    | DT<br>(MIN) | DX<br>(FT) | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|------|-------------|------------|---------------|--------------------------|----------------|------------------------------|
| MAIN    | 4.92  | 1.25 | 1.89        | 327.50     | 7.20          | 364.27                   | .52            | 5.78                         |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |  |      |        |     |  |
|------|------|------|------|--|------|--------|-----|--|
| MAIN | 4.92 | 1.25 | 2.00 |  | 7.19 | 364.00 | .52 |  |
|------|------|------|------|--|------|--------|-----|--|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6256E+00 EXCESS= .0000E+00 OUTFLOW= .6225E+00 BASIN STORAGE= .3074E-02 PERCENT ERROR= .0

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HYDROGRAPH AT STATION P1A

| EAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|-------------------|--------------|----------------------|-------|-------|---------|
|                   |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 7.                | 6.07         | (CFS)<br>1.          | 1.    | 1.    | 1.      |
|                   |              | (INCHES)<br>.524     | .524  | .524  | .524    |
|                   |              | (ΔC-FT)<br>1         | 1     | 1     | 1       |

R6-18

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\* \*  
\* C2 \*  
\* \*  
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40 KK

42 KO

OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
IPLOT 1 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

43 BA

SUBBASIN CHARACTERISTICS

TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

44 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

| HYDRO-35 |        |        | TP-40 |      |      |       | TP-49 |       |       |       |        |
|----------|--------|--------|-------|------|------|-------|-------|-------|-------|-------|--------|
| 5-MIN    | 15-MIN | 60-MIN | 2-HR  | 3-HR | 6-HR | 12-HR | 24-HR | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
| .39      | .76    | 1.34   | 1.40  | 1.44 | 1.56 | 1.69  | 2.01  | .00   | .00   | .00   | .00    |

STORM AREA = .00

45 LS

SCS LOSS RATE

STRTL .08 INITIAL ABSTRACTION  
CRVNBR 96.00 CURVE NUMBER  
RTIMP .00 PERCENT IMPERVIOUS AREA

46 UD

SCS DIMENSIONLESS UNITGRAPH

TLAG .61 LAG

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UNIT HYDROGRAPH  
93 END-OF-PERIOD ORDINATES

|    |    |    |    |    |    |    |    |    |    |
|----|----|----|----|----|----|----|----|----|----|
| 0. | 0. | 0. | 0. | 1. | 1. | 1. | 1. | 2. | 2. |
| 2. | 3. | 3. | 3. | 4. | 4. | 4. | 4. | 4. | 4. |
| 4. | 4. | 3. | 3. | 3. | 3. | 3. | 3. | 2. | 2. |
| 2. | 2. | 2. | 1. | 1. | 1. | 1. | 1. | 1. | 1. |
| 1. | 1. | 1. | 1. | 1. | 1. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |

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RG-19



TOTAL RAINFALL = 1.65, TOTAL LOSS = .41, TOTAL EXCESS = 1.24

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 4.                 | 5.63         | (CFS) 1.             | 0.    | 0.    | 0.      |
|                    |              | (INCHES) 1.225       | 1.225 | 1.225 | 1.225   |
|                    |              | (AC-FT) 0.           | 0.    | 0.    | 0.      |

CUMULATIVE AREA = .00 SQ MI

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47 KK \* C2 \*

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49 KO OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 3    | PRINT CONTROL                   |
| IPLOT  | 1    | PLOT CONTROL                    |
| QSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

50 HC HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION C2

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 9.                 | 6.03         | (CFS) 2.             | 1.    | 1.    | 1.      |
|                    |              | (INCHES) .650        | .650  | .650  | .650    |
|                    |              | (AC-FT) 1.           | 1.    | 1.    | 1.      |

CUMULATIVE AREA = .03 SQ MI

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51 KK \* C13 \*

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53 KO OUTPUT CONTROL VARIABLES

|       |   |               |
|-------|---|---------------|
| IPRNT | 3 | PRINT CONTROL |
| IPLOT | 1 | PLOT CONTROL  |

R6-20

IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 TOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

SUBBASIN RUNOFF DATA

54 BA SUBBASIN CHARACTERISTICS  
 TAREA .02 SUBBASIN AREA

PRECIPITATION DATA

55 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM  
 ..... HYDRO-35 ..... TP-40 ..... TP-49 .....  
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY  
 .39 .76 1.34 1.40 1.44 1.56 1.69 2.01 .00 .00 .00 .00

STORM AREA = .02

56 LS SCS LOSS RATE  
 STRTL .08 INITIAL ABSTRACTION  
 CRVNBR 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

57 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .47 LAG

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UNIT HYDROGRAPH  
73 END-OF-PERIOD ORDINATES

|     |     |     |     |     |     |     |     |     |     |
|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| 0.  | 1.  | 2.  | 3.  | 5.  | 7.  | 9.  | 11. | 14. | 16. |
| 18. | 19. | 20. | 20. | 20. | 20. | 19. | 19. | 18. | 16. |
| 15. | 14. | 12. | 11. | 9.  | 8.  | 7.  | 7.  | 6.  | 5.  |
| 5.  | 4.  | 4.  | 3.  | 3.  | 3.  | 2.  | 2.  | 2.  | 2.  |
| 2.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  | 1.  |
| 1.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  | 0.  |
| 0.  | 0.  | 0.  |     |     |     |     |     |     |     |

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HYDROGRAPH AT STATION C13

TOTAL RAINFALL = 1.65, TOTAL LOSS = .41, TOTAL EXCESS = 1.24

| PEAK FLOW (CFS) | TIME (HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|-----------------|-----------|----------------------|-------|-------|---------|
|                 |           | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 18.             | 5.50      | (CFS) 3.             | 2.    | 2.    | 2.      |
|                 |           | (INCHES) 1.228       | 1.229 | 1.229 | 1.229   |
|                 |           | (AC-FT) 1.           | 1.    | 1.    | 1.      |

CUMULATIVE AREA = .02 SQ MI

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\* \*  
\* 012 \*

R6-21

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60 KO OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

61 HC HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION C12

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 22.                | 5.53         | (CFS) 5.             | 3.    | 3.    | 3.      |
|                    |              | (INCHES) .899        | .900  | .900  | .900    |
|                    |              | (AC-FT) 2.           | 2.    | 2.    | 2.      |

CUMULATIVE AREA = .05 SQ MI

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62 KK

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 \* P2A \*  
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64 KO OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

65 RD MUSKINGUM-CUNGE CHANNEL ROUTING

L 277. CHANNEL LENGTH  
 S .0054 SLOPE  
 N .015 CHANNEL ROUGHNESS COEFFICIENT  
 CA .00 CONTRIBUTING AREA  
 SHAPE CIRC CHANNEL SHAPE  
 WD 3.00 BOTTOM WIDTH OR DIAMETER  
 Z .00 SIDE SLOPE

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R6-22

| ELEMENT | ALPHA | COMPUTATION TIME STEP |             |            | PEAK<br>(CFS) | TIME TO<br>PEAK<br>(MIN) | VOLUME<br>(IN) | MAXIMUM<br>CELERITY<br>(FPS) |
|---------|-------|-----------------------|-------------|------------|---------------|--------------------------|----------------|------------------------------|
|         |       | M                     | DT<br>(MIN) | DX<br>(FT) |               |                          |                |                              |
| MAIN    | 4.73  | 1.25                  | .66         | 277.00     | 21.55         | 331.76                   | 6.97           |                              |

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

|      |      |      |      |  |       |        |     |
|------|------|------|------|--|-------|--------|-----|
| MAIN | 4.73 | 1.25 | 2.00 |  | 21.55 | 332.00 | .90 |
|------|------|------|------|--|-------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2294E+01 EXCESS= .0000E+00 OUTFLOW= .2293E+01 BASIN STORAGE= .1864E-02 PERCENT ERROR= .0

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HYDROGRAPH AT STATION P2A

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
| 22.                | 5.53         | (CFS) 5.             | 3.    | 3.    | 3.      |
|                    |              | (INCHES) .899        | .899  | .899  | .899    |
|                    |              | (AC-FT) 2.           | 2.    | 2.    | 2.      |

CUMULATIVE AREA = .05 SQ MI

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\* \*  
66 KK \* E1 \*  
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68 KO OUTPUT CONTROL VARIABLES

|        |      |                                 |
|--------|------|---------------------------------|
| IPRNT  | 3    | PRINT CONTROL                   |
| IPLT   | 1    | PLOT CONTROL                    |
| QSCAL  | 0.   | HYDROGRAPH PLOT SCALE           |
| IPNCH  | 1    | PUNCH COMPUTED HYDROGRAPH       |
| IOUT   | 21   | SAVE HYDROGRAPH ON THIS UNIT    |
| ISAV1  | 1    | FIRST ORDINATE PUNCHED OR SAVED |
| ISAV2  | 300  | LAST ORDINATE PUNCHED OR SAVED  |
| TIMINT | .033 | TIME INTERVAL IN HOURS          |

SUBBASIN RUNOFF DATA

69 BA SUBBASIN CHARACTERISTICS

TAREA .00 SUBBASIN AREA

PRECIPITATION DATA

70 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

| HYDRO-35 |        |        | TP-40 |      |      |       | TP-49 |       |       |       |        |
|----------|--------|--------|-------|------|------|-------|-------|-------|-------|-------|--------|
| 5-MIN    | 15-MIN | 60-MIN | 2-HR  | 3-HR | 6-HR | 12-HR | 24-HR | 2-DAY | 4-DAY | 7-DAY | 10-DAY |
| .39      | .76    | 1.34   | 1.40  | 1.44 | 1.56 | 1.69  | 2.01  | .00   | .00   | .00   | .00    |

R6-23

71 LS SCS LOSS RATE  
 STRTL .08 INITIAL ABSTRACTION  
 CRVNBR 96.00 CURVE NUMBER  
 RTIMP .00 PERCENT IMPERVIOUS AREA

72 UD SCS DIMENSIONLESS UNITGRAPH  
 TLAG .13 LAG

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UNIT HYDROGRAPH  
 21 END-OF-PERIOD ORDINATES

|    |    |    |    |    |    |    |    |    |    |
|----|----|----|----|----|----|----|----|----|----|
| 0. | 1. | 2. | 3. | 3. | 2. | 2. | 1. | 1. | 1. |
| 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. | 0. |
| 0. |    |    |    |    |    |    |    |    |    |

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HYDROGRAPH AT STATION E1

TOTAL RAINFALL = 1.65, TOTAL LOSS = .41, TOTAL EXCESS = 1.24

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |       |
|--------------------|--------------|----------------------|-------|-------|---------|-------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |       |
| 2.                 | 5.13         | 0.                   | 0.    | 0.    | 0.      |       |
|                    |              | (INCHES)             | 1.235 | 1.238 | 1.238   | 1.238 |
|                    |              | (AC-FT)              | 0.    | 0.    | 0.      | 0.    |

CUMULATIVE AREA = .00 SQ MI

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73 KK \*                    \*  
 \*                    E1 \*  
 \*                    \*  
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75 KO OUTPUT CONTROL VARIABLES  
 IPRNT 3 PRINT CONTROL  
 IPLOT 1 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

76 HC HYDROGRAPH COMBINATION  
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION E1

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |
|--------------------|--------------|----------------------|-------|-------|---------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |
|                    |              |                      |       |       |         |

R6-24

(INCHES) .905 .906 .906 .906  
 (AC-FT) 2. 2. 2. 2.

CUMULATIVE AREA = .05 SQ MI

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 \* \*  
 \* RES5 \*  
 \* \*  
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77 KK

79 KO

OUTPUT CONTROL VARIABLES

IPRNT 1 PRINT CONTROL  
 IPLOT 2 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 1 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT .033 TIME INTERVAL IN HOURS

HYDROGRAPH ROUTING DATA

80 RS

STORAGE ROUTING

NSTPS 1 NUMBER OF SUBREACHES  
 ITYP ELEV TYPE OF INITIAL CONDITION  
 RSVRIC 11.24 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

81 SV

STORAGE .0 .1 .2 .3 .5 .8

82 SE

ELEVATION 12.00 13.00 14.00 15.00 16.00 17.00

83 SL

LOW-LEVEL OUTLET

ELEVL 12.24 ELEVATION AT CENTER OF OUTLET  
 CAREA .06 CROSS-SECTIONAL AREA  
 COQL .60 COEFFICIENT  
 EXPL .50 EXPONENT OF HEAD

84 SS

SPILLWAY

CREL 16.00 SPILLWAY CREST ELEVATION  
 SPWID 12.29 SPILLWAY WIDTH  
 COQW 2.70 WEIR COEFFICIENT  
 EXPW 1.50 EXPONENT OF HEAD

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COMPUTED OUTFLOW-ELEVATION DATA

|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| OUTFLOW   | .00   | .00   | .12   | .13   | .15   | .18   | .21   | .27   | .37   | .58   |
| ELEVATION | 12.00 | 12.24 | 12.39 | 12.43 | 12.49 | 12.58 | 12.74 | 13.05 | 13.75 | 16.00 |
| OUTFLOW   | .62   | .86   | 1.50  | 2.73  | 4.77  | 7.80  | 12.02 | 17.64 | 24.85 | 33.84 |
| ELEVATION | 16.01 | 16.04 | 16.09 | 16.16 | 16.25 | 16.36 | 16.49 | 16.64 | 16.81 | 17.00 |

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

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|           |       |       |       |       |       |       |       |       |       |       |
|-----------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| OUTFLOW   | .00   | .00   | .12   | .13   | .15   | .18   | .21   | .26   | .27   | .37   |
| ELEVATION | 12.00 | 12.24 | 12.39 | 12.43 | 12.49 | 12.58 | 12.74 | 13.00 | 13.05 | 13.75 |
| STORAGE   | .16   | .31   | .52   | .52   | .53   | .55   | .56   | .59   | .62   | .65   |
| OUTFLOW   | .40   | .50   | .58   | .62   | .86   | 1.50  | 2.73  | 4.77  | 7.80  | 12.02 |
| ELEVATION | 14.00 | 15.00 | 16.00 | 16.01 | 16.04 | 16.09 | 16.16 | 16.25 | 16.36 | 16.49 |
| STORAGE   | .69   | .74   | .79   |       |       |       |       |       |       |       |
| OUTFLOW   | 17.64 | 24.85 | 33.84 |       |       |       |       |       |       |       |
| ELEVATION | 16.64 | 16.81 | 17.00 |       |       |       |       |       |       |       |

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HYDROGRAPH AT STATION RES5

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| * * * * * |     |      |     |         |         |       |   |    |     |      |     |         | * * * * * |       |   |    |     |      |     |         |         |       |
|-----------|-----|------|-----|---------|---------|-------|---|----|-----|------|-----|---------|-----------|-------|---|----|-----|------|-----|---------|---------|-------|
| DA        | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE | * | DA | MON | HRMN | ORD | OUTFLOW | STORAGE   | STAGE | * | DA | MON | HRMN | ORD | OUTFLOW | STORAGE | STAGE |
| 30        | JUN | 1200 | 1   | 0.      | .0      | 12.0  | * | 30 | JUN | 1520 | 101 | 0.      | .0        | 12.0  | * | 30 | JUN | 1840 | 201 | 6.      | .6      | 16.3  |
| 30        | JUN | 1202 | 2   | 0.      | .0      | 12.0  | * | 30 | JUN | 1522 | 102 | 0.      | .0        | 12.0  | * | 30 | JUN | 1842 | 202 | 6.      | .6      | 16.3  |
| 30        | JUN | 1204 | 3   | 0.      | .0      | 12.0  | * | 30 | JUN | 1524 | 103 | 0.      | .0        | 12.0  | * | 30 | JUN | 1844 | 203 | 5.      | .6      | 16.3  |
| 30        | JUN | 1206 | 4   | 0.      | .0      | 12.0  | * | 30 | JUN | 1526 | 104 | 0.      | .0        | 12.0  | * | 30 | JUN | 1846 | 204 | 5.      | .6      | 16.3  |
| 30        | JUN | 1208 | 5   | 0.      | .0      | 12.0  | * | 30 | JUN | 1528 | 105 | 0.      | .0        | 12.0  | * | 30 | JUN | 1848 | 205 | 5.      | .6      | 16.2  |
| 30        | JUN | 1210 | 6   | 0.      | .0      | 12.0  | * | 30 | JUN | 1530 | 106 | 0.      | .0        | 12.0  | * | 30 | JUN | 1850 | 206 | 4.      | .6      | 16.2  |
| 30        | JUN | 1212 | 7   | 0.      | .0      | 12.0  | * | 30 | JUN | 1532 | 107 | 0.      | .0        | 12.0  | * | 30 | JUN | 1852 | 207 | 4.      | .6      | 16.2  |
| 30        | JUN | 1214 | 8   | 0.      | .0      | 12.0  | * | 30 | JUN | 1534 | 108 | 0.      | .0        | 12.0  | * | 30 | JUN | 1854 | 208 | 4.      | .6      | 16.2  |
| 30        | JUN | 1216 | 9   | 0.      | .0      | 12.0  | * | 30 | JUN | 1536 | 109 | 0.      | .0        | 12.0  | * | 30 | JUN | 1856 | 209 | 4.      | .6      | 16.2  |
| 30        | JUN | 1218 | 10  | 0.      | .0      | 12.0  | * | 30 | JUN | 1538 | 110 | 0.      | .0        | 12.0  | * | 30 | JUN | 1858 | 210 | 4.      | .6      | 16.2  |
| 30        | JUN | 1220 | 11  | 0.      | .0      | 12.0  | * | 30 | JUN | 1540 | 111 | 0.      | .0        | 12.0  | * | 30 | JUN | 1900 | 211 | 3.      | .6      | 16.2  |
| 30        | JUN | 1222 | 12  | 0.      | .0      | 12.0  | * | 30 | JUN | 1542 | 112 | 0.      | .0        | 12.0  | * | 30 | JUN | 1902 | 212 | 3.      | .6      | 16.2  |
| 30        | JUN | 1224 | 13  | 0.      | .0      | 12.0  | * | 30 | JUN | 1544 | 113 | 0.      | .0        | 12.0  | * | 30 | JUN | 1904 | 213 | 3.      | .6      | 16.2  |
| 30        | JUN | 1226 | 14  | 0.      | .0      | 12.0  | * | 30 | JUN | 1546 | 114 | 0.      | .0        | 12.0  | * | 30 | JUN | 1906 | 214 | 3.      | .6      | 16.2  |
| 30        | JUN | 1228 | 15  | 0.      | .0      | 12.0  | * | 30 | JUN | 1548 | 115 | 0.      | .0        | 12.0  | * | 30 | JUN | 1908 | 215 | 3.      | .6      | 16.2  |
| 30        | JUN | 1230 | 16  | 0.      | .0      | 12.0  | * | 30 | JUN | 1550 | 116 | 0.      | .0        | 12.0  | * | 30 | JUN | 1910 | 216 | 3.      | .6      | 16.2  |
| 30        | JUN | 1232 | 17  | 0.      | .0      | 12.0  | * | 30 | JUN | 1552 | 117 | 0.      | .0        | 12.0  | * | 30 | JUN | 1912 | 217 | 3.      | .6      | 16.2  |
| 30        | JUN | 1234 | 18  | 0.      | .0      | 12.0  | * | 30 | JUN | 1554 | 118 | 0.      | .0        | 12.0  | * | 30 | JUN | 1914 | 218 | 3.      | .6      | 16.2  |
| 30        | JUN | 1236 | 19  | 0.      | .0      | 12.0  | * | 30 | JUN | 1556 | 119 | 0.      | .0        | 12.0  | * | 30 | JUN | 1916 | 219 | 2.      | .6      | 16.1  |
| 30        | JUN | 1238 | 20  | 0.      | .0      | 12.0  | * | 30 | JUN | 1558 | 120 | 0.      | .0        | 12.0  | * | 30 | JUN | 1918 | 220 | 2.      | .6      | 16.1  |
| 30        | JUN | 1240 | 21  | 0.      | .0      | 12.0  | * | 30 | JUN | 1600 | 121 | 0.      | .0        | 12.0  | * | 30 | JUN | 1920 | 221 | 2.      | .6      | 16.1  |
| 30        | JUN | 1242 | 22  | 0.      | .0      | 12.0  | * | 30 | JUN | 1602 | 122 | 0.      | .0        | 12.0  | * | 30 | JUN | 1922 | 222 | 2.      | .6      | 16.1  |
| 30        | JUN | 1244 | 23  | 0.      | .0      | 12.0  | * | 30 | JUN | 1604 | 123 | 0.      | .0        | 12.0  | * | 30 | JUN | 1924 | 223 | 2.      | .6      | 16.1  |
| 30        | JUN | 1246 | 24  | 0.      | .0      | 12.0  | * | 30 | JUN | 1606 | 124 | 0.      | .0        | 12.0  | * | 30 | JUN | 1926 | 224 | 2.      | .6      | 16.1  |
| 30        | JUN | 1248 | 25  | 0.      | .0      | 12.0  | * | 30 | JUN | 1608 | 125 | 0.      | .0        | 12.0  | * | 30 | JUN | 1928 | 225 | 2.      | .6      | 16.1  |
| 30        | JUN | 1250 | 26  | 0.      | .0      | 12.0  | * | 30 | JUN | 1610 | 126 | 0.      | .0        | 12.1  | * | 30 | JUN | 1930 | 226 | 2.      | .6      | 16.1  |
| 30        | JUN | 1252 | 27  | 0.      | .0      | 12.0  | * | 30 | JUN | 1612 | 127 | 0.      | .0        | 12.1  | * | 30 | JUN | 1932 | 227 | 2.      | .6      | 16.1  |
| 30        | JUN | 1254 | 28  | 0.      | .0      | 12.0  | * | 30 | JUN | 1614 | 128 | 0.      | .0        | 12.1  | * | 30 | JUN | 1934 | 228 | 2.      | .6      | 16.1  |
| 30        | JUN | 1256 | 29  | 0.      | .0      | 12.0  | * | 30 | JUN | 1616 | 129 | 0.      | .0        | 12.1  | * | 30 | JUN | 1936 | 229 | 2.      | .6      | 16.1  |
| 30        | JUN | 1258 | 30  | 0.      | .0      | 12.0  | * | 30 | JUN | 1618 | 130 | 0.      | .0        | 12.1  | * | 30 | JUN | 1938 | 230 | 2.      | .5      | 16.1  |
| 30        | JUN | 1300 | 31  | 0.      | .0      | 12.0  | * | 30 | JUN | 1620 | 131 | 0.      | .0        | 12.1  | * | 30 | JUN | 1940 | 231 | 2.      | .5      | 16.1  |
| 30        | JUN | 1302 | 32  | 0.      | .0      | 12.0  | * | 30 | JUN | 1622 | 132 | 0.      | .0        | 12.1  | * | 30 | JUN | 1942 | 232 | 2.      | .5      | 16.1  |
| 30        | JUN | 1304 | 33  | 0.      | .0      | 12.0  | * | 30 | JUN | 1624 | 133 | 0.      | .0        | 12.1  | * | 30 | JUN | 1944 | 233 | 2.      | .5      | 16.1  |
| 30        | JUN | 1306 | 34  | 0.      | .0      | 12.0  | * | 30 | JUN | 1626 | 134 | 0.      | .0        | 12.1  | * | 30 | JUN | 1946 | 234 | 2.      | .5      | 16.1  |
| 30        | JUN | 1308 | 35  | 0.      | .0      | 12.0  | * | 30 | JUN | 1628 | 135 | 0.      | .0        | 12.1  | * | 30 | JUN | 1948 | 235 | 2.      | .5      | 16.1  |
| 30        | JUN | 1310 | 36  | 0.      | .0      | 12.0  | * | 30 | JUN | 1630 | 136 | 0.      | .0        | 12.1  | * | 30 | JUN | 1950 | 236 | 2.      | .5      | 16.1  |
| 30        | JUN | 1312 | 37  | 0.      | .0      | 12.0  | * | 30 | JUN | 1632 | 137 | 0.      | .0        | 12.1  | * | 30 | JUN | 1952 | 237 | 2.      | .5      | 16.1  |
| 30        | JUN | 1314 | 38  | 0.      | .0      | 12.0  | * | 30 | JUN | 1634 | 138 | 0.      | .0        | 12.1  | * | 30 | JUN | 1954 | 238 | 2.      | .5      | 16.1  |
| 30        | JUN | 1316 | 39  | 0.      | .0      | 12.0  | * | 30 | JUN | 1636 | 139 | 0.      | .0        | 12.2  | * | 30 | JUN | 1956 | 239 | 2.      | .5      | 16.1  |
| 30        | JUN | 1318 | 40  | 0.      | .0      | 12.0  | * | 30 | JUN | 1638 | 140 | 0.      | .0        | 12.2  | * | 30 | JUN | 1958 | 240 | 1.      | .5      | 16.1  |
| 30        | JUN | 1320 | 41  | 0.      | .0      | 12.0  | * | 30 | JUN | 1640 | 141 | 0.      | .0        | 12.2  | * | 30 | JUN | 2000 | 241 | 1.      | .5      | 16.1  |
| 30        | JUN | 1322 | 42  | 0.      | .0      | 12.0  | * | 30 | JUN | 1642 | 142 | 0.      | .0        | 12.2  | * | 30 | JUN | 2002 | 242 | 1.      | .5      | 16.1  |
| 30        | JUN | 1324 | 43  | 0.      | .0      | 12.0  | * | 30 | JUN | 1644 | 143 | 0.      | .0        | 12.2  | * | 30 | JUN | 2004 | 243 | 1.      | .5      | 16.1  |
| 30        | JUN | 1326 | 44  | 0.      | .0      | 12.0  | * | 30 | JUN | 1646 | 144 | 0.      | .0        | 12.3  | * | 30 | JUN | 2006 | 244 | 1.      | .5      | 16.1  |
| 30        | JUN | 1328 | 45  | 0.      | .0      | 12.0  | * | 30 | JUN | 1648 | 145 | 0.      | .0        | 12.3  | * | 30 | JUN | 2008 | 245 | 1.      | .5      | 16.1  |
| 30        | JUN | 1330 | 46  | 0.      | .0      | 12.0  | * | 30 | JUN | 1650 | 146 | 0.      | .0        | 12.4  | * | 30 | JUN | 2010 | 246 | 1.      | .5      | 16.1  |

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|             |     |    |    |      |   |             |     |     |    |      |   |             |     |    |    |      |
|-------------|-----|----|----|------|---|-------------|-----|-----|----|------|---|-------------|-----|----|----|------|
| 30 JUN 1334 | 48  | 0. | .0 | 12.0 | * | 30 JUN 1654 | 148 | 0.  | .0 | 12.5 | * | 30 JUN 2014 | 248 | 1. | .5 | 16.1 |
| 30 JUN 1336 | 49  | 0. | .0 | 12.0 | * | 30 JUN 1656 | 149 | 0.  | .1 | 12.6 | * | 30 JUN 2016 | 249 | 1. | .5 | 16.1 |
| 30 JUN 1338 | 50  | 0. | .0 | 12.0 | * | 30 JUN 1658 | 150 | 0.  | .1 | 12.8 | * | 30 JUN 2018 | 250 | 1. | .5 | 16.1 |
| 30 JUN 1340 | 51  | 0. | .0 | 12.0 | * | 30 JUN 1700 | 151 | 0.  | .1 | 12.9 | * | 30 JUN 2020 | 251 | 1. | .5 | 16.1 |
| 30 JUN 1342 | 52  | 0. | .0 | 12.0 | * | 30 JUN 1702 | 152 | 0.  | .1 | 13.1 | * | 30 JUN 2022 | 252 | 1. | .5 | 16.1 |
| 30 JUN 1344 | 53  | 0. | .0 | 12.0 | * | 30 JUN 1704 | 153 | 0.  | .1 | 13.2 | * | 30 JUN 2024 | 253 | 1. | .5 | 16.1 |
| 30 JUN 1346 | 54  | 0. | .0 | 12.0 | * | 30 JUN 1706 | 154 | 0.  | .1 | 13.4 | * | 30 JUN 2026 | 254 | 1. | .5 | 16.1 |
| 30 JUN 1348 | 55  | 0. | .0 | 12.0 | * | 30 JUN 1708 | 155 | 0.  | .1 | 13.6 | * | 30 JUN 2028 | 255 | 1. | .5 | 16.1 |
| 30 JUN 1350 | 56  | 0. | .0 | 12.0 | * | 30 JUN 1710 | 156 | 0.  | .2 | 13.9 | * | 30 JUN 2030 | 256 | 1. | .5 | 16.1 |
| 30 JUN 1352 | 57  | 0. | .0 | 12.0 | * | 30 JUN 1712 | 157 | 0.  | .2 | 14.1 | * | 30 JUN 2032 | 257 | 1. | .5 | 16.1 |
| 30 JUN 1354 | 58  | 0. | .0 | 12.0 | * | 30 JUN 1714 | 158 | 0.  | .2 | 14.4 | * | 30 JUN 2034 | 258 | 1. | .5 | 16.1 |
| 30 JUN 1356 | 59  | 0. | .0 | 12.0 | * | 30 JUN 1716 | 159 | 0.  | .3 | 14.6 | * | 30 JUN 2036 | 259 | 1. | .5 | 16.1 |
| 30 JUN 1358 | 60  | 0. | .0 | 12.0 | * | 30 JUN 1718 | 160 | 0.  | .3 | 14.9 | * | 30 JUN 2038 | 260 | 1. | .5 | 16.1 |
| 30 JUN 1400 | 61  | 0. | .0 | 12.0 | * | 30 JUN 1720 | 161 | 1.  | .3 | 15.1 | * | 30 JUN 2040 | 261 | 1. | .5 | 16.1 |
| 30 JUN 1402 | 62  | 0. | .0 | 12.0 | * | 30 JUN 1722 | 162 | 1.  | .4 | 15.4 | * | 30 JUN 2042 | 262 | 1. | .5 | 16.1 |
| 30 JUN 1404 | 63  | 0. | .0 | 12.0 | * | 30 JUN 1724 | 163 | 1.  | .4 | 15.6 | * | 30 JUN 2044 | 263 | 1. | .5 | 16.1 |
| 30 JUN 1406 | 64  | 0. | .0 | 12.0 | * | 30 JUN 1726 | 164 | 1.  | .5 | 15.9 | * | 30 JUN 2046 | 264 | 1. | .5 | 16.1 |
| 30 JUN 1408 | 65  | 0. | .0 | 12.0 | * | 30 JUN 1728 | 165 | 2.  | .6 | 16.1 | * | 30 JUN 2048 | 265 | 1. | .5 | 16.1 |
| 30 JUN 1410 | 66  | 0. | .0 | 12.0 | * | 30 JUN 1730 | 166 | 6.  | .6 | 16.3 | * | 30 JUN 2050 | 266 | 1. | .5 | 16.1 |
| 30 JUN 1412 | 67  | 0. | .0 | 12.0 | * | 30 JUN 1732 | 167 | 11. | .6 | 16.4 | * | 30 JUN 2052 | 267 | 1. | .5 | 16.1 |
| 30 JUN 1414 | 68  | 0. | .0 | 12.0 | * | 30 JUN 1734 | 168 | 14. | .7 | 16.5 | * | 30 JUN 2054 | 268 | 1. | .5 | 16.1 |
| 30 JUN 1416 | 69  | 0. | .0 | 12.0 | * | 30 JUN 1736 | 169 | 16. | .7 | 16.6 | * | 30 JUN 2056 | 269 | 1. | .5 | 16.1 |
| 30 JUN 1418 | 70  | 0. | .0 | 12.0 | * | 30 JUN 1738 | 170 | 18. | .7 | 16.6 | * | 30 JUN 2058 | 270 | 1. | .5 | 16.1 |
| 30 JUN 1420 | 71  | 0. | .0 | 12.0 | * | 30 JUN 1740 | 171 | 19. | .7 | 16.7 | * | 30 JUN 2100 | 271 | 1. | .5 | 16.1 |
| 30 JUN 1422 | 72  | 0. | .0 | 12.0 | * | 30 JUN 1742 | 172 | 19. | .7 | 16.7 | * | 30 JUN 2102 | 272 | 1. | .5 | 16.1 |
| 30 JUN 1424 | 73  | 0. | .0 | 12.0 | * | 30 JUN 1744 | 173 | 19. | .7 | 16.7 | * | 30 JUN 2104 | 273 | 1. | .5 | 16.1 |
| 30 JUN 1426 | 74  | 0. | .0 | 12.0 | * | 30 JUN 1746 | 174 | 19. | .7 | 16.7 | * | 30 JUN 2106 | 274 | 1. | .5 | 16.1 |
| 30 JUN 1428 | 75  | 0. | .0 | 12.0 | * | 30 JUN 1748 | 175 | 18. | .7 | 16.6 | * | 30 JUN 2108 | 275 | 1. | .5 | 16.1 |
| 30 JUN 1430 | 76  | 0. | .0 | 12.0 | * | 30 JUN 1750 | 176 | 17. | .7 | 16.6 | * | 30 JUN 2110 | 276 | 1. | .5 | 16.1 |
| 30 JUN 1432 | 77  | 0. | .0 | 12.0 | * | 30 JUN 1752 | 177 | 17. | .7 | 16.6 | * | 30 JUN 2112 | 277 | 1. | .5 | 16.1 |
| 30 JUN 1434 | 78  | 0. | .0 | 12.0 | * | 30 JUN 1754 | 178 | 16. | .7 | 16.6 | * | 30 JUN 2114 | 278 | 1. | .5 | 16.1 |
| 30 JUN 1436 | 79  | 0. | .0 | 12.0 | * | 30 JUN 1756 | 179 | 16. | .7 | 16.6 | * | 30 JUN 2116 | 279 | 1. | .5 | 16.1 |
| 30 JUN 1438 | 80  | 0. | .0 | 12.0 | * | 30 JUN 1758 | 180 | 16. | .7 | 16.6 | * | 30 JUN 2118 | 280 | 1. | .5 | 16.1 |
| 30 JUN 1440 | 81  | 0. | .0 | 12.0 | * | 30 JUN 1800 | 181 | 16. | .7 | 16.6 | * | 30 JUN 2120 | 281 | 1. | .5 | 16.1 |
| 30 JUN 1442 | 82  | 0. | .0 | 12.0 | * | 30 JUN 1802 | 182 | 16. | .7 | 16.6 | * | 30 JUN 2122 | 282 | 1. | .5 | 16.1 |
| 30 JUN 1444 | 83  | 0. | .0 | 12.0 | * | 30 JUN 1804 | 183 | 16. | .7 | 16.6 | * | 30 JUN 2124 | 283 | 1. | .5 | 16.1 |
| 30 JUN 1446 | 84  | 0. | .0 | 12.0 | * | 30 JUN 1806 | 184 | 16. | .7 | 16.6 | * | 30 JUN 2126 | 284 | 1. | .5 | 16.1 |
| 30 JUN 1448 | 85  | 0. | .0 | 12.0 | * | 30 JUN 1808 | 185 | 15. | .7 | 16.6 | * | 30 JUN 2128 | 285 | 1. | .5 | 16.1 |
| 30 JUN 1450 | 86  | 0. | .0 | 12.0 | * | 30 JUN 1810 | 186 | 15. | .7 | 16.6 | * | 30 JUN 2130 | 286 | 1. | .5 | 16.1 |
| 30 JUN 1452 | 87  | 0. | .0 | 12.0 | * | 30 JUN 1812 | 187 | 14. | .7 | 16.5 | * | 30 JUN 2132 | 287 | 1. | .5 | 16.1 |
| 30 JUN 1454 | 88  | 0. | .0 | 12.0 | * | 30 JUN 1814 | 188 | 13. | .7 | 16.5 | * | 30 JUN 2134 | 288 | 1. | .5 | 16.1 |
| 30 JUN 1456 | 89  | 0. | .0 | 12.0 | * | 30 JUN 1816 | 189 | 13. | .7 | 16.5 | * | 30 JUN 2136 | 289 | 1. | .5 | 16.1 |
| 30 JUN 1458 | 90  | 0. | .0 | 12.0 | * | 30 JUN 1818 | 190 | 12. | .7 | 16.5 | * | 30 JUN 2138 | 290 | 1. | .5 | 16.1 |
| 30 JUN 1500 | 91  | 0. | .0 | 12.0 | * | 30 JUN 1820 | 191 | 11. | .6 | 16.5 | * | 30 JUN 2140 | 291 | 1. | .5 | 16.1 |
| 30 JUN 1502 | 92  | 0. | .0 | 12.0 | * | 30 JUN 1822 | 192 | 11. | .6 | 16.4 | * | 30 JUN 2142 | 292 | 1. | .5 | 16.1 |
| 30 JUN 1504 | 93  | 0. | .0 | 12.0 | * | 30 JUN 1824 | 193 | 10. | .6 | 16.4 | * | 30 JUN 2144 | 293 | 1. | .5 | 16.1 |
| 30 JUN 1506 | 94  | 0. | .0 | 12.0 | * | 30 JUN 1826 | 194 | 9.  | .6 | 16.4 | * | 30 JUN 2146 | 294 | 1. | .5 | 16.1 |
| 30 JUN 1508 | 95  | 0. | .0 | 12.0 | * | 30 JUN 1828 | 195 | 9.  | .6 | 16.4 | * | 30 JUN 2148 | 295 | 1. | .5 | 16.1 |
| 30 JUN 1510 | 96  | 0. | .0 | 12.0 | * | 30 JUN 1830 | 196 | 8.  | .6 | 16.4 | * | 30 JUN 2150 | 296 | 1. | .5 | 16.1 |
| 30 JUN 1512 | 97  | 0. | .0 | 12.0 | * | 30 JUN 1832 | 197 | 8.  | .6 | 16.4 | * | 30 JUN 2152 | 297 | 1. | .5 | 16.1 |
| 30 JUN 1514 | 98  | 0. | .0 | 12.0 | * | 30 JUN 1834 | 198 | 7.  | .6 | 16.3 | * | 30 JUN 2154 | 298 | 1. | .5 | 16.1 |
| 30 JUN 1516 | 99  | 0. | .0 | 12.0 | * | 30 JUN 1836 | 199 | 7.  | .6 | 16.3 | * | 30 JUN 2156 | 299 | 1. | .5 | 16.1 |
| 30 JUN 1518 | 100 | 0. | .0 | 12.0 | * | 30 JUN 1838 | 200 | 6.  | .6 | 16.3 | * | 30 JUN 2158 | 300 | 1. | .5 | 16.1 |

\* \* \*

\*\*\*\*\*

| PEAK FLOW<br>(CFS) | TIME<br>(HR) | MAXIMUM AVERAGE FLOW |       |       |         |      |
|--------------------|--------------|----------------------|-------|-------|---------|------|
|                    |              | 6-HR                 | 24-HR | 72-HR | 9.97-HR |      |
| 19.                | 5.70         | 4.                   | 2.    | 2.    | 2.      |      |
|                    |              | (INCHES)             | .706  | .706  | .706    | .706 |
|                    |              | (AC-FT)              | 2.    | 2.    | 2.      | 2.   |

| PEAK STORAGE<br>(AC-FT) | TIME<br>(HR) | MAXIMUM AVERAGE STORAGE |       |       |         |
|-------------------------|--------------|-------------------------|-------|-------|---------|
|                         |              | 6-HR                    | 24-HR | 72-HR | 9.97-HR |
| 1.                      | 5.70         | 0                       | 0.    | 0     | 0       |

R6-27



| PEAK STAGE<br>(FEET) | TIME<br>(HR) | MAXIMUM AVERAGE STAGE |       |       |         |
|----------------------|--------------|-----------------------|-------|-------|---------|
|                      |              | 6-HR                  | 24-HR | 72-HR | 9.97-HR |
| 16.68                | 5.70         | 15.38                 | 14.04 | 14.04 | 14.04   |

CUMULATIVE AREA = .05 SQ MI

R6-28

(I) INFLOW, (O) OUTFLOW

|            | 0. | 4. | 8. | 12. | 16. | 20. | 24. | 0.          | 0. | 0. | 0. | 0. | 0. |
|------------|----|----|----|-----|-----|-----|-----|-------------|----|----|----|----|----|
|            |    |    |    |     |     |     |     | (S) STORAGE |    |    |    |    |    |
|            | .0 | .0 | .0 | .0  | .0  | .0  | .0  | .2          | .4 | .6 | .8 | .0 | .0 |
| 301200 1I  |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1202 2I    |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1204 3I    |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301206 4I  |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1208 5I    |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1210 6I    |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301212 7I  |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1214 8I    |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1216 9I    |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301218 10I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1220 11I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1222 12I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301224 13I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1226 14I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1228 15I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301230 16I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1232 17I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1234 18I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301236 19I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1238 20I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1240 21I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301242 22I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1244 23I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1246 24I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301248 25I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1250 26I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1252 27I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301254 28I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1256 29I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1258 30I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301300 31I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1302 32I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1304 33I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301306 34I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1308 35I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1310 36I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301312 37I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1314 38I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1316 39I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301318 40I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1320 41I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1322 42I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301324 43I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1326 44I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1328 45I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301330 46I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1332 47I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1334 48I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301336 49I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1338 50I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1340 51I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301342 52I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1344 53I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1346 54I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 301348 55I |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1350 56I   |    |    |    |     |     |     |     |             |    |    |    |    |    |
| 1352 57I   |    |    |    |     |     |     |     |             |    |    |    |    |    |

R6-29





|        |      |     |   |
|--------|------|-----|---|
| 1820   | 191. | I 0 | S |
| 1822   | 192. | I 0 | S |
| 301824 | 193. | I 0 | S |
| 1826   | 194. | I 0 | S |
| 1828   | 195. | I 0 | S |
| 301830 | 196. | I 0 | S |
| 1832   | 197. | I 0 | S |
| 1834   | 198. | I 0 | S |
| 301836 | 199. | I 0 | S |
| 1838   | 200. | I 0 | S |
| 1840   | 201. | I 0 | S |
| 301842 | 202. | I 0 | S |
| 1844   | 203. | I 0 | S |
| 1846   | 204. | I 0 | S |
| 301848 | 205. | I 0 | S |
| 1850   | 206. | I 0 | S |
| 1852   | 207. | I 0 | S |
| 301854 | 208. | I 0 | S |
| 1856   | 209. | I 0 | S |
| 1858   | 210. | I 0 | S |
| 301900 | 211. | I 0 | S |
| 1902   | 212. | I 0 | S |
| 1904   | 213. | I 0 | S |
| 301906 | 214. | I 0 | S |
| 1908   | 215. | I 0 | S |
| 1910   | 216. | I 0 | S |
| 301912 | 217. | I 0 | S |
| 1914   | 218. | I 0 | S |
| 1916   | 219. | I 0 | S |
| 301918 | 220. | I 0 | S |
| 1920   | 221. | I 0 | S |
| 1922   | 222. | I 0 | S |
| 301924 | 223. | I 0 | S |
| 1926   | 224. | I 0 | S |
| 1928   | 225. | I 0 | S |
| 301930 | 226. | I 0 | S |
| 1932   | 227. | I 0 | S |
| 1934   | 228. | I 0 | S |
| 301936 | 229. | I 0 | S |
| 1938   | 230. | I 0 | S |
| 1940   | 231. | I 0 | S |
| 301942 | 232. | I 0 | S |
| 1944   | 233. | I 0 | S |
| 1946   | 234. | I 0 | S |
| 301948 | 235. | I 0 | S |
| 1950   | 236. | I 0 | S |
| 1952   | 237. | I 0 | S |
| 301954 | 238. | I 0 | S |
| 1956   | 239. | I 0 | S |
| 1958   | 240. | I 0 | S |
| 302000 | 241. | I 0 | S |
| 2002   | 242. | I 0 | S |
| 2004   | 243. | I 0 | S |
| 302006 | 244. | I 0 | S |
| 2008   | 245. | I 0 | S |
| 2010   | 246. | I 0 | S |
| 302012 | 247. | I 0 | S |
| 2014   | 248. | I 0 | S |
| 2016   | 249. | I 0 | S |
| 302018 | 250. | I 0 | S |
| 2020   | 251. | I 0 | S |
| 2022   | 252. | I 0 | S |
| 302024 | 253. | I 0 | S |
| 2026   | 254. | I 0 | S |
| 2028   | 255. | I 0 | S |

R.6-32



RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

| OPERATION     | STATION | PEAK FLOW | TIME OF PEAK | AVERAGE FLOW FOR MAXIMUM PERIOD |         |         | BASIN AREA | MAXIMUM STAGE | TIME OF MAX STAGE |
|---------------|---------|-----------|--------------|---------------------------------|---------|---------|------------|---------------|-------------------|
|               |         |           |              | 6-HOUR                          | 24-HOUR | 72-HOUR |            |               |                   |
| HYDROGRAPH AT | D123    | 16.       | 5.50         | 2.                              | 1.      | 1.      | .02        |               |                   |
| ROUTED TO     | CH1A    | 16.       | 5.50         | 2.                              | 1.      | 1.      | .02        |               |                   |
| HYDROGRAPH AT | D4      | 3.        | 5.50         | 0.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | D4      | 19.       | 5.50         | 3.                              | 2.      | 2.      | .02        |               |                   |
| ROUTED TO     | RES4    | 7.        | 6.03         | 1.                              | 1.      | 1.      | .02        | 23.84         | 6.03              |
| ROUTED TO     | P1A     | 7.        | 6.07         | 1.                              | 1.      | 1.      | .02        |               |                   |
| HYDROGRAPH AT | C2      | 4.        | 5.63         | 1.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | C2      | 9.        | 6.03         | 2.                              | 1.      | 1.      | .03        |               |                   |
| HYDROGRAPH AT | C13     | 18.       | 5.50         | 3.                              | 2.      | 2.      | .02        |               |                   |
| 2 COMBINED AT | C12     | 22.       | 5.53         | 5.                              | 3.      | 3.      | .05        |               |                   |
| ROUTED TO     | P2A     | 22.       | 5.53         | 5.                              | 3.      | 3.      | .05        |               |                   |
| HYDROGRAPH AT | E1      | 2.        | 5.13         | 0.                              | 0.      | 0.      | .00        |               |                   |
| 2 COMBINED AT | E1      | 22.       | 5.53         | 5.                              | 3.      | 3.      | .05        |               |                   |
| ROUTED TO     | RES5    | 19.       | 5.70         | 4.                              | 2.      | 2.      | .05        | 16.68         | 5.70              |

R6-34

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO  
COMPUTATION INTERVAL

| ISTAQ | ELEMENT | DT    | PEAK  | TIME TO | VOLUME | DT    | PEAK  | TIME TO | VOLUME |
|-------|---------|-------|-------|---------|--------|-------|-------|---------|--------|
|       |         | (MIN) | (CFS) | PEAK    | (IN)   | (MIN) | (CFS) | PEAK    | (IN)   |
| CH1A  | MANE    | 1.70  | 16.37 | 330.76  | 1.23   | 2.00  | 16.35 | 330.00  | 1.23   |

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1226E+01 EXCESS= .0000E+00 OUTFLOW= .1225E+01 BASIN STORAGE= .1911E-02 PERCENT ERROR= .0

|     |      |      |      |        |     |      |      |        |     |
|-----|------|------|------|--------|-----|------|------|--------|-----|
| PIA | MANE | 1.89 | 7.20 | 364.27 | .52 | 2.00 | 7.19 | 364.00 | .52 |
|-----|------|------|------|--------|-----|------|------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6256E+00 EXCESS= .0000E+00 OUTFLOW= .6225E+00 BASIN STORAGE= .3074E-02 PERCENT ERROR= .0

|     |      |     |       |        |     |      |       |        |     |
|-----|------|-----|-------|--------|-----|------|-------|--------|-----|
| P2A | MANE | .66 | 21.55 | 331.76 | .90 | 2.00 | 21.55 | 332.00 | .90 |
|-----|------|-----|-------|--------|-----|------|-------|--------|-----|

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2294E+01 EXCESS= .0000E+00 OUTFLOW= .2293E+01 BASIN STORAGE= .1864E-02 PERCENT ERROR= .0

\*\*\* NORMAL END OF HEC-1 \*\*\*

R6-35



**TRAFFIC STUDY**  
**FOR**  
**GRANDVIEW SUBDIVISION**

June 30, 1994

Original  
Do NOT Remove  
From Office

85  
94-2

Prepared for:  
Don dela Motte

Prepared by:  
**HART GROUP, PC**  
ENGINEERS DESIGNERS PLANNERS  
A DIVISION OF  
**LANDesign**  
200 North 6th St.  
Suite 102  
Grand Junction, Colorado 81501

I certify that this study has been prepared by me or under my supervision.

Prepared by: \_\_\_\_\_

Philip M. Hart P.E  
State of Colorado, #19346

## **TABLE OF CONTENTS**

- A. INTRODUCTION**
- B. ROADWAY IMPROVEMENTS**
- C. TRIP GENERATION AND DESIGN HOUR VOLUMES**
- D. TRAFFIC VOLUMES**
- E. CAPACITY ANALYSIS**
- F. TRAFFIC ACCIDENT REPORTS**
- G. TRAFFIC SIGNAL WARRANTS ANALYSIS**
- H. CONCLUSIONS AND RECOMMENDATIONS**
  
- APPENDIX A**
  
- APPENDIX B**

## **A. INTRODUCTION**

### **1. Land Use**

Grandview Subdivision site is located 1/4 mile north of Patterson Road, east of 28 Road in the City of Grand Junction. The 64.8 acre site was formerly know as "Fox Estates."

The existing land use is agricultural and contains one single family dwelling and two out buildings. The topography of the site is relatively flat and gently slopes towards the southwest at a average rate of 1%. The historic use of the site has been crop production. The site has little vegetation except for a grove of cottonwood trees near the single family residence.

The proposed use of the site calls for the ultimate development of 200 single family building sites on the 64.8 acres. The resulting density is 3.1 dwelling units per acre. Lots range in size from 8,000 square feet to 12,500 square feet. The proposed zoning for the parcel is RSF-5.

The surrounding land uses of the site are primarily single family developments on moderately sized lots. Some acreage sized parcels adjoin the site to the north on 28 Road. Land to the south consists of a large parcel which currently has no development plans. Land to the west consists of single family residential lots. To the east the Matchett Village Development which is currently in the County review process. This development consists of mixed single family, multi-family and non-residential uses. There are no existing non-residential uses in the surrounding area. Figure 1 shows the site with the surrounding zoning and land uses.

### **2. Access**

Primary access to the site is gained from 28 Road runs north\south along the west side of the site. Patterson Road located 1/4 mile to the south serves as a major east/west

# SURROUNDING ZONING MAP

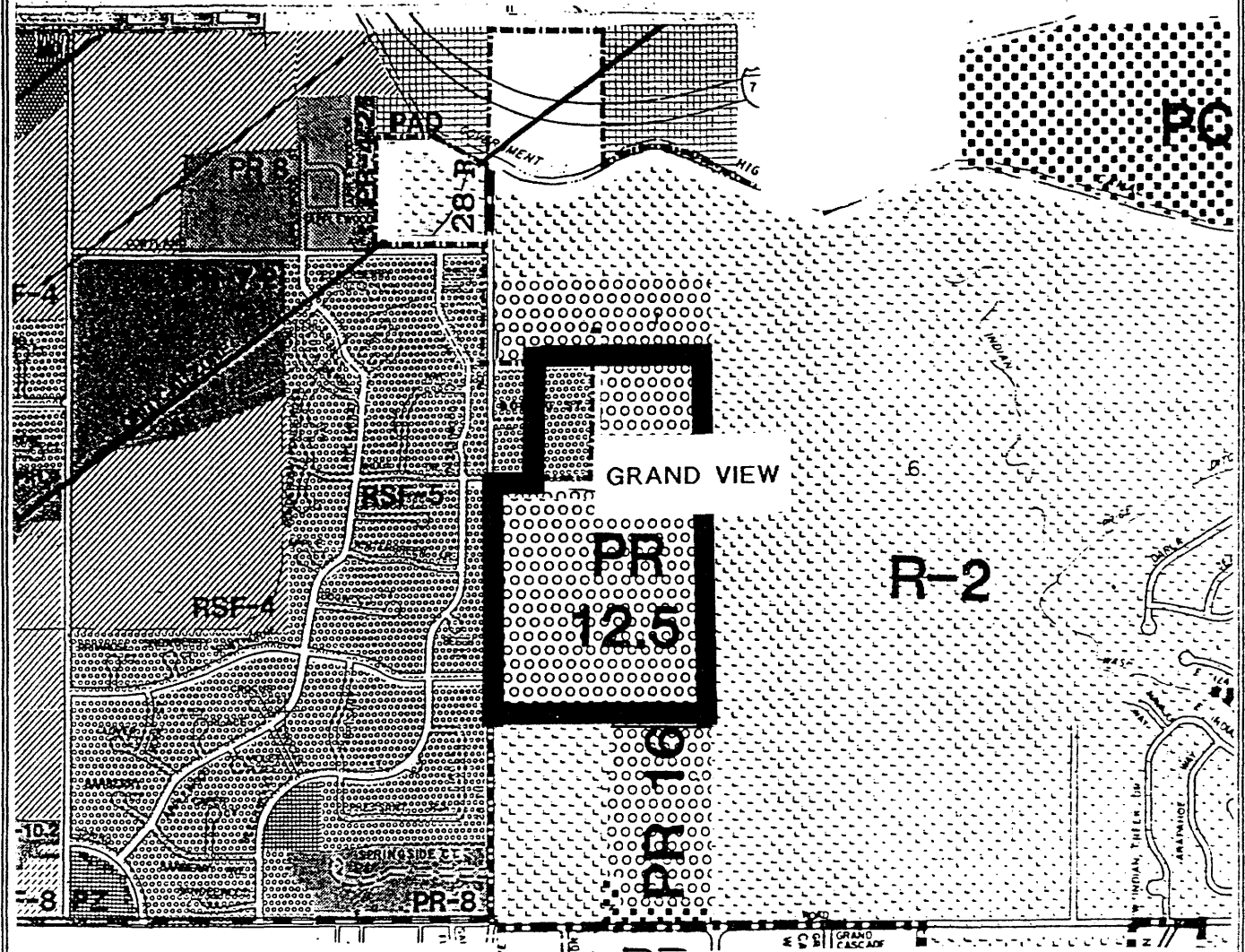
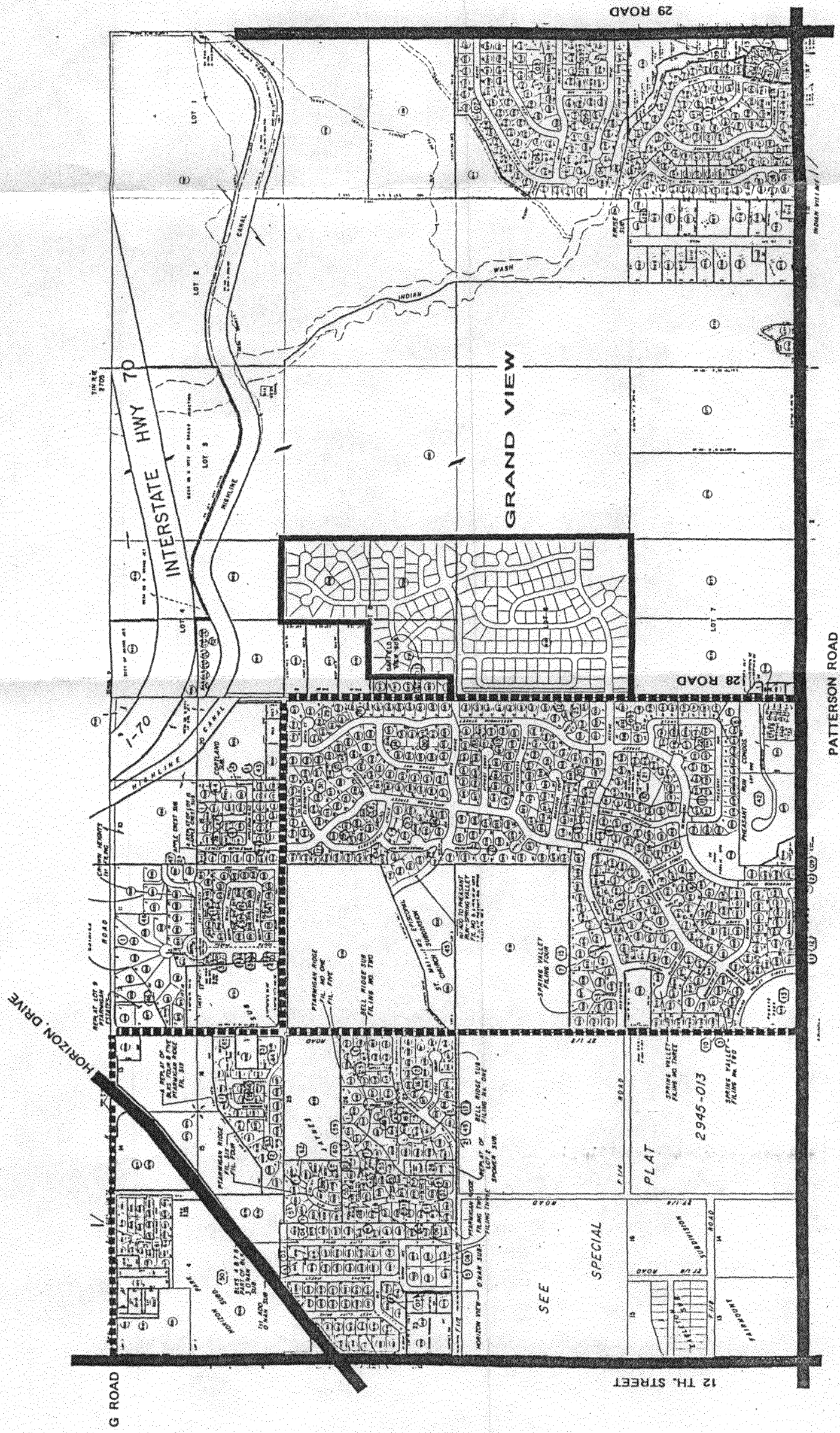


Figure 1  
Surrounding Land Use and Zoning

arterial roadway in Grand Junction and connects 28 Road to the Other access is gained from Ridge Drive, Hawthorn Avenue and Cortland Avenue. Figure 2 shown the current street configuration in the general area of the site. The intersections of 28 Road and Ridge Drive and 28 Road and Hawthorn Avenue will be studied for traffic impact in this report as well as a traffic signal warrants analysis at the intersection of 28 Road and Patterson Avenue. The existing and proposed street alignment for 28 Road and Ridge Drive, 28 Road and Hawthorn Avenue and Patterson Avenue and 28 Road are shown in Figure 3 , Figure 4 and Figure 5.

## **B. ROADWAY IMPROVEMENTS**

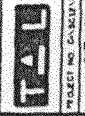
Improvements to the roadway system are to be made due to the proposed development. Additionally, improvements will be made to the east side of 28 Road by widening the pavement by 9' and adding a 2' vertical curb and gutter section where 28 Road is adjacent to the planned development. Currently, a curb and gutter section is located on the west side of 28 Road adjacent to the existing development. This improvement to 28 Road will provide curb and gutter on both sides of 28 Road. This addition will improve the drainage on 28 Road and provide a vertical barrier to traffic on 28 Road.



SCALE: 1"=400'

LOCATION MAP

GRAND VIEW SUBDIVISION

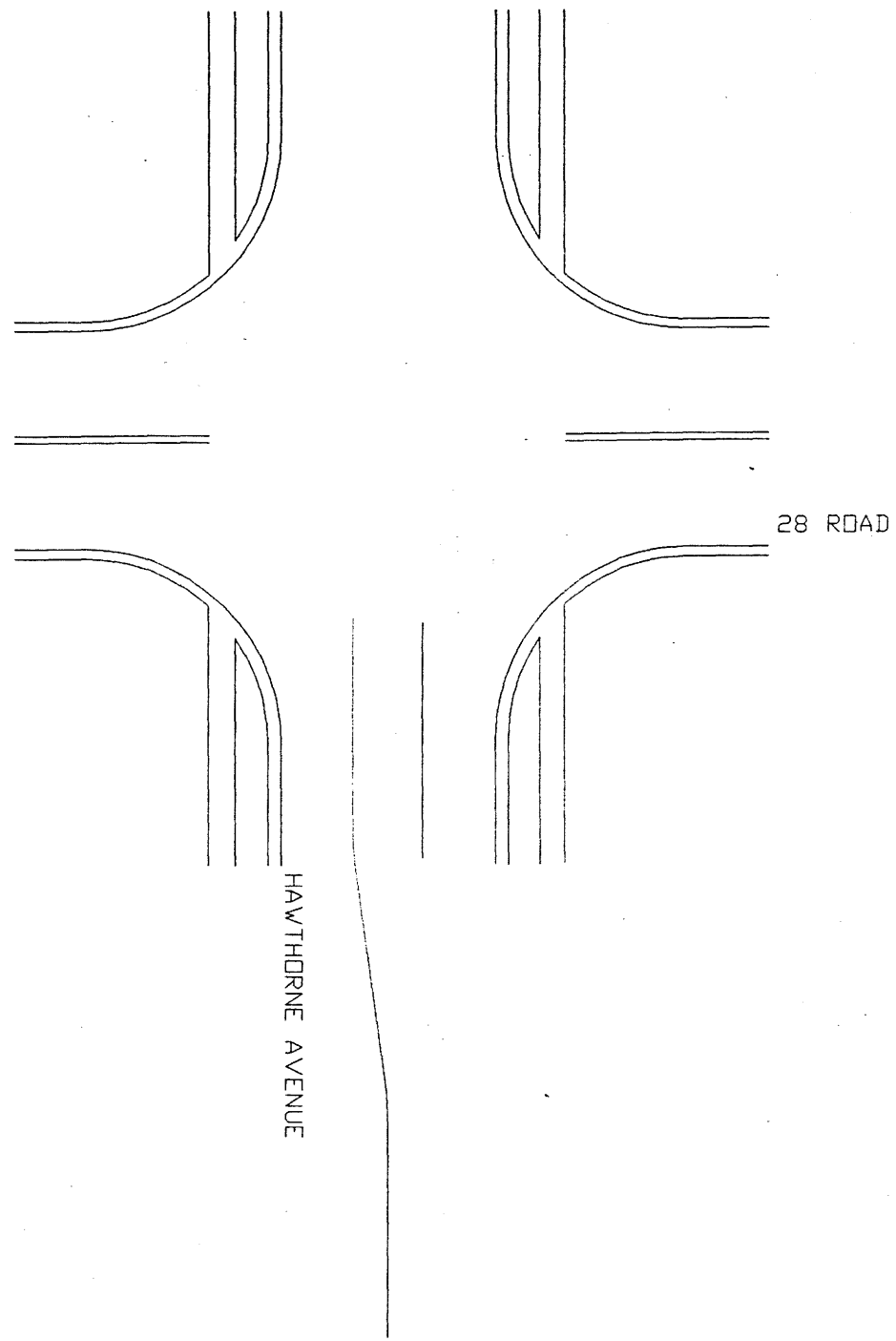


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PROJECT NO. 2003-0001-0003-001  
DATE: APR. 1994

|       |   |    |   |
|-------|---|----|---|
| SHEET | 1 | OF | 5 |
|-------|---|----|---|

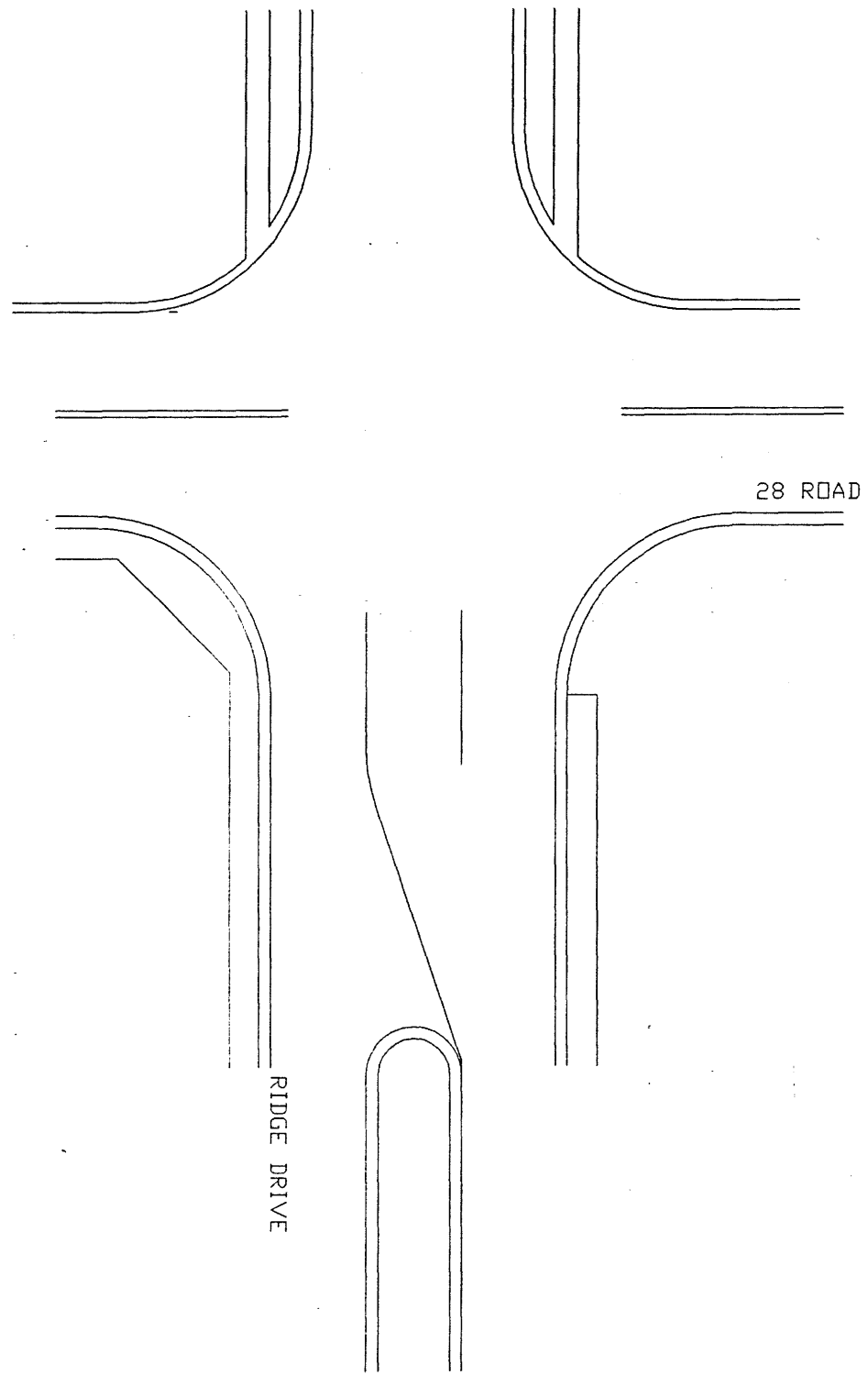
Figure 2  
Vicinity Street

Fig 2



**Figure 3**  
**28 Road and Hawthorn Avenue Intersection**





**Figure 4**  
**28 Road and Ridge Drive Intersection**

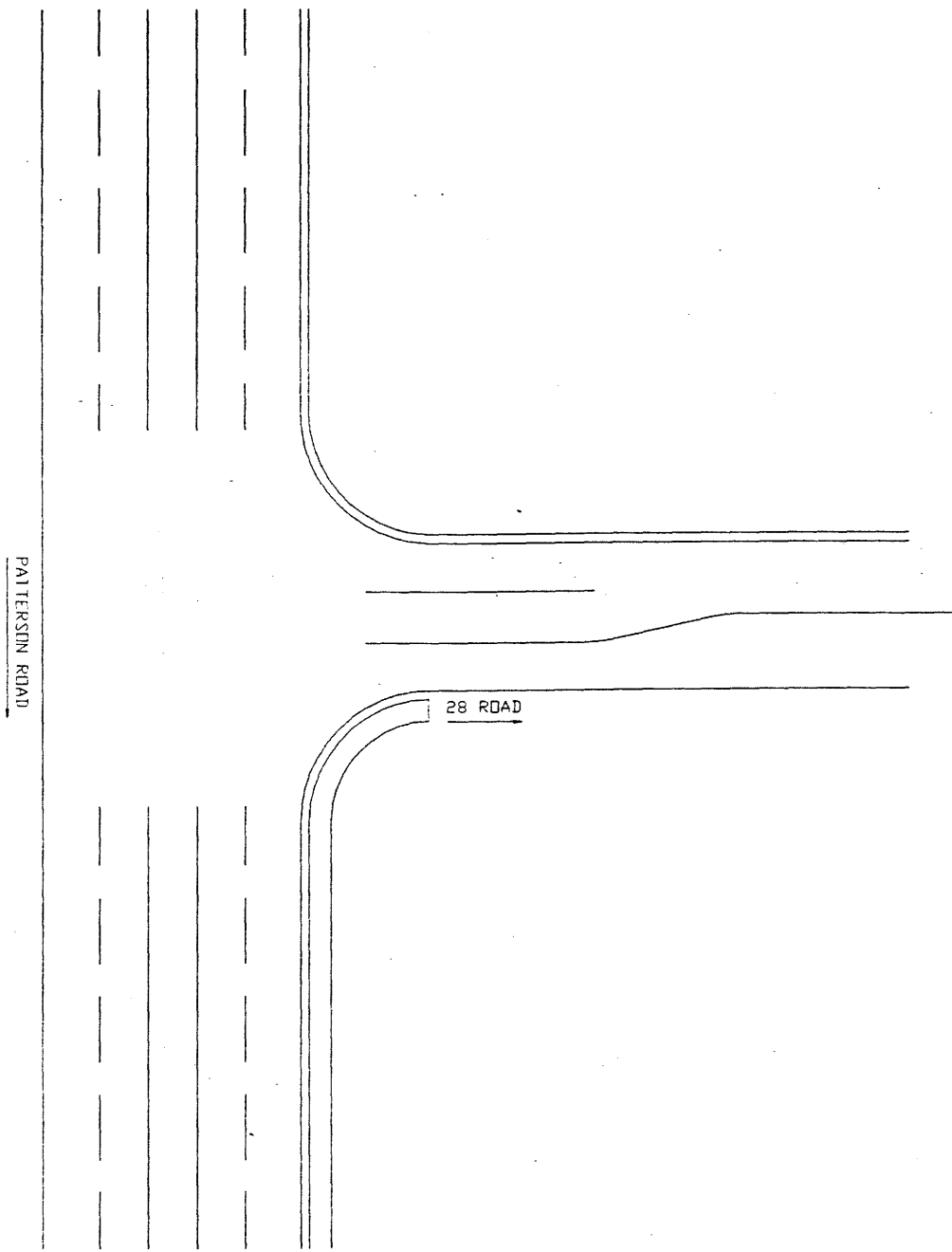


Figure 5  
Patterson Road and 28 Road Intersection

## C. TRIP GENERATION AND DESIGN HOUR VOLUMES

### 1. Trip Generation Rates

Trip generation rates are provided in Trip Generation , January 1991, Institute of Transportation Engineers. (see Appendix A)

Assumption is made that 1/2 of the daily trips generated by this development will use Ridge Drive and 1/2 will use Hawthorn Avenue. The resultant pattern provides 100 lots accessing 28 Road by Hawthorn Avenue and 1/2 accessing 28 Road by Ridge Drive.

**TRIP GENERATION RATE**

| Intersection          | # of Lots | Ave Day | Ave Day Trips | Peak Hour (am) | Peak Hour (am) Trips | Peak Hour (pm) | Peak Hour (pm) Trips |
|-----------------------|-----------|---------|---------------|----------------|----------------------|----------------|----------------------|
| Hawthorn\28 Road      | 100       | 9.55    | 955           | .76            | 76                   | 1.02           | 102                  |
| Ridge\28 Road         | 100       | 9.55    | 955           | .76            | 76                   | 1.02           | 102                  |
| 28 Road\<br>Patterson | 200       | 9.55    | 1910          | .76            | 152                  | 1.02           | 204                  |

**Table A**

### 2. Trip Distribution

Trip distribution on the new development was determined by available socioeconomic and demographic data for the influence area and by site visits to determine the current traffic flow and percentages of movements. Emphasis was given to fastest route to and from the traffic generators. Table B, C and D shown below represents the Trip Distribution by percentages for the new development.

**Trip Distribution for Hawthorn and 28 Road Intersection**

|  |      |
|--|------|
| To and From Hawthorn North via 28 Road | 2%   |
| To and From Hawthorn South via 28 Road | 94%  |
| To and From Hawthorn West via Hawthorn | 4%   |
|  | 100% |

**Table B**

**Trip Distribution for Ridge and 28 Road Intersection**

|                                     |      |
|-------------------------------------|------|
| To and From Ridge North via 28 Road | 2%   |
| To and From Ridge South via 28 Road | 94%  |
| To and From Ridge West via Ridge    | 4%   |
|                                     | 100% |

**Table C**

**Trip Distribution for 28 Road and Patterson Road Intersection**

|   |     |
|---|-----|
| To and From 28 Road(at new devel.) West via Patterson | 29% |
| To and From 28 Road(at new devel.) East via Patterson | 71% |
|   |     |
|   |     |

**Table D**

Note: These percentages determined by on site traffic count for Ave AM and Ave PM Peak Hour.

### 3. Trip Assignment

The following Figure 6 shows the new development traffic assignment in the development vicinity based on the Trip Distribution discussed above.

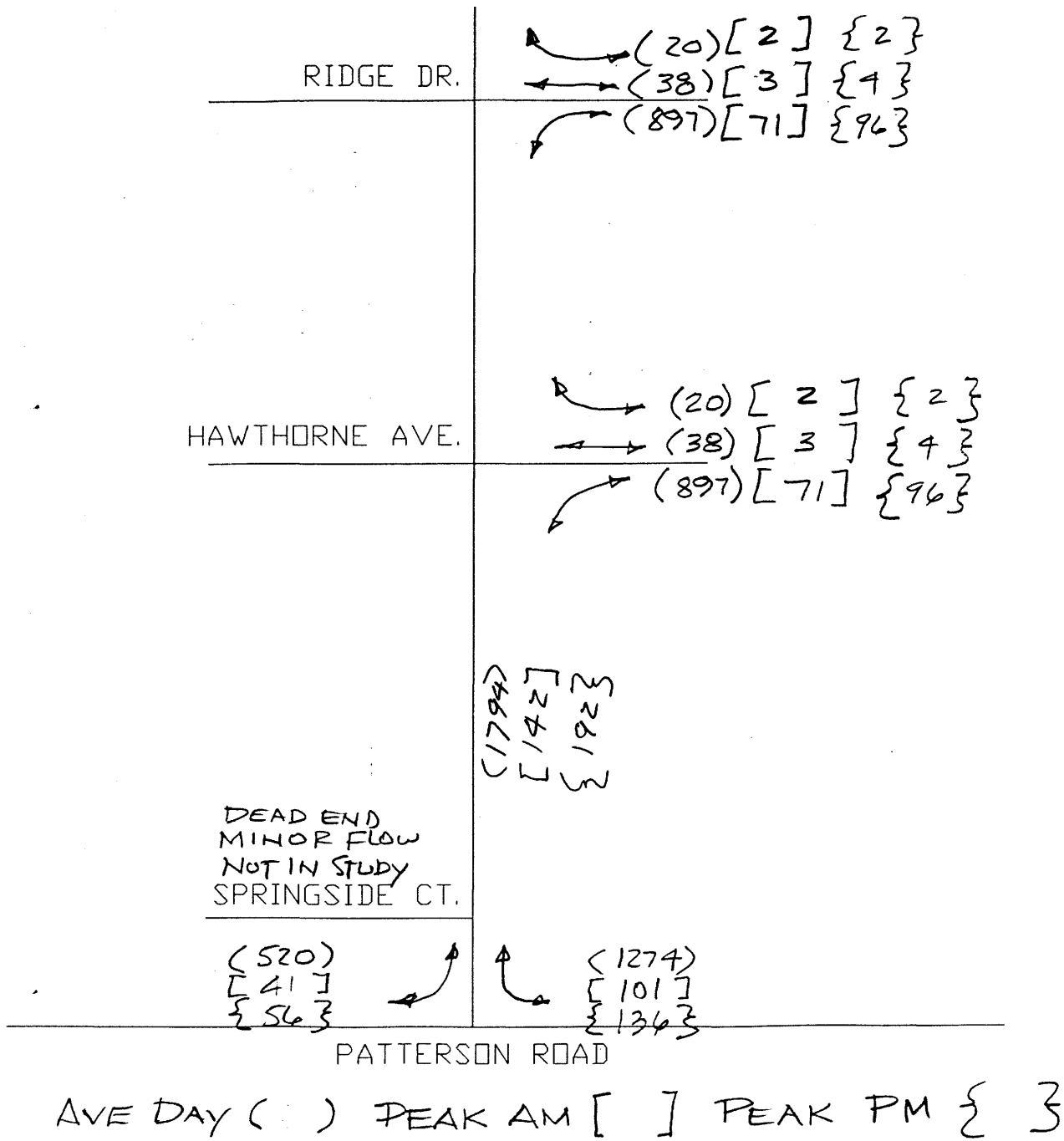


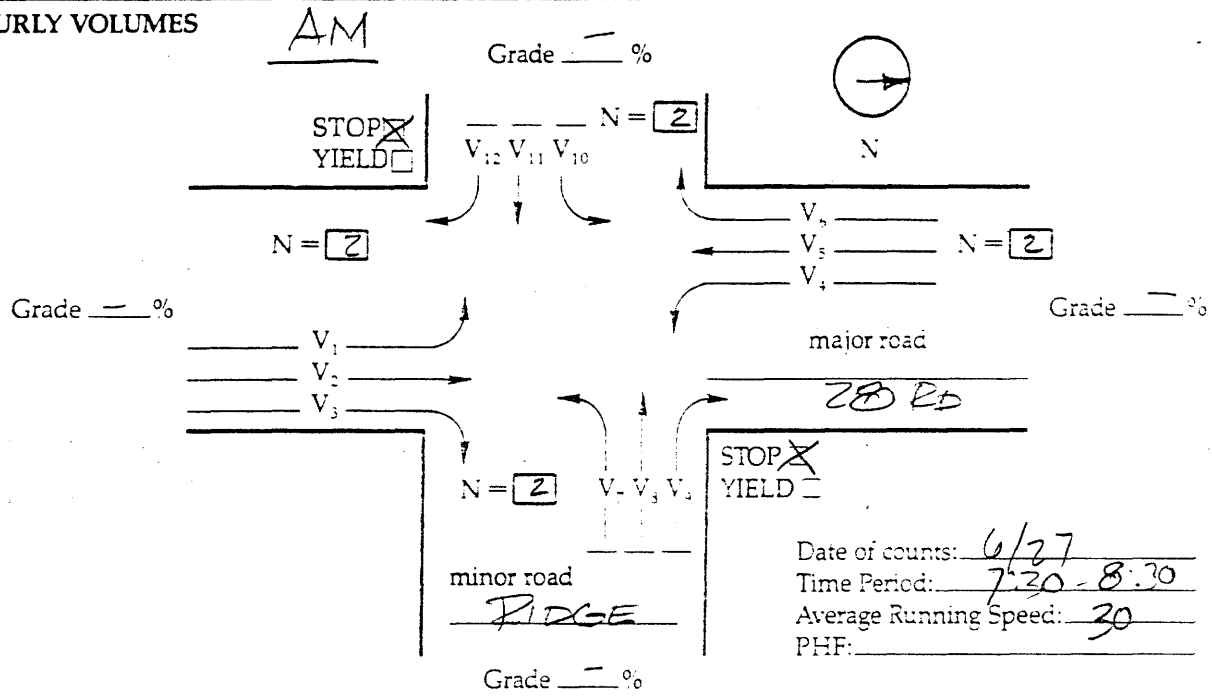
Figure 6

Trip Assignment for the New Development

WORKSHEET FOR FOUR-LEG INTERSECTIONS

Location: RIDGE E 28 RD Name: P. HART

HOURLY VOLUMES

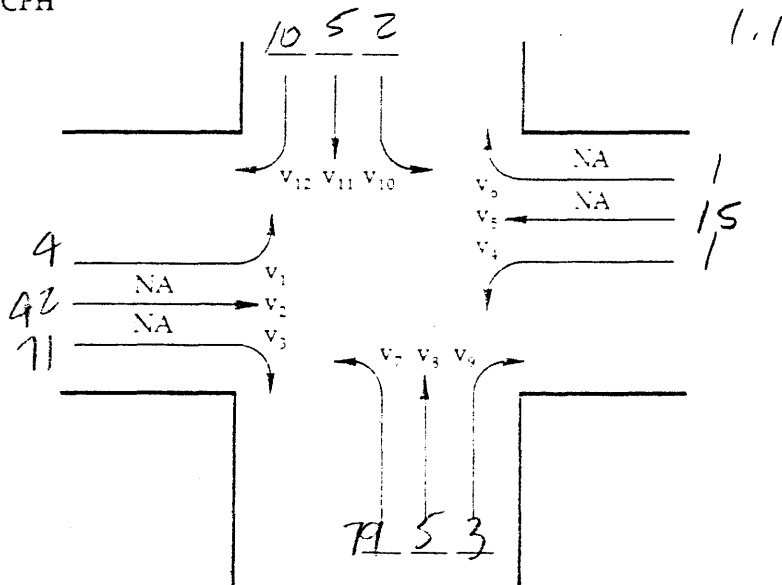


Date of counts: 6/27  
 Time Period: 7:30-8:30  
 Average Running Speed: 30  
 PHF: \_\_\_\_\_

VOLUME ADJUSTMENTS

| Movement No.                | 1 | 2  | 3 | 4 | 5  | 6 | 7  | 8 | 9 | 10 | 11 | 12 |
|-----------------------------|---|----|---|---|----|---|----|---|---|----|----|----|
| Volume (vph)                | 4 | 42 | 7 | 1 | 15 | 1 | 71 | 4 | 2 | 1  | 4  | 9  |
| Vol. (pcph), see Table 10-1 | 5 |    |   |   | 1  |   | 79 | 5 | 3 | 2  | 5  | 10 |

VOLUMES IN PCPH



WORKSHEET FOR FOUR-LEG INTERSECTIONS

| STEP 1: RT From Minor Street   | $\Gamma V_9$   | $J V_{12}$   |
|--|--|--|
| Conflicting Flows, $V_c$<br>Critical Gap, $T_c$ (Tab. 10-2)<br>Potential Capacity, $c_p$ (Fig. 10-3)<br>Percent of $c_p$ Utilized<br>Impedance Factor, $P$ (Fig. 10-5)<br>Actual Capacity, $c_m$ | $1/2 V_3 + V_2 = V_{c9}$<br>$35.5 + 42 = 77.5$ vph<br>$5.5$ (sec)<br>$c_{p9} = 1000^+$ pcph<br>$(v_9/c_{p9}) \times 100 = .3$ %<br>$P_9 = .99$<br>$c_{m9} = c_{p9} = 1000$ pcph  | $1/2 V_6 + V_5 = V_{c12}$<br>$0 + 15 = 15$ vph<br>$5.5$ (sec)<br>$c_{p12} = 1000^+$ pcph<br>$(v_{12}/c_{p12}) \times 100 = 1.0$ %<br>$P_{12} = .99$<br>$c_{m12} = c_{p12} = 1000$ pcph   |
| STEP 2: LT From Major Street   | $\Gamma V_4$   | $J V_1$  |
| Conflicting Flows, $V_c$<br>Critical Gap, $T_c$ (Tab. 10-2)<br>Potential Capacity, $c_p$ (Fig. 10-3)<br>Percent of $c_p$ Utilized<br>Impedance Factor, $P$ (Fig. 10-5)<br>Actual Capacity, $c_m$ | $V_3 + V_2 = V_{c4}$<br>$71 + 42 = 113$ vph<br>$5.5$ (sec)<br>$c_{p4} = 970$ pcph<br>$(v_4/c_{p4}) \times 100 = .1$ %<br>$P_4 = .99$<br>$c_{m4} = c_{p4} = 970$ pcph   | $V_6 + V_5 = V_{c1}$<br>$1 + 15 = 16$ vph<br>$5.5$ (sec)<br>$c_{p1} = 1000$ pcph<br>$(v_1/c_{p1}) \times 100 = .5$ %<br>$P_1 = .99$<br>$c_{m1} = c_{p1} = 1000$ pcph   |
| STEP 3: TH From Minor Street   | $\uparrow V_8$   | $\downarrow V_{11}$  |
| Conflicting Flows, $V_c$<br>Critical Gap, $T_c$ (Tab. 10-2)<br>Potential Capacity, $c_p$ (Fig. 10-3)<br>Percent of $c_p$ Utilized<br>Impedance Factor, $P$ (Fig. 10-5)<br>Actual Capacity, $c_m$ | $1/2 V_3 + V_2 + V_1 + V_6 + V_5 + V_4 = V_{c8}$<br>$36 + 41 + 4 + 1 + 15 + 1 = 98$ vph<br>$6.0$ (sec)<br>$c_{p8} = 900$ pcph<br>$(v_8/c_{p8}) \times 100 = .56$ %<br>$P_8 = .99$<br>$c_{m8} = c_{p8} \times P_1 \times P_4$<br>$882 = 900 \times .99 \times .99$ (pcph) | $1/2 V_6 + V_5 - V_4 + V_3 + V_2 + V_1 = V_{c11}$<br>$1 + 15 - 1 + 71 + 42 - 4 = 134$ vph<br>$6.0$ (sec)<br>$c_{p11} = 860$ pcph<br>$(v_{11}/c_{p11}) \times 100 = .58$ %<br>$P_{11} = .99$<br>$c_{m11} = c_{p11} \times P_1 \times P_4$<br>$843 = 860 \times .99 \times .99$ (pcph) |
| STEP 4: LT From Minor Street   | $\Gamma V_7$   | $J V_{10}$   |
| Conflicting Flows, $V_c$<br>Critical Gap, $T_c$ (Tab. 10-2)<br>Potential Capacity, $c_p$ (Fig. 10-3)<br>Actual Capacity, $c_m$   | $V_{c8}$ (step 3) + $V_{11} + V_{12} = V_{c7}$<br>$98 + 5 + 10 = 113$ vph<br>$6.5$ (sec)<br>$c_{p7} = 800$ pcph<br>$c_{m7} = c_{p7} \times P_1 \times P_4 \times P_{11} \times P_{12}$<br>$768 = 800 \times .99 \times .99 \times .99 \times .99$ (pcph)                 | $V_{c11}$ (step 3) - $V_4 + V_3 = V_{c10}$<br>$134 - 5 - 3 = 126$ vph<br>$6.5$ (sec)<br>$c_{p10} = 752$ pcph<br>$c_{m10} = c_{p10} \times P_1 \times P_4 \times P_8 \times P_{11}$<br>$720 = 752 \times .99 \times .99 \times .99 \times .99$ (pcph)                                 |

## WORKSHEET FOR FOUR-LEG INTERSECTIONS

Page 3

## SHARED-LANE CAPACITY

$$C_{SH} = \frac{v_i + v_j}{(v_i/c_{mi}) + (v_j/c_{mj})} \quad \text{where 2 movements share a lane}$$

$$C_{SH} = \frac{v_r + v_i + v_k}{(v_i/c_{mi}) + (v_j/c_{mj}) + (v_k/c_{mk})} \quad \text{where 3 movements share a lane}$$

## MINOR STREET APPROACH MOVEMENTS 7, 8, 9

| Movement | v (pcph) | $c_m$ (pcph) | $C_{SH}$ (pcph) | $C_R = C_{SH} - v$ | LOS |
|----------|----------|--------------|-----------------|--------------------|-----|
| 7        | 79       | 768          | 771             | 692                | A   |
| 8        | 5        | 882          | 771             | 766                | A   |
| 9        | 3        | 1000         | 771             | 768                | A   |

## MINOR STREET APPROACH MOVEMENTS 10, 11, 12

| Movement | v (pcph) | $c_m$ (pcph) | $C_{SH}$ (pcph) | $C_R = C_{SH} - v$ | LOS |
|----------|----------|--------------|-----------------|--------------------|-----|
| 10       | 2        | 750          | 914             | 912                | A   |
| 11       | 5        | 843          | 914             | 909                | A   |
| 12       | 10       | 1000         | 914             | 904                | A   |

## MAJOR STREET LEFT TURNS 1, 4

| Movement | v (pcph) | $c_m$ (pcph) | $C_R = c_m - v$ | LOS |
|----------|----------|--------------|-----------------|-----|
| 1        | 4        | 1000         | 996             | A   |
| 4        | 1        | 970          | 969             | A   |

## COMMENTS:

LEVEL OF SERVICE OK  
FOR ALL MOVEMENTS WITH  
ADDED DEVELOPMENT



### **3. 28 Road and Patterson Intersection**

The 28 Road and Patterson intersection is a "T" intersection and will be analyzed as unchanged from the current configuration using the developed traffic flow.

| WORKSHEET FOR ANALYSIS OF T-INTERSECTIONS   |          |                       |   |                |     |           |
|---|----------|-----------------------|---|----------------|-----|-----------|
| LOCATION: <u>CURRENT PATTERSON / 28 RD</u>  |          |                       | NAME: <u>P. HART</u>  |                |     |           |
| HOURLY VOLUMES<br>Major Street: <u>PATTERSON</u> N<br>N = <u>2 1/2</u><br>Grade _____ %<br>Date of Counts: <u>6/26/94</u><br>Time Period: <u>7:30-8:30 AM</u><br>Average Running Speed: _____<br>PHF: _____ Grade _____ % |          |                       | VOLUMES IN PCPH<br>   |                |     |           |
| Minor Street: <u>28 RD</u><br>N = <u>2</u><br><input checked="" type="checkbox"/> STOP<br><input type="checkbox"/> YIELD  |          |                       |   |                |     |           |
| VOLUME ADJUSTMENTS  |          |                       |   |                |     |           |
| Movement No.  | 2        | 3                     | 4   | 5              | 7   | 9         |
| Volume (vph)  | 1170     | 46                    | 6   | 370            | 32  | 13        |
| Vol. (pcph), see Table 10-1   |          |                       | 6.6   |                |     | 35.2 14.3 |
| STEP 1: RT from Minor Street  |          |                       | ← V <sub>5</sub>  |                |     |           |
| Conflicting Flow, V <sub>c</sub>  |          |                       | $1/2 V_3 - V_2 = 23 - 1170 = 1193$ vph (V <sub>5</sub> )                                  |                |     |           |
| Critical Gap, T <sub>c</sub> , and Potential Capacity, c <sub>p</sub>   |          |                       | T <sub>c</sub> = <u>5.5</u> sec (Table 10-2) c <sub>p</sub> = <u>290</u> pcph (Fig. 10-3) |                |     |           |
| Actual Capacity, c <sub>m</sub>   |          |                       | c <sub>m</sub> = c <sub>p</sub> = <u>290</u> pcph   |                |     |           |
| STEP 2: LT From Major Street  |          |                       | ↓ V <sub>4</sub>  |                |     |           |
| Conflicting Flow, V <sub>c</sub>  |          |                       | $V_3 - V_2 = 1170 + 46 = 1216$ vph (V <sub>4</sub> )                                      |                |     |           |
| Critical Gap, T <sub>c</sub> , and Potential Capacity, c <sub>p</sub>   |          |                       | T <sub>c</sub> = <u>5.5</u> sec (Table 10-2) c <sub>p</sub> = <u>245</u> pcph (Fig. 10-3) |                |     |           |
| Percent of c <sub>p</sub> Utilized and Impedance Factor (Fig. 10-5)   |          |                       | $(v_4/c_{p4}) \times 100 = 2.5^{0\%} P_4 = .99$   |                |     |           |
| Actual Capacity, c <sub>m</sub>   |          |                       | c <sub>m</sub> = c <sub>p</sub> = <u>245</u> pcph   |                |     |           |
| STEP 3: LT From Minor Street  |          |                       | ↘ V <sub>1</sub>  |                |     |           |
| Conflicting Flow, V <sub>c</sub>  |          |                       | $1/2 V_3 - V_2 + V_5 - V_4 = 23 - 1170 - 370 - 6 = 1569$ vph (V <sub>1</sub> )            |                |     |           |
| Critical Gap, T <sub>c</sub> , and Potential Capacity, c <sub>p</sub>   |          |                       | T <sub>c</sub> = <u>7</u> sec (Table 10-2) c <sub>p</sub> = <u>90</u> pcph (Fig. 10-3)    |                |     |           |
| Actual Capacity, c <sub>m</sub>   |          |                       | c <sub>m</sub> = c <sub>p</sub> × P <sub>4</sub> = <u>90 × .99 = 89</u> pcph              |                |     |           |
| SHARED-LANE CAPACITY  |          |                       |   |                |     |           |
| $SH = \frac{v - v_0}{(v_7/c_{m7}) - (v_0/c_{m0})}$ if lane is shared  |          |                       |   |                |     |           |
| Movement No.  | v (pcph) | c <sub>m</sub> (pcph) | c <sub>su</sub> (pcph)  | c <sub>0</sub> | LOS |           |
| 7   | 35.2     | 89                    | -   | 53.8           | E   |           |
| 9   | 14.3     | 290                   | -   | 275.7          | C   |           |
| 4   | 6.6      | 245                   | -   | 238            | C   |           |

| WORKSHEET FOR ANALYSIS OF T-INTERSECTIONS  |  |                       |                        |                |     |        |
|--|--|-----------------------|------------------------|----------------|-----|--------|
| LOCATION: <u>W/ DEVELOPMENT PATTERSON / 28 RD</u>  |  |                       | NAME: <u>P. HART</u>   |                |     |        |
| HOURLY VOLUMES<br>Major Street: <u>PATTERSON</u> N<br>N = <input type="checkbox"/><br>Grade _____ %<br>Date of Counts: <u>ESTIMATED</u><br>Time Period: <u>7:00 - 9:00 AM</u><br>Average Running Speed: _____<br>PHF: _____ Grade _____ % <u>28 Rd</u> |  |                       | VOLUMES IN PCPH<br>    |                |     |        |
| VOLUME ADJUSTMENTS   |  |                       |                        |                |     |        |
| Movement No.   | 2  | 3                     | 4                      | 5              | 7   | 9      |
| Volume (vph)   | 1170   | 147                   | 47                     | 370            | 132 | 54     |
| Vol. (pcph), see Table 10-1  | ██████████   |                       | 52                     | ██████████     |     | 145 59 |
| STEP 1: RT from Minor Street   |  |                       | ← V <sub>5</sub>       |                |     |        |
| Conflicting Flow, V <sub>c</sub>   | 1/2 V <sub>3</sub> + V <sub>2</sub> = <u>74</u> + <u>1170</u> = <u>1244</u> vph (V <sub>c</sub> )  |                       |                        |                |     |        |
| Critical Gap, T <sub>c</sub> , and Potential Capacity, c <sub>p</sub>  | T <sub>c</sub> = <u>5.5</u> sec (Table 10-2) c <sub>p</sub> = <u>220</u> pcph (Fig. 10-3)  |                       |                        |                |     |        |
| Actual Capacity, c <sub>m</sub>  | c <sub>m9</sub> = c <sub>p9</sub> = <u>220</u> pcph  |                       |                        |                |     |        |
| STEP 2: LT From Major Street   |  |                       | ↓ V <sub>4</sub>       |                |     |        |
| Conflicting Flow, V <sub>c</sub>   | V <sub>3</sub> + V <sub>2</sub> = <u>1170</u> + <u>147</u> = <u>1317</u> vph (V <sub>c</sub> )   |                       |                        |                |     |        |
| Critical Gap, T <sub>c</sub> , and Potential Capacity, c <sub>p</sub>  | T <sub>c</sub> = <u>5.5</u> sec (Table 10-2) c <sub>p4</sub> = <u>215</u> pcph (Fig. 10-3)   |                       |                        |                |     |        |
| Percent of c <sub>p</sub> Utilized and Impedance Factor (Fig. 10-5)  | (v <sub>4</sub> /c <sub>p4</sub> ) × 100 = <u>2.4%</u> P <sub>4</sub> = <u>.94</u>   |                       |                        |                |     |        |
| Actual Capacity, c <sub>m</sub>  | c <sub>m4</sub> = c <sub>p4</sub> = <u>215</u> pcph  |                       |                        |                |     |        |
| STEP 3: LT From Minor Street   |  |                       | ↘ V <sub>1</sub>       |                |     |        |
| Conflicting Flow, V <sub>c</sub>   | 1/2 V <sub>3</sub> - V <sub>2</sub> - V <sub>3</sub> - V <sub>4</sub> = <u>74</u> - <u>1170</u> - <u>370</u> - <u>52</u> = <u>1066</u> vph (V <sub>c</sub> ) |                       |                        |                |     |        |
| Critical Gap, T <sub>c</sub> , and Potential Capacity, c <sub>p</sub>  | T <sub>c</sub> = <u>7</u> sec (Table 10-2) c <sub>p7</sub> = <u>80</u> pcph (Fig. 10-3)  |                       |                        |                |     |        |
| Actual Capacity, c <sub>m</sub>  | c <sub>m7</sub> = c <sub>p7</sub> × P <sub>7</sub> = <u>80</u> × <u>.94</u> = <u>75</u> pcph   |                       |                        |                |     |        |
| SHARED-LANE CAPACITY   |  |                       |                        |                |     |        |
| SH = $\frac{v_i - v_j}{(v_i/c_{m7}) - (v_j/c_{m9})}$ if lane is shared   |  |                       |                        |                |     |        |
| Movement No.   | v (pcph)   | c <sub>m</sub> (pcph) | c <sub>sh</sub> (pcph) | c <sub>o</sub> | LOS |        |
| 7  | 145  | 75                    | -                      | -70            | F   |        |
| 9  | 69.3   | 220                   | -                      | 150            | D   |        |
| 4  | 19.8   | 215                   | -                      | 195            | D   |        |

## F. TRAFFIC ACCIDENT REPORTS FOR PATTERSON AND 28 ROAD

1. Traffic accident reports for Patterson Road and 28 Road intersection are included in Appendix B of this report.

## G. TRAFFIC SIGNAL WARRANT ANALYSIS

A traffic signal warrant analysis is required on the intersection at Patterson Road and 28 Road to determine if a traffic signal is warranted at this intersection. The following warrants analysis contained in the Manual on Uniform Traffic Control Devices, 1989, U.S. Department of Transportation is used to determine this requirement.

- |           |   |
|-----------|---|
| Warrant 1 | Minimum Vehicular Volume  |
| Not Met   | Vehicles per hour approaching the intersection from the minor street does exceed 200 vehicles per hour.                                   |
| Warrant 2 | Interupption of Continuous Traffic  |
| Met       | Vehicles per hour approaching the intersection from the minor street does exceed 100 vehicles per hour                                    |
| Warrant 3 | Minimum Pedestrian Volume   |
| Not Met   | Study shows that during 2- 2 hour periods (7:30 to 9:30AM and 4:30 to 6:30PM) a total of 3 pedestrians used the intersection each period. |
| Warrant 4 | School Crossing   |
| Not Met   | No school crossing is located at this intersection, the nearest school is located more than 1 mile from this intersection.                |
| Warrant 5 | Progressive Movement  |
| Not Met   | Platooning of vehicles on Patterson is provided by the signals at 27 1/2 Road to the west and 28 1/4 Road.                                |

- Warrant 6    Accident Experience  
 Not Met     Accident reports for the intersection show 6 accidents in 3 years which is less than the minimum of 5 accidents in 1 year.
- Warrant 7    Systems Warrant  
 Met          Patterson Road is a part of the highway system which serves as the principal network for through traffic flow.
- Warrant 8    Combination of Warrants  
 Met          Warrant 1 can be met using the .8 factor and Warrant 2 is met
- Warrant 9    Four Hour Volumes  
 Not Met     Volumes fall below the line graph on figure 4-7, MOUTCD
- Warrant 10   Peak Hour Delay  
 Met          All 3 items in this Warrant are met during the AM and PM volumes
- Warrant 11   Peak Hour Volume  
 Met          One Hour Peak AM volume of 187 falls above the the curve on figur 4-5, MOUTCD

Factors governing the selection of type of control show that a traffic actuated control may be the type of control which may assist the left hand turn from the minor street (28 Road) unto Patterson Road during the AM and PM peak hours.

## **H. CONCLUSIONS AND RECOMMENDATIONS**

The intersections an Hawthorn and 28 Road and Ridge and 28 Road are within the LOS A and will provide proper service with no further changes or improvements after the proposed development is completed.

It must be decided by the City if a traffic signal at Patterson Road and 28 Road would improve service. The Warrant Analysis shows that 5 of the Warrants are met showing that a traffic signal of some sort be considered. Some movements would be improved by the installation of a signal, others, while somewhat safer, would be adversely affected and delayed. It is recommended that a traffic actuated signal to assist in the left hand turn from 28 Road onto Patterson Road be considered when actual traffic conditions occur after build-out of partial build-out of the Grandview Subdivision.

**APPENDIX A**

# Single-Family Detached Housing (< 300 Units) (210)

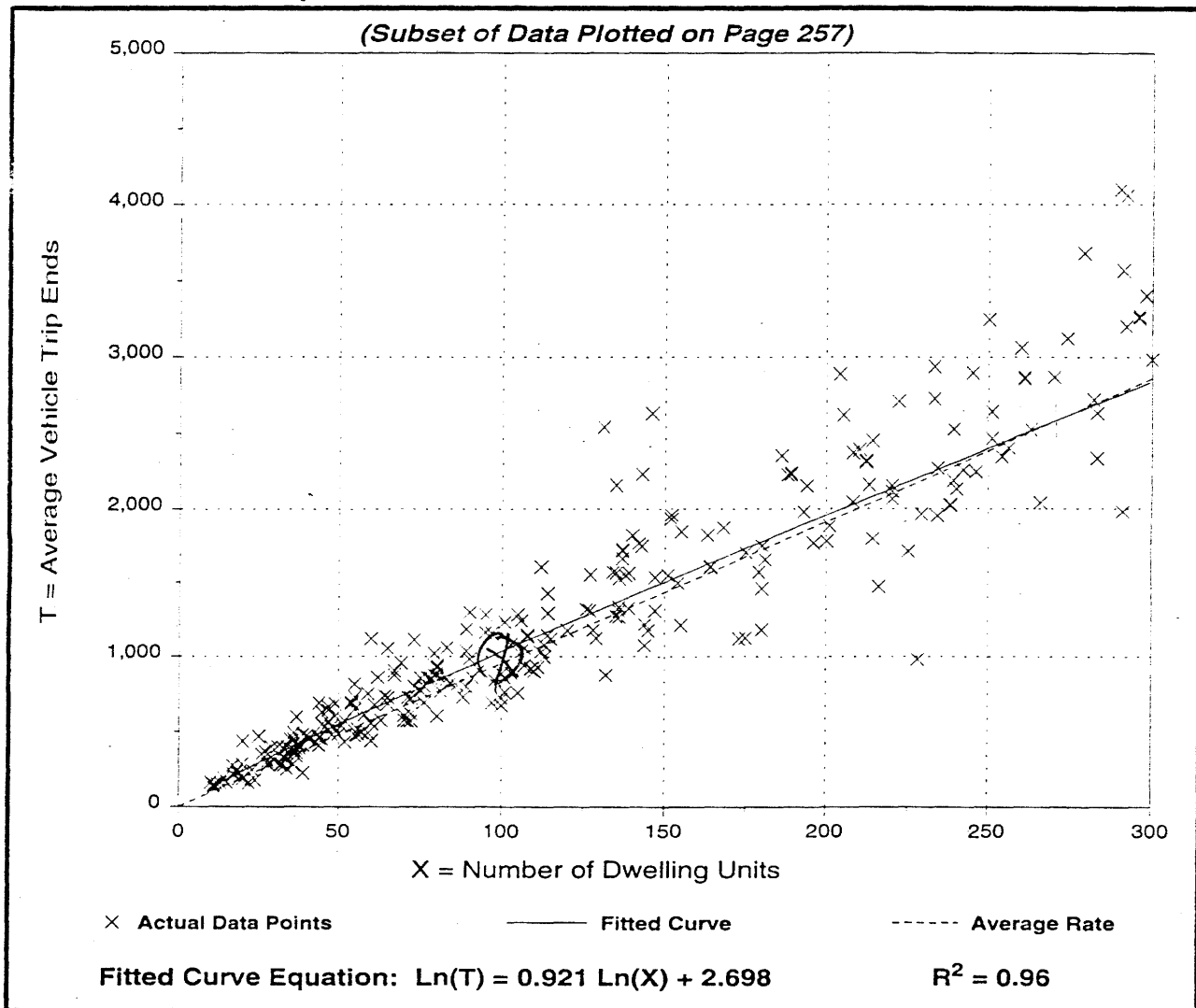
Average Vehicle Trip Ends vs: Dwelling Units  
On a: Weekday

Number of Studies: 348  
Average Number of Dwelling Units: 206  
Directional Distribution: 50% entering, 50% exiting

## Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
|--------------|----------------|--------------------|
| 9.55         | 4.31 - 21.85   | 3.36               |

## Data Plot and Equation





# Single-Family Detached Housing (< 300 Units) (210)

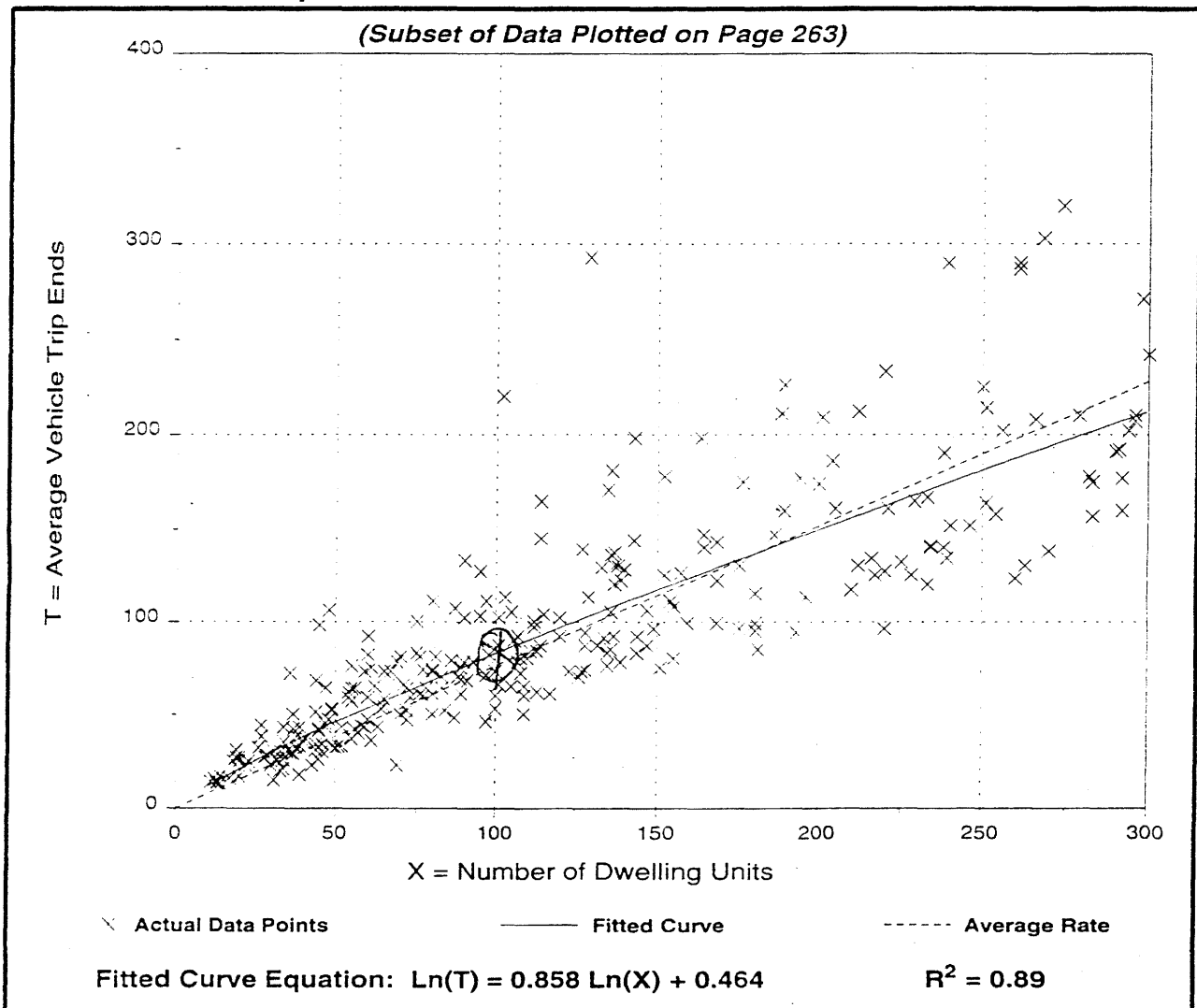
Average Vehicle Trip Ends vs: Dwelling Units  
On a: Weekday,  
A.M. Peak Hour of Generator

Number of Studies: 339  
Average Number of Dwelling Units: 190  
Directional Distribution: 26% entering, 74% exiting

## Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
|--------------|----------------|--------------------|
| 0.76         | 0.33 - 2.27    | 0.91               |

## Data Plot and Equation



# Single-Family Detached Housing (< 300 Units) (210)

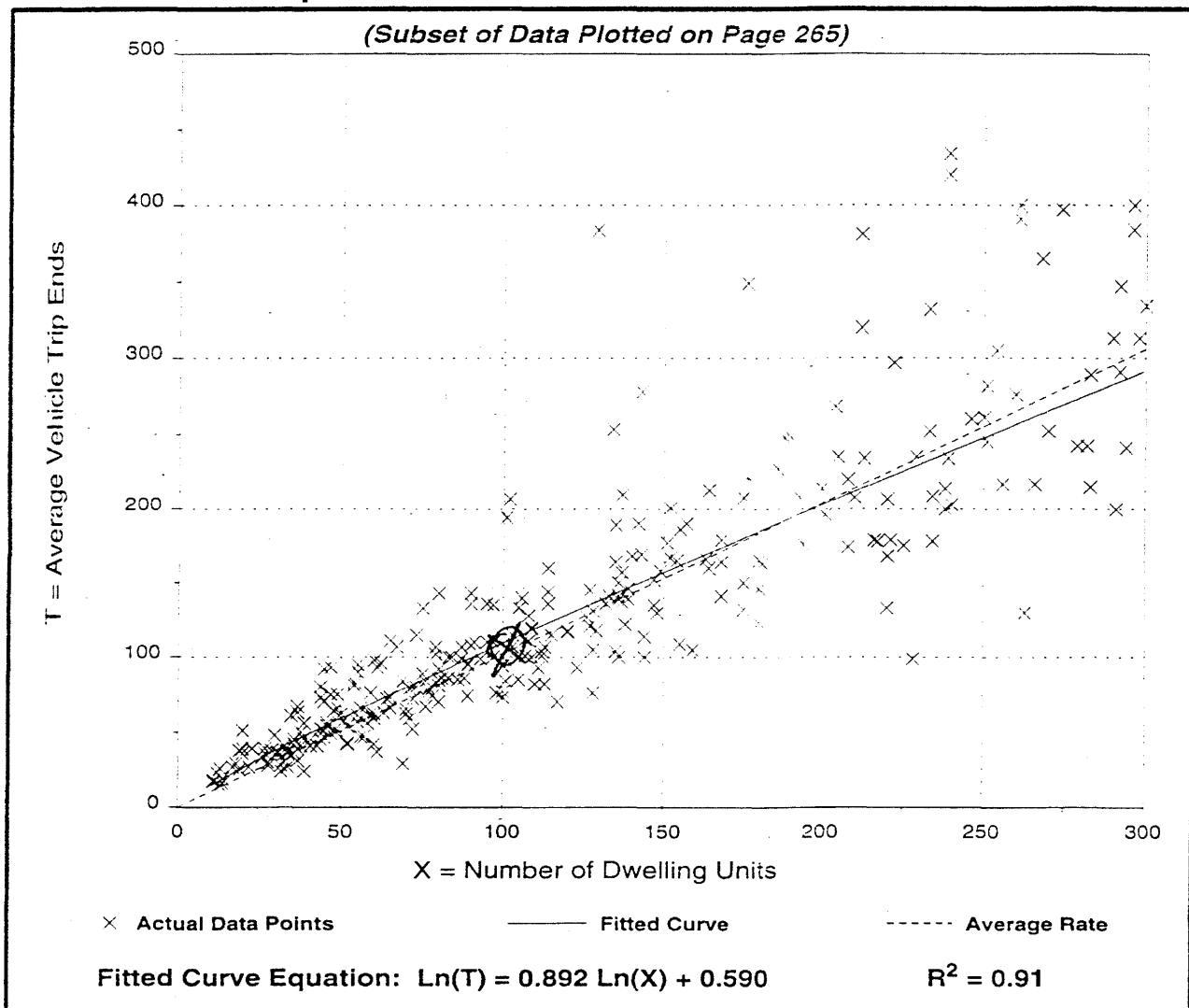
Average Vehicle Trip Ends vs: Dwelling Units  
On a: Weekday,  
P.M. Peak Hour of Generator

Number of Studies: 357  
Average Number of Dwelling Units: 183  
Directional Distribution: 64% entering, 36% exiting

## Trip Generation per Dwelling Unit

| Average Rate | Range of Rates | Standard Deviation |
|--------------|----------------|--------------------|
| 1.02         | 0.42 - 2.98    | 1.05               |

## Data Plot and Equation



**APPENDIX B**

## **2. Ridge and 28 Road Intersection**

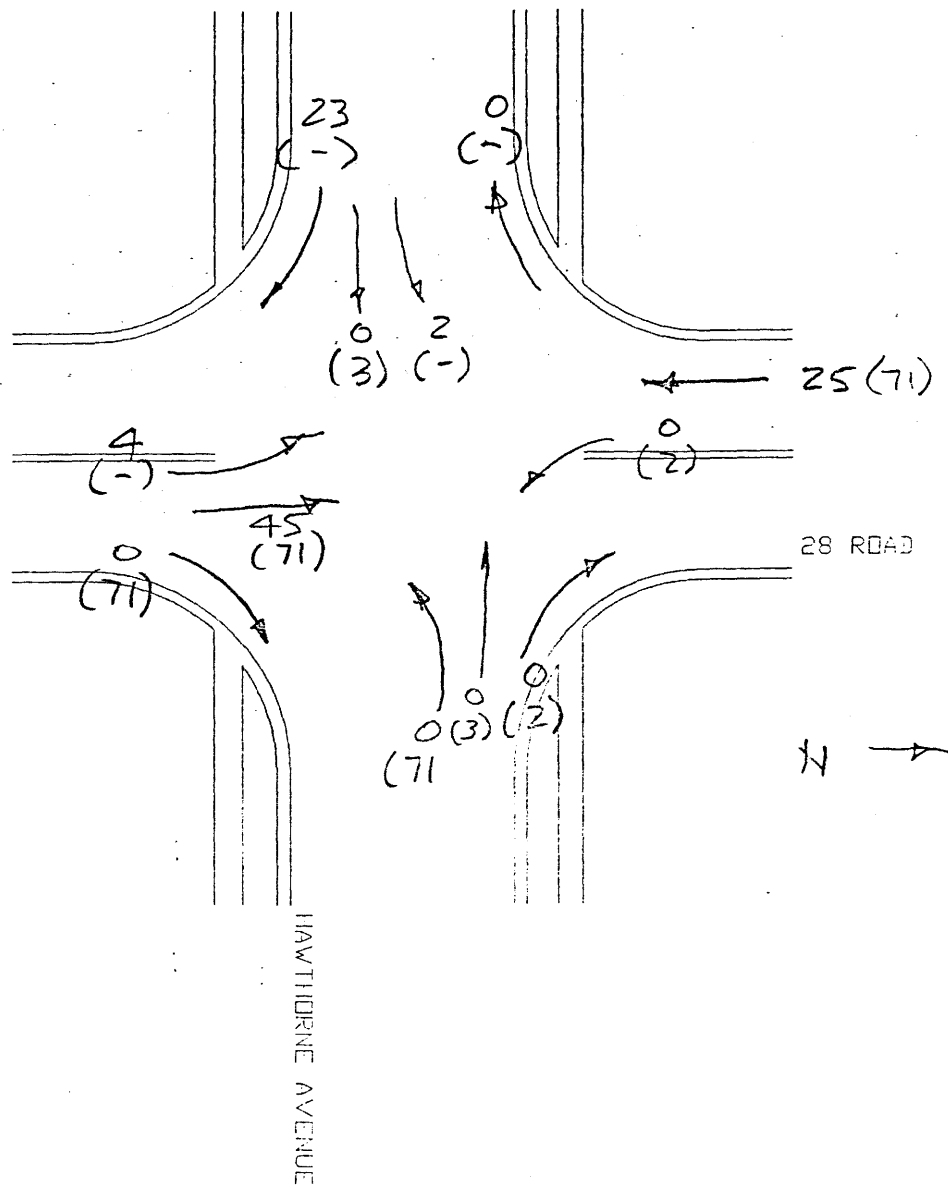
The Ridge and 28 Road Intersection is a "T" intersection which will become a standard "Four Leg" intersection when the new development is constructed. It will be analyzed in the constructed condition using the developed traffic flow.

Traffic volume for the left turn warrants a left turn lane from Hawthorn and 28 Road. Volume of left turns are greatest from Ridge Drive onto 28 Road at the AM Peak Hour. Standard criteria dictates a 2 minute storage at the Peak Hour.  $2 (71/60) = 2.36$  cars therefor a left turn lane which stores 3 cars (75 feet long) is recommended. This would necessitate moving the proposed median on Ridge back to allow a left turn lane in the street section at 28 Road.

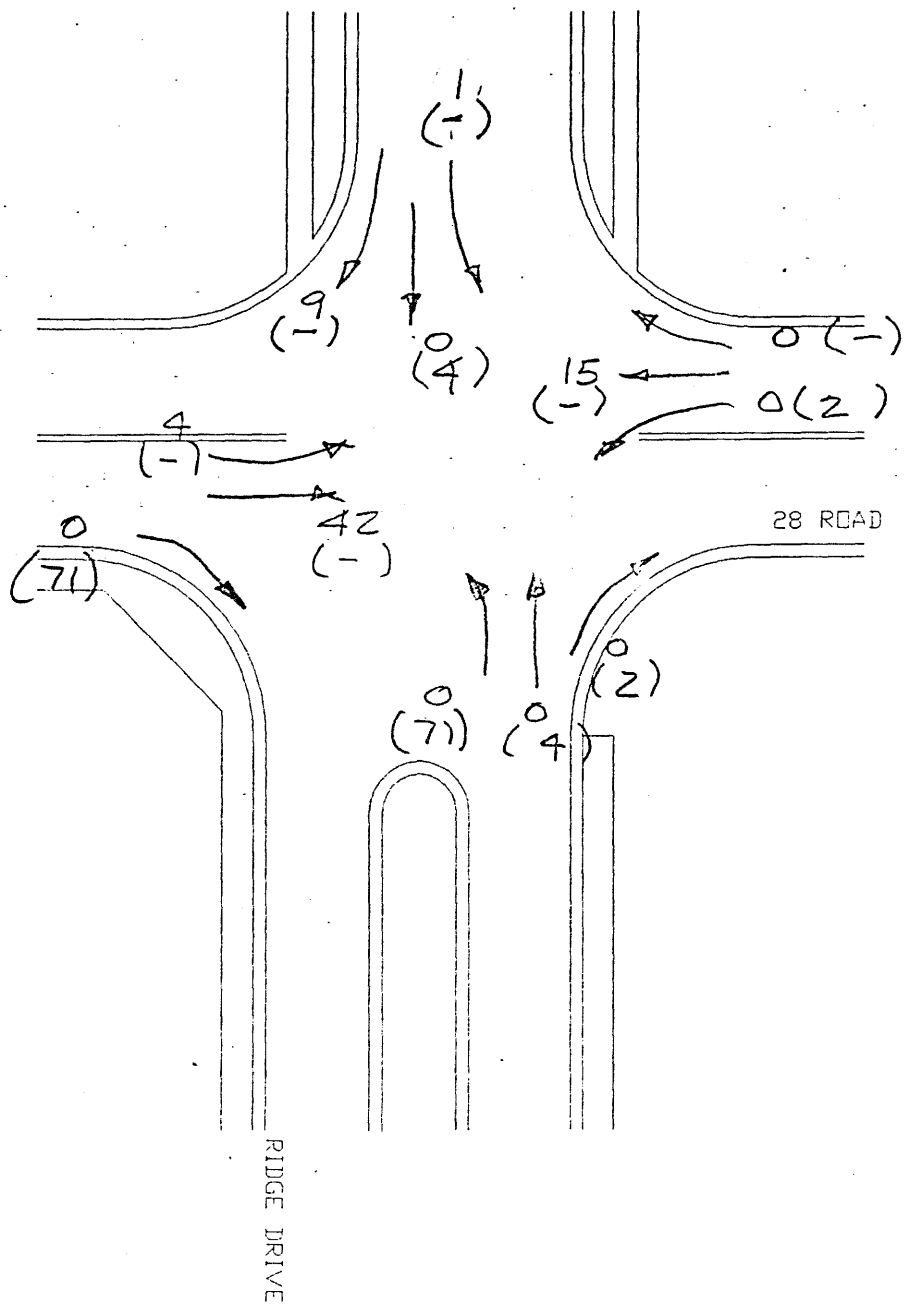
## **D. TRAFFIC VOLUME**

### **1. Peak Hour AM Traffic Volume**

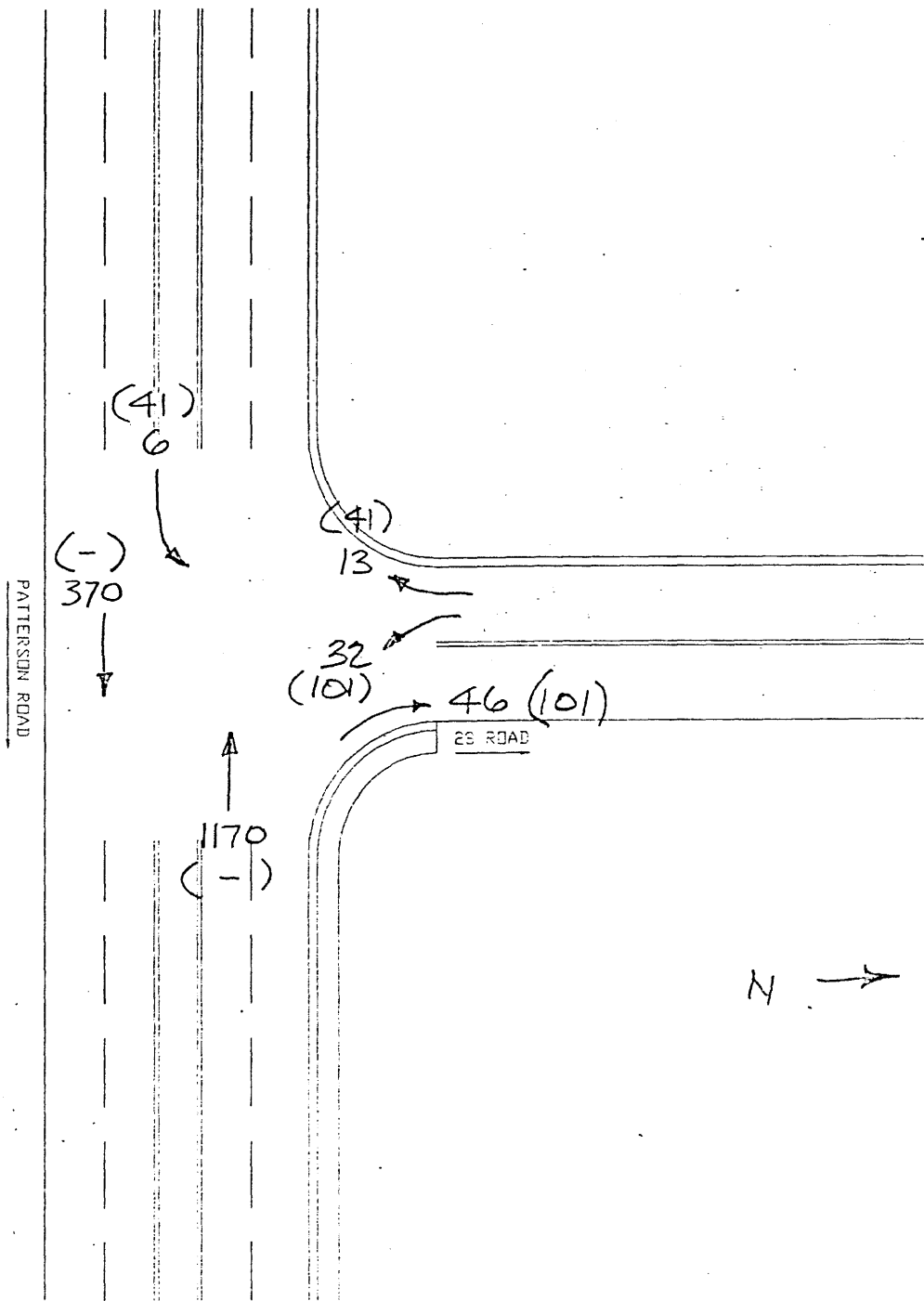
The following Figures 7, 8, and 9 depict peak hour am total traffic volume for the existing and for the Build-out condition. Existing traffic counts were taken June 27, 1994 for the peak hour AM. Build-out counts were taken from the trip distribution and trip assignment calculations shown earlier in this report.



**Figure 7**  
**Peak Hour AM traffic Volume (existing and at buildout)**  
**Hawthorn Ave\28 Road**



**Figure 8**  
**Peak Hour AM Traffic Volume (existing and at buildout)**  
**Ridge Drive\28 Road**

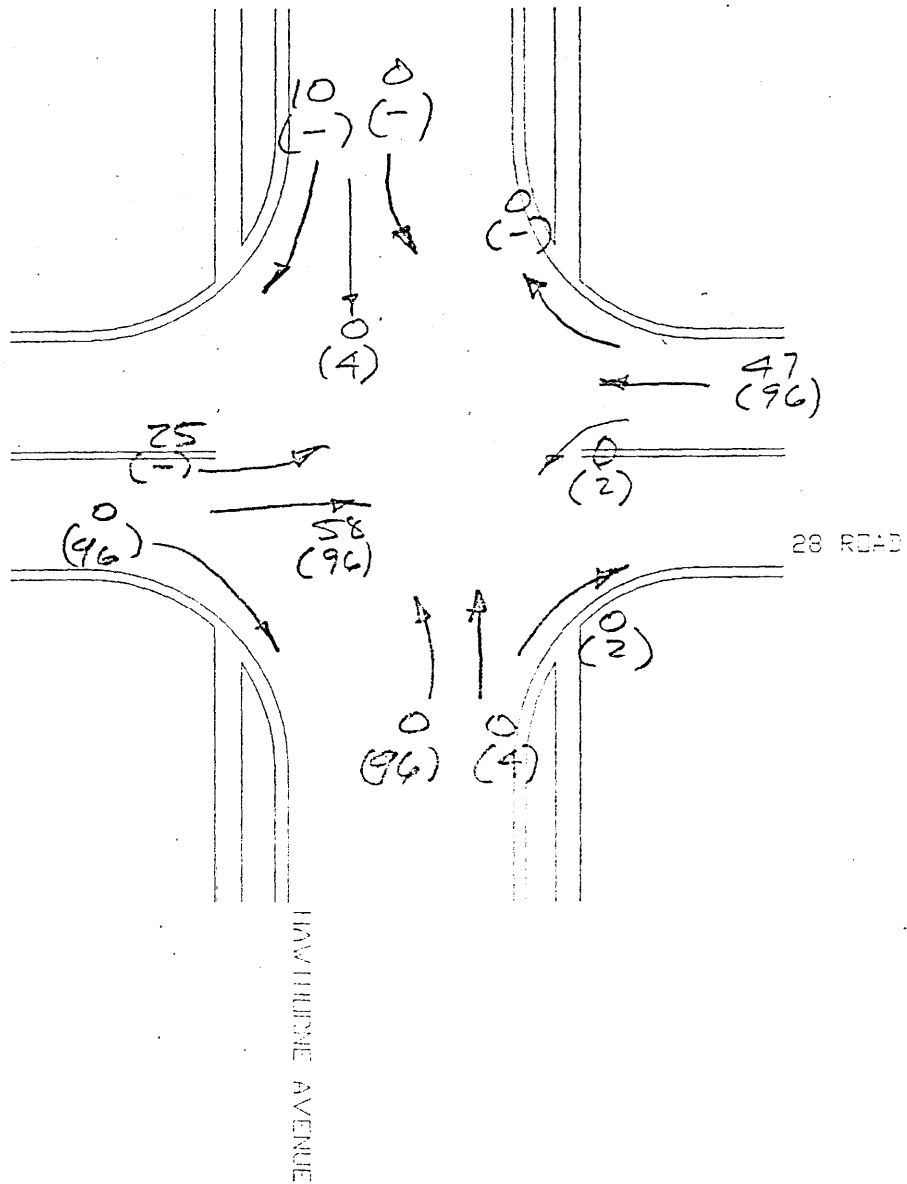


**Figure 9**  
**Peak Hour AM Traffic Volume (existing and at buildout)**  
**Patterson Road\28 Road**

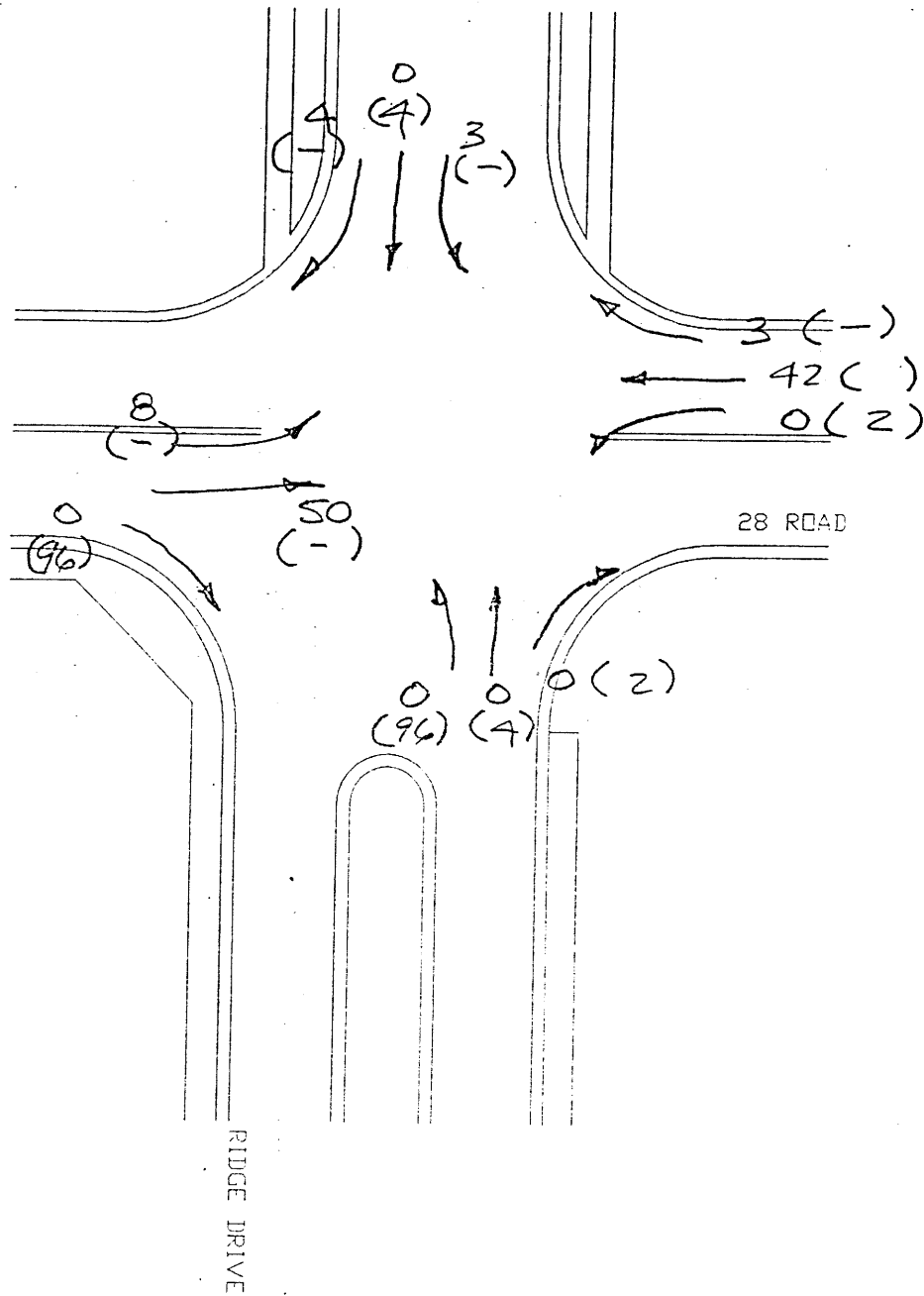


## **2. Peak Hour PM Traffic Volume**

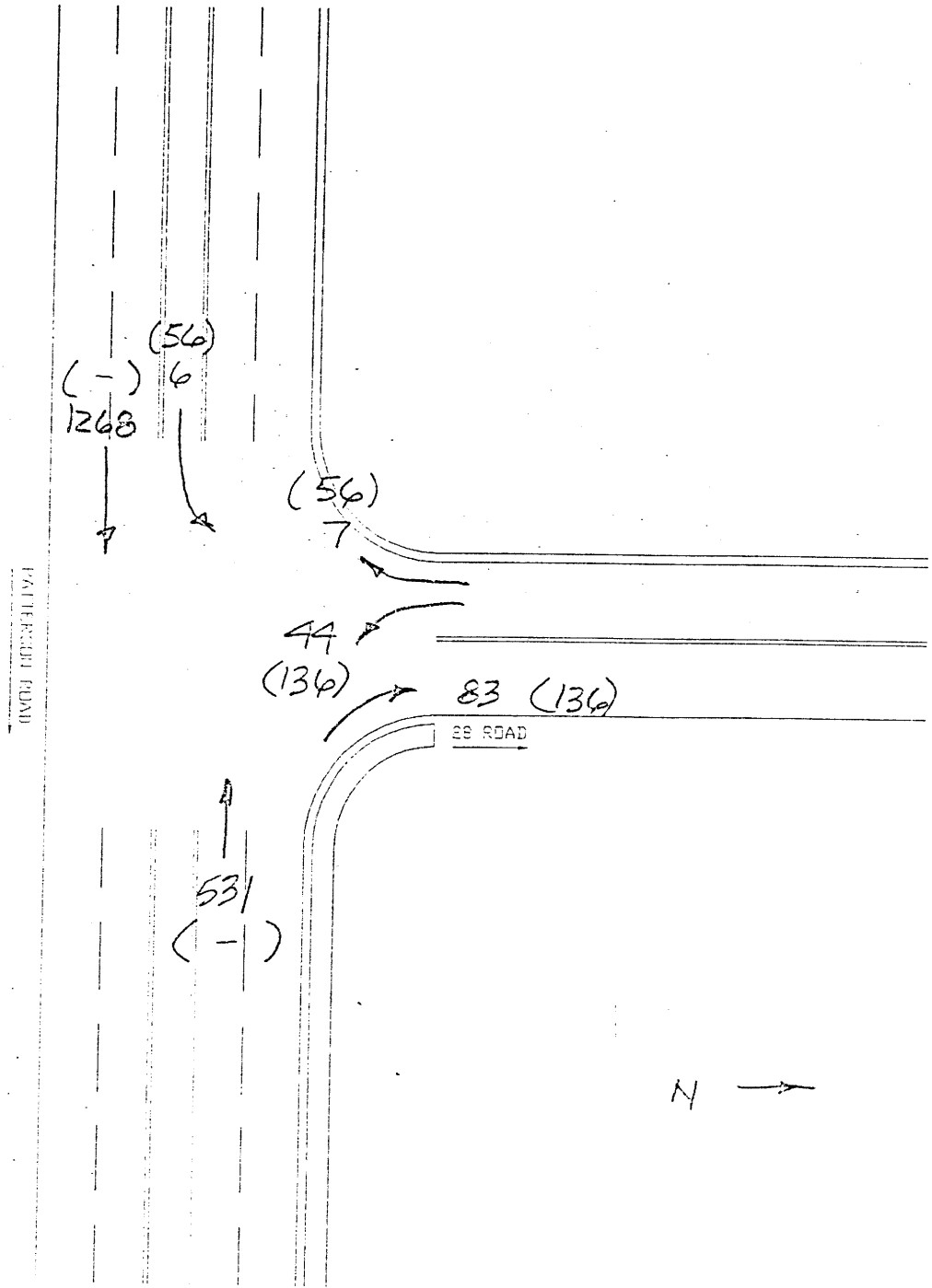
The following Figures 10, 11 and 12 depict peak hour pm total traffic volume for the existing and for the Build-out condition. Existing traffic counts were taken June 27, 1994 for the peak hour PM. Build-out counts were taken from the trip distribution and trip assignment calculations shown earlier in this report.



**Figure 10**  
**Peak Hour PM traffic Volume (existing and at buildout)**  
**Hawthorn Ave\28 Road**



**Figure 11**  
**Peak Hour PM Traffic Volume (existing and at buildout)**  
**Ridge Drive\28 Road**



**Figure 12**  
**Peak Hour AM Traffic Volume (existing and at buildout)**  
**Patterson Road\28 Road**

## **E. CAPACITY ANALYSIS**

### **1. Hawthorn and 28 Road Intersection**

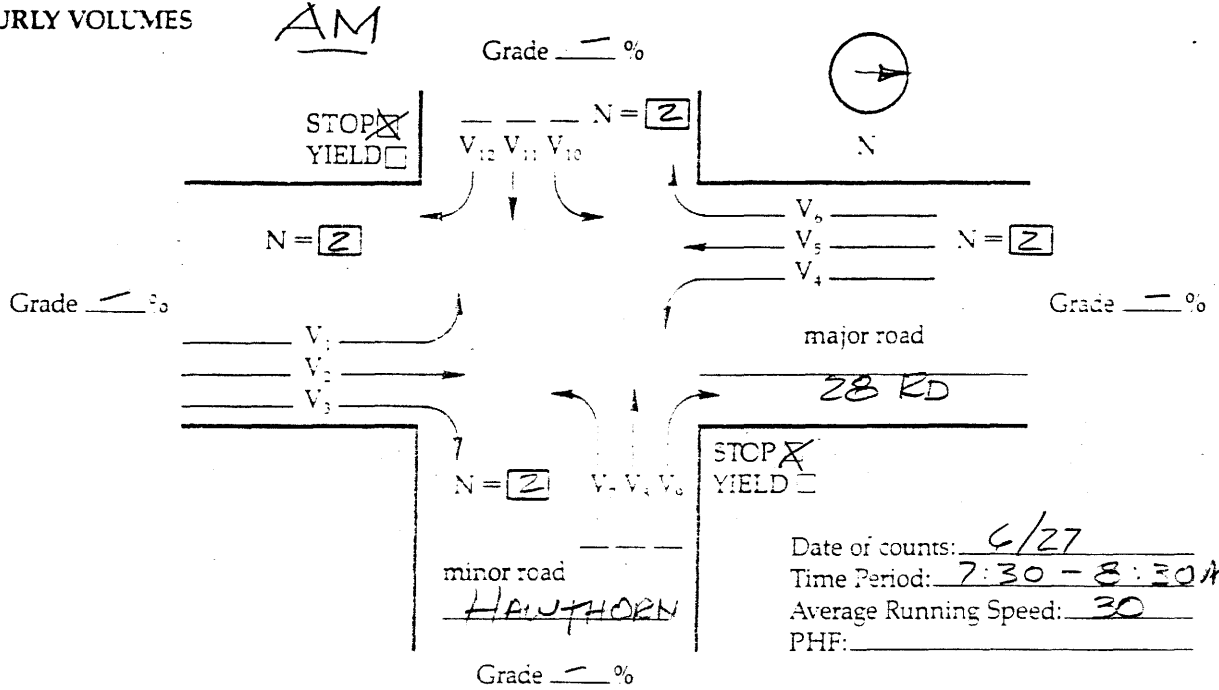
The Hawthorn and 28 Road Intersection is a "T" intersection which will become a standard "Four Leg" intersection when the new development is constructed. It will be analyzed in the constructed condition using the developed traffic flow.

Traffic volume for the left turn warrants a left turn lane from Hawthorn and 28 Road. Volume of left turns are greatest from Hawthorn onto 28 Road at the AM Peak Hour. Standard criteria dictates a 2 minute storage at the Peak Hour.  $2 (71/60) = 2.36$  cars therefor a left turn lane which stores 3 cars (75 feet long) is recommended.

WORKSHEET FOR FOUR-LEG INTERSECTIONS

Location: HAWTHORN E 28 RD Name: P. HART

HOURLY VOLUMES

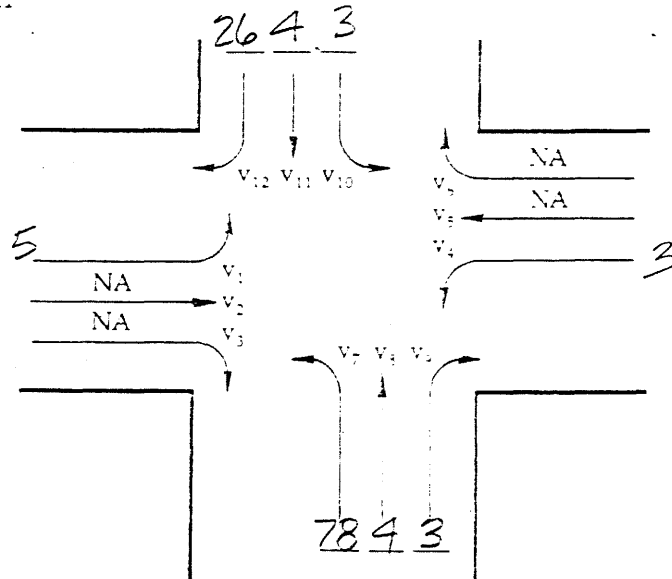


Date of counts: 6/27  
 Time Period: 7:30 - 8:30 AM  
 Average Running Speed: 30  
 PHF: \_\_\_\_\_

VOLUME ADJUSTMENTS

| Movement No.                | 1 | 2         | 3  | 4 | 5         | 6 | 7  | 8 | 9 | 10 | 11 | 12 |
|-----------------------------|---|-----------|----|---|-----------|---|----|---|---|----|----|----|
| Volume (vph)                | 4 | 116       | 71 | 2 | 96        | 0 | 71 | 3 | 2 | 2  | 3  | 23 |
| Vol. (pcph), see Table 10-1 | 5 | [hatched] |    | 3 | [hatched] |   | 78 | 4 | 3 | 3  | 4  | 26 |

VOLUMES IN PCPH



| WORKSHEET FOR FOUR-LEG INTERSECTIONS  |   |   |
|---------------------------------------|---|---|
|                                       |   | Page 2  |
| <b>STEP 1: RT From Minor Street</b>   | $\Gamma V_9$  | $J V_{12}$  |
| Conflicting Flows, $V_c$              | $1/2 V_3 + V_2 = V_{c9}$<br>$14.4 + 116 = 130.4$ vph  | $1/2 V_6 + V_5 = V_{c12}$<br>$0 + 96 = 96$ vph  |
| Critical Gap, $T_c$ (Tab. 10-2)       | $5.5$ (sec)   | $5.5$ (sec)   |
| Potential Capacity, $c_p$ (Fig. 10-3) | $c_{p9} = 960$ pcph   | $c_{p12} = 990$ pcph  |
| Percent of $c_p$ Utilized             | $(v_9/c_{p9}) \times 100 = .3$ %  | $(v_{12}/c_{p12}) \times 100 = 2.6$ %   |
| Impedance Factor, $P$ (Fig. 10-5)     | $P_9 = .99$   | $P_{12} = .98$  |
| Actual Capacity, $c_m$                | $c_{m9} = c_{p9} = 960$ pcph  | $c_{m12} = c_{p12} = 990$ pcph  |
| <b>STEP 2: LT From Major Street</b>   | $\Gamma V_4$  | $J V_1$   |
| Conflicting Flows, $V_c$              | $V_3 + V_2 = V_{c4}$<br>$71 + 116 = 187$ vph  | $V_6 + V_5 = V_{c1}$<br>$0 + 96 = 96$ vph   |
| Critical Gap, $T_c$ (Tab. 10-2)       | $5.5$ (sec)   | $5.5$ (sec)   |
| Potential Capacity, $c_p$ (Fig. 10-3) | $c_{p4} = 970$ pcph   | $c_{p1} = 990$ pcph   |
| Percent of $c_p$ Utilized             | $(v_4/c_{p4}) \times 100 = .3$ %  | $(v_1/c_{p1}) \times 100 = .5$ %  |
| Impedance Factor, $P$ (Fig. 10-5)     | $P_4 = .99$   | $P_1 = .99$   |
| Actual Capacity, $c_m$                | $c_{m4} = c_{p4} = 970$ pcph  | $c_{m1} = c_{p1} = 990$ pcph  |
| <b>STEP 3: TH From Minor Street</b>   | $\uparrow V_8$  | $\downarrow V_{11}$   |
| Conflicting Flows, $V_c$              | $1/2 V_3 + V_2 + V_1 + V_6 + V_5 + V_4 = V_{c8}$<br>$35.5 + 116 + 5 + 0 + 96 + 3 = 256$ vph   | $1/2 V_6 + V_5 - V_4 + V_3 + V_2 + V_1 = V_{c11}$<br>$0 + 96 + 3 + 71 + 116 + 5 = 291$ vph  |
| Critical Gap, $T_c$ (Tab. 10-2)       | $6.0$ (sec)   | $6.0$ (sec)   |
| Potential Capacity, $c_p$ (Fig. 10-3) | $c_{p8} = 730$ pcph   | $c_{p11} = 700$ pcph  |
| Percent of $c_p$ Utilized             | $(v_8/c_{p8}) \times 100 = .55$ %   | $(v_{11}/c_{p11}) \times 100 = .57$ %   |
| Impedance Factor, $P$ (Fig. 10-5)     | $P_8 = .99$   | $P_{11} = .99$  |
| Actual Capacity, $c_m$                | $c_{m8} = c_{p8} \times P_1 \times P_4$<br>$715 = 730 \times .99 \times .99$ (pcph)   | $c_{m11} = c_{p11} \times P_1 \times P_4$<br>$686 = 700 \times .99 \times .99$ (pcph)   |
| <b>STEP 4: LT From Minor Street</b>   | $\Gamma V_7$  | $L V_{10}$  |
| Conflicting Flows, $V_c$              | $V_{c8}$ (step 3) + $V_{11} - V_{12} = V_{c7}$<br>$256 + 4 + 216 = 286$ vph   | $V_{c11}$ (step 3) - $V_8 + V_6 = V_{c10}$<br>$291 + 4 + 3 = 298$ vph   |
| Critical Gap, $T_c$ (Tab. 10-2)       | $6.5$ (sec)   | $6.5$ (sec)   |
| Potential Capacity, $c_p$ (Fig. 10-3) | $c_{p7} = 630$ pcph   | $c_{p10} = 630$ pcph  |
| Actual Capacity, $c_m$                | $c_{m7} = c_{p7} \times P_1 \times P_4 \times P_{11} \times P_{12}$<br>$599 = 630 \times .99 \times .99 \times .99 \times .98$ (pcph) | $c_{m10} = c_{p10} \times P_4 \times P_1 \times P_8 \times P_6$<br>$605 = 630 \times .99 \times .99 \times .99 \times .99$ (pcph) |

## WORKSHEET FOR FOUR-LEG INTERSECTIONS

Page 3

## SHARED-LANE CAPACITY

$$c_{SH} = \frac{v_i + v_j}{(v_i/c_{mi}) + (v_j/c_{mj})} \quad \text{where 2 movements share a lane}$$

$$c_{SH} = \frac{v_r + v_i + v_k}{(v_i/c_{mi}) + (v_j/c_{mj}) + (v_k/c_{mk})} \quad \text{where 3 movements share a lane}$$

## MINOR STREET APPROACH MOVEMENTS 7, 8, 9

| Movement | v(pcph) | c <sub>m</sub> (pcph) | c <sub>SH</sub> (pcph) | c <sub>R</sub> = c <sub>SH</sub> - v | LOS |
|----------|---------|-----------------------|------------------------|--------------------------------------|-----|
| 7        | 78      | 599                   | 515                    | 427                                  | A   |
| 8        | 4       | 715                   | 515                    | 511                                  | A   |
| 9        | 3       | 960                   | 515                    | 512                                  | A   |

## MINOR STREET APPROACH MOVEMENTS 10, 11, 12

| Movement | v(pcph) | c <sub>m</sub> (pcph) | c <sub>SH</sub> (pcph) | c <sub>R</sub> = c <sub>SH</sub> - v | LOS |
|----------|---------|-----------------------|------------------------|--------------------------------------|-----|
| 10       | 3       | 605                   | 825                    | 822                                  | A   |
| 11       | 4       | 686                   | 825                    | 821                                  | A   |
| 12       | 26      | 990                   | 825                    | 799                                  | A   |

## MAJOR STREET LEFT TURNS 1, 4

| Movement | v(pcph) | c <sub>m</sub> (pcph) | c <sub>R</sub> = c <sub>m</sub> - v | LOS |
|----------|---------|-----------------------|-------------------------------------|-----|
| 1        | 5       | 990                   | 985                                 | A   |
| 4        | 3       | 990                   | 917                                 | A   |

## COMMENTS:

LEVEL OF SERVICE OK FOR  
ALL MOVEMENTS WITH  
ADDED DEVELOPMENT



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 Denver, Colorado 80222  
 DR-447 (1/80)

STATE OF COLORADO  
 INVESTIGATOR'S  
 TRAFFIC ACCIDENT REPORT

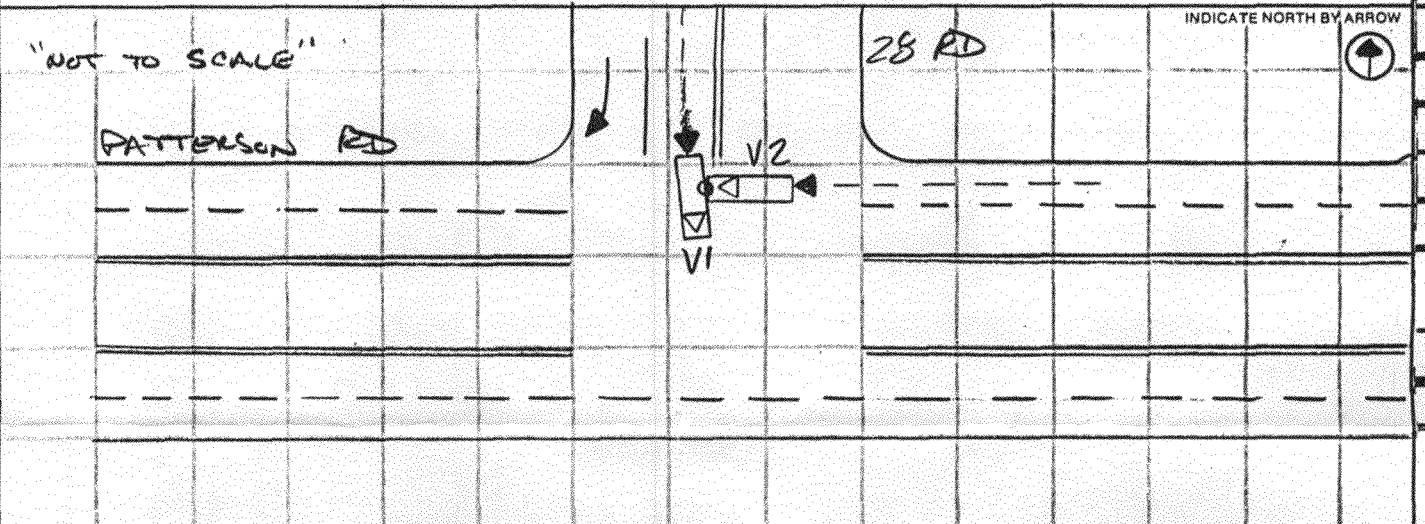
SHEET 1 OF 1 SHEETS

|   |  |   |  |   |  |   |  |
|---|--|---|--|---|--|---|--|
| A   |  | ROAD CODE   |  | CITY SERIAL NUMBER<br>94-02478          |  | STATE SERIAL NUMBER                                     |  |
| DATE OF ACCIDENT<br>031394                    |  | TIME<br>1207  |  | DAY OF WEEK<br>SUNDAY                   |  | CITY<br>GRAND JUNCTION                                  |  |
| DATE NOTIFIED OF ACCIDENT<br>031394           |  | TIME<br>1208  |  | INVESTIGATED AT SCENE<br>X              |  | LAW ENFORCEMENT AGENCY<br>GJPD                          |  |
| DATE ARRIVED AT SCENE<br>031394               |  | TIME<br>1210  |  | OFFICER NUMBER<br>405                   |  | NAME<br>A. CLAYTON                                      |  |
| DATE OF REPORT<br>031394                      |  | LOCATION: ROUTE, STREET, ROAD<br>PATTERSON RD.          |  | MILES FEET N S E W                      |  | SIGNATURE<br>A. Clayton                                 |  |
| NUMBER KILLED<br>0                            |  | NUMBER INJURED<br>0                                     |  | AT THE INTERSECTION WITH<br>28 RD.      |  | DETAIL<br>P   |  |
| PUBLIC PROPERTY PUBLIC EMPLOYEE               |  | TOTAL VEHICLES<br>2                                     |  | DISTRICT NUMBER<br>280FO                |  | INCOMPLETE REPORT                                       |  |
| VEH # OR PARKED UNATTENDED PEDESTRIAN #       |  | VEH # OR PARKED UNATTENDED PEDESTRIAN #                 |  | VEH # OR PARKED UNATTENDED PEDESTRIAN # |  | VEH # OR PARKED UNATTENDED PEDESTRIAN #                 |  |
| LAST NAME<br>MILLER II ROBERT                 |  | FIRST<br>ROBERT   |  | MI<br>ROSS                              |  | LAST NAME<br>DARNELL                                    |  |
| STREET ADDRESS<br>2214 DOGWOOD CT.            |  | RES. PHONE<br>2416430                                   |  | CITY<br>GRAND JCT CO                    |  | STATE<br>CO   |  |
| DRIVER LIC. NO.<br>M509109                    |  | STATE<br>CO   |  | SEX<br>M                                |  | DATE OF BIRTH<br>103176                                 |  |
| VIOLATION(S)<br>FAILED YIELD ROW AT STOP SIGN |  | CITATION NUMBER(S)<br>BO2041                            |  | COMMON CODE(S)<br>373                   |  | VIOLATION(S)<br>AGE<br>81                               |  |
| LIC. PLATE NO.<br>JEL 2012                    |  | STATE<br>CO   |  | BODY TYPE<br>S/W                        |  | COLOR<br>BLUE   |  |
| VEH. IDENT. NO.<br>TF1A W#3B2B8418439         |  | VEH. OWNER LAST NAME<br>SAME                            |  | FIRST<br>SAME                           |  | MI<br>SAME  |  |
| INS. CO.<br>STATE FARM                        |  | DAMAGE SEVERITY:<br>1-SLIGHT<br>2-MODERATE<br>3-EXTREME |  | INS. CO.<br>ARTFORD                     |  | DAMAGE SEVERITY:<br>1-SLIGHT<br>2-MODERATE<br>3-EXTREME |  |
| POLICY #<br>PND05NL6                          |  | EXP. DATE<br>032494                                     |  | POLICY #<br>SSAN/17557121804            |  | EXP. DATE<br>100694                                     |  |

|   |  |   |  |   |  |
|---|--|---|--|---|--|
| G   |  | 20 Undercarriage                          |  | 20 Undercarriage                          |  |
| VEH # TOWED BY/TO                         |  | VEH # TOWED BY/TO                         |  | VEH # TOWED BY/TO                         |  |
| OWNER OF DAMAGED PROP. LAST NAME FIRST MI |  | OWNER OF DAMAGED PROP. LAST NAME FIRST MI |  | OWNER OF DAMAGED PROP. LAST NAME FIRST MI |  |

H 1 DESCRIBE ACCIDENT  
 VEH #2 WAS W/B PATTERSON RD AT 28 RD IN #2 LANE. VEH #1 WAS S/B 28 RD AT PATTERSON

I 1 VEH #1 ATTEMPTED TO TURN R/B ONTO PATTERSON AND WAS HIT BY VEH #2.



| VEH # (1) | Pos (2) | Res (3) | Ejct (4) | Inj Sev (5) | Phy Inj (6) | Stat (7) | Trp Rpt (8) | Age (9) | Sex (10) | Name/Add (11) |
|-----------|---------|---------|----------|-------------|-------------|----------|-------------|---------|----------|---------------|
| 1         | 1       | 1       | 2        | 1           |             |          |             | 17      | M        | DRIVER #1     |
| 2         | 1       | 1       | 2        | 1           |             |          |             | 51      | F        | DRIVER #2     |

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 Denver, Colorado 80222  
 DR-447 (1/90)

# STATE OF COLORADO INVESTIGATOR'S TRAFFIC ACCIDENT REPORT

SHEET 1 OF 1 SHEETS

|  |                                       |
|--|---------------------------------------|
| CITY GENERAL NUMBER<br><b>CC. D.A.</b> | STATE SERIAL NUMBER<br><b>CLEARED</b> |
| 93-31881                               |                                       |

|                                       |   |  |                                |   |               |   |
|---------------------------------------|---|--|--------------------------------|---|---------------|---|
| <b>A</b> ROAD CODE                    |   |  |                                |   |               | 1 |
| DATE OF ACCIDENT<br>11-15-93          | TIME<br>0032                                  | DAY OF WEEK<br>SUN   | CITY<br>G. Jet                 | COUNTY<br>Mesa                          | CO. CODE<br>8 | K |
| DATE NOTIFIED OF ACCIDENT<br>11-15-93 | TIME<br>0032                                  | INVESTIGATED AT SCENE<br>yes   | LAW ENFORCEMENT AGENCY<br>GJPD | REVIEWING OFFICER<br><i>[Signature]</i> |               | K |
| DATE ARRIVED AT SCENE<br>11-15-93     | TIME<br>0040                                  | OFFICER NUMBER<br>369  | NAME<br>Woloszczuk             | SIGNATURE<br><i>[Signature]</i>         | DETAIL<br>PAT | K |
| DATE OF REPORT<br>11-15-93            | LOCATION: ROUTE, STREET, ROAD<br>Patterson Rd |  |                                |   |               | K |
| NUMBER KILLED<br>0                    | NUMBER INJURED<br>0                           | MILES _____ FEET _____ N _____ S _____ E _____ W<br>X AT THE INTERSECTION WITH 28 Rd |                                |   |               | K |

|                                 |                     |                          |                   |                   |             |           |                  |                   |   |
|---------------------------------|---------------------|--------------------------|-------------------|-------------------|-------------|-----------|------------------|-------------------|---|
| PUBLIC PROPERTY PUBLIC EMPLOYEE | TOTAL VEHICLES<br>1 | DISTRICT NUMBER<br>380FO | PHOTO TAKEN<br>NO | RAILROAD CROSSING | CONST. ZONE | WILD GAME | PRIVATE PROPERTY | INCOMPLETE REPORT | 7 |
|---------------------------------|---------------------|--------------------------|-------------------|-------------------|-------------|-----------|------------------|-------------------|---|

|   |                  |          |                 |  |    |   |  |
|---|------------------|----------|-----------------|--|----|---|--|
| <b>B</b> VEH #1 OR X PARKED _____ UNATTENDED _____ PEDESTRIAN # _____ |                  |          |                 | VEH #2 OR _____ PARKED _____ UNATTENDED _____ PEDESTRIAN # _____ |    |   |  |
| LAST NAME<br>WEST   | FIRST<br>Patrick | MI<br>D. | LAST NAME       | FIRST  | MI | L |  |
| STREET ADDRESS<br>3223 Chipeta #23                                    |                  |          | STREET ADDRESS  |  |    | L |  |
| CITY<br>Clifton Co  |                  |          | CITY            |  |    | L |  |
| DRIVER LIC. NO.<br>NONE   |                  |          | DRIVER LIC. NO. |  |    | L |  |

|  |                    |   |
|--|--------------------|---|
| VIOLATION(S)<br>Careless No Lic, No Ins. | VIOLATION(S)       | N |
| VIOLATION CODE(S)<br>42-4-12.04          | VIOLATION CODE(S)  | N |
| CITATION NUMBER(S)<br>B00137 / B00135    | CITATION NUMBER(S) | N |
| COMMON CODE(S)<br>141                    | COMMON CODE(S)     | N |
| YEAR<br>1979                             | YEAR               | N |
| MAKE<br>Toy                              | MAKE               | N |
| MODEL<br>Celica                          | MODEL              | N |

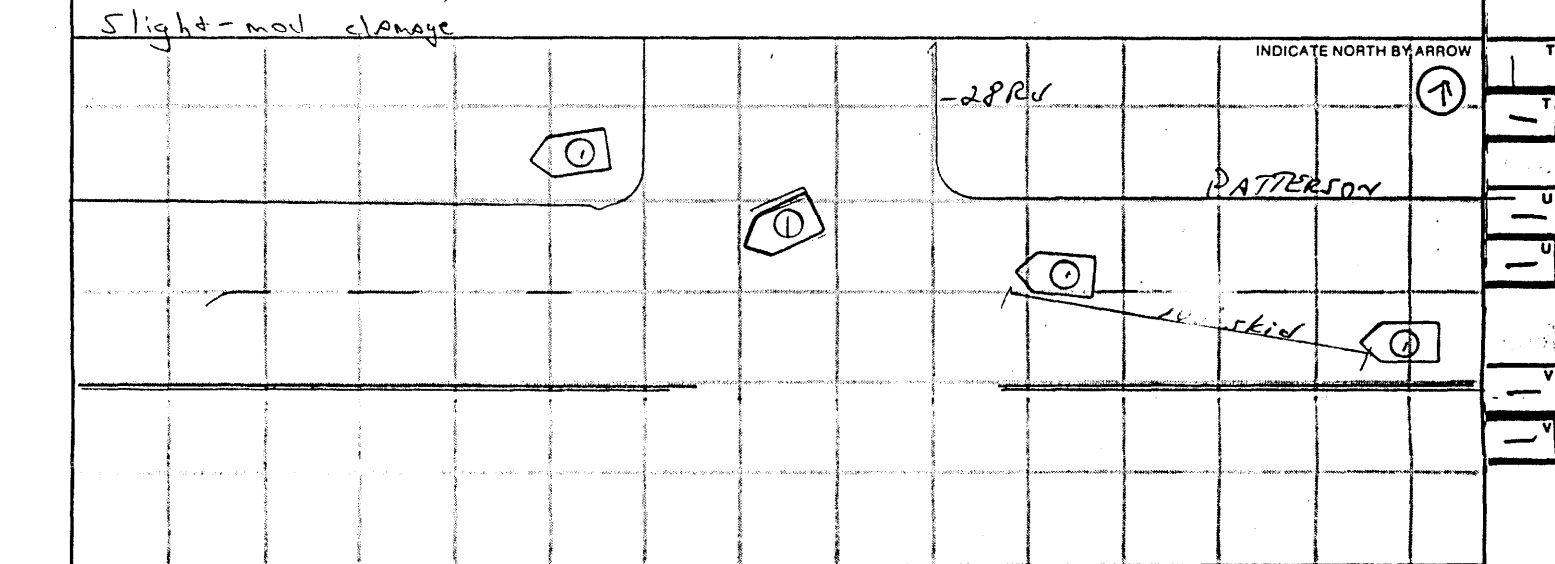
|                                |               |                   |              |                 |       |           |       |   |
|--------------------------------|---------------|-------------------|--------------|-----------------|-------|-----------|-------|---|
| LIC. PLATE NO.<br>WET8040      | STATE<br>COLO | BODY TYPE<br>2-Dr | COLOR<br>Blk | LIC. PLATE NO.  | STATE | BODY TYPE | COLOR | P |
| VEH. IDENT. NO.<br>AA 42145198 |               |                   |              | VEH. IDENT. NO. |       |           |       | P |

|                                |                 |          |                      |       |    |   |
|--------------------------------|-----------------|----------|----------------------|-------|----|---|
| VEH. OWNER LAST NAME<br>Palmer | FIRST<br>Shawna | MI<br>M. | VEH. OWNER LAST NAME | FIRST | MI | P |
| ADDRESS<br>1938 N. 1st         |                 |          | ADDRESS              |       |    | P |
| CITY<br>G. Jet                 |                 |          | CITY                 |       |    | P |
| STATE ZIP CODE<br>Co 81501     |                 |          | STATE ZIP CODE       |       |    | P |

|                  |   |          |   |   |
|------------------|---|----------|---|---|
| INS. CO.<br>NONE | DAMAGE SEVERITY:<br>1-SLIGHT<br>2-MODERATE<br>3-EXTREME | INS. CO. | DAMAGE SEVERITY:<br>1-SLIGHT<br>2-MODERATE<br>3-EXTREME | O |
| POLICY #         | EXP. DATE   | POLICY # | EXP. DATE   | O |

|  |                             |   |
|--|-----------------------------|---|
| 20 Undercarriage<br>1-2                      | 20 Undercarriage            | R |
| VEH # _____ TOWED BY/TO<br>Bill's Body       | VEH # _____ TOWED BY/TO     | R |
| OWNER OF DAMAGED PROP. LAST NAME<br>FIRST MI | ADDRESS CITY STATE ZIP CODE | R |

**H** 3 DESCRIBE ACCIDENT  
 VEH #1 w/R 3800 Blk Patterson (left lane) when driver lost control of vehicle. VEH #1 N/W Bound at 28/Patterson striking curb and coming to rest.



| VEH # (1) | Pos (2) | Res (3) | Ejct (4) | Inj Sev (5) | Phy Inj (6) | Stat (7) | Trp Rpt (8) | Age (9) | Sex (10) | Name/ Add (11)                        |
|-----------|---------|---------|----------|-------------|-------------|----------|-------------|---------|----------|---------------------------------------|
| 1         | 1       | 1       | 2        | 1           |             |          |             |         |          | Same as #1                            |
| 1         | 3       | 1       | 2        | 1           |             |          |             | 22      | M        | Bradley Kent 3223 Chipeta #23 Clifton |

2-  
NF  
Mail to:  
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Denver, Colorado 80222  
DR-447 (1/90)

STATE OF COLORADO  
INVESTIGATOR'S  
TRAFFIC ACCIDENT REPORT

SHEET 1 OF 1 SHEETS

|                                |  |                     |  |
|--------------------------------|--|---------------------|--|
| CITY SERIAL NUMBER<br>93-27870 |  | STATE SERIAL NUMBER |  |
|--------------------------------|--|---------------------|--|

|               |  |                              |  |              |                         |                    |                |               |
|---------------|--|------------------------------|--|--------------|-------------------------|--------------------|----------------|---------------|
| A 1 ROAD CODE |  | DATE OF ACCIDENT<br>07-29-93 |  | TIME<br>1314 | DAY OF WEEK<br>Thursday | CITY<br>Grand Jct. | COUNTY<br>Mesa | CO. CODE<br>8 |
|---------------|--|------------------------------|--|--------------|-------------------------|--------------------|----------------|---------------|

|   |  |              |                            |                                    |                                  |
|---|--|--------------|----------------------------|------------------------------------|----------------------------------|
| B 5 DATE NOTIFIED OF ACCIDENT<br>07-29-93 |  | TIME<br>1314 | INVESTIGATED AT SCENE<br>x | LAW ENFORCEMENT AGENCY<br>G.J.P.O. | REVIEWING OFFICER<br>[Signature] |
| DATE ARRIVED AT SCENE<br>07-29-93         |  | TIME<br>1320 | OFFICER NUMBER<br>#372     | NAME<br>Moreno                     | SIGNATURE<br>[Signature]         |

|                            |  |       |      |   |   |   |   |
|----------------------------|--|-------|------|---|---|---|---|
| DATE OF REPORT<br>07-29-93 | LOCATION: ROUTE, STREET, ROAD<br>Patterson Rd. | MILES | FEET | N | S | E | W |
|----------------------------|--|-------|------|---|---|---|---|

|                                 |                     |                          |             |                   |             |           |                  |                        |
|---------------------------------|---------------------|--------------------------|-------------|-------------------|-------------|-----------|------------------|------------------------|
| PUBLIC PROPERTY PUBLIC EMPLOYEE | TOTAL VEHICLES<br>2 | DISTRICT NUMBER<br>280FO | PHOTO TAKEN | RAILROAD CROSSING | CONST. ZONE | WILD GAME | PRIVATE PROPERTY | INCOMPLETE REPORT<br>x |
|---------------------------------|---------------------|--------------------------|-------------|-------------------|-------------|-----------|------------------|------------------------|

|             |        |            |              |             |        |            |              |
|-------------|--------|------------|--------------|-------------|--------|------------|--------------|
| VEH #1 OR 1 | PARKED | UNATTENDED | PEDESTRIAN # | VEH #2 OR 2 | PARKED | UNATTENDED | PEDESTRIAN # |
|-------------|--------|------------|--------------|-------------|--------|------------|--------------|

|                     |                  |          |                      |                  |          |
|---------------------|------------------|----------|----------------------|------------------|----------|
| LAST NAME<br>McLeod | FIRST<br>Michael | MI<br>N. | LAST NAME<br>Settles | FIRST<br>Darlene | MI<br>C. |
|---------------------|------------------|----------|----------------------|------------------|----------|

|                                    |                        |                                       |                        |
|------------------------------------|------------------------|---------------------------------------|------------------------|
| STREET ADDRESS<br>2871 Orchard Ave | RES. PHONE<br>241-4964 | STREET ADDRESS<br>3850 Mont Vista Ct. | RES. PHONE<br>464-5070 |
|------------------------------------|------------------------|---------------------------------------|------------------------|

|                       |             |                   |                        |                  |             |                   |                        |
|-----------------------|-------------|-------------------|------------------------|------------------|-------------|-------------------|------------------------|
| CITY<br>Grand Jct, CO | STATE<br>CO | ZIP CODE<br>81501 | BUS. PHONE<br>241-5711 | CITY<br>Palisade | STATE<br>CO | ZIP CODE<br>81526 | BUS. PHONE<br>434-6024 |
|-----------------------|-------------|-------------------|------------------------|------------------|-------------|-------------------|------------------------|

|                            |             |          |                           |                            |             |          |                           |
|----------------------------|-------------|----------|---------------------------|----------------------------|-------------|----------|---------------------------|
| DRIVER LIC. NO.<br>U381709 | STATE<br>CO | SEX<br>M | DATE OF BIRTH<br>09-14-52 | DRIVER LIC. NO.<br>K243491 | STATE<br>CO | SEX<br>F | DATE OF BIRTH<br>06-02-50 |
|----------------------------|-------------|----------|---------------------------|----------------------------|-------------|----------|---------------------------|

|  |           |              |           |
|--|-----------|--------------|-----------|
| C 1 VIOLATION(S)<br>Failed to yield Row at stop sign | AGE<br>40 | VIOLATION(S) | AGE<br>43 |
|--|-----------|--------------|-----------|

|              |             |              |              |              |                 |
|--------------|-------------|--------------|--------------|--------------|-----------------|
| YEAR<br>1991 | MAKE<br>GMC | MODEL<br>P10 | YEAR<br>1989 | MAKE<br>Ford | MODEL<br>Taurus |
|--------------|-------------|--------------|--------------|--------------|-----------------|

|                              |             |                  |             |                          |             |                  |                |
|------------------------------|-------------|------------------|-------------|--------------------------|-------------|------------------|----------------|
| D 1 LIC. PLATE NO.<br>862AJG | STATE<br>NM | BODY TYPE<br>P10 | COLOR<br>BK | LIC. PLATE NO.<br>U66271 | STATE<br>CO | BODY TYPE<br>4-D | COLOR<br>Brown |
|------------------------------|-------------|------------------|-------------|--------------------------|-------------|------------------|----------------|

|                                      |                                      |
|--------------------------------------|--------------------------------------|
| VEH. IDENT. NO.<br>26FFK29KXM1526912 | VEH. IDENT. NO.<br>1FABP54Y5KA178572 |
|--------------------------------------|--------------------------------------|

|                                  |             |                              |             |
|----------------------------------|-------------|------------------------------|-------------|
| E 2 VEH. OWNER LAST NAME<br>Same | FIRST<br>MI | VEH. OWNER LAST NAME<br>Same | FIRST<br>MI |
|----------------------------------|-------------|------------------------------|-------------|

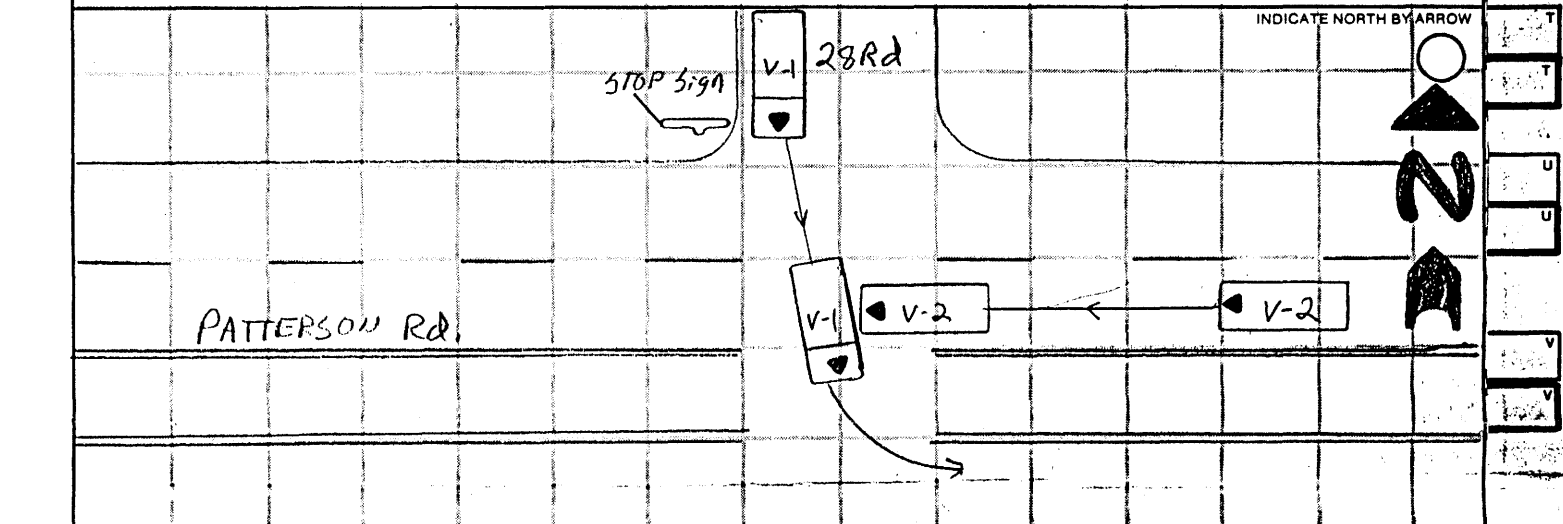
|         |      |       |          |         |      |       |          |
|---------|------|-------|----------|---------|------|-------|----------|
| ADDRESS | CITY | STATE | ZIP CODE | ADDRESS | CITY | STATE | ZIP CODE |
|---------|------|-------|----------|---------|------|-------|----------|

|                             |   |                             |   |
|-----------------------------|---|-----------------------------|---|
| F 2 INS. CO.<br>Continental | DAMAGE SEVERITY:<br>1-SLIGHT<br>2-MODERATE<br>3-EXTREME | INS. CO.<br>Employer's Fire | DAMAGE SEVERITY:<br>1-SLIGHT<br>2-MODERATE<br>3-EXTREME |
|-----------------------------|---|-----------------------------|---|

|           |           |
|-----------|-----------|
| EXP. DATE | EXP. DATE |
|-----------|-----------|

|                      |                  |
|----------------------|------------------|
| G 1 20 Undercarriage | 20 Undercarriage |
|----------------------|------------------|

|   |
|---|
| H 1 DESCRIBE ACCIDENT<br>Vehicle #1 was stopped at a stop sign facing south at 28 Rd. and Patterson Rd. Vehicle #2 was west bound on Patterson Rd. Vehicle #1 entered the intersection in front of vehicle #2. Vehicle #2 Broad sided Vehicle #1. |
|---|



| VEH # (1) | Pos (2) | Res (3) | Ejct (4) | Inj Sev (5) | Phy Inj (6) | Stat (7) | Trp Rpt (8) | Age (9) | Sex (10) | Name/Add (11)     |
|-----------|---------|---------|----------|-------------|-------------|----------|-------------|---------|----------|-------------------|
| 1         | 1       | 1       | 2        | 1           |             |          |             |         |          | Same as Driver #1 |
| 2         | 1       | 1       | 2        | 1           |             |          |             |         |          | Same as Driver #2 |

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 Denver, Colorado 80222  
 DR-447 (1/80)

STATE OF COLORADO  
 INVESTIGATOR'S  
 TRAFFIC ACCIDENT REPORT

GC: D.A. SHEET 1 OF 2 SHEETS

|                                |                     |
|--------------------------------|---------------------|
| CITY SERIAL NUMBER<br>92-54612 | STATE SERIAL NUMBER |
|--------------------------------|---------------------|

|   |           |                              |              |                          |                        |                |               |
|---|-----------|------------------------------|--------------|--------------------------|------------------------|----------------|---------------|
| A | ROAD CODE | DATE OF ACCIDENT<br>12-16-92 | TIME<br>1854 | DAY OF WEEK<br>Wednesday | CITY<br>Grand Junction | COUNTY<br>Mesa | CO. CODE<br>8 |
|---|-----------|------------------------------|--------------|--------------------------|------------------------|----------------|---------------|

|   |   |                                       |              |                            |   |                                  |
|---|---|---------------------------------------|--------------|----------------------------|---|----------------------------------|
| B | 5 | DATE NOTIFIED OF ACCIDENT<br>12-16-92 | TIME<br>1854 | INVESTIGATED AT SCENE<br>X | LAW ENFORCEMENT AGENCY<br>Grand Jct. P.D. | REVIEWING OFFICER<br>[Signature] |
|---|---|---------------------------------------|--------------|----------------------------|---|----------------------------------|

|   |   |                                   |              |                       |                   |                          |                  |
|---|---|-----------------------------------|--------------|-----------------------|-------------------|--------------------------|------------------|
| B | 5 | DATE ARRIVED AT SCENE<br>12-16-92 | TIME<br>1854 | OFFICER NUMBER<br>359 | NAME<br>D. Osvalt | SIGNATURE<br>[Signature] | DETAIL<br>Patrol |
|---|---|-----------------------------------|--------------|-----------------------|-------------------|--------------------------|------------------|

|   |   |                            |   |       |      |   |   |   |   |                                       |
|---|---|----------------------------|---|-------|------|---|---|---|---|---------------------------------------|
| B | 5 | DATE OF REPORT<br>12-16-92 | LOCATION: ROUTE, STREET, ROAD<br>Patterson Road | MILES | FEET | N | S | E | W | OF ROUTE, ST, RD, MILEPOST<br>28 Road |
|---|---|----------------------------|---|-------|------|---|---|---|---|---------------------------------------|

|   |   |                                 |                     |                          |             |                   |             |           |                  |                   |
|---|---|---------------------------------|---------------------|--------------------------|-------------|-------------------|-------------|-----------|------------------|-------------------|
| B | 5 | PUBLIC PROPERTY PUBLIC EMPLOYEE | TOTAL VEHICLES<br>2 | DISTRICT NUMBER<br>280F0 | PHOTO TAKEN | RAILROAD CROSSING | CONST. ZONE | WILD GAME | PRIVATE PROPERTY | INCOMPLETE REPORT |
|---|---|---------------------------------|---------------------|--------------------------|-------------|-------------------|-------------|-----------|------------------|-------------------|

|   |   |                                    |                |          |                                 |                 |          |
|---|---|------------------------------------|----------------|----------|---------------------------------|-----------------|----------|
| B | 7 | VEH #1 OR<br>LAST NAME<br>Galveson | FIRST<br>Danny | MI<br>H. | VEH #2 OR<br>LAST NAME<br>Brady | FIRST<br>Joleen | MI<br>R. |
|---|---|------------------------------------|----------------|----------|---------------------------------|-----------------|----------|

|   |   |   |                        |                                   |                        |
|---|---|---|------------------------|-----------------------------------|------------------------|
| B | 4 | STREET ADDRESS<br>2828 Orchard Ave. #63 | RES. PHONE<br>243-4578 | STREET ADDRESS<br>508 Blevins Rd. | RES. PHONE<br>241-7756 |
|---|---|---|------------------------|-----------------------------------|------------------------|

|   |   |                       |             |                   |                        |                       |             |                   |                        |
|---|---|-----------------------|-------------|-------------------|------------------------|-----------------------|-------------|-------------------|------------------------|
| B | 1 | CITY<br>Grand Jct, CO | STATE<br>CO | ZIP CODE<br>81501 | BUS. PHONE<br>241-2996 | CITY<br>Grand Jct, CO | STATE<br>CO | ZIP CODE<br>81501 | BUS. PHONE<br>245-0900 |
|---|---|-----------------------|-------------|-------------------|------------------------|-----------------------|-------------|-------------------|------------------------|

|   |   |                             |             |          |                           |                            |             |          |                           |
|---|---|-----------------------------|-------------|----------|---------------------------|----------------------------|-------------|----------|---------------------------|
| B | 1 | DRIVER LIC. NO.<br>JG613983 | STATE<br>CO | SEX<br>M | DATE OF BIRTH<br>09-28-54 | DRIVER LIC. NO.<br>P937937 | STATE<br>CO | SEX<br>F | DATE OF BIRTH<br>12-11-73 |
|---|---|-----------------------------|-------------|----------|---------------------------|----------------------------|-------------|----------|---------------------------|

|   |   |   |           |              |           |
|---|---|---|-----------|--------------|-----------|
| C | 1 | VIOLATION(S)<br>D.W.A.I.D. / Failed to Yield ROW. | AGE<br>38 | VIOLATION(S) | AGE<br>19 |
|---|---|---|-----------|--------------|-----------|

|   |   |   |                               |                |                   |                    |                |
|---|---|---|-------------------------------|----------------|-------------------|--------------------|----------------|
| C | 1 | VIOLATION CODE(S)<br>42-4-1202/42-4-603 | CITATION NUMBER(S)<br>A 07948 | COMMON CODE(S) | VIOLATION CODE(S) | CITATION NUMBER(S) | COMMON CODE(S) |
|---|---|---|-------------------------------|----------------|-------------------|--------------------|----------------|

|   |   |              |                  |                  |              |               |                  |
|---|---|--------------|------------------|------------------|--------------|---------------|------------------|
| C | 1 | YEAR<br>1985 | MAKE<br>Plymouth | MODEL<br>Reliant | YEAR<br>1986 | MAKE<br>Honda | MODEL<br>Prelude |
|---|---|--------------|------------------|------------------|--------------|---------------|------------------|

|   |   |                           |             |                   |               |                           |             |                   |              |
|---|---|---------------------------|-------------|-------------------|---------------|---------------------------|-------------|-------------------|--------------|
| D | 1 | LIC. PLATE NO.<br>UKP 445 | STATE<br>CO | BODY TYPE<br>4 DR | COLOR<br>Blue | LIC. PLATE NO.<br>ULM 202 | STATE<br>CO | BODY TYPE<br>2 DR | COLOR<br>Red |
|---|---|---------------------------|-------------|-------------------|---------------|---------------------------|-------------|-------------------|--------------|

|   |   |                                      |                                      |
|---|---|--------------------------------------|--------------------------------------|
| D | 1 | VEH. IDENT. NO.<br>1P3BP46DIFF292043 | VEH. IDENT. NO.<br>JHMBB52339C035596 |
|---|---|--------------------------------------|--------------------------------------|

|   |   |                                  |                 |    |                               |                |          |
|---|---|----------------------------------|-----------------|----|-------------------------------|----------------|----------|
| E | 2 | VEH. OWNER LAST NAME<br>Galveson | FIRST<br>Howard | MI | VEH. OWNER LAST NAME<br>Brady | FIRST<br>Scott | MI<br>L. |
|---|---|----------------------------------|-----------------|----|-------------------------------|----------------|----------|

|   |   |  |                 |
|---|---|--|-----------------|
| E | 2 | ADDRESS<br>539 Spain St. Grand Jct, CO 81501 | ADDRESS<br>Same |
|---|---|--|-----------------|

|   |   |                         |  |                         |  |
|---|---|-------------------------|--|-------------------------|--|
| E | 2 | INS. CO.<br>Mid-Century | DAMAGE SEVERITY:<br>1-SLIGHT 2-MODERATE 3-EXTREME<br>3 3 3 3 3 3 3 3 3 3 | INS. CO.<br>Mid-Century | DAMAGE SEVERITY:<br>1-SLIGHT 2-MODERATE 3-EXTREME<br>3 3 3 3 3 3 3 3 3 3 |
|---|---|-------------------------|--|-------------------------|--|

|   |   |                            |                            |
|---|---|----------------------------|----------------------------|
| F | 2 | POLICY #<br>07 11445 97 26 | POLICY #<br>07 13002 07 97 |
|---|---|----------------------------|----------------------------|

|   |   |                       |                       |
|---|---|-----------------------|-----------------------|
| F | 2 | EXP. DATE<br>04-29-93 | EXP. DATE<br>05-19-93 |
|---|---|-----------------------|-----------------------|

|   |   |                  |                  |
|---|---|------------------|------------------|
| G | 1 | 20 Undercarriage | 20 Undercarriage |
|---|---|------------------|------------------|

|   |   |  |  |
|---|---|--|--|
| G | 1 | VEH # 1 TOWED BY/TO<br>Vic's / 2150 North Ave. | VEH # 2 TOWED BY/TO<br>All Truck / 1007 S. 5th St. |
|---|---|--|--|

|   |   |  |                                |
|---|---|--|--------------------------------|
| G | 1 | OWNER OF DAMAGED PROP. LAST NAME<br>FIRST MI | ADDRESS<br>CITY STATE ZIP CODE |
|---|---|--|--------------------------------|

|   |   |  |
|---|---|--|
| H | 3 | DESCRIBE ACCIDENT<br>Vehicle #2 W/B, #1 lane, 2800 Block Patterson Road. |
|---|---|--|

|   |   |  |
|---|---|--|
| H | 3 | Vehicle #1 S/B on 28 Road and turned E/B onto Patterson Road |
|---|---|--|

|   |   |                      |
|---|---|----------------------|
| H | 3 | Striking Vehicle #2. |
|---|---|----------------------|

| VEH # (1) | Pos (2) | Res (3) | Ejct (4) | Inj Sev (5) | Phy Inj (6) | Stat (7) | Trp Rpt (8) | Age (9) | Sex (10) | Name/ Add (11) |
|-----------|---------|---------|----------|-------------|-------------|----------|-------------|---------|----------|----------------|
| 1         | 1       | 1       | 2        | 1           |             |          |             |         |          | Driver         |
| 2         | 1       | 2       | 2        | 2           | 4           |          |             |         |          | Driver         |

Diagram on Back

INDICATE NORTH BY ARROW





STATE OF COLORADO  
 INVESTIGATOR'S  
 TRAFFIC ACCIDENT REPORT

CITY SERIAL NUMBER: 91-43814  
 STATE SERIAL NUMBER: [Blank]  
 CC DAS OFFICER  
 VICTIM WITH  
 BUSINESS COORDINATOR

**A** ROAD CODE: 1  
 DATE OF ACCIDENT: 10-24-91 TIME: 1610 DAY OF WEEK: Thur CITY: Grand Jct. COUNTY: Mesa CO. CODE: 8  
 DATE NOTIFIED OF ACCIDENT: 10-24-91 TIME: 1612 INVESTIGATED AT SCENE: X LAW ENFORCEMENT AGENCY: Grand Jct. PD REVIEWING OFFICER: S. Cowgill  
**B** 5 DATE ARRIVED AT SCENE: 10-24-91 TIME: 1620 OFFICER NUMBER: 361 NAME: S. Cowgill SIGNATURE: S. Cowgill DETAIL: Patrol  
 DATE OF REPORT: 10-24-91 LOCATION: ROUTE, STREET, ROAD: 28 Rd X AT THE INTERSECTION WITH F Rd.  
 NUMBER KILLED: 0 NUMBER INJURED: 3

**B** PUBLIC PROPERTY PUBLIC EMPLOYEE: 2 TOTAL VEHICLES: 2 DISTRICT NUMBER: 391 PHOTO TAKEN: RAILROAD CROSSING: CONST. ZONE: WILD GAME: PRIVATE PROPERTY: INCOMPLETE REPORT: 5

**B** VEH #1 OR PARKED UNATTENDED PEDESTRIAN # VEH #2 OR PARKED UNATTENDED PEDESTRIAN #  
 LAST NAME: Shawcroft Donna F MI LAST NAME: Purdy Donald H MI  
 STREET ADDRESS: 568 Sunvalley RES. PHONE: 434-8176 STREET ADDRESS: 2897 F Rd. Res. Phone: 242-7311  
 CITY: Grand Jct. STATE: CO. ZIP CODE: 81504 BUS. PHONE: 243-5236 CITY: Grand Jct. STATE: CO. ZIP CODE: 81506 BUS. PHONE: none  
 DRIVER LIC. NO.: L 433973 STATE: CO. SEX: F DATE OF BIRTH: 07-18-57 DRIVER LIC. NO.: J 29920 STATE: CO. SEX: M DATE OF BIRTH: 04-16-29

**C** 1 VIOLATION(S): Failed to yield R/W from stop sign 34 VIOLATION(S): AGE: 62  
 VIOLATION CODE(S): 2-2 CITATION NUMBER(S): 29985 COMMON CODE(S): 373 VIOLATION CODE(S): CITATION NUMBER(S): COMMON CODE(S):  
 YEAR: 89 MAKE: Pontiac MODEL: Sunbird YEAR: 91 MAKE: Mer MODEL: Tracer

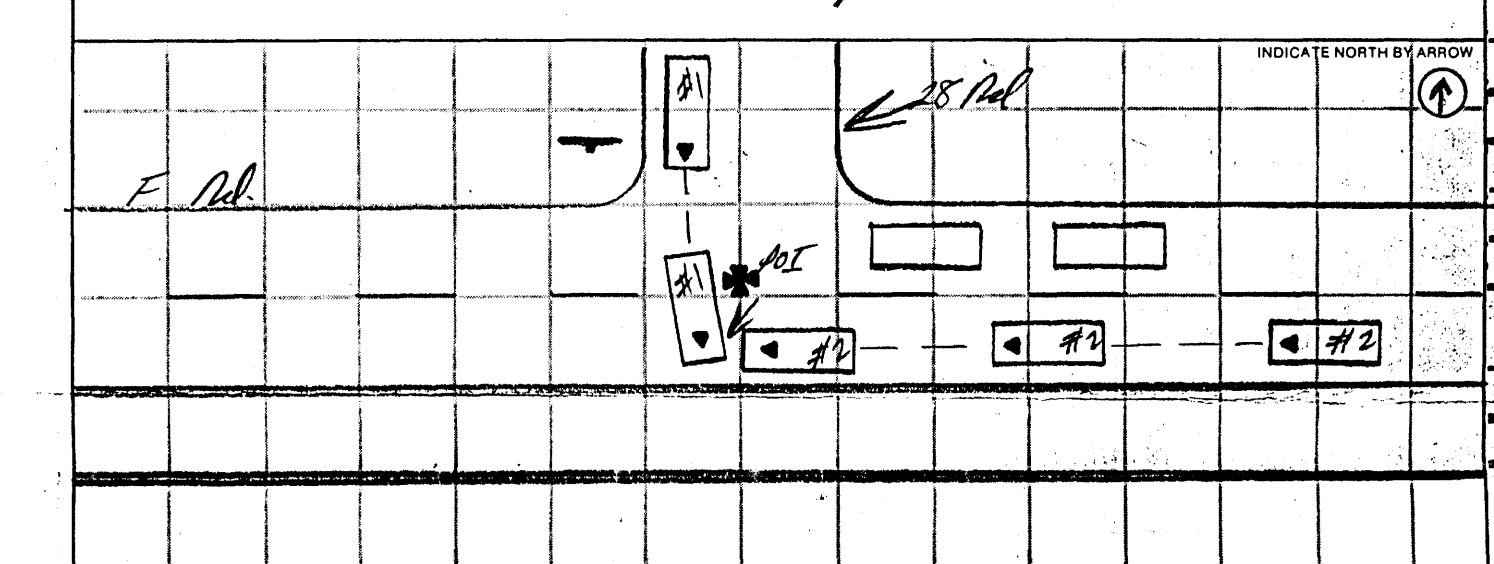
**D** 1 LIC. PLATE NO.: NR 2515 STATE: CO. BODY TYPE: 4dr COLOR: Grey LIC. PLATE NO.: N14305 STATE: CO. BODY TYPE: 4dr COLOR: Silver  
 VEH. IDENT. NO.: 1625851K7K7537202 VEH. IDENT. NO.: 3MAPM15J6M2644098

**E** 1 VEH. OWNER LAST NAME: Same FIRST MI VEH. OWNER LAST NAME: Same FIRST MI  
 ADDRESS: CITY: STATE: ZIP CODE: ADDRESS: CITY: STATE: ZIP CODE:

**F** 2 INS. CO.: Civil Service DAMAGE SEVERITY: 1-SLIGHT 2-MODERATE 3-EXTREME POLICY: COA-0022590 EXP. DATE: 03-23-92  
 INS. CO.: Hartford Ins. DAMAGE SEVERITY: 1-SLIGHT 2-MODERATE 3-EXTREME POLICY: 55PH0368758- EXP. DATE: 07-26-92

**G** 1 20 Undercarriage: VEH #1 TOWED BY/TO: Via Towing OWNER OF DAMAGED PROP. LAST NAME: FIRST MI ADDRESS: CITY: STATE: ZIP CODE:  
 VEH #2 TOWED BY/TO: A-1 Towing

**H** 1 DESCRIBE ACCIDENT: V#1 was SB on 28 Rd and stopped at a stop sign at 28 Rd & F Rd. V#1 then proceeded through the intersection and was struck by V#2 which was WB on F Rd in the left lane.



| VEH # (1) | Pos (2) | Res (3) | Ejct (4) | Inj Sev (5) | Phy Inj (6) | Stat (7) | Trp Rpt (8) | Age (9) | Sex (10) | Name/ Add (11)                        |
|-----------|---------|---------|----------|-------------|-------------|----------|-------------|---------|----------|---------------------------------------|
| 1         | 1       | 1       | 2        | 1           |             |          |             | F 34    |          | Driver                                |
| 1         | 4       | 1       | 2        | 2           | 1           |          | N/A         | 5 F     |          | Laura Shawcroft, 568 Sunvalley, GJ Co |
| 1         | 5       | 1       | 2        | 1           |             |          |             | none M  |          | Cody Shawcroft 568 Sunvalley, GJ Co   |
| 2         | 1       | 1       | 2        | 4           | 4           | 1        | 91-0506     | M 62    |          | Driver                                |
| 2         | 3       | 1       | 2        | 4           | 4           | 1        | 91-0506     | F 60    |          | Bonell Purdy 2897 F Rd GJ Co          |

# STATE OF COLORADO INVESTIGATOR'S TRAFFIC ACCIDENT REPORT

|   |   |  |   |                                     |
|---|---|--|---|-------------------------------------|
| A | 1 | ROAD CODE                                    | CITY SERIAL NUMBER<br><b>9131641</b>  | STATE SERIAL NUMBER                 |
|   |   | DATE OF ACCIDENT<br><b>08-10-91</b>          | TIME<br><b>0900</b>   | DAY OF WEEK<br><b>SAT</b>           |
|   |   | DATE NOTIFIED OF ACCIDENT<br><b>08-10-91</b> | TIME<br><b>0906</b>   | INVESTIGATED AT SCENE<br><b>YLS</b> |
|   |   | DATE ARRIVED AT SCENE<br><b>08-10-91</b>     | TIME<br><b>0915</b>   | OFFICER NUMBER<br><b>352</b>        |
|   |   | DATE OF REPORT<br><b>08-10-91</b>            | LOCATION: ROUTE, STREET, ROAD<br><b>F ROAD X AT THE INTERSECTION WITH 28 RD</b> |                                     |
|   |   | NUMBER KILLED<br><b>0</b>                    | NUMBER INJURED<br><b>0</b>  | DETAIL<br><b>352</b>                |
|   |   | PUBLIC PROPERTY PUBLIC EMPLOYEE              | TOTAL VEHICLES<br><b>2</b>  | DISTRICT NUMBER<br><b>391</b>       |
|   |   | PHOTO TAKEN                                  | RAILROAD CROSSING   | CONST. ZONE                         |
|   |   | WILD GAME                                    | PRIVATE PROPERTY  | INCOMPLETE REPORT                   |

|   |    |   |   |
|---|----|---|---|
| B | 31 | VEH #1 OR PARKED UNATTENDED PEDESTRIAN #          | VEH #2 OR PARKED UNATTENDED PEDESTRIAN #              |
|   |    | LAST NAME FIRST MI<br><b>SCHMALZ RICHARD A</b>    | LAST NAME FIRST MI<br><b>BROWN ERNEST W</b>           |
|   |    | STREET ADDRESS<br><b>463 32 1/8 RD #2</b>         | STREET ADDRESS<br><b>2160 TEXAS AVE</b>               |
|   |    | CITY STATE ZIP CODE<br><b>CLIFTON CO 81520</b>    | CITY STATE ZIP CODE<br><b>GRAND JUNCTION CO 81501</b> |
|   |    | DRIVER LIC NO. STATE SEX<br><b>P. 924616 CO M</b> | DRIVER LIC NO. STATE SEX<br><b>H. 241212 CO M</b>     |

|   |   |  |  |
|---|---|--|--|
| C | 2 | VIOLATION(S)<br><b>No INSURANCE</b>        | VIOLATION(S)<br><b>RECORDS</b>             |
|   |   | VIOLATION CODE(S)<br><b>42-4-12(3)(1)</b>  | VIOLATION CODE(S)                          |
|   |   | CITATION NUMBER(S)<br><b>Z-4093</b>        | CITATION NUMBER(S)                         |
|   |   | COMMON CODE(S)<br><b>954</b>               | COMMON CODE(S)                             |
|   |   | YEAR MAKE MODEL<br><b>69 DODGE 1/2 TON</b> | YEAR MAKE MODEL<br><b>87 OLDSMOBILE 98</b> |

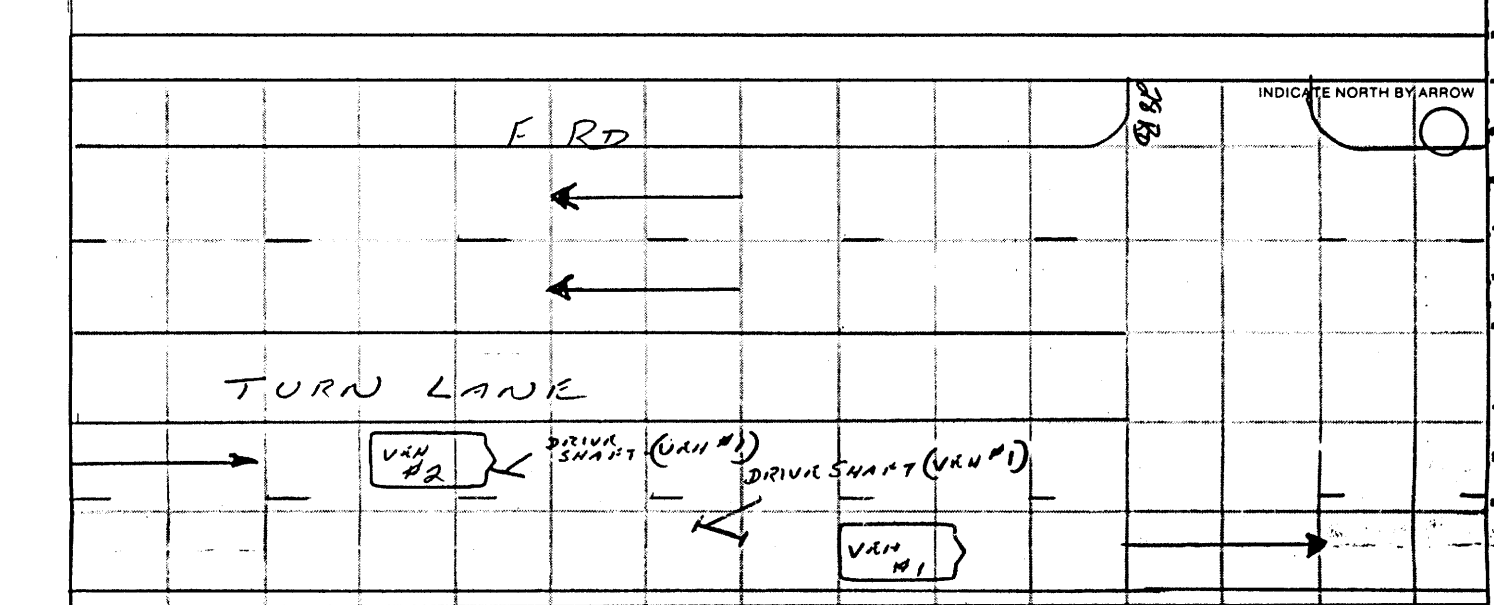
|   |   |   |  |
|---|---|---|--|
| D | 4 | LIC. PLATE NO. STATE BODY TYPE COLOR<br><b>0652 NM CO P1U WHITE</b> | LIC. PLATE NO. STATE BODY TYPE COLOR<br><b>NH-1789 CO 41D GOLD</b> |
|   |   | VEH. IDENT. NO.<br><b>1181887297</b>                                | VEH. IDENT. NO.<br><b>1630WS130H431174</b>                         |

|   |   |  |  |
|---|---|--|--|
| E | 1 | VEH. OWNER LAST NAME FIRST MI<br><b>SCHMALZ JAVED</b>  | VEH. OWNER LAST NAME FIRST MI<br><b>SAME</b> |
|   |   | ADDRESS CITY STATE ZIP CODE<br><b>2910 NORTH 13<sup>TH</sup> AVE GRAND JUNCTION CO 81506</b> | ADDRESS CITY STATE ZIP CODE                  |

|   |   |  |   |
|---|---|--|---|
| F | 2 | INS. CO. DAMAGE SEVERITY:<br><b>None</b> | INS. CO. DAMAGE SEVERITY:<br><b>HAIRTIORD</b> |
|   |   | POLICY #<br><b>-</b>                     | POLICY #<br><b>55PH55737151680</b>            |
|   |   | EXP. DATE<br><b>-</b>                    | EXP. DATE<br><b>08-18-91</b>                  |

|   |   |   |                             |
|---|---|---|-----------------------------|
| G | 1 | 20 Undercarriage                          | 20 Undercarriage            |
|   |   | VEH # TOWED BY/TO                         | VEH # TOWED BY/TO           |
|   |   | OWNER OF DAMAGED PROP. LAST NAME FIRST MI | ADDRESS CITY STATE ZIP CODE |

H 1 DESCRIBE ACCIDENT  
**VEH #1 WAS I/B F RD WEST OF 28 RD. VEH #1 LOST THE DRIVER SHAFT WHICH FELL FROM AND WAS STRUCK BY VEH #2. VEH #2 WAS BEHIND VEH #1 IN THE #1 LANE**



| VEH # (1) | Pos (2) | Res (3) | Ejct (4) | Inj Sev (5) | Phy Inj (6) | Stat (7) | Trp Rpt (8) | Age (9) | Sex (10) | Name/ Add (11) |
|-----------|---------|---------|----------|-------------|-------------|----------|-------------|---------|----------|----------------|
| 1         | 1       | 1       | 2        | 1           |             |          |             |         |          | DRIVER #1      |
| 2         | 1       | 1       | 2        | 1           |             |          |             |         |          | DRIVER #2      |

**APPENDIX B**

**LANDSCAPING STANDARDS**

CSU

# COOPERATIVE EXTENSION SERVICE



619 Main  
Grand Junction, Colorado 81501  
244-1834

1001 North 2nd  
Friendship Hall  
Montrose, Colorado 81401  
249-3935

County Courthouse Annex  
5th & Palmer  
Delta, Colorado 81416  
874-3519

Address Reply to  
Grand Junction  
Office

## TRI RIVER AREA

November 7, 1983 - Updated

### MESA COUNTY LANDSCAPE SPECIFICATIONS (GUIDELINES)

DEVELOPED BY:

CURTIS E. SWIFT, HORTICULTURIST  
TRI RIVER AREA EXTENSION SERVICE

These specifications were developed to update landscaping specifications presently in use. The attached are not designed to totally replace specifications presently in use, but to supplement and update those sections deficient. These guidelines are designed to be used by developers, landscape architects/designers/contractors and others involved with the designing and installation of landscape plant materials.

These specifications provide the most up-to-date guidelines available. Updates will be provided as needed. These specifications were developed in cooperation with Dr. James Feucht, Professor - Horticulture, C.S.U., Golden, Co., and Dr. Jack Butler, Professor - Horticulture, C.S.U., Ft. Collins, Co., and area landscape architects/contractors. The detail drawings were done by Angeline Barrett.

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APPENDIX A

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| 1 - Licensed Arborists                                     |  |
| 2 - Shrub/ground cover planting area - detail drawing      |  |
| 3 - Soil Testing Laboratories                              |  |
| 4 - a - Trees and Shrubs for High Salt                     |  |
| b - Growing Turf on Salt-Affected (Alkali) Sites           |  |
| 5 - Buffalograss lawns - CSU SA#7.224                      |  |
| 6 - Sod growers with salt-tolerant turf                    |  |
| 7 - Laying sod - detail drawing                            |  |
| 8 - Container Grown & Balled and Burlaped - Detail Drawing |  |
| a - Container shrub  |  |
| b - Container shrub - dryland area                         |  |
| c - Container shrub - on slope                             |  |
| d - B&B or container tree - dryland area                   |  |
| e - B&B or container tree - irrigated area                 |  |
| f - B&B conifer - on slope                                 |  |
| g - Creeping ground cover                                  |  |
| h - Lightweight soil mixes - container grown stock         |  |
| 9 - Removing wire - planting detail                        |  |

APPENDIX A (cont'd.)

- 10 - Bare root - planting details
  - a - Tree-lawn area
  - b - Shrub
- 11 - Tile drain aeration system - detail drawing
- 12 - Raising/lowering grade - detail drawings
- 13 - Impact Rotary Head

APPENDIX B - Recommended Reading/Lists

- 1 - Suggested list of drought tolerant plants
- 2 - Suggested watering schedule for drought tolerant species
- 3 - Soil Sampling - CSU - SA#.500
- 4 - Soil Preparation in Colorado's Grand Valley and Adjacent Areas
- 5 - Salt-Affected Soils - CSU SA# .503
- 6 - Irrigation Design/Installation Problems
- 7 - Normal Precipitation and Temperatures - Grand Junction, Co.

LANDSCAPING SPECIFICATIONS  
FOR  
MESA COUNTY, COLORADO

SECTION I - GENERAL REQUIREMENTS

1. QUALITY CONTROL

a) Developer Responsibilities

- 1) Submit landscape and irrigation design plans during the review process. These plans will show actual location and plants selected.
- 2) Select and utilize plant materials (to include grass) based on soluble salt tests (soil test analysis), and available irrigation.
- 3) Provide a listing of recommended salt tolerant and/or drought tolerant plant materials to prospective buyers if moderate to high salt conditions (above 4 mmhos/cm) exist, or if insufficient irrigation water is available.
- 4) Developers will avoid mass plantings (monocultures) of single cultivars/varieties of trees and shrubs to reduce endemic disease and insect problems.
- 5) Select disease/insect resistant plants if feasible.

b) Laboratory Testing Services

Tests called for shall be made by approved laboratories with the full cooperation of the contractor. The laboratory charges, unless otherwise specified, shall be borne by the owner.

c) Inspections

In addition to normal progress inspections, schedule and conduct the following formal inspections, giving the owner or representative at least 24 hours prior notice of readiness for inspection:

- 1) Inspection of plants prior to planting.
- 2) Inspection of plant locations to verify compliance with the drawings.
- 3) Final inspection after completion of planting. Schedule this inspection sufficiently in advance, and in cooperation with the owner or representative so that the final inspection may be conducted within 24 hours after completion of planting.
- 4) Final inspection at the end of the maintenance period, provided that all previous deficiencies have been corrected.

END OF SECTION

SECTION II - SITE CLEARING

1. CLEARING

- a) Remove from the site trees, shrubs, etc. as indicated on the drawings. Arborists doing the work shall be licensed by the City of Grand Junction Forestry Board.
- b) Clearing work shall be restricted to area within property lines or within construction limits indicated on drawings.
- c) No burning is allowed on site. All trees, shrubs, etc. will be transported to an approved disposal site.

2. PROTECTING EXISTING TREES

- a) Heavy equipment will be kept off root zone areas of trees as much as possible. Snow fence will be used, if necessary, to delineate this area.
- b) Excavations necessary for curbing, irrigation lines, etc. will be done with minimal damage to roots. Large roots defined as 2" or more in caliper will not be cut.
- c) Sprinkling lines will be installed under roots if necessary.

3. PRUNING EXISTING TREES

- a) Trees to be saved as shown on drawings will be pruned to remove dead wood and reshape where needed.
- b) Arborist hired will be licensed by the Grand Junction Forestry Board. A Class 'A' license is required. (Appendix A1).
- c) Guidelines of the National Arborist Association will be followed. These include the following:
  - 1) Remove dead, dying, diseased, interfering and weak branches.
  - 2) All cuts must be made sufficiently close to the parent stem so that the wound may heal. The branch collar will be taken into account.
  - 3) Cuts on all limbs one inch or more in diameter shall be done in three stages to prevent splitting.
  - 4) All branches 3½ or more in diameter shall be lowered to the ground with ropes.
  - 5) Pruning tools shall be disinfected after each tree before moving on to the next. Denatured alcohol, a 10 per cent bleach solution, or other disinfectant will be used.
  - 6) If tops or large side branches must be reduced, the drop crotch method shall be used. The leader shall be cut back to a strong lateral branch 1/3 or more the size of the leader, avoiding any stub.

- 7) No spikes will be used for climbing live trees.
- 8) Pruning paints will not be used.
- 9) Pruning cuts made in stone fruits will be treated with Mertect 340F or a paste made of water and benomyl (Benlate) to help prevent Cytospora canker.

END OF SECTION

SECTION III - EARTHWORK

1. SOIL PREPARATION

- a) Topsoil shall be a sandy loam, clay loam, loam, silt loam or sandy clay loam. It shall not have a mixture of subsoil and shall contain no slag, cinders, stones, lumps of soil, sticks, roots, trash or other extraneous materials larger than 1 inch in any dimension.
- b) Soil preparation will consist of the addition of good quality organic matter and nutrients (by means of commercial fertilizers), and tilling to a depth of 4 to 6 inches. All areas to be seeded, sodded or used as planting beds will receive this treatment.
- c) Good quality organic matter will be applied to the soil at the rate of 3 cubic yards per 1,000 square feet of area (2.3 cu. meters/90 square meters). Canadian/Michigan sphagnum peat moss is preferred, however, manure or mountain peat can be used. Manure should be well composted and NOT from a feedlot. Manures or mountain peat should be shredded.
- d) If mountain peat or barnyard manure is used, a soluble salt test will be done. Materials with a salt reading (EC=mmhos/cm) above 3 shall not be used.
- e) Phosphorus will be tilled into the soil (unless otherwise shown by soil test) at a rate per 1,000 square feet as follows:  
  
Superphosphate (0-18-0) - 20 to 30 lbs.  
  
or  
  
Treble Superphosphate (0-46-0) - 10 to 15 lbs.
- f) Additional nutrients will be added according to soil test results submitted through C.S.U. or a private lab.
- g) Stones and other debris over one (1) inch in diameter will be removed.
- h) The use of heavy equipment will be kept to a minimum to avoid root and trunk damage to existing trees.

2. TOPSOIL

- a) Topsoil brought to site shall be free of noxious weeds.
- b) Topsoil shall have a low salt content. Samples will be tested by C.S.U. Soil Laboratory, or private laboratory for soluble salts at the expense of the contractor. Soil with an Ec (mmhos/cm) above 3 will not be used without explicit written permission of consulting horticulturist.

3. EXCAVATION

- a) Areas to be sodded shall be graded  $1\frac{1}{2}$  -  $1\frac{1}{2}$ " below grade to allow space for sod. The final grade with sod in place shall be  $\frac{1}{2}$  to 1" below sidewalks and drives.
- b) Areas to be seeded shall be graded  $\frac{1}{2}$  to 1" below sidewalks and drives.
- c) Areas to be planted will be graded as shown on planting and grading details.
- d) Stockpile topsoil - first 2 inches taken off in separate pile and replace as topsoil after final grading.
- e) Stockpile excavated soil material satisfactory for backfill or fill until required. Place, grade and shape stockpiles for proper drainage.
- f) Remove and dispose of material unsatisfactory for backfill or fill, trash and debris, and all excess material in appropriate off-site landfills or dumping areas.
- g) Keep excavations free of water while work is being performed.
- h) Where underground streams or springs are found, provide temporary drainage and notify owner or representative.
- i) Excavate so that banks of excavation will not be undercut and strata for foundations will not be disturbed.
- j) Excavate unsatisfactory soil materials encountered that extend below required elevations to the additional depth as directed by the owner or representative.
- k) Remove shoring and all form materials upon completion of the work.

4. EXCAVATION FOR TRENCHES

Care will be taken to prevent damage to large roots. See Section II - SITE CLEARING.

END OF SECTION

SECTION IV - CONCRETE FORM WORK

1. GENERAL

- a) Care will be taken to prevent damage to large roots (2" or over in diameter)

END OF SECTION

SECTION V - CAST-IN-PLACE CONCRETE

1. CURBING - MOWING STRIPS, ETC.

- a) Concrete curbing will be installed in such a manner as to prevent damage to large roots (2" or over) of existing trees. Notching forms may be necessary
- b) Deep roots interfering with proposed curbing will be wrapped with a minimum of two (2) inches of biodegradable material prior to pouring concrete. This will allow future root expansion.
- c) Large shallow roots interfering with proposed curbing will be protected if at all possible. In these instances, an opening (gap) will be left in curbing in lieu of cutting the root.

END OF SECTION

SECTION VI - FINISH GRADE

1. FINISH GRADE

- a) All finish grading will be performed to the tolerances described.
- b) Concrete, lumber, large stones and other debris will be removed to a depth of 12 inches.
- c) Fine Grading

Upon completion of finish grading, perform all fine grading required in planting areas.

- d) Shrub and ground cover planting areas:

Level of planting areas shall be 2 - 4" below grade level of lawn, sidewalk, edging, etc. See Drawings for specific information. Planting areas will be covered with 2 - 4" of bark mulch to bring up to grade. (Appendix A2)

END OF SECTION

SECTION VII - PLANTS & PLANTING

1. SOIL TESTS

- 1) A soil analysis will be done in accordance with C.S.U. S.A.# .500 to determine soluble salt and nutrient levels. Test will be done by C.S.U. or a private laboratory facility. (Appendix A3).
- 2) Deficient nutrients will be corrected with the addition of the appropriate fertilizer materials.
- 3) Ornamentals and turf will be selected based on the soluble salt level determined by the soil test. (Appendix A4a & 4b).

2. QUALITY ASSURANCE

a) STANDARDS

- 1) All plants and planting material shall meet or exceed the specifications of Federal, State and County laws requiring inspection for plant disease, insect control and weeds. All plants will be free of insect and disease problems and guaranteed healthy. Any misshapen insect or disease infested, or non-thrifty plants will be rejected.
- 2) Quality and size of woody plants shall conform with the current edition of 'Horticultural Standards' for number one grade nursery stock as adopted by the American Association of Nurserymen.
- 3) All specifications of Article 26, Colorado Nursery Act, pertaining to root spread, definitions, etc. will be followed.
- 4) All plants shall be true to name and one of each bundle or lot shall be tagged with the name and size of plants in accordance with the standards of practice of the American Association of Nurserymen. In all cases, botanical names shall take precedence over common names.

3. PRODUCT HANDLING

a) DELIVERY AND STORAGE

- 1) Deliver all items to the site in their original containers with all labels intact and legible at time of owner or representative's inspection.
- 2) Immediately remove from the site all plants which are not true to name and all materials which do not comply with the provisions of this Section of the Specifications.
- 3) Use all means necessary to protect plant materials before, during and after installation and to protect the installed work and materials of all other trades.



4) Specific treatments of plants include the following:

- a) Trees and shrubs will be delivered in the early morning. Efforts will be taken to prevent wind burn of plant material during transit from nursery to planting site.
- b) Plant material will be placed in shade when arriving at destination.
- c) Care will be taken by nursery personnel to avoid damage to root ball, trunk and branches. This will include rubbing due to improper handling.
- d) Broken branches will be properly pruned by nursery personnel using the drop crotch method.

b) REPLACEMENTS

-In the event of damage or rejection, immediately make all repairs and replacements necessary to the approval of the owner or representative and at no additional cost to the owner.

4. LAWN GRASSES

a) GENERAL

- 1) Grasses will be selected according to salt tolerance as determined by soil test.
- 2) Seeding mixes for salt-affected (alkali) sites will be in accordance with Colorado State University Extension Service SA #7.227 titled "Growing Turf on Salt-Affected (Alkali) sites". (Appendix A4b)
- 3) Seeding of cool season grasses will be done in late summer when possible. Proper irrigation will be provided to allow establishment.
- 4) General soil preparation will be done in accordance with Section III, sub paragraph 1 'Soil Preparation'. Additional specs for seeding and sodding will be done in accordance with following guidelines.
- 5) Specifications for buffalograss and grasses for naturalized areas will be based on C.S.U. SA's #7.224 and \$7.227. (Appendix A5 & 4b)
- 6) Sod will be selected in accordance with salt level and tolerances of sod selected (See Appendix A6).

b) KILLING EXISTING SOD

- 1) Actively growing sod will be treated with glyphosate four weeks prior to soil preparation work commencing.
- 2) Two treatments will be made. These will be made at 7 to 10 day intervals.

- 3) Dead sod can be stripped and removed from location, tilled under or cut with a verticut machine (before seeding).
- 4) Soil will be prepared in accordance with section on Soil Preparation, Seeding and/or Sodding.

c) SODDING - SOIL PREPARATION/INSTALLING

1) GENERAL

Preparatory to sodding, all irregularities in the ground surface shall be corrected. Sticks, stones, debris and other similar material more than one inch (1") in diameter shall be removed. Any objectionable depressions or other variances from a smooth grade shall be corrected. Areas to be sodded shall be smooth before any sodding is done. The soil will be worked well and left firm, but not tightly packed. If walking over the lawn leaves foot prints more than 1" deep after the final working, the soil shall be firmed with a roller.

- 2) The first strip of sod will be laid in a straight line. Thereafter, the sod will be butted together tightly and laid in the manner of bricks in a wall. See figure on detail sheet. (Appendix A7).
- 3) Fertilizing - nutrients will be added during soil preparation in accordance with Section III-Earthwork. After laying sod, periodic fertilizer applications will be made to maintain an attractive and healthy lawn.
- 4) If sod and soil are moist, the sod can be lightly rolled after it is laid; if dry, rolling should be delayed until just before the second watering.
- 5) The sod will be soaked thoroughly as soon as possible after it is laid. Sprinkler system, either new or old, will be in good working order before any sod is planted. This will be demonstrated to owner by contractor. As soon as watering is completed the first time, a few strips of sod will be turned back and the sod and soil examined to be sure they have been soaked.
- 6) If weather is hot and dry, sod will be soaked as necessary. The soil will be wet to 5-6" deep at each watering. In cool weather, sod may not have to be watered more than once every two or three days.
- 7) Sod will be checked 14 days after being laid to see how well it has knit. If sod is well knit, the grass will be watered as needed. If blue areas occur, the lawn will be watered immediately.

d) SOD - KENTUCKY BLUEGRASS

- 1) Kentucky Bluegrass (*Poa pratensis*) sod will not be used if a soil analysis indicates a salt level above 4 mmhos/cm and poor subsurface drainage exists

- 2) Sod will be selected according to disease resistance to Melting Out disease, Dollar Spot Fungi, Stripe Smut and Fusarium Blight. The percentage of bluegrass cultivars will be presented with bid to assist in selecting sod. The percentage of grasses (determined from seed planted) other than bluegrass will also be presented.
  - 3) Sod will be cut, delivered and installed within 24 hours of harvesting.
  - 4) Provide sod that is strongly rooted, free of weeds including undesirable grasses. Provide only sod that is capable of growth and development when planted (not dormant).
  - 5) Standard size sections of sod shall be strong enough to support their own weight when suspended vertically from a firm grasp on the upper 10% of section.
  - 6) When delivered, sod will be stored in the shade and the exposed sod surfaces lightly watered. A moist burlap cover will be used to keep the sod from drying.
  - 7) Sod shall be free of glass, stones, diseases, nematodes, weeds and insect problems.
  - 8) Soil thickness of sod shall not be less than 3/4" nor more than 1".
- e) SOD - SALT TOLERANT
- 1) Sod will be selected based on salt level as determined by soil analysis. C.S.U. SA#7.227 will be used as a guide in selecting grass types. (See Appendix A4b).
  - 2) Sod will be selected according to disease resistance to Melting Out disease, Dollar Spot Fungi, Stripe Smut and Fusarium Blight. The percentage of bluegrass cultivars will be presented with bid to assist in selecting sod. The percentage of grasses (determined from seed planted) other than bluegrass will also be presented.
  - 3) Sod will be cut, delivered and installed within 24 hours of harvesting.
  - 4) Provide sod that is strongly rooted, free of weeds including undesirable grasses. Provide only sod that is capable of growth and development when planted (not dormant).
  - 5) Standard size sections of sod shall be strong enough to support their own weight when suspended vertically from a firm grasp on the upper 10% of section.
  - 6) When delivered, sod will be stored in the shade and the exposed sod surfaces lightly watered. A moist burlap cover will be used to keep the sod from drying.
  - 7) Sod shall be free of glass, stones, diseases, nematodes, weeds and insect problems.

f) SEEDING

- 1) Bluegrass (*Poa pratensis*) alone will not be used if a soil test shows an electrical conductivity in excess of 4 mmhos/cm and poor subsurface drainage exists. Grasses will be selected in accordance with C.S.U. SA#7.227 (Appendix A4b). Colorado Department of Agriculture specifications will be followed in regards to viability and weed seed content.
- 2) Sprinkler system (if present) shall be tested and demonstrated to owner as to effectiveness of water coverage.
- 3) Soil preparation - preparatory to seeding, all areas that have been excavated will be thoroughly soaked to settle fill dirt. After settling, top soil will be added and firmed before seeding.

A disk, rototiller or other appropriate piece of equipment will be used to work the soil to a depth of five to six inches. The soil surface will be smooth with few clods bigger than one inch in diameter remaining.

The soil will be well worked and left firm, but not packed. If walking over the lawn site leaves footprints more than one inch deep after the final working, the soil can be firmed with a roller.

- 4) Fertilizing - a starter fertilizer (e.g. 18-46-0) will be applied at the rate recommended on the bag. The fertilizer will be raked or otherwise worked into the soil surface before seeding.
- 5) Watering - frequent light watering will be made - as many as 3 or 4 a day - until seed has germinated.

5 TREES, SHRUBS AND GROUNDCOVERS

a) GENERAL

- 1) Standards/quality of plant materials will be in accordance with Section VII - Plants & Planting Subsection - 2 - Quality Assurance
- 2) Plant nursery stock immediately upon delivery to site and approval by the owner or representative.
- 3) Regularly water all nursery stock in containers and place them in a cool area protected from the sun and drying winds.
- 4) Do not fertilize young trees or shrubs until the following spring. Do not add fertilizer to backfill.
- 5) Backfill soil will be prepared by thoroughly mixing 1/3 coarse decomposed organic matter or Canadian/Michigan sphagnum peat to the existing soil. This backfill will be tested to determine the soluble salt level before use.

b) CONTAINER GROWN AND BALLED AND BURLAPED

- 1) The planting hole will be made 2 - 4" shallower than the soil ball - refer to detail drawings (Appendix A8).
- 2) Container grown plants - Containers will be slit on at least three (3) sides if made of papermache, peat or wood slats (baskets); containers will be removed if made of metal, tarpaper or plastic. Bottoms do not need to be removed. Papermache rims will be removed to prevent any portion from protruding above ground and acting as a wick, drying the soil ball.
- 3) For plants wrapped in burlap, leave the burlap in place except for the top portion. Make sure twine around trunk is cut and removed. Plastic burlap should be removed if possible to avoid root girdling.
- 4) Wire and plastic netting used to hold soil ball together will be removed from the top 2/3rd of the soil ball. (Appendix A9)
- 5) Rootballs will be scored with a knife vertically every 2 to 3 inches (circumference) equidistant around the ball. Cut to a depth not to exceed 1".

c) BARE-ROOT PLANTS

Bare-root plants should be planted in spring only.

- 1) Holes for bare-root plants will be large enough to permit the roots to spread out without crowding or curving around wall of hole. See detail drawing (Appendix A10a & A10b).
- 2) Backfill soil will be prepared by thoroughly mixing 1/3 coarse decomposed organic matter to existing soil (if needed).
- 3) Plant will be set so that crown is at least 2 inches and no more than 4 inches ABOVE surrounding soil or lawn level.
- 4) Add backfill until plant is anchored and will stand by itself. Apply water slowly, using hose to work out large air pockets.
- 5) Add remaining backfill, sloping level up to crown. Settle soil with water. DO NOT TAMP backfill.
- 6) Form a water basin just beyond edge of planting hole if in planting beds. If in lawn area, DO NOT build basins. Basins can be used in those instances where trees and shrubs are watered with sprinklers independently of the lawn (i.e. on a separate valve).

x) LIGHTWEIGHT SOIL MIXES - CONTAINER GROWN STOCK

- 1) Dig a wide but shallow hole.
- 2) Remove container and split rootball with a sharp spade from the bottom half-way up (Appendix 8h).
- 3) 'Butterfly' the split ball and plant being sure that no air space occurs in the 'V' beneath the split ball. Be sure the crown of plant is above final grade of lawn/planting area (Appendix 8h).
- 4) Backfill and water. Do NOT tamp backfill.

d) PLANTING

- 1) Put enough backfill into the hole to hold plant in place. Add water slowly, using a hose to work out large air pockets. When water has receded, add remaining backfill and soak slowly again with water. DO NOT TAMP backfill. Form a water basin just beyond the edge of the planting hole if in planting beds. If in lawn area, do NOT build basins.
- 2) In 5 - 7 days, fill the reservoir with water and resoak. Reapply water only when the soil begins to feel dry at a 2" depth. Avoid frequent, light waterings.
- 3) After planting, apply bark mulch to a depth of 3-5". See Drawings. Do not use plastic over roots unless of type that allows air to reach roots (e.g. Propex®).

e) STAKING, GUY WIRES AND TREE WRAP

- 1) Note drawing details for specifics required for tree staking (Appendix A8 and A10).
- 2) All deciduous trees will be wrapped from the ground to the second whorl of branches. A commercial treated wrap will be used.
- 3) Wrap will be applied on November 1 and removed April 1 (approximate dates). Trees will not be wrapped in summer.
- 4) Wrap will be attached with a single tack and staple at top and bottom or masking tape will be used. Twistems, wire, twine or strapping tape will not be used.
- 5) All B&B/container grown trees will be guyed using 2-3" wide strips of carpet, nylon or indoor-outdoor carpet. Straps will be attached as diagrammed (Appendix A8 & A10). Three wires will be used or all B&B/container grown deciduous trees will be guyed using two stakes in accordance with the drawings (Appendix A8d and e).
- 6) B&B conifers will be guyed using three wires in accordance with detail drawing (Appendix A8f).
- 7) Bare root trees - less than 2 inches in diameter will be supported with two wooden stakes driven into solid ground below planting hole. Ties of broad, soft strapping such as a strip of indoor-outdoor carpeting will be used to guy trees to guying wire. See drawing for details. (Appendix A10).

Trees over 2 inches in diameter will be supported with 3 guys. Guys will be attached to broad, soft strapping such as strips of indoor-outdoor carpeting. See drawing for details. (Appendix 4).

- 8) Guys will be flagged to warn pedestrians. These will be of a reflective quality visible at night.

f) EXISTING TREES

Tile Aeration System - raising grade and hard surfacing.

- 1) Aeration system will be used around all existing trees when the grade is to be raised, or a hard surface, i.e. concrete or asphalt, is to be put over any area within a radius of  $\frac{1}{2}$  the height of the tree.
- 2) Drain tiles or perforated drain tubes will be placed as shown in the Drawings (Appendix All). Care will be taken so as to not damage the roots of the trees that will remain.

Sample Specs #1

- 3) Tile - aeration system - under concrete, asphalt and any other hard surface.
  - a) Grading and preparation of areas to be hard surface will be done in such a manner that minimal crushing/damage of roots will occur.
  - b) Areas after grading soil will be raked to ensure good air infiltration.
  - c) Perforated corrugated pipe or perforated PVC - 2" diameter or larger - will be used. The pipe will be laid on the raked soil as indicated on diagram (Appendix All). Uprights (outlets) will be installed where indicated. Uprights (outlets) will be installed where indicated. Uprights will be PVC and extend 4 inches above finished grade. All joints will be glued together using the proper materials.
  - d) Street outlets will consist of 4" solid PVC.
  - e) Wire mesh caps will be installed on all outlets and uprights. This mesh will be fine and strong enough to prevent trash from plugging aeration system.
  - f) All lines will be installed so that they drain water away from the tree trunk. Lines will be raised or lowered to accomplish this.
  - g) The aeration system will be sunk 2" into the graded soil. A 2" layer of  $\frac{3}{4}$ " gravel will be added. Eight inch wide strips of Propex<sup>®</sup> ) or other woven plastic material will be put over the drainage system to prevent the poured concrete from plugging the system.
  - h) Care will be taken to prevent crushing of the perforated pipe. Damaged pipe will be replaced.
  - i) Only approved 'sterilants' will be used under hard surfaces to control regrowth of weeds. Label directions will be followed in regards to application rates and uses. Casoron<sup>®</sup> ) is recommended for this use.



Sample Specs #2

- 4) Tile - aeration system - when grade is raised
  - a) Grading and preparation of areas to be raised in grade will be done in such a manner that minimal crushing/damage of roots will occur.
  - b) Areas after grading soil will be raked to ensure good air infiltration.
  - c) Perforated corrugated pipe (2") or 2" perforated PVC will be used. The pipe will be laid on raked soil as indicated on diagram (Appendix A12). These will tie in to a drywall at the trunk and extend radially beyond the drip line.
  - d) Ends near drip zone will extend 4 inches above finished grade. All joints will be glued using proper materials.
  - e) Wire mesh caps will be installed on all outlets and uprights. This mesh will be fine yet strong enough to prevent trash from plugging aeration system.
  - f) Perforated pipe will open into drywall every foot around well's circumference. This pipe will be laid directly on disked original grade.
  - g) With large trees a second set of radials will extend from drip line halfway back to tree. These will be open to surface at drip line end.
  - h) All lines will be installed to drain water away from tree trunk. Line will be raised or lowered to accomplish this.
  - i) Two inch (2") diameter gravel will be spread between and over the tile system to a depth of \_\_\_\_\_ inches. Care will be taken to prevent the crushing of the perforated pipe. Damaged pipe will be replaced. A final layer of 3/4 inch gravel will be spread and topped with a permeable material that will prevent soil from filtering into the gravel layer and clogging the pores necessary for air distribution. Propex<sup>®</sup> or other such material will be used for this purpose.
  - j) Fill dirt and top soil will be added to this tile/gravel system according to the architect's specifications.
- g) EXISTING TREES - LOWERING GRADE
  - 1) Trees will be evaluated on site by a qualified horticulturist as to survivability based on amount of grade change.
  - 2) Trees to remain (after inspection) will:

- a) be treated in such a manner as to prevent damage to trunk and root system. This will include keeping heavy equipment away from trunk and out as far as possible beyond drip zone. Grading in this area will be done by hand or small equipment.
- b) have a terrace system designed according to root spread characteristics of tree (Appendix A12).

6. MAINTENANCE

a) GENERAL

Maintain all plantings starting with the planting operations and continuing for 30 calendar days after all planting is complete and approved by the owner or representative.

b) WORK INCLUDED:

- 1) Maintenance shall include all watering, weeding, cultivation, spraying and pruning necessary to keep the plant materials in a healthy growing condition and to keep the planted areas neat and attractive throughout the maintenance period.
- 2) Provide all equipment and means for proper application of water to those planted areas not equipped with an irrigation system.
- 3) Protect all planted areas against damage, including erosion and trespassing and vandalism by providing and maintaining proper safeguards.

c) REPLACEMENTS

- 1) At the end of the maintenance period, all plant materials shall be in a healthy growing condition.
- 2) During the maintenance period, should the appearance of any plant indicate weakness and probability of dying, immediately replace that plant with a new and healthy plant of the same type and size without additional cost to the owner.
- 3) Replacements required because of vandalism or other causes beyond control of the contractor are not part of the contract.

d) EXTENSION OF MAINTENANCE PERIOD:

Continue the maintenance period at no additional cost to the owner until all previously noted deficiencies have been corrected, at which time the final inspection shall be made.

END OF SECTION

**STORMWATER MANAGEMENT PLAN**

**FOR**

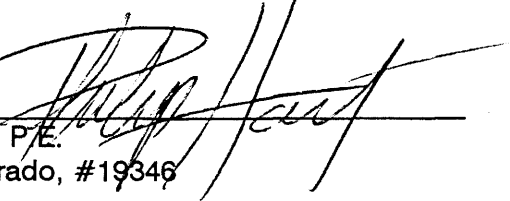
**Grandview Subdivision, Filing No. 1**

July 30, 1994

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## **A. Site and Project Description**

### **1. Site Location:**

Grandview Subdivision is located in the City of Grand Junction, County of Mesa, State of Colorado, more particularly being located in the W 1/2 of Section 6, T.1 S., R.1 E. of the Ute Meridian.

Existing streets within the area of the project include 28 Road to the west and Patterson Road to the south which runs west to east and is to be used as primary access to the site.

Grandview Subdivision is bounded to the west across 28 Road by the Spring Valley Subdivision and to the east, north and south by undeveloped lands. One quarter mile to the south of the Subdivision is Patterson Road.

### **2. Description of Property:**

The Grandview Subdivision Filing 1 contains approximately 11.4 acres including 3 acres of area designated for open-space. Filing 1 contains 27 single family residential lots.

### **3. Description of Proposed Construction Activity:**

Activity shall include the construction of roadway, water, sanitary sewer, storm sewer, irrigation, dry utility infrastructures followed by the construction of 27 single family residential structures and associated landscaping.

### **4. Proposed Sequence of Major Construction Activities:**

Phase I Clearing and grubbing of proposed roadway alignments, storm water detention facilities and disposal of construction debris.

Phase II Construction of roadways to proposed subgrade elevations including cut and fill activities as required. Excess embankment material to be stockpiled in designated areas.

Phase III Utility infrastructures to be installed including storm sewers and culverts, swales and permanent erosion control features.

Phase IV Curb, gutter and sidewalks installed.

Phase V Clearing, Grubbing and overlot grading of single or multiple lots as sales and market conditions allow.

Phase VI Construction of single or multiple building structures as sales and market conditions allow.

Phase VII Final landscaping of individual lots as required by the project Covenants, Conditions and Restrictions.

**5. Estimate of Areas Subject to Clearing, Grubbing and Excavation:**

Grandview Subdivision filing 1 contains a total of 11.4 acres. Construction Phases I through IV will consist of approximately 5.15 acres. Phases V through VII will consist of the residual area of 6.25 acres.

**6. Preconstruction and Postconstruction Runoff Coefficients:**

As defined in the Final Drainage Report For Grandview Filing 1 (References 9) the historic runoff coefficients for the 2 year and 100 year storm events respectively are 0.22 and 0.28. With the construction of proposed roadways coefficients are expected to increase to 0.40 and 0.49 respectively.

**7. Soil Erosion Potential:**

The site soils are classified as (Bc) Billings Silty Clay loam, 0 to 2 percent slopes which falls within the hydrological soil group "C", (Rf), Ravola very fine sandy loam, 0 to 2 percent slopes which falls within the hydrological soil group "B", and (Fc) Fruita and Ravola loams, 2 to 5 percent slopes which falls in hydrological soil group "B".

Erosion of these soils during construction may be moderate if allowed to remain open with no vegetation.

**8. Existing Vegetation:**

Vegetation consists primarily of row crops and pockets of grass ground cover. Isolated pockets of wetland vegetation is found to exist within the existing irrigation and drainage channel along the south boundary line of the project.

**9. Storage of Fuel Oils, Chemicals, Fertilizers or Other Potential Pollution Sources:**

The storage of fuel oils, chemicals, fertilizers or other potential pollutants is prohibited without prior written notice to the owner by the contractor, subcontractor or other persons doing work on the site. In the event it becomes necessary to store such items, storage areas shall be designated. Storage areas shall be located above and away from drainages, waterways and other apparent conveyance elements. Appropriate measures shall be taken to protect such areas from spills or vandalism including but not limited to spill control berms and fencing.

**10. Anticipated Non-Stormwater Components of Discharge:**

Irrigation facilities include a pressurized under ground system supplied by a storage pond located at the southeast corner of Filing One. Offsite residual irrigation runoff is collected and routed underground to the storage pond upon entering the site.

## **11. Name and Location of Receiving Waters:**

Receiving waters of the runoff from the site is first an existing storm water conveyance system routed through Spring Valley Subdivision and eventually into one of the irrigation canals located south of the site.

As defined in the detailed drainage study entitled "Flood Hazard Information, Colorado River and Tributaries" (Reference 2), Grandview Filing 1 is not within the 100 and 500 year floodplains.

### **B. Management During Construction**

#### **1. Anticipated Problems and Corrective (BMPs) Best Management Practices:**

Structural Erosion Control Areas below the toe of fill slopes shall be isolated from fill areas by the installation of prefabricated silt fences as shown on the Drainage and Grading Plan and as detailed on the Erosion Control Plan. Straw bales shall be installed along side and rear yard swales at the locations shown on the plans. Straw bale outlet barriers shall be installed immediately below discharge points and pipe outlets.

Non-Structural Erosion Control Disturbed areas not designated for immediate construction or permanent landscaping shall be temporarily re-vegetated. In the event construction activity ceases for a period of 60 calendar days disturbed areas including cut and fill slopes shall be revegetated with a annual and perennial seed mixture as indicated on the Erosion Control Plan.

Dust Abatement The contractor shall be required to provide a consistent and reliable source of construction water. Watering to prevent dust shall be ongoing for the duration of the project. In the event high winds and heavy traffic loads create a situation where watering by itself is not sufficient the contractor is to apply an approved dust palliative other than or in addition to water.

Soil Tracking Access to Filing No. One shall be from 28 Road which is currently 2 lane asphalt roadway. Where construction traffic enters or exits unimproved areas onto asphalted public roadways a crushed rock construction staging pad shall be installed to minimize soil tracking.

Waste Disposal Construction debris shall be stockpiled in a central location. Debris shall be removed from the site and disposed of at appropriate locations secured by the contractor.

Sedimentation Control The contractor shall be responsible for inspecting the entire site on a weekly basis to ensure compliance and identify existing or potential sedimentation problems. The Final Drainage Reports For Grandview Subdivision (Reference 8 and 9) identifies the 2 existing drainage ditches which will be used for stormwater runoff and detention. Based on field investigations the mannings (N) value for each approaches 0.08. These drainages will provide an excellent sediment control and filtering effect and are to be maintained in their natural state.

### **Final Stabilization and Long Term Management**

The project's Covenants Conditions and Restrictions (Reference 12) obligate each lot owner to fully landscape front yard within 60 days and the rear yard within 1 year from the issuance of a Certificate of Occupancy. Other areas including open-space are to be landscaped by the developer and maintained by the Homeowners Association.

Permanent structural BMP's include pipe outlet protection, rip-rap over filter fabric and grassed swales as shown on the Drainage and Grading Plan.

**Inspection and Maintenance**

The Contractor shall be ultimately responsible for compliance and maintenance during construction. The owners representative and the contractor shall make weekly inspections of the site to assure compliance and implementation of the proposed BMPs.

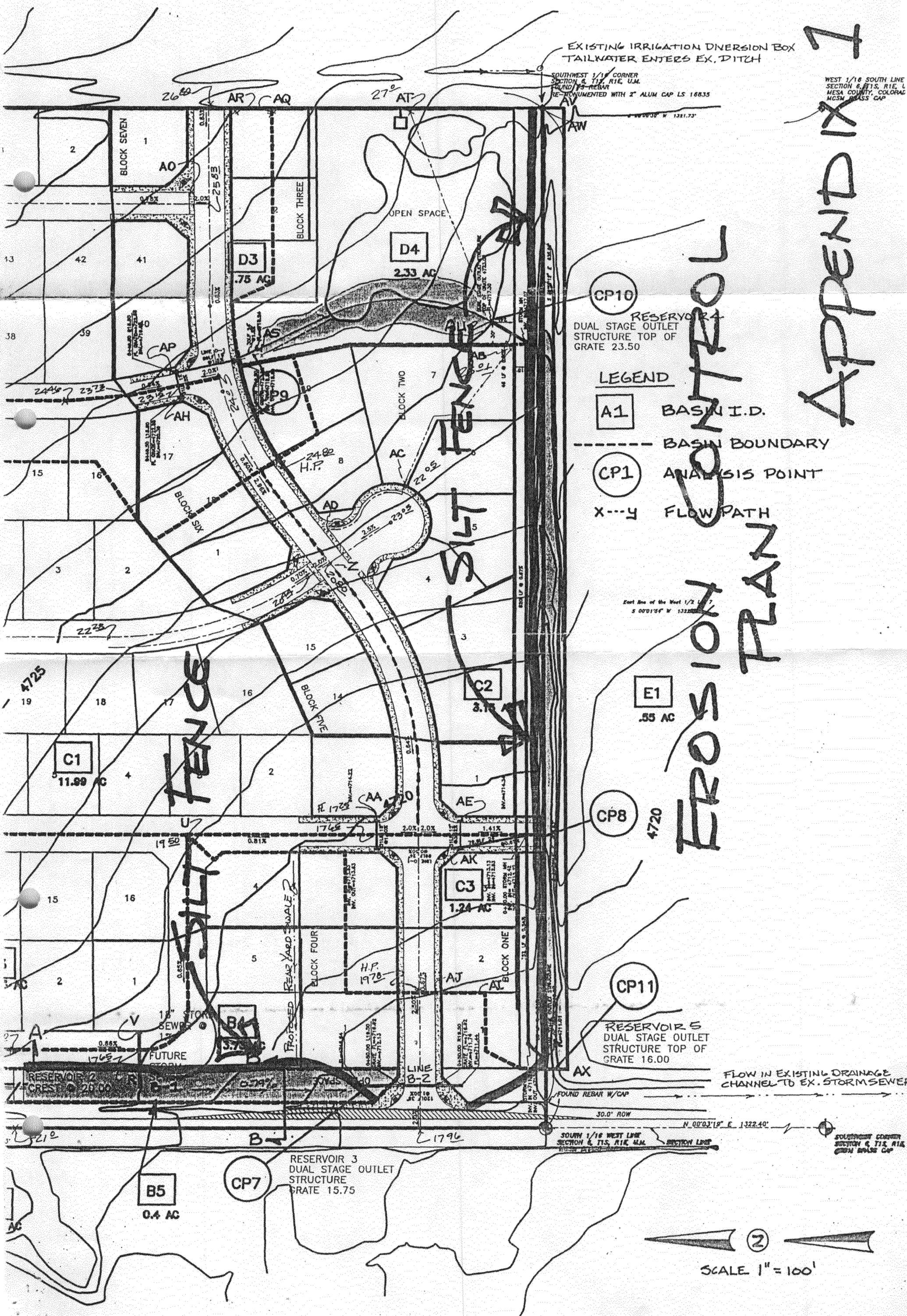


## V. References

1. Mesa County Storm Drainage Criteria Manual, Final Draft, Mesa County, Colorado, March 1992.
2. Flood Hazard Information, Colorado River and Tributaries, Grand Junction, Colorado, prepared for the City of Grand Junction and Mesa County, by The Department Of The Army, Sacramento District, Corps Of Engineers, Sacramento, California, November, 1976.
3. Flood Insurance Rate Map, Mesa County, Colorado, (Unincorporated Areas), Community Panel Number 080115 0480 C, Federal Emergency Management Agency, Map Revised July 15th, 1992.
4. Soil Survey, Grand Junction Area, Colorado, Series 1940, No. 19, U.S. Department of Agriculture, issued November, 1955.
5. Urban Storm Drainage Criteria Manual, Urban Drainage and Flood Control District, prepared by Wright-McLaughlin Engineers, March 1969, Revised May, 1984.
6. Interim Outline of Grading and Drainage Criteria, City of Grand Junction, July 1992.
7. Douglas County Storm Drainage Design and Technical Criteria, Addendum A, Erosion Control Criteria, prepared by HydroDynamics Incorporated, Parker, Colorado, October, 1992.
8. Grandview Subdivision Filing No. One Hydraulic Calculations prepared by HART GROUP, PC, Engineers Designers Planners, A Division of LANDesign, Grand Junction, Colorado, July, 1994.
9. Master Drainage Report for Grandview Subdivision prepared by HART GROUP, PC, Engineers Designers Planners, A Division Of LANDesign, Grand Junction, Colorado, July, 1994.
10. Subsurface Soils Exploration, Grandview Subdivision, Grand Junction, Colorado, prepared by Lincoln-DeVore, Inc., Grand Junction, Colorado, August 3, 1993.
11. Colorado Department of Transportation, Erosion Control and Stormwater Quality Guide, Draft version, November 27, 1992.
12. Declaration Of Covenants, Conditions, And Restrictions Of Grandview Subdivision, Recorded in Book 2055, Pages 317 to 414 of the Mesa County Clerk and Records Office.

**APPENDIX**

APPENDIX 7



EXISTING IRRIGATION DIVERSION BOX  
TAILWATER ENTERS EX. DITCH

SOUTHWEST 1/4 CORNER  
SECTION 6, T15, R1E, U.M.  
FOUND REBAR  
RE-MONUMENTED WITH 2" ALUM CAP LS 18835

WEST 1/16 SOUTH LINE  
SECTION 6, T15, R1E, U.M.  
MESA COUNTY, COLORADO  
HCSM MASS CAP

CP10  
RESERVOIR 4  
DUAL STAGE OUTLET  
STRUCTURE TOP OF  
GRATE 23.50

LEGEND

- A1 BASIN I.D.
- BASIN BOUNDARY
- CP1 ANALYSIS POINT
- X---Y FLOW PATH

E1  
.55 AC

CP8  
4720

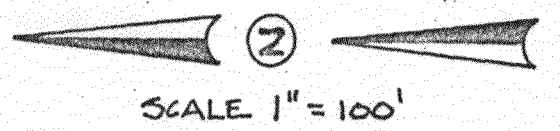
CP11

RESERVOIR 5  
DUAL STAGE OUTLET  
STRUCTURE TOP OF  
GRATE 16.00

FLOW IN EXISTING DRAINAGE  
CHANNEL TO EX. STORM SEWER

SOUTH 1/16 WEST LINE  
SECTION 6, T15, R1E, U.M.  
SECTION LINE

SOUTHWEST CORNER  
SECTION 6, T15, R1E, U.M.  
FOUND REBAR W/ CAP



EROSION CONTROL PLAN

## ROADWAY IMPROVEMENTS

| ITEM               | DESCRIPTION            | UNIT | QUAN. | UNIT PRICE | TOTAL               |
|--------------------|------------------------|------|-------|------------|---------------------|
| 1                  | Excavation             | CY   | 7235  | \$1.25     | \$9,043.75          |
| 2                  | Sub-Grade Preperation  | SY   | 7624  | \$1.95     | \$14,866.80         |
| 3                  | Class 2 Pit Run Gravel | CY   | 1850  | \$9.00     | \$16,650.00         |
| 4                  | Class 6 ABC            | CY   | 1977  | \$18.50    | \$36,574.50         |
| 5                  | Overlot Grading        | CY   | 15000 | \$1.25     | \$18,750.00         |
| 6                  | Grading C HBP          | TON  | 1140  | \$25.50    | \$29,070.00         |
| 7                  | 36" RCP Storm Sewer    | LF   | 219   | \$42.00    | \$9,198.00          |
| 8                  | 24" RCP Storm Sewer    | LF   | 229   | \$30.00    | \$6,870.00          |
| 9                  | 18" RCP Storm Sewer    | LF   | 170   | \$25.00    | \$4,250.00          |
| 10                 | 12" RCP Storm Sewer    | LF   | 62    | \$22.50    | \$1,395.00          |
| 11                 | Storm Sewer Manholes   | EA   | 1     | \$1,200.00 | \$1,200.00          |
| 12                 | Standard Inlets        | EA   | 9     | \$1,200.00 | \$10,800.00         |
| 13                 | Trench Compaction      | LF   | 680   | \$2.10     | \$1,428.00          |
| 14                 | 18" Rip-Rap            | CY   | 100   | \$30.00    | \$3,000.00          |
| 15                 | Flared End Section     | EA   | 3     | \$200.00   | \$600.00            |
| 16                 | 7'-0" Curbwalk         | LF   | 2028  | \$16.00    | \$32,448.00         |
| 17                 | 6'-6" Curbwalk         | LF   | 800   | \$15.50    | \$12,400.00         |
| 18                 | Fillets and Cross Pans | SF   | 7736  | \$3.50     | \$27,076.00         |
| 19                 | Drainage Control Str.  | EA   | 3     | \$2,500.00 | \$7,500.00          |
| 20                 | 3' "V" Pan             | LF   | 305   | \$10.50    | \$3,202.50          |
| 21                 | Traffic Control Signs  | EA   | 34    | \$125.00   | \$4,250.00          |
| 22                 | Adjust MH's & Valves   | EA   | 22    | \$135.00   | \$2,970.00          |
| <b>TOTAL ROADS</b> |                        |      |       |            | <b>\$253,542.55</b> |

**SANITARY SEWER**

| ITEM                        | DESCRIPTION            | UNIT | QUAN. | UNIT PRICE | TOTAL              |
|-----------------------------|------------------------|------|-------|------------|--------------------|
| 1                           | 8" Sanitary Sewer Main | LF   | 2233  | \$10.00    | \$22,330.00        |
| 2                           | 4" Sanitary Sewer Main | LF   | 1270  | \$8.25     | \$10,477.50        |
| 3                           | Standard Manhole       | EA   | 9     | \$1,250.00 | \$11,250.00        |
| 5                           | Trench Compaction      | LF   | 3503  | \$3.50     | \$12,260.50        |
| 6                           | Pipe Bedding           | CY   | 1040  | \$8.00     | \$8,320.00         |
| 7                           | Pavement Replacement   | LF   | 135   | \$24.00    | \$3,240.00         |
| 8                           | Join Existing          | EA   | 1     | \$500.00   | \$500.00           |
| <b>TOTAL SANITARY SEWER</b> |                        |      |       |            | <b>\$68,378.00</b> |

**DOMESTIC WATER**

| ITEM                        | DESCRIPTION              | UNIT | QUAN. | UNIT PRICE | TOTAL              |
|-----------------------------|--------------------------|------|-------|------------|--------------------|
| 1                           | 8" PVC Water Main        | LF   | 1844  | \$14.50    | \$26,738.00        |
| 2                           | 4" PVC Water Main        | LF   | 135   | \$7.50     | \$1,012.50         |
| 3                           | 8" Gate Valve w/Box      | EA   | 9     | \$450.00   | \$4,050.00         |
| 4                           | 4" Gate Valve w/Box      | EA   | 1     | \$250.00   | \$250.00           |
| 5                           | Join Existing Water Main | EA   | 1     | \$3,500.00 | \$3,500.00         |
| 6                           | Service Connection       | EA   | 28    | \$335.00   | \$9,380.00         |
| 7                           | Trench Compaction        | LF   | 2925  | \$2.00     | \$5,850.00         |
| 8                           | Fire Hydrant Assembly    | EA   | 3     | \$1,250.00 | \$3,750.00         |
| 9                           | Asphalt Replacement      | LF   | 25    | \$25.00    | \$625.00           |
| <b>TOTAL DOMESTIC WATER</b> |                          |      |       |            | <b>\$55,155.50</b> |

**MISCELLANEOUS**

---

| ITEM | DESCRIPTION                 | UNIT | QUAN. | UNIT PRICE | TOTAL              |
|------|-----------------------------|------|-------|------------|--------------------|
| 1    | Design/Engineering          | LS   |       |            | \$15,600.00        |
| 2    | Surveying                   | LS   |       |            | \$6,000.00         |
| 3    | Developer's Inspection Cost | LS   |       |            | \$5,000.00         |
| 4    | Quality Control Testing     | LS   |       |            | \$6,000.00         |
| 5    | City Inspection Fees        | LS   |       |            | \$1,000.00         |
| 6    | General Const. Supervision  | EA   |       |            | \$10,000.00        |
|      | <b>TOTAL MISCELLANEOUS</b>  |      |       |            | <b>\$43,600.00</b> |

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|                    |  |  |  |  |                     |
|--------------------|--|--|--|--|---------------------|
| <b>GRAND TOTAL</b> |  |  |  |  | <b>\$420,696.05</b> |
|--------------------|--|--|--|--|---------------------|

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# GRAND VIEW SUBDIVISION

LOCATION : 28 Rd N of Patterson

Length of Roadways as measured on centerline  
on approved "Preliminary Site Development Plan"  
dated April 1994 (revised) (prepared by Thomas A. Logue)

= 11,300 ft  $\approx$  2.14 miles

Measured by: MTD 7/5/94





### North Area Residents—Here's Something You Should Know . . .

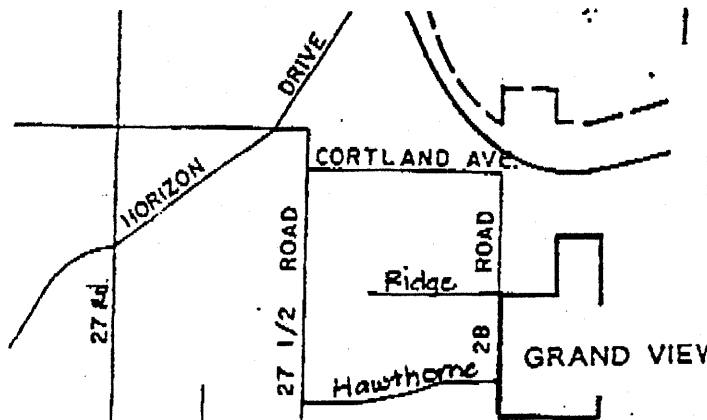
A concerned group of your neighbors have been following the development plans for Matchett's Field (the large parcel of land east of 28 Rd. and north of Patterson Rd.). Grand View, a 200 single family unit subdivision, will begin construction this year (see map below). Please understand that we don't oppose the development. Grand View's density (R3) is less than Spring Valley's, and the development group has a proven local track record. Despite this, we feel that there are serious issues that must be addressed to insure that all of us can maintain the same quality of life we now enjoy. Here are our concerns:

**TRAFFIC:** Primary access to Grand View will be on 28 Road. Hawthorne Ave. and Ridge Ave. will be extended from Spring Valley into Grand View and will serve as the only two entrances into the subdivision. City planners estimate that a subdivision of this size will add approximately 1,900 daily car trips on 28 Road. This impacts the surrounding areas in four ways: 1) Increased traffic will make the intersection at 28 Rd. and Patterson even more difficult to negotiate. 2) Safety of young children becomes an issue if residents of Grand View choose to take Hawthorne or Ridge through Spring Valley to use the light at 27½ Rd. and Patterson. 3) The elevated road bed at the south end of 28 Rd. will aggravate an existing traffic noise problem for residents in that area. 4) All area streets and intersections will be impacted by an increase in traffic.

**SCHOOLS:** The school district estimates, rather conservatively, that Grand View will add 111 students to our already overcrowded schools: 51 to Orchard Avenue (this will push their enrollment to 100 over capacity), 26 to East Middle, and 34 to GJHS. It just seems irresponsible to ask our children to carry the burden of our prosperity.

**PLEASE NOTE:** The developers have petitioned for and have been primarily approved for annexation to the city. The final vote on this will be during the city council meeting on Wednesday, July 6th at 7pm in the City Auditorium. This is our last opportunity before the annexation to voice our concerns. Residents from Spring Valley, Crestview, Ptarmigan Estates, Applecrest, Mantey Heights, and Crown Heights have agreed to speak on your behalf. BUT your presence is still needed. Our experience shows us that an auditorium full of concerned faces backing up the speakers sends a very powerful message. Would you plan on being there? Because you have chosen to live in these areas, we know that you care about your quality of life.

Questions? Call Janet Terry 245-8998



Post-It™ brand fax transmittal memo 7671 # of pages > 1

|                        |                         |
|------------------------|-------------------------|
| To <i>Dave Thorton</i> | From <i>Tom Logue</i>   |
| Co.                    | Co.                     |
| Dept.                  | Phone # <i>244-1599</i> |
| Fax #                  | Fax #                   |



# REVIEW COMMENTS

Page 1 of 3

FILE #85-94

TITLE HEADING: Preliminary Plan/Plat - Grand  
View Subdivision

LOCATION: 28 & F 1/4 Roads

PETITIONER: Ben Carnes & Discovery 76 Corp.

PETITIONER'S ADDRESS/TELEPHONE: 2499 Highway 6 & 50  
Grand Junction, CO 81505  
243-0456

PETITIONER'S REPRESENTATIVE: Tom Logue

STAFF REPRESENTATIVE: Michael Drollinger

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**NOTE: WRITTEN RESPONSE BY THE PETITIONER TO THE REVIEW COMMENTS IS  
REQUIRED ON OR BEFORE 5:00 P.M., MAY 27, 1994.**

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**U.S. WEST**  
**Leon Peach**

**5/4/94**  
**244-4964**

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New or additional telephone facilities necessitated by this project may result in a "contract" and up-front monies required from developer, prior to ordering or placing of said facilities. For more information, please call: Leon Peach, 244-4964.

**U.S. POSTAL SERVICE**  
**Cheryl Fiegel**

**5/4/94**  
**244-3435**

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1. Delivery options are centralized - preferred - immediate deliver - curbside or behind sidewalk -50% complete prior to delivery.
2. Townhomes will need to be centralized due to parking concerns.
3. Condos must be centralized and customer provides the equipment.

**CITY UTILITY ENGINEER**  
**Bill Cheney**

**5/9/94**  
**244-1590**

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1. Show utilities for "Preliminary Plan" submittal as required in the SSID manual.
2. Sewer lines to be stubbed to the east property line for all future road extensions.
3. A portion of the sewer lines may have to be enlarged to accommodate development to the east. Sewer system will pay for upgrades if needed.
4. More hydrants will be required than are shown on the "Utility Plan" which is O.K. since the submittal is conceptual only.

**GRAND JUNCTION FIRE DEPARTMENT**  
**George Bennett**

**5/6/94**  
**244-1400**

---

The fire hydrant placement for this subdivision needs to be reconfigured to meet with the standards. These standards require that fire hydrants be placed at each intersection and no greater than 500 feet apart thereafter. Please contact our office for assistance in placing the fire hydrants.

**MESA COUNTY PLANNING**  
**Karl G. Metzner**

**5/11/94**  
**244-1867**

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1. A Grand View Drive already exists at C.25 and 31.75.
2. If the cul-de-sac portion of Boeing Street is vacated, all of the street should be vacated.
3. Cortland Avenue right-of-way extension should be provided to connect future 28 1/4 Road to existing Cortland Avenue.
4. Recommend 3 lots adjacent to open space be deleted to create a larger and more useable open space area around the pond.

**UTE WATER DISTRICT**  
**Gary R. Mathews**

**5/11/94**  
**242-7491**

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1. Fire hydrants should be placed at intersections and no more than 500' apart.
2. Water mains are installed 2-3 foot in oil on the North and East side of the street.
3. All valves are installed at the main line and fire hydrants are required to be valved.
4. Policies and fees in effect at the time of application will apply.

**PARKS & RECREATION DEPARTMENT**  
**Don Hobbs**

**5/12/94**  
**244-1542**

---

Dedication and development, to City standards, of a minimum of 5 continuous acres for a public park should be part of this development. It would be desirable if this could be adjoining to Matchett Village.

**GRAND JUNCTION POLICE DEPARTMENT**  
**Dave Stassen**

**5/16/94**  
**244-3587**

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In reference to the open space that runs along 28 Road, I would recommend that landscaping be of that type does not totally block vision from 28 Road to the homes (i.e. intermittent trees or shrubs as opposed to a solid wall of green). Keeping with this idea, if fencing is to be used at this location, I would recommend a type that allows for unobstructed vision from the street (wrought iron or similar in nature).

I would also suggest some low, decorative, see through fencing around the irrigation pond to keep children from the water.

**CITY UTILITY ENGINEER**

**5/17/94**

**Jody Kliska**

**244-1591**

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See attached comments, red-lined text and red-lined drawings.

**CITY DEVELOPMENT ENGINEER**

**5/18/94**

**Michael Drollinger**

**244-1439**

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See attached comments.

**MESA COUNTY SCHOOL DISTRICT**

**5/16/94**

**Lou Grasso**

**242-8500**

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See attached comments.

85-94 GRAND VIEW SUBDIVISION - 28 Road and F 1/4 Road  
COMMUNITY DEVELOPMENT DEPARTMENT COMMENTS

1. Ridge Drive Boulevard - landscaping should be provided along edges of roadway to provide screening between the roadway and fences (rear yards of adjoining residences). Provide 10 ft. landscaping easement adjacent to roadway along with typical planting detail. Street tree spacing should be no greater than 50 feet. As per the Ordinance, all landscaped areas must have an underground, pressurized irrigation system.
2. A planting detail is required for all proposed landscaped areas including the type and size of all plants.
3. The maintenance of all common private open space areas should be the responsibility of the homeowner's association.
4. The design and location of all landscaping must be such that intersection sight distances are not impaired.
5. The "Entrance Feature" sign proposed is set back over 75 feet from 28 Road. Given the location and distance, it is questionable whether the sign will assist drivers to easily identify the development. The developer should consider an alternative sign location. Also, is a similar sign proposed for Hawthorne Drive?
6. Garfield View Subdivision Lot 1 (adjacent to Ridge Drive) - proposed roadway is immediately adjacent to the lot, no screening is proposed. A fence and/or screening should be provided to buffer adjacent home from Ridge Drive. Issues regarding the driveway location on the adjacent lot also need to be addressed (see Engineer's report). Applicant should attempt to arrange this with homeowner. Details of any arrangement should be indicated on plans.
7. At the time of final, the applicant should attempt to coordinate with adjacent property owners so that arrangements can be made to vacate the entire length of Boring Street.
8. Twenty foot pedestrian access easements should be provided at the ends of Dillon Court (between Lots 38 & 39) and Tamarron Ct. (betw. Lots 7 & 8). The Tamarron Court easement should be coordinated with the adjacent development to provide a linkage to the proposed open space in Matchett Village.
9. "Private Open Space" provided should be relabeled as "Irrigation and Stormwater Detention" unless the applicant has plans to provide open space amenities within the area.
10. "Buffer and Landscape Area" along 28 Road should be redesigned as a berm to adequately screen the homes along Grand View Circle. Detention storage area capacity could be handled in below grade structures (see engineer's report). Area should be relabeled "Detention Storage and Buffer Area". Regarding the police

department comments with reference to this area, we will be in contact with the police to discuss our recommendation.

11. Disposition of small strip of land between Block 9, Lot 8 and 28 Road not indicated on plans.

12. Grand View Drive shall be extended as a stub street to the southern property boundary (through Block 1, Lot 4)

13. A revised project narrative shall be provided.

May 17, 1994

**REVIEW COMMENTS FOR:** Grand View Subdivision #85-94

**TYPE OF REVIEW:** Preliminary Plans

**REVIEWED BY:** Jody Kliska

### **Site Plan**

The median islands on Ridge and Hawthorne need to be dedicated to the homeowner's association and they will have maintenance responsibility.

Please adhere to the attached table for minimum curve radius. The radii on Grand View Circle need to be longer or the curves bulbed out like a cul-de-sac.

Grand View should provide a connection to the south for future development of adjacent property.

### **Street Details**

The intersection of Ridge with 28 Road is of concern, particularly the complications caused by the property immediately to the north. Because this property is not a part of this development, the street intersection is slightly offset, and there is inadequate right-of-way to continue the radius and sidewalk. Access to this property is a concern, since their driveway is so close to the intersection. Is it possible to work with this property owner to resolve this?

An R4-7 sign will be required on the median island. Because of the width of the approach, the final plan should include a striping detail for better intersection definition, at least a stop bar and double yellow stripe. The traffic evaluation for this intersection should also include an analysis of the need for and the design of a left-turn lane, including storage length requirements.

The 50' sight zone from the curb line must be maintained - no trees, no obstructions over 30" in height. Please note on the detail. The 40' sight triangle must be observed at other intersections within the project.

On section B-B, please note trees must be a minimum of 5' from the back of walk.

Please show the dimensions of the entrance sign. The height above the roadway must not exceed 30".

May 17, 1994

**REVIEW COMMENTS FOR: Grand View Subdivision #85-94**

**TYPE OF REVIEW: Preliminary Plans**

**REVIEWED BY: Jody Kliska**

**page 2**

On the final plans, the roadway plans for the 28 Rd. widening must be shown and these should address pavement markings as well.

### **Grading and Drainage Plan & Report**

Page 9 of the interim outline for Storm Drainage allows the Rational Method for areas up to 25 acres. This development exceeds that.

On the final plan, please provide a detail of the buffer and landscape area for use as detention. Because of the proximity of Grand View Circle to 28 Road, the buffering will be important for noise, headlights, and other distractions. The conceptual plan looks like a series of detention areas. If so, how are these connected?

In the final plan and report, please verify the flows at the intersections of Tamarron Drive at Ridge and Hawthorne are such that the v-pans will carry the flow.

At staff field review the suggestion of a regional detention facility in conjunction with Matchett Village development was discussed. This should be considered prior to final submittal.

### **Traffic**

A traffic impact analysis is required for the final plan submittal. The SSID checklist with notes is attached.

STAFF REVIEW

---

FILE: 85-94-2

DATE: August 30, 1994

REQUEST: Final Plan/Plat Filing #1  
Revised Preliminary Plan  
GRAND VIEW SUBDIVISION

LOCATION: 28 Road at Hawthorn Avenue

APPLICANT: Donada Inc. c/o Don della Motte  
634 Avalon Dr.  
Grand Junction, CO 81501

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EXISTING LAND USE: Vacant, except for a single family residence and two accessory buildings

PROPOSED LAND USE: Single Family Residential

## SURROUNDING LAND USE:

NORTH: Vacant/Single Family Residential  
SOUTH: Vacant/Agricultural  
EAST: Vacant/Agricultural  
WEST: Single Family Residential

EXISTING ZONING: RSF-5

PROPOSED ZONING: No change

## SURROUNDING ZONING:

NORTH: R-2 (County)  
SOUTH: R-2/PR-16 (County)  
EAST: R-2 (County)  
WEST: RSF-5

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## RELATIONSHIP TO COMPREHENSIVE PLAN:

No comprehensive plan exists for this area

## STAFF ANALYSIS:

The site is located north of Patterson Road and east of 28 Road and consists of approximately



65 acres. The property was recently annexed into the City and is zoned RSF-5.

The petitioner is requesting Final Plat/Plan approval for Filing #1 consisting of 27 single family lots. The street layout in Filing #1 (and in the remainder of the development) has been modified since Preliminary Plan approval was granted in June (File #85-94). Section 6-8-1(F) of the Zoning and Development Code permits the Administrator to require submittal of materials to adequately review any changes to the to the plan and specify the process for approval of the modifications (based on the scope of the change). The petitioner has been advised that a revised Preliminary Plan will be required to be approved by Planning Commission illustrating all proposed changes. The applicant has submitted a revised Preliminary Plan for the project for consideration by Planning Commission.

#### Revised Preliminary Plan

The major changes to the Preliminary Plan from the original are as follows:

1. The number of units has increased from 200 to 204 as a result of road realignments. The gross density of the site remains at 3.1 units/acre
2. Tamarron Court cul-de-sac now designed as an "eyebrow".
3. Pagosa Court cul-de-sac (south of Hawthorn) shortened
4. Peak Court cul-de-sac eliminated, now an "eyebrow".
5. Cimarron Court/Pagosa Court (north of Hawthorn) reconfigured to loop road, renamed Pagosa Drive.

There are also a number of other minor realignments/reconfiguration of the street network. The plan should be revised to show a 30 ft. half-street ROW for Courtland Avenue consistent with a draft Major Street Plan for the area.

The applicant has addressed other staff comments/recommendations regarding this submittal.

#### Final Plan - Filing #1

Filing #1 consists of 27 lots on 11.4 acres. An irrigation and drainage facility will also be constructed in conjunction with Filing #1. The applicant has adequately addressed staff comments/recommendations regarding this submittal.

#### STAFF RECOMMENDATION:

Staff recommends approval of the revised Preliminary Plan and Final Plan for Filing #1 subject to the items identified above.

#### SUGGESTED PLANNING COMMISSION MOTION:

Mr. Chairman, on item #85-94-2, a request for revised preliminary approval and final approval for Filing #1, I move that the application be approved subject to the conditions in the staff report.

# STATE OF COLORADO

## COLORADO GEOLOGICAL SURVEY

Division of Minerals and Geology

Department of Natural Resources  
1313 Sherman Street, Room 715  
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DEPARTMENT OF  
NATURAL  
RESOURCES

Roy Romer  
Governor

James S. Lochhead  
Executive Director

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Division Director

Vicki Cowart  
State Geologist  
and Director

FAXED  
8/30/94

August 30, 1994

MA-95-0009

Mr. Michael Drollinger  
City of Grand Junction  
Community Development Department  
250 North 5th Street  
Grand Junction, Colorado 81501

Re: Proposed Grand View Subdivision Filing 1 -- North of Patterson Road and East of  
28 Road, Northeast of the Hawthorne Avenue Alignment, Grand Junction

Dear Mr. Drollinger:

At your request, we have reviewed the materials submitted for and made a field inspection of the site of the proposed first filing of the residential subdivision indicated above. The following comments summarize our findings.

(1) The geologic conditions of this site are simple and straightforward, but may present serious problems for housing construction if not considered adequately in overall development planning and structural designs. The near-surface "soils" present are typically relatively low density clayey materials derived from alluvium. These soils are subject to settlement if loaded with relatively heavy or concentrated loads. These surficial materials overlie Mancos Shale bedrock. The Mancos exhibits low permeability and, because of past and present irrigation in the vicinity, a shallow perched water table occurs on the shale bedrock. This condition can be expected to persist after subdivision buildout because of landscaping irrigation in this and nearby contiguous subdivisions.

(2) Because of the conditions indicated above, the most suitable type of house design in this subdivision will be a single story frame structure without a basement. Such buildings can be safely founded on correctly sized conventional spread footings. Heavier structures will require more elaborate (and expensive) foundation systems. Various options are adequately discussed in the submitted Lincoln-Devore, Inc., report. In all cases, we recommend that


Mr. Michael Drollinger  
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individual building sites be investigated by a qualified soils and foundation engineer prior to selection of (each) foundation design.

(3) It will be most important to maintain good positive drainage away from houses. Roof runoff and irrigation water should not be allowed to pond or puddle near foundations and proper grades in the streets should be maintained to carry away storm runoff. This site in its undeveloped condition is susceptible to shallow flooding during heavy storms and some initial site regrading will be necessary to improve drainage prior to development as a housing subdivision. If it has not been done already, a drainage plan that not only considers drainage of this parcel, but also in contiguous properties should be made.

The recommendations made in the submitted Lincoln Devore, Inc., report are valid and thorough and should be followed. If the recommendations made above are followed and made conditions of approval of this proposal, then we have no geology-related objection to it.

Sincerely,

  
James M. Soule  
Engineering Geologist

#85-94-2

FINAL PLAN/PLAT, REVISED PRELIMINARY PLAN  
GRAND VIEW SUBDIVISION: 28 Rd. at Hawthorn Ave.  
COMMUNITY DEVELOPMENT DEPARTMENT COMMENTS

Revised Preliminary Plan

1. The title of the drawing should be amended to read "Revised Preliminary Plan."
2. Items 12 and 18 on the Preliminary Plan Drawing Standards Checklist (copy attached) must be indicated on the revised Preliminary Plan. These items were shown on the original drawing.
3. The tracts which will be under common ownership shall be appropriately labeled to reflect their intended use. The tract between Grand View Circle and 28 Road shall be labeled as "Detention Storage and Buffer Area." Tract "A" shall be relabeled "Private Open Space/Detention/ Irrigation"; Tracts "B" & "C" shall be relabeled "Detention Storage and Buffer Area."
4. Twenty (20) foot pedestrian easements shall be identified on the Plan at the end of Dillon Court between Lots 38 & 39 and between Lots 7 & 8 in Block 7 as required during the preliminary approval process.
5. The full Ridge Drive ROW is not identified on the plans adjacent to 28 Road and shall be indicated on the plans.
6. The existing Boeing Street cul-de-sac ROW should be identified on the plans and be labeled as "to be vacated."
7. The zoning district for the property should be identified on the plans (RSF-5).
8. The project density in the "Land Use Summary" table should be corrected to read "3.1".
9. The "Principal Building Setbacks" table incorrectly identifies the RSF-5 zone setbacks. The table should be revised as follows:

**RSF-5**

Front yard: Local street - 45 ft. from ROW centerline  
Collector street - 50 ft. from ROW centerline  
Side Yard - 5 ft.  
Rear yard - 25 ft.  
NO lots permitted to front on Ridge Drive

Final Plat - Filing #1

1. Setback table on plat incorrect, should be revised as per comment #9 above.

2. Based on the revised preliminary plan submitted, will the name for Cimmaron Drive remain?

3. Landscape Plan (Sheet LS-2) -

a. General Notes - reference to "Mesa County Land Development Code" should be deleted

b. "Type C" wall detail should be clearly labeled for identification

Burse

**WALKER FIELD AIRPORT AUTHORITY**  
**Mike Sutherland**

**8/04/94**  
**244-9100**

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This development lies within the Airport Area of Influence, as well as underlying common aircraft traffic patterns, so may be affected by overflight of aircraft. An Avigation Easement is required to be recorded at or before filing of the subdivision plat. Please send a copy of the recorded document to Walker Field Airport Authority following recording.

It is our recommendation that, due to this residential development's proximity to aircraft flight paths and the airport proper, that additional soundproofing insulation - as well as planned landscape features - be designed into each residence and site to help mitigate potential sound-level perceptions.

**CITY FIRE DEPARTMENT**  
**Hank Masterson**

**8/04/94**  
**244-1400**

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A hydrant needs to be placed at the intersection of 28 Road and Hawthorne Avenue. Also, move the hydrant located at block 4, lot 3 to block 5, lot 1.

Since the overall development plan layout has changed from the April, 1994 submittals, a new overall utility composite needs to be submitted showing all hydrant locations.

**MESA COUNTY PLANNING DEPT.**  
**Linda Dannenberger**

**8/03/94**  
**244-1771**

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Connection to church property is desirable for future neighborhood interconnection with the Matchett property.

Will Grandview Drive be wide enough?

No other concerns.

**U.S. POST OFFICE**  
**Cheryl Fiegel**

**8/05/94**  
**244-3435**

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Mail delivery will be:           centralized  
  curbside  
  behind sidewalk - paired at lot lines

Option #'s 2 & 3 cannot receive mail delivery until 14 houses are complete.

Street name of Cimarron is also proposed for Valley Meadow Subdivision & North Valley Subdivision - duplicate street names cause a lot of confusion.

**GRAND VALLEY WATER USERS**  
**G.W. Klapwyk**

**8/16/94**  
**242-5065**

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See attached comments.



Re: Grand <sup>View</sup>~~Valley~~ Filing One Proposal

There remain questions to be clarified and further addressed relative to the drainage channel located along the south boundary of Grand View Filing One, as well as above and below such location.

Item #11 of the "Storm Management Plan" is not entirely correct as said conveyance system is used to carry a combination of seepage water, irrigation return flows and storm water and is not exclusively for storm water. Also, the conveyance beyond Spring Valley Subdivision never discharges into any irrigation canal(s), but rather is mostly underground pipeline with a modest amount of open channel until it reaches the Colorado River. Without regard to the total fore-said system, the immediate controlling limit is the pipeline under Spring Valley Subdivision. Planning for run-off should assure that historic inflow at the 28 Road inlet to such pipeline is not exceeded, as we are advised that the conveyance system beyond Spring Valley has been previously loaded to and/or beyond its capacity.

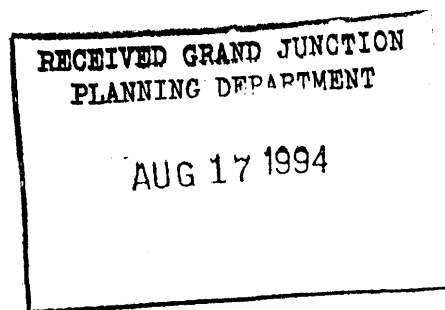
This Association will be glad to continue to discuss and work toward resolution of our questions concerning the drainage channel in the vicinity of Grand View Subdivision - Filing One. However, we will be unable to approve final plan for the subdivision until such questions are suitably resolved.

We look forward to working with subdivision's representatives to reach settlement on our questions and concerns.

Thank You



Grand Valley Water Users' Assn.



Revised

**SCHOOL DISTRICT NO. 51**  
**Lou Grasso**

**8/14/94**  
**242-8500**

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See attached.

**GRAND JUNCTION DRAINAGE DISTRICT**  
**John L. Ballagh**

**8/16/94**  
**242-4343**

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The site is outside the boundaries of the Grand Junction Drainage District but the surface runoff flows into the District maintained Buthorn Drain. The "downstream major drainway" is not an irrigation canal. The crossing under the GVIC canal at 1001 F Road is a 24" CMP. The Buthorn Drain follows Little Bookcliff Dr. to Bookcliff, along a meandering rear lot line alignment to First & Orchard, through West Park to behind Sam's Club.

The system has been upgraded in the last 8 years by the City and the Drainage District. However there is a significant base flow of return flow irrigation water. The system cannot stand increases in flow. Detention outside the District in the upper reaches of the basin is warranted and justifiable.

See attachment.

**CITY DEVELOPMENT ENGINEER**  
**Jody Kliska**

**8/15/94**  
**244-1591**

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See attached comments and red-lined drawings.

**COMMUNITY DEVELOPMENT DEPARTMENT**  
**Michael Drollinger**

**8/15/94**  
**244-1439**

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See attached comments.

**CITY UTILITY ENGINEER**  
**Bill Cheney**

**8/17/94**  
**244-1590**

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Water: Ute water uses corporation stops at meter pit instead of curb stops.  
Sewer: Extend line further into Pagosa Ct. to reduce length of service lines to lots 5, 6 and 7.  
Show water line on profile for Lines "F" and "G".  
Provide information pertaining to sewer line elevations for future filings.

**CITY POLICE DEPT.**  
**Dave Stassen**

**8/12/94**  
**244-3587**

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The only question I have is whether there is going to be any type of safety fencing around the irrigation pond. This fencing would add a measure of safety as well as provide some security against vandalism to this facility.



*\* Revised*

**PUBLIC SERVICE**  
**Dale Clawson**

**8/13/94**  
**244-2695**

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Electric and Gas: Require Tract B and Tract C be designated as utility easement so that utilities can be installed across them. If water meter pits are installed close to side property lines as shown on sewer and water plan, electrical pedestals and transformers as well as phone and television pedestals will have to be located more into the center of the lots!

AVIGATION EASEMENT

THIS EASEMENT is made and entered into by and between the WALKER FIELD, COLORADO, PUBLIC AIRPORT AUTHORITY, a body corporate and politic and constituting a political subdivision of the State of Colorado, hereinafter called GRANTEE, and DONADA, INC.

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hereinafter, GRANTOR;

WHEREAS, Grantee is the owner and operator of Walker Field Airport situated in the County of Mesa, State of Colorado, and in close proximity to the land of Grantor, and Grantee desires to obtain and preserve for the use and benefit of the public a right of free and unobstructed flight for aircraft landing upon, taking off from, or maneuvering about said airport; and

WHEREAS, Grantor is the owner in fee simple of that certain parcel of land situated in the County of Mesa, State of Colorado, to wit:

*See Exhibit "A"*

NOW, THEREFORE, in consideration of the sum of One Dollar (\$1.00) and other good and valuable consideration, the receipt of which is hereby acknowledged, the Grantor, for himself, his heirs, administrators, executors, successors and assigns, does hereby grant, bargain, sell and convey unto the Grantee, its successors and assigns, for the use and benefit of the public, an easement and right of way appurtenant to Walker Field Airport, for the passage of all aircraft ("aircraft" being defined for the purposes of this instrument as any device known or hereafter invented, used or designed for navigation or flight in the air) by whomsoever owned and operated, in the navigable airspace above the surface of Grantor's Property to an infinite height above said Grantor's property, together with the right to cause in said airspace such noise and vibrations, smoke, fumes, glare, dust, fuel particles and all other effects that may be caused by the normal operation of aircraft landing at or taking off from or operating at or on said Walker Field Airport, and Grantor hereby waives, remises and releases any right or cause of action which Grantor now has or which Grantor may have in the future against Grantee, its successors and assigns, due to such noise, vibrations, smoke, fumes, glare, dust, fuel particles and all other effects caused by the normal operation of such aircraft.

FURTHER, Grantor hereby covenants, for and during the life of this easement, that Grantor:

(a) shall not hereafter construct, permit or suffer to maintain upon said land any obstruction that extends into navigable airspace required for use of said airport runway surfaces; (Navigable airspace is defined for the purpose of this instrument



*EXHIBIT "A"*

DESCRIPTION OF PROPERTY

Being the portions of Section 6, Township 1 South, Range 1 East, Ute Meridian, described as follows:

The NE1/4, SW1/4, NW1/4, Section 6,

The SE1/4, SW1/4, NW1/4, Section 6,

The NW1/4, SW1/4, Section 6,

The SW1/4, SW1/4, NW1/4, EXCEPT: Beginning at a point being the West 1/4, Section 6, T1S, R1E, Ute Meridian, thence North 536.25 feet, thence East 165.0 feet, thence South 210.25 feet, thence 261.8 feet along the arc of a 50 foot radius curve to the right, the chord of which bears South 50 feet, thence South 276.0 feet to the South line of the NW1/4 of Section 6, T1S, R1W, Ute Meridian, thence West 165.00 feet to the Point of Beginning,

ALSO EXCEPT: Beginning at the Northwest corner of the SW1/4, SW1/4, NW1/4, Section 6, T1S, R1W, Ute Meridian, thence East 165 feet, thence South 125 feet, thence West 165 feet, thence North 125 feet to the Point of Beginning.