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P	S	A few items are denoted with an asterisk (*), which mea	ans	th	ey are to be scanned for permanent record on the						
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s	n	are also documents specific to certain files, not found on	the	e s	tandard list. For this reason, a checklist has been						
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		quick guide for the contents of each file. Files denoted with (**) are to be located using the ISVS (0	-	System - Planning Clearance will need to be tuned						
		in full as well as other entries such as Ordinances Resolu	Uue	ns.	Board of Appeals and etc						
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X		Receipts for fees paid for anything									
X	X	*Submittal checklist									
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					11/15/96
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APRIL 1995



DEVELOPMEN **G** APPLICATION

Community Development Department 250 North 5th Street, Grand Junction, CO 81501 (303) 244-1430

Receipt	· · · · · · · · · · · · · · · · · · ·
Date	
Rec'd By	
File No.	699-95-182

We, the undersigned, being the owners of property

	situatea în M	esa County, Sta	le of Colorado, as descri	bea nerein ao na	creby petition this:					
PETITION	PHASE	SIZE	LOCATION	2	LONE	LAND USE				
Subdivision Plat/Plan	☐ Minor X Major ☐ Resub	11.4 ac	REDLANDS PKWY. AT RIO LINDA LN	MESA COUN	ГҮ R-2	RESIDENTIAL				
				From:	То:					
Planned Development	DDP Prelim	11.4 ac	REDLANDS PKWY. AT RIO LINDA LN	MESA COUN	ГҮ R-2	RESIDENTIAL				
Conditional Use										
Zone of Annex										
□ Variance										
Special Use										
□ Vacation						□ Right-of Way □ Easement				
Revocable Permit										
PROPERTY OWNE	R	X	DEVELOPER			ESENTATIVE				
B & P DEVELOPMENT	CO., LLC	AL	PINE C.M., INC.	NICHOLS	NICHOLS ASSOCIATES, INC					
Name		Na	ime	Name	Name					
702 GOLFMORE DRIV	Έ	11	11 S. 12TH STREET	751 HOR	751 HORIZON COURT					
Address		Ac	Idress	Address	Address					
GRAND JUNCTION, C City/State/Zip	0 81506	GR. Ci	AND JUNCTION, CO ty/State/Zip	GRAND J City/State/	GRAND JUNCTION, CO 81 City/State/Zip					
245-2505		24	5-2505	245-710	245-7101					
Business Phone No.		Bu	isiness Phone No.	Business P	Business Phone No.					

NOTE: Legal property owner is owner of record on date of submittal.

We hereby acknowledge that we have familiarized ourselves with the rules and regulations with respect to the preparation of this submittal, that the forego information is true and complete to the best of our knowledge, and that we assume the responsibility to monitor the status of the application and the rev computes. We recognize that we or our representative(s) must be present at all required hearings. In the event that the petitioner is not represented, the i will be applied from the agenda, and an additional fee charged to cover rescheduling expenses before it can again be placed on the agenda.

Signature of Person Completing Application

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2 aDate

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for DX & Develop AOA Signature & Property Dwner(s) - attach additional sheets if necessary

9-27 Date

PROJECT NARRATIVE VISTA DEL RIO SUBDIVISION FILING THREE

<u>GENERAL</u>

The proposed Vista Del Rio Subdivision is vacant land on a site adjacent to the west side of the Redlands Parkway, along the south bluff of the Colorado River and abuts Filing One, Filing Two and existing residential development on the west. A smaller portion of the site is located on the east side of the Parkway and is bounded on the east by existing residential and open areas on the south and east. The entire property underwent the development process in 1984 and was accepted as a single family and condominium-type project, but the final plat was never recorded. Filings One and Two of Vista Del Rio Subdivision, approved by Mesa County, were started in 1994 with construction completed in 1995. Several lots are sold and homes are constructed or are being constructed.

The existing county zoning was R-2, which allows 3.5 residential units per acre. The recorded and approved ODP for the county contained 54 residential units (53 proposed single family lots and one existing unit). The original proposed density was approximately 1.86 units/acre.

The proposed filings will lower the approved density. The breakdown is as follows: Filing One - (9) units on 4.5 acres = 2.0 units/acre; Filing Two - (10) units on 6.9 acres = 1.45 units/acre; Filing Three - (23) units on 11.4 acres = 2.02 units/acre; Filing Four (Future Filing east of the Parkway) - (2) units on 3 acres = 0.67 units/acre; and the existing unit - (1) unit on 2.4 acres = 0.42 units/acre.

Several acres of the site will be left as Private Open Space. Areas adjacent to the Redlands Parkway access and along Rio Linda Lane have been or will be irrigated. These areas will be owned and maintained by the homeowners association. Other open areas will be left in their native condition with existing natural vegetation.

PROJECT DESIGN/LAYOUT

The layout and design of roads and lots in all filings in the Subdivision have been predominately dictated by existing internal and external forces.

Rio Linda Lane was installed by the County in the early 1980's as a second (and now primary) access to Loma Rio and adjacent subdivisions. Rio Linda Lane serves as the primary access to Vista Del Rio Subdivision as well.

It was determined early in the project to utilize the existing sewer lines on the site, rather than bear the expense to re-do them. The lines were video taped and found to be serviceable, once they were cleaned. The existing road crossing of the sewer line at Rio Linda Lane was maintained, as well as the existing curb cuts to minimize disruptions to the traffic for the existing neighborhoods.

The existing Public Service easement for the distribution line roughly bisects the property and must be maintained.

Some irrigation and storm water run-off from Loma Rio Subdivision has been flowing east down Rio Linda Lane for the past 12-14 years. The water collected in a low area that was cut as a road bed for the proposed development in 1984. Since that time, trees and vegetation have grown, and the spot is now being classified as a wetland. The area is located in our Filing Three. We are not developing the area, but rather are leaving it as a conservation easement attached to two lots.

The forces listed above have all shaped the development that has occurred in Filings One and Two and continues to dictate lot layouts in this proposed third filing.

BENEFICIAL USE

When we started this project, the property was vacant ground located between existing residential subdivisions and was not suited for agricultural or commercial uses. We felt its highest and best use was as a residential development, closely matching the existing neighborhoods.

We feel in-fill projects such as this should be encouraged by local governing bodies. They can be tied to existing utilities and are within established service districts for police, fire, sanitation, and education and slow down the growth of 'urban sprawl'. Our lot sizes and amenities closely mirror the surrounding developments.

ELECTRICAL DISTRIBUTION LINE

A Public Service Company (PSCO) overhead transmission line previously crossed the site from east to west, bisecting our proposed Phase Three. During 1995 the line was downloaded from a 69-KV transmission line to a 13-KV distribution line. We always felt the overhead line was visually objectionable and wanted to bury it. When it was downloaded this became financially feasible and, in August 1995, we coordinated with PSCO to trench the line. The line is now buried and the easement was changed from 30' to 10'. We feel the improvements this created for the quality of our project far outweigh the costs.

SOILS/GEOLOGIC CONSIDERATIONS

A subsurface soils investigation for the site was performed by Lincoln-DeVore, Inc. in early 1994. Boring locations were included in the previous two filings as well as the proposed Filing Three. The soils report was referenced in and recorded with the Covenants. We require engineered foundations for all homes as well as submittals for grading, drainage, irrigation and landscaping to, hopefully, insure one home doesn't impact another.

PROPOSED STRUCTURES

The structures proposed for this filing will be single family homes on individual lots. Homes may be one or two stories (exclusive of walk-out basements) and will be limited to 35' in height. Minimum heated living space will be a minimum of 1,600 square feet. Exteriors must be sided with at least 30% stucco or masonry. Roof coverings will be architectural grade shingles or better.

The rear elevations of homes on lots that abut the Redlands Parkway require added 'curb appeal' when viewed from behind.

'Zeroscape' or low water plantings will be encouraged, as well as limited amounts of irrigated areas on geologically sensitive portions of the site.

Please refer to the recorded Covenants for the project for more specific information.

SEWERS

Most of the sewers on the project were constructed in 1984 when the property underwent a previous development proposal. Filings One and Two were connected to these lines. After the lines were cleaned and easements written where they crossed through our proposed Filing Three, the City of Grand Junction accepted the sewers. Filing Three will utilize portions of these existing lines, as well as new lines to be constructed into the additional cul-de-sacs.

REQUIRED IMPROVEMENTS

Mesa County required numerous off-site and on-site improvements during the development process for Filings One and Two and are itemized as follows:

Dedicate an additional 20' R.O.W. on the west side of the Redlands Parkway for possible future expansion of the road to four lanes. If after the Parkway was widened and the additional R.O.W. was not used, it could be vacated and returned to the adjacent lots. The additional R.O.W. is included in Filing One and the three plats.

Cut back the existing west bank of the Parkway south of Rio Linda Lane in the road R.O.W. to create a longer sight distance for traffic on Rio Linda Lane turning north. This work was completed in early 1995.

Install an approximately 20 lineal foot extension of El Rio Court off the southwest corner of Filing One that had not previously been completed. This work included curbs, gutters, base course and asphalt paving, as well as drainage structures and easements to convey storm water runoff from El Rio Court across our project and down to the catch basin at the intersection of Rio Linda Lane and the Parkway. This work was completed in early 1995.

The point on Rio Linda Lane where it turns due west up toward Loma Rio Subdivision used to be a 3-way intersection. When Rio Linda Lane was constructed in the early 1980's, it was assumed a future road would go north from the intersection. The north extension was never constructed and Loma Rio runoff ran off the north end of the intersection for years creating the wetlands. Mesa County Planning staff didn't want any new Vista Del Rio roads going north out the 3-way intersection because the northbound road would look like the main road, not the road heading west into Loma Rio, which was the main thoroughfare. They feared people wouldn't know the main road turned left and would go north. They instead required us to access lots north of the 3-way intersection via a culde-sac coming in from the east and take out the intersection altogether and put in a curve in the road. We were also required to install catch basins and direct Loma Rio runoff into the wetlands. This work was completed in the spring of 1995.

A traffic study was required. We were to study the impact our 50+ lots would have on the Redlands Parkway from the intersection at Broadway to the next intersection (the interchange by Mesa Mall). The study showed our subdivision would have a negligible impact on the Parkway and a traffic light at Rio Linda Lane was not warranted. This study was completed in the spring of 1994.

To date, we have spent approximately \$39,200.00 on the above outlined improvements plus the value of the land at the additional right-of-ways which lost two building lots. We request that the City of Grand Junction review the additional improvements we have completed and that those monies already expended be credited towards the required fees.

BICYCLE PATH

At the County Commissioners hearing for the Filing One approval, the subject of applying our Development Impact Fees (DIF) towards the construction of a bicycle path was discussed. The proposed path was to parallel the west side of the Redlands Parkway from Rio Linda Lane south to Greenbelt Drive. The purpose of the path was for pedestrians and cyclists to be able to travel south to a stop light and cross the Parkway to reach the main bike path on the east side of the Parkway. Neighbors to the west of Vista Del Rio considered the Parkway too hard and dangerous to cross on foot or bicycle without a stop light. (The traffic study we commissioned said a stop light was not warranted.)

At the end of the discussion the Commissioners requested us to start cost estimates. At this time, everyone thought a bike path would be constructed.

At the County Commissioners hearing for the Filing Two approval, we presented the cost of the bike path, as well as some design constraints and safety concerns it posed. Discussions at the meeting then centered on reasons not to build the path, as the money would be more effective if spent on other improvements. The bike path was almost nixed, when Commissioner Spehar noted, "We promised a bike path to the neighbors in an open meeting last month, and it would not be fair to those people to change what they are expecting, even though it doesn't make as

much sense anymore".

At the end of the meeting we were wondering what we were supposed to do about the bike path. We listened to the tape transcripts of both meetings and still we were unclear as to the direction we should take. Meanwhile, County staff were telling us "Don't forget the bike path". When we asked them what we should provide, we were not given clear direction or designs.

During the course of the project, we were required to provide additional improvements. (See section above - Required Improvements) We felt our expenditure on these items more than satisfied our DIF requirements, but County staff still make occasional references to the bike path.

IRRIGATION SYSTEMS

The subdivision has excellent water rights on Goat Wash and the Colorado River. Water from either source is adequate to supply the entire subdivision. We have installed underground pressurized irrigation lines to all lots in Phases One and Two and will do the same for Filing Three. Open space at the entrance on Rio Linda Lane and the Parkway is irrigated and planted. More irrigated and landscaped areas are planned for the road entrance to Filing Three.

At present, one 7-1/2 HP pump in Goat Wash supplies water to Filing One and the common areas. When water is needed in Filing Two (up on the hill) an additional 30 HP will be installed. It was designed to handle the entire subdivision. Should water ever dry up in Goat Wash, the system is designed so the pump can be moved to the Colorado River and still supply the whole project.

FIRE PROTECTION

We requested fire hydrant flow tests from the City of Grand Junction Fire Department for fire hydrants in Filings One and Two. The hydrants tested to be more than adequate to meet fire department standards. Mr. Hank Masterson said no further test would be required for lower filings due to the good flows in Filing Two, the highest point in the subdivision.

DEVELOPMENT SCHEDULE

We anticipate completing the development approval process for Filing Three by the end of 1995. Construction on Filing Three is expected to start in the spring of 1996 when the weather warms, with completion by late summer of 1996.

PHASE FOUR

Our original ODP to the County included five proposed phases. Phase One and Two became Filing One and Two; Phase Three and Four became Filing Three; and Phase Five is now Phase Four.

Our proposed Phase Four is an approximately (3) acre parcel on the east side of the

Redlands Parkway and is accessed from 23 Road. With the Filing Three final submittal, we are submitting an amended preliminary plan of Phase Four for approval.

Goat Wash runs the length of this parcel near the edge of the Redlands Parkway right-ofway. Irrigation water for Vista Del Rio is drawn from the wash. We would expect to plat the wash bottom as private open space with access to it for the homeowners association. The buildable portion of the site is a knoll on the east side of the site.

We would anticipate Phase Four will not be governed by the covenants of Filings One, Two and Three, and may not be part of the homeowners association, due to being located in an entirely different neighborhood. These items, though, will be resolved when it is submitted for final approval in 1996.

DRAINAGE

Vista Del Rio Subdivision is adjacent to the Colorado River. The majority of our runoff will be channeled directly into the river, and a smaller portion runs into Goat Wash and then directly into the river. (We are also collecting the neighbors runoff on Rio Linda Lane and El Rio Court, and these are channeled through our structures.)

We feel that no drainage fees or detention/retention structures should be required for this project because our water is routed directly to the Colorado River and has no impact on downstream properties.

In as much as this was also the opinion of the Mesa County Development Engineer no provisions have been made to do any more than discharge directly to the Colorado River.

BLM GRAND JUNCTION, CO 81501

JAMES F./ESTHER M. FOSTER 556 RIO OSO LANE GRAND JUNCTION, CO 81503

GEORGE E./CAROL M. NARVAES 562 RIO OSO LANE GRAND JUNCTION, CO 81503

JEFFERY B. BURWELL 2282 RIO LINDA LANE GRAND JUNCTION, CO 81503

DARREL E./TERRI CARLSON 2283 EL MONTE COURT GRAND JUNCTION, CO 81503

STEVEN P. COLONY P.O. BOX 177 GRAND JUNCTION, CO 81502

ARNOLD L./MARY L. BROWN 1006 21 ROAD FRUITA, CO 81521 JAMES L./BARBARA J. COMSTOCK 552 RIO OSO LANE GRAND JUNCTION, CO 81503

KENNETH J./JUDITH A. BROTSKY 558 RIO OSO LANE GRAND JUNCTION, CO 81503

JOHN R./PATRICIA V. GRIEST 564 RIO OSO LANE GRAND JUNCTION, CO 81503

RANDY O./JANE M. SCHADE 2284 RIO LINDA LANE GRAND JUNCTION, CO 81503

HENRY G./JUDITH K. DRAKE 555 BLUFF COURT GRAND JUNCTION, CO 81503

CLAUDE/DEBORAH U-REN 2261 BROADWAY GRAND JUNCTION, CO 81503

JAMES R./LINDA S. PRINGLE 9266 QUAIL RUN DRIVE SANDY, UT 84093

Alpine C.M. Inc.		Nichols Associates		
1111 S. 12th Street		751 Horizon Ct.		
Grand Junction, CO	81501	Grand Junction, CO	81505	

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JOANNE S. ATKINSON DENNIS W. LOHSE 554 RIO OSO LANE GRAND JUNCTION, CO 81503

PAUL A./JUDY L. BAUMAN 560 RIO OSO LANE GRAND JUNCTION, CO 81503

DEAN G./GLORIA J. REES 566 RIO OSO LANE GRAND JUNCTION, CO 81503

STANLEY L. SELIGMAN 3032 I-70 BUSINESS LOOP GRAND JUNCTION, CO 81504

DOUGLAS/RAMONA L. OSBORN 562 BLUFF COURT GRAND JUNCTION, CO 81503

MARLIN/JANET SCOTTING 2907¹/₂ HERMOSA COURT GRAND JUNCTION, CO 81504

B & P DEVELOPMENT CO. 702 GOLFMORE DRIVE GRAND JUNCTION, CO 81506

City of Grand Junction Community Development Dept. 250 N 5th St. Grand Junction, C0 81501



ROW AND EASEMENT VACATION VISTA DEL RIO FILING #3 FPP-95-182



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LOCATION MAP VISTA DEL RIO FILING #3 FPP-95-182







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FINAL DRAINAGE REPORT

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VISTA DEL RIO SUBDIVISION FILING 3 Mesa County, CO

Prepared for:

Alpine C.M., Inc. Grand Junction, CO

Prepared by:



751 Horizon Ct, Suite 102 Grand Junction, CO

October 6, 1995

Certification Sheet

2 October, 1995

Development Staff City of Grand Junction, Colorado

Ladies and Gentlemen:

A storm drainage system for Vista del Rio, Filing 3 has been designed to collect and convey storm water and discharge it to the Colorado River. The drainage system has been designed to accept historical offsite inflow through an existing wetlands area on the proposed site.

Detention for the increased peak flow rate was not considered since the outfall will continue to be the Colorado River.

I certify this report for the final drainage design of Vista del Rio, Filing 3 was prepared under my direct supervision.

Prepared by: Terry Nichols State of Colorado, Number 12093 Registered Professional Engineer

Nichols Associates, Inc.

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Final Drainage Report

2

VISTA DEL RIO, FILING 3 Mesa County, CO

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I. GENERAL DESCRIPTION AND LOCATION

A. Site and Major Basin Location

Vista del Rio Subdivision, Filing 3 is the final phase of the Vista del Rio Subdivision located in the northeast quarter of section 7, Township 1 South, Range 1 West, Ute Meridian. The subdivision is approximately three and a half miles west of downtown Grand Junction, is bounded on the north by the Colorado River, on the east by the Redlands Parkway and on the west by Loma Rio Subdivision. Rio Linda Lane crosses through the subdivision and is the primary access to the parkway for residents of Rio Linda Subdivision.

Other developments in the vicinity include El Rio Villas to the south and The Bluffs on the north side of the Redlands Parkway. The neighboring developments consist of single family dwellings on lots in the 0.2 to 0.5 acre range. The property across the river to the north is currently undeveloped and used for agricultural and light industrial operations.

B. Site and Major Basin Description

Filing 3 has an area of 13.7 acres. The site was partially developed in the early 80's with rough grading and sanitary sewer main installation. As a result, the historical site properties may not be typical of existing site conditions. Existing vegetation consists of less than 10% cover of native grasses and forbes in most of the property. Inspection of nearby undeveloped, non-irrigated properties indicate site historical conditions may have been as much as 25% ground cover. A observed wetland area of approximately 0.18 acres is located within the Filing 3 boundary. The wetland area is a product of excess lawn irrigation water flowing onto the property from Rio Linda Subdivision.

The major basin has an area of 34.3 acres encompassing Vista del Rio Subdivision and most of Loma Rio Subdivision. Most of the major basin has been developed, therefore existing ground cover is mostly landscaped lawns and impervious areas.

Soils at the site consist of Mesa Clay Loam the and Mesa Cobbly Clay Loam. Both soils are classified as hydrologic soil type "B" by the U.S. Natural Resources Conservation Service.

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II. EXISTING DRAINAGE CONDITIONS

A. Major Basin

The topography of the major basin is a series of terraces separated above the Colorado River with moderate to steep hillsides and natural gullies into the river. The general topography drops to the northeast, toward the Colorado River. Within the developed portions of the major basin, stormwater is diverted to gutters adjacent to the existing streets. Storm water is then routed through the site and outfalls at the Colorado River.

The property as well as the major basin are zoned X (i.e. outside of the 500-year floodplain) by the National Flood Insurance Program. Though the Flood Insurance Rate Maps (FIRMs) do not necessarily identify all areas subject to flooding, no local features have been identified to suggest the FIRM is incorrect.

B. Site

Current drainage patterns at the site are a consequence of partial development of the site in the early 1980's and the inflow from Loma Rio Subdivision to the west. The general topography is similar to the major basin and slopes to the northeast. Runoff from the site collects in mild sloping natural drainage paths that proceed to the edge of the terrace where they quickly increase up to 50% slopes and outfall into the river.

There is one major source of offsite inflow where runoff from Loma Rio Subdivision is routed into the site from Rio Linda Lane. Runoff within Loma Rio Subdivision is routed to concrete gutters adjacent to the streets and travels quickly through the subdivisions 2-5% street grades. Inflow is routed from Rio Linda Lane to the wetlands area of the proposed Vista del Rio Filing 3 where the runoff spreads out into the wetlands area. A much smaller amount of water enters the site from a small, poorly defined drainage labeled outfall "F" on the Historic Conditions exhibit.

Runoff water outfalls the site through a system of gullies on the north side of the terrace. The gullies drop up to 100 feet from the edge of the terrace to the bank of the river.

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III.PROPOSED DRAINAGE CONDITIONS

A. Changes in Drainage Patterns

Drainage patterns in the major basin will be effected at the outfalls into the river. The smaller basins on the north side of the terrace that comprise the outfall basins will be altered and runoff flows will be decreased in some basins. Consequently, the proposed outfall will drain a larger area.

The property for the proposed development currently drains to the northeast, toward the Colorado River. The development will not alter the general slope direction and the current offsite inflow will be accepted into the site. Inflow will be routed through the wetlands area and then into a storm sewer at the wetlands outfall.

The storm sewer is designed to convey the 100 year event. Storm water generated on the site will be routed with street gutters to inlet grates leading into the storm sewer. The storm sewer will extend to the north end north-south street and will not collect runoff beyond this point. The storm sewer will remain buried and follow a natural drainage down to a stilling basin at the river high water mark.

B. Maintenance Issues

The drainage system will be located within dedicated easements to insure access to all parts of the system. A homeowners association will be formed and will accept responsibility of maintenance of the drainage system. Maintenance of the system will include:

clearing debris from the inlets, inspecting for obstructions, and inspecting for structural integrity.

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IV. DESIGN CRITERIA & APPROACH

A. General Considerations

Master planning issues are limited in scope due to the planned discharge into the river and the absence of developed downstream subbasins. The criteria affecting master planning are the same criteria driving the requirements to submit a drainage report.

The most significant site consideration was accepting the large amount of uncontrolled inflow from upstream.

B. Hydrology

Design storm durations conform with Table VI-2 of the City of Grand Junction Storm Water Management Manual, June 1994 (SWMM). Rainfall intensity information was also obtained from the SWMM without adjustment for basin area. Input parameters for the modeling methods were chosen in accordance with the procedures as outlined in the SWMM.

The Rational Method was used to determine storm water quantities using the equation: $\Omega = CiA$

Where:

- C = Runoff coefficient
- A = Area in acres
- i = Intensity at the time of concentration;
- Q = Runoff rate, cfs;

C. Hydraulics

Hydraulic calculations and methods followed those recommended in the SWMM. Mannings Equation was used for pipes and the Modified Mannings Equation was used to determine flows in gutters. The energy and momentum equations were used to examine surcharge in curb boxes and manholes as well as flow velocities Input parameters were selected in accordance with standard engineering practices for the materials chosen for inlets, conveyance, and outlets

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V. RESULTS AND CONCLUSIONS

A. Existing and Proposed Runoff Rates (2- and 100-year storm)

Γ	Runoff Rates					
Γ	2-Year Event	100-Year Event				
	(cfs)	(cfs)				
Existing total site	3.5	11				
Existing discharging to Colorado	16	43				
River						
Proposed total site	3.9	11				
Proposed discharging to	17	46				
Colorado River						

B. Overall Compliance

The design of the proposed drainage system conforms to the requirements of the Grand Junction Stormwater Management Manual. The methods used to analyze stormwater quantities, rates, and volumes have been used in accordance with policy in Sections I through V of the SWMM. Criteria for approved methods were followed as outlined in Tables I-1, and I-2 of the SWMM.

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VII. APPENDICES

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4. HYDRAULICS OF STORM SEWERS

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SENER C مر^نه Although the friction slope Sf appears as a second order term in the expression for ' \tilde{C} ' the resulting discharge is not sensitive to this term. Table 4.11 shows the difference (%) in discharge computed using the Kutter equation compared with that obtained by Manning. The table gives the relationship between the diameter (D) and the hydraulic radius (R) assuming full flow in a circular pipe. The values in Table 4.11 are also valid for noncircular pipes flowing partially full.

WETLAND

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Alignment chart for energy loss in pipes, for Manning's formula. Note: Use chart for flow computations, $H_L = S$

Figure 4.8 Nomograph for solution of Manning's formula.

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Although the friction slope Sf appears as a second order term in the expression for 'C' the resulting discharge is not sensitive to this term. Table 4.11 shows the difference (%) in discharge computed using the Kutter equation compared with that obtained by Manning. The table gives the relationship between the diameter (D) and the hydraulic radius (R) assuming full flow in a circular pipe. The values in Table 4.11 are also valid for noncircular pipes flowing partially full.





4. HYDRAULICS OF STORM SEWERS

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Although the friction slope Sf appears as a second order term in the expression for 'C' the resulting discharge is not sensitive to this term. Table 4.11 shows the difference (%) in discharge computed using the Kutter equation compared with that obtained by Manning. The table gives the relationship between the diameter (D) and the hydraulic radius (R) assuming full flow in a circular pipe. The values in Table 4.11 are also valid for noncircular pipes flowing partially full.



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VISTA DEL RIO - FILING 3

							2 -	· YEAR I	EVENT -	HIST	ORICAL	CONDIT	IONS						2	- YEAR EV	ENT -	DEVELOP	ED CON	DITION	IS
		RLI	RL2	RL	.3		RL5		VDRI		RL4	VDR3		RL6		VDR2	F3		F3.1	INFLOW	F3.2	INFLOW	F3.3	F3.4	1
Surface Type 1 - Landscaped	C	0.2	0.2		0.2		0.2		0.2		0.2	0.2		0.2		0.2	0.3		0.2	то	0.2	TO	0.2	0.2	1
Surface Type 1 area	(ac)	1.91	1.88		2.08		0.56		2.88		3.31	0		1.73		1.1	10.37		2.23	OUTFALL	1.01	REACH	2.85	2.21	
Surface Type 2 - Paved	C	0.93	0.93		0.93		0.93		0.93		0.93	0.93		0.93		0.93	0.93		0.93	G	0.93	I-J	0.93	0.93	
Surface Type 2 area	(ac)	1.03	0.99		1.41		0.67		1.19		1.9	0.13		0.25		0.9	0		0.25	FROM	0.48	FROM	0.72	0.62	
Average Coffecient	C	0.455748	0.451812	0.	.494928		0.597642		0.41344		0.466219	0.93		0.292172		0.5285	0.3		0.273589	OUTFALLS	0.435168	OUTFALL	0.347227	0.359929	
overland travel length	(ft)	90	90		90		30		120		100	375		300		120	300		300	D,E,F	120	H	100	100	L
overland Slope	(%)	1	1		l		<u> </u>		1		1	0.5		1		1	3		1	COMBINED	1	COMBINED	1	1	
T=1.8*(1.1-C)*L^0.5*S^-0.3333		15.36867	15.36867	1:	5.36867		8.873105		17.74621		16.2			28.05922		17.746211	17.2941255		28.05922	WITH	17.74621	WITH	16.2	16.2	ļ
																				FLOW		FLOW			
Channel Shape		parabola	parabola	pai	rabola		parabola		gutter		parabola	parabola	culvert	parabola		gutter	parabola		parabola	FROM	gutter	FROM	gutter	gutter	
width, T	(ft)	2	2	3	2	3.5	2	3.5	1.405188		3	3	3.5	2		1.2933308	2		2	F3.1	1.057429	I	1.195117	1.214808	
depth, d	(ft)	0.2	0.2	0.3	0.22	0.33	0.15	0.33	0.162286		0.23	0.23	0.33	0.2		0.1377555	0.2		0.2		0.092431		0.117821	0.121697	
Cross-sectional now area, a	$\frac{(\pi^2)}{(\pi^2)}$	0.266667	0.266667	0.0 0.	.293333	7 501205	2.020(07	2 691296	0.152029		0.46	0.40	2 501205	0.200007		0.118//30	0.20000007		0.200007		0.065159		0.0938/3	0.098559	<u> </u>
Hudenulia Padina T a/Du	(Π) (Δ)	2.052121	2.052121	3.078182 2	142202	0.215007	2.029007	3.381283	1.453079		3.04638	0.1500080	0.315007	0.120047		0.0902072	2.05212120		0.120047		0.060411		0.076606	0.070064	+
Channel slone S	(11)	0.129947	0.129947	0.19492 0.	0.04	0.213007	0.096341	0.215007	0.104582		0.150999	0.1509989	0.215007	0.123347		0.0892073	0.12994084		0.129947		0.000411		0.070000	0.079004	
Mannings Boughness Coefficient n	(1011)	0.00	0.00	0.05	0.04	0.03	0.03	0.05	0.01		0.04	0.04	0.05	0.00		0.01	0.02		0.02,		0.05		0.05	0.02	
$V = 1.49r^{(2/3)}S^{(1/2)}/n$	(fpc)	7 107909	7 107808	5 780831 6	241129	6 171646	4 23201	7 96756	2.06559		6 496035	6 4960355	7 96756	7 197808		1 8577338	4 15565652		4 155657		2 481032		2 906901	2 423998	
Assumed velocity	(fps)	1.197000	4	5.700051 0.	4	0.1/1040	4	1.90130	2.00000		2	. 4	1.50150	4		1.0577550	4.15505052		4.155057		2.401052		3	3	<u></u>
Reach	(193)	RL1-A	RI.2 - B	A-B R	13-C	B-C	R1.5 - D	C-D	VDR1-D	D	RIA.F	VDR3-E	D-E	F-G	G	VDR2-H	VDR4-7	7.	F3.1-G	G	F3.2-I	I-J	F3.3-J	F3.4-J	Z
flow length, L	(ft)	494	494	283	1010		825	150	600		1010	1010	150	300		320	350		200		500		940	360	+
Travel time L/(60V)	(min)	1.143867	1.143867	0.815915 2.	.697161	0.299758	3.249047	0.313772	3.333333		2.591324	2.5913241	0.313772	0.694656		1.7777778	1.40370921		0.833333		2.777778		5.222222	2	1
Overland Travel Time	(min)	15.36867	15.36867	0, 1:	5.36867	0	8.873105	0	17.74621		16.2	0	0	28.05922		17.746211	17.2941255		28.05922		17.74621		16.2	16.2	
Time of concentration (5 minimum)	(min)	16.51	16.51	0.82	18.07	0.30	12.12	0.31	21.08		18.79	5.00	0.31	28.75		19.52	18.70		28.89		20.52		21.42	18.20	
Intensity	(in/hr)	1.21	1.21	1.21	1.17	1.17	1.41	1.41	1.08		1.14	1.95	1.14	0.9		1.11	1.14		0.9		1.08		1.08	1.17	1
total area	(ac)	2.94	2.87	0	3.49	0	1.23	0	4.07	14.6	5.21	0.13	0	1.98	21.92	2	10.37	34.29	2.48	24.4	1.49	27.89	3.57	2.83	34.2
Q=Va	(cfs)	1.919416	1.919416	3.468498 1.	.830731	4.752167	0.846402	6.135021	0.314029		2.988176	2.9881763	6.135021	1.919416		0.2206535	1.10817507		1.108175		0.161662		0.272881	0.238907	
Q=Cia	(cfs)	1.621279	1.569007	3.190286 2	.020941	5.211227	1.036491	6.247718	1.817316	8.065034	2.76906	0.235755	8.065034	0.52065	11.5905	1.17327	3.54654	16.31031	0.61065	12.201149	0.700272	14.074691	1.338768	1.191762	16.6052
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Surface Type 1 - Landscaped	C	RL1 0.24	RL2 0.24	RL	.3		10 RL5 0.24	0 - YEAF	VDR1 0.24	- HIST	RL4 0.24	VDR3	TIONS	RL6 0.24		VDR2 0.24	F3 0.36		10 F3.1 0.24	0 - YEAR E INFLOW TO	VENT - F3.2 0.24	DEVELO INFLOW TO	PED CO F3.3 0.24	NDITIO F3.4 0.24	NS
Surface Type I - Landscaped	C (ac)	RL1 0.24 1.91	RL2 0.24 1.88	RL	.3 0.24 2.08		10 RL5 0.24 0.56	0 - YEAF	VDR1 0.24 2.88	- HIST	RL4 0.24 3.31	VDR3 0.24 0.25	TIONS	RL6 0.24 1.73		VDR2 0.24 1.1	F3 0.36 10.37		10 F3.1 0.24 2.23	0 - YEAR E INFLOW TO OUTFALL	VENT - F3.2 0.24 1.01	DEVELO INFLOW TO REACH	PED CO F3.3 0.24 2.85	NDITION F3.4 0.24 2.21	NS
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 - Paved	C (ac) C	RL1 0.24 1.91 0.95	RL2 0.24 1.88 0.95	RI	.3 0.24 2.08 0.95		100 RL5 0.24 0.56 0.95	0 - YEAF	EVENT VDRI 0.24 2.88 0.95	- HIST	RL4 0.24 3.31 0.95	VDR3 0.24 0 0.95	TIONS	RL6 0.24 1.73 0.95		VDR2 0.24 1.1 0.95	F3 0.36 10.37 0.95		10 F3.1 0.24 2.23 0.95 0.25	0 - YEAR E INFLOW TO OUTFALL G	VENT - F3.2 0.24 1.01 0.95	DEVELO INFLOW TO REACH I-J	PED CO F3.3 0.24 2.85 0.95 0.72	NDITION F3.4 0.24 2.21 0.95	NS
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average cofficient	C (ac) C (ac)	RL1 0.24 1.91 0.95 1.03	RL2 0.24 1.88 0.95 0.99	RL	_3 0.24 2.08 0.95 1.41 526848		100 RL5 0.56 0.95 0.67 0.626748	0 - YEAF	VDRI 0.24 2.88 0.95 1.19	- HIST	RL4 0.24 3.31 0.95 1.9	VDR3 0.24 0 0.95 0.13	TIONS	RL6 0.24 1.73 0.95 0.25		VDR2 0.24 1.1 0.95 0.9	F3 0.36 10.37 0.95 0 0		10 F3.1 0.24 2.23 0.95 0.25 0.25	0 - YEAR E INFLOW TO OUTFALL G FROM	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725	DEVELO INFLOW TO REACH I-J FROM	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193	NDITIO F3.4 0.24 2.21 0.95 0.62 0.395548	NS
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient Overland travel length	C (ac) C (ac) C (ac)	RL1 0.24 1.91 0.95 1.03 0.488741	RL2 0.24 1.88 0.95 0.99 0.484913 90	RL	23 0.24 2.08 0.95 1.41 526848 90		100 RL5 0.24 0.56 0.95 0.67 0.626748 30	0 - YEAF	EVENT VDRI 0.24 2.88 0.95 1.19 0.447592 120	- HIST	RL4 0.24 3.31 0.95 1.9 0.498925	VDR3 0.24 0 0.95 0.13 0.95 375	TIONS	RL6 0.24 1.73 0.95 0.25 0.329646 300		VDR2 0.24 1.1 0.95 0.9 0.5595	F3 0.36 10.37 0.95 0 0.36 300		10 F3.1 0.24 2.23 0.95 0.25 0.311573 300	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D F F	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120	DEVELO INFLOW TO REACH I-J FROM OUTFALL H	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100	NDITIO F3.4 0.24 2.21 0.95 0.62 0.395548	NS
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland travel length overland Slope	C (ac) C (ac) C (ft) (%)	RL1 0.24 1.91 0.95 1.03 0.488741 90	RL2 0.24 1.88 0.95 0.99 0.484913 90	RI	_3 0.24 2.08 0.95 1.41 526848 90		100 RL5 0.24 0.56 0.95 0.67 0.626748 30	0 - YEAF	EVENT VDR1 0.24 2.88 0.95 1.19 0.447592 120 1	- HIST	RL4 0.24 3.31 0.95 1.9 0.498925 100	VDR3 0.24 0 0.95 0.13 0.95 375 0.5	TIONS	RL6 0.24 1.73 0.95 0.25 0.329646 300		VDR2 0.24 1.1 0.95 0.9 0.5595 120	F3 0.36 10.37 0.95 0 0.36 300		F3.1 0.24 2.23 0.95 0.25 0.311573 300	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100	NDITION F3.4 0.24 2.21 0.95 0.62 0.395548 100	NS
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland travel length overland Slope = 1 & (1 LC)* (0 5*\$0-0 3333	C (ac) C (ac) C (ft) (%)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 1468562	0.	23 0.24 2.08 0.95 1.41 526848 90 1 4.68562		100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745	0 - YEAF	EVENT VDR1 0.24 2.88 0.95 1.19 0.447592 120 1 16 95749	- HIST	RL4 0.24 3.31 0.95 1.9 0.498925 100 1 15.48	CONDIT VDR3 0.24 0.95 0.13 0.95 375 0.5	TIONS	RL6 0.24 1.73 0.95 0.25 0.329646 300 1 26.81215		VDR2 0.24 1.1 0.95 0.99 0.5595 120 1 16 95749	F3 0.36 10.37 0.95 0 0.36 300 3 15 997066		F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26 81215	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 1695749	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 1 1 5 48	NDITION F3.4 0.24 2.21 0.95 0.62 0.395548 100 1 15.48	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333	C (ac) C (ac) C (ft) (%)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562	0.	_3 0.24 2.08 0.95 1.41 _526848 90 1 4.68562		100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745	0 - YEAF	VDRI 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749	- HIST	RL4 0.24 3.31 0.95 1.9 0.498925 100 1 15.48	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5	TIONS	RL6 0.24 1.73 0.95 0.25 0.329646 300 1 26.81215		VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749	F3 0.36 10.37 0.95 0 0.36 300 3 15.9970661		F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 1 15.48	NDITIO F3.4 2.21 0.95 0.62 0.395548 100 1 15.48	NS
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape	C (ac) C (ac) C (ft) (%)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 parabola	RL 0. 14	_3 0.24 _2.08 _0.95 _1.41 _526848 _90 1 4.68562 		100 RL5 0.56 0.95 0.627 0.626748 30 1 8.478745 parabola	0 - YEAF	VDRI 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749 gutter	- HIST	RL4 0.24 3.31 0.95 1.9 0.498925 100 1 15.48 parabola	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5	culvert	RL6 0.24 1.73 0.95 0.25 0.329646 300 1 26.81215 parabola		VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749 gutter	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 parabola		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 parabola	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW FROM	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW FROM	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 1 15.48 gutter	NDITION F3.4 2.21 0.95 0.62 0.395548 100 1 15.48 gutter	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T	C (ac) C (ac) C (ft) (%)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 parabola 2	RL 0. 1. 	-3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 rabola 2	3.5	100 RL5 0.56 0.95 0.62 0.626748 30 1 8.478745 parabola 2	0 - YEAF	VDR1 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749 gutter 1.405188	- HIST	RL4 0.24 3.31 0.95 1.9 0.498925 100 1 15.48 parabola 3	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 0.5 parabola 3	culvert 3.5	RL6 0.24 1.73 0.95 0.329646 300 1 26.81215 parabola 2		VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749 gutter 1.2933308	F3 0.36 10.37 0.95 0 0 0 3 3 15.9970661 parabola 2		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 parabola 2	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW FROM I	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 1 15.48 gutter 1.195117	NDITIO F3.4 0.24 2.21 0.95 0.62 0.395548 100 1 15.48 gutter 1.214808	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d	C (ac) C (ac) C (ft) (%)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.2	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 parabola 2 0.2	RL 0. 14 pau 3 0.3	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 rabola 2 0.22	3.5	100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 parabola 2 0.15	0 - YEAF	VDR1 0.24. 2.88 0.955 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.162286	- HIST	RL4 0.24 3.31 0.95 1.99 0.498925 100 1 15.48 parabola 3 0.23	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 parabola 3 0.23	culvert 3.5 0.33	RL6 0.24 1.73 0.95 0.25 0.329646 300 1 26.81215 parabola 2 0.2		VDR2 0.24 1.1 0.95 0.99 0.5595 120 1 16.95749 gutter 1.2933308 0.1377555	F3 0.36 10.37 0.95 0 0.36 300 3 15.9970661 parabola 2 0.2		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 parabola 2 0.2	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW FROM I	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 1 15.48 gutter 1.195117 0.117821	NDITIO F3.4 0.24 2.21 0.95 0.62 0.395548 100 1 15.48 gutter 1.214808 0.121697	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a	C (ac) C (ac) C (ft) (%) (ft) (ft) (ft^2)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.2 0.266667	RL2 0.24 1.88 0.95 0.484913 900 1 14.68562 parabola 2 0.266667	RL 0. 14 pa 3 0.3 0.6 0.	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 rabola 2 0.22 293333	3.5 0.33 0.77	100 RL5 0.24 0.56 0.67 0.626748 30 1 8.478745 parabola 0.15 0.2	0 - YEAF 3.5 0.33 0.77	VDR1 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.162286 0.152029	- HIST	Image: Number of the system RL4 0.24 3.31 0.95 1.9 0.498925 1000 1 15.48 parabola 3 0.23 0.436	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 0.5 0.5 0.23 0.23 0.46	culvert 3.5 0.33 0.77	RL6 0.24 1.73 0.95 0.25 0.329646 300 1 26.81215 parabola 2 0.2 0.2 0.266667		VDR2 0.24 1.1 0.95 0.99 0.5595 120 1 16.95749 gutter 1.2933308 0.1377555 0.1187756	F3 0.36 10.37 0.95 0 0.36 300 3 15.9970661 parabola 2 0.2 0.26666667		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 parabola 2 0.2 0.266667	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431 0.065159	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW FROM I	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 1 15.48 gutter 1.195117 0.117821 0.093873	NDITIO F3.4 0.24 2.21 0.95 0.62 0.395548 100 1 15.48 gutter 1.214808 0.121697 0.098559	NS
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw	C (ac) C (ac) C (ft) (%) (ft) (ft) (ft^2) (ft)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.22 0.266667 2.052121	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 1 14.68562 parabola 2 0.266667 2.052121	RL 0. 14 pau 3 0.3 0.6 0.6 0.3 0.6 0.3 0.78182 2.	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 	3.5 0.33 0.77 3.581285	104 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 parabola 2 0.15 0.22 2.029607	0 - YEAR 3.5 0.33 0.77 3.581285	EVENT VDR1 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.162286 0.152029 1.453679	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 100 1 1.5.48 parabola 3.04638	CONDIT VDR3 0.24 0.95 0.13 0.95 375 0.5 0.5 parabola 3.0.23 0.46 3.0463802	culvert 3.5 0.33 0.77 3.581285	RL6 0.24 1.73 0.955 0.255 0.329646 3000 1 26.81215 parabola 2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0		VDR2 0.24 1.1 0.95 0.9 0.5595 120 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 parabola 2 0.26666667 2.05212126		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 parabola 2 0.26 0.266667 2.052121	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D.E.F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 16.95749 16.95749 0.092431 0.065159 1.078595	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW FROM I	PED CO F3.3 0.24 2.85 0.955 0.72 0.383193 100 1 15.48 gutter 1.195117 0.117821 0.093873 1.225406	NDITIO1 F3.4 0.24 2.21 0.955 0.62 0.395548 100 1 15.48 gutter 1.214808 0.12168 0.124877	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw	C (ac) C (ac) C (ft) (%) (ft) (ft) (ft) (ft^2) (ft) (ft)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 parabola 2 0.2 0.266667 2.052121 0.129947	RI 0.0 14 9a 3 0.3 0.6 0.6 0.6 0.3.078182 2.0.19492 0.19492	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 rabola 2 0.22 293333 293333 142203	3.5 0.33 0.77 3.581285 0.215007	104 RL5 0.24 0.56 0.95 0.626748 30 1 8.478745 2 0.15 0.2 2.029607 0.098541	0 - YEAR 3.5 0.33 0.77 3.581285 0.215007	EVENT VDR1 0.24 2.88 0.955 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.152029 1.435679 0.104582	- HIST	RL4 0.24 0.95 0.95 1.9 0.498925 100 1 15.48 3 0.23 0.46 3.04638 0.150999	CONDIT VDR3 0.24 0.95 0.13 0.95 375 0.5 parabola 3.023 0.23 0.46 3.0463802 0.1509989	Culvert 3.5 0.33 0.77 3.581285 0.215007	RL6 0.24 1.73 0.25 0.25 0.329646 300 1 26.81215 parabola 2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0		VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 parabola 2 0.26666667 2.02512126 0.12994684		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 26.81215 20.266667 2.052121 0.129947	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431 0.065159 1.078595 0.060411	DEVELO INFLOW TO REACH IJ FROM OUTFALL H COMBINED WITH FLOW FROM I	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 15.48 gutter 1.195117 0.117821 0.093873 1.225406 0.093873	NDITIO1 F3.4 0.24 0.95 0.62 0.395548 100 1 15.48 gutter 1.214808 0.121697 0.098559 1.246577 0.079064	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland Islope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S	C (ac) (ac) C (ft) (%) (ft) (ft) (ft) (ft) (ft) (ft) (ft)(ft)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.06	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 2 0.26667 2.052121 0.129947 0.06	RI 0. 14 pai 3 0.3 0.6 0. 3.078182 2 0.19492 0. 0.03	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 2 0.22 293333 0.62778 142203 0.04	3.5 0.33 0.77 3.581285 0.215007 0.03	100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 2.029607 0.02 2.029607 0.098541 0.03	0 - YEAR 3.5 0.33 0.77 3.581285 0.215007 0.05	REVENT VDR1 0.24 2.88 0.955 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.162286 0.152029 0.104582 0.01	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 100 1 15.48 0.23 0.23 0.498925 100 1 0.498925 0.01 1 0.23 0.43 0.23 0.46 3.046138 0.150999 0.04	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 9arabola 3 0.23 0.46 3.0463802 0.1509989 0.04	Culvert 3.5 0.33 0.77 3.581285 0.215007 0.05	RL6 0.24 1.73 0.25 0.25 0.329646 300 1 26.81215 parabola 2 0.266667 2.052121 0.2052121 0.129947 0.06		VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073 0.01	F3 0.36 10.37 0.95 0 0.336 300 3 15.9970661 parabola 2 0.26666667 2.05212126 0.12994684 0.02		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 26.81215 20.266667 2.052121 0.129947 0.02	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431 0.065159 1.078595 0.060411 0.03	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW FROM 1	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 15.48 gutter 1.195117 0.117821 0.093873 1.225406 0.076606 0.03	NDITION F3.4 0.24 2.21 0.95 0.62 0.395548 100 1 15.48 gutter 1.214808 0.121697 0.098559 1.246577 0.079064 0.02	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannings Roughness Coefficient, n	C (ac) C (ac) C (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.06 0.013	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.00 0.013	RL 0.0 14 pai 3 0.3 0.6 0. 3.078182 2. 0.19492 0. 0.03 0.015	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 rabola 2 0.22 293333 062778 142203 0.04 0.013	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015	100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 2 0.15 0.2 2.029607 0.098541 0.03 0.013	0 - YEAF 3.5 0.33 0.77 3.581285 0.21507 0.2055 0.005	REVENT VDR1 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.162286 0.162286 0.104882 0.016	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 100 1 15.48 3 0.23 0.46 3.04638 0.150999 0.041	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 0.5 0.5 0.5 0.5 0.4 0.4 3.0463802 0.1509989 0.04 0.013	culvert 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015	RL6 0.24 1.73 0.95 0.25 0.329646 300 1 26.81215 parabola 2 0.266667 2.052121 0.129947 0.06 0.013		VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073 0.01 0.016	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 parabola 2 0.26666667 2.05212126 0.12994684 0.022 0.021		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 parabola 2 0.266667 2.052121 0.129947 0.02 0.013	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431 0.065159 1.078595 0.060411 0.03 0.016	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FROM I I	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 1 15.48 gutter 1.195117 0.117821 0.093873 1.225406 0.076606 0.03 0.016	NDITIO1 F3.4 0.24 2.21 0.95 0.62 0.395548 100 1 15.48 9 utter 1.214808 0.121697 0.098559 1.246577 0.079064 0.02 0.02	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannings Roughness Coefficient, n V = 1.49r'(2/3)S^(1/2) / n	C (ac) C (ac) C (ft) (%) (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 9 0.266667 2.052121 0.266667 2.052121 0.29947 0.066 0.013 7.197808	RL2 0.24 1.88 0.95 0.484913 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.066 0.013 7.197808	RL 0. 0. 14 0.3 0.3 0.6 0.3 0.78182 2. 0.09492 0. 0.03 0.015 5.780831 6.	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 rabola 2 0.22 293333 062778 142203 0.04 0.013 241129	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015 6.171646	100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 2.029607 0.098541 0.03 0.013 4.23201	0 - YEAF 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015 7.96756	REVENT VDR1 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.162286 0.152029 1.433679 0.104582 0.01 0.016 2.06559	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 100 1 15.48 parabola 3 0.23 0.4638 0.150999 0.044 0.013 6.496035	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.	culvert 3.5 0.33 0.77 3.581285 0.21500 0.05 0.015 7.96756	RL6 0.24 1.73 0.95 0.255 0.329646 300 1 26.81215 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197808		VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073 0.01 0.016 1.8577338	F3 0.36 10.37 0.95 0 0 0 0.36 300 3 15.9970661 parabola 2 0.26666667 2.05212126 0.12994684 0.022 0.021 0.021 0.021 0.021		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 parabola 2 0.266667 2.052121 0.129947 0.02 0.013 4.155657	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431 0.065159 1.078595 0.060411 0.03 0.016 2.481032	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW FROM I	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 1 15.48 gutter 1.195117 0.117821 0.093873 1.225406 0.0736066 0.03 0.016 2.906901	NDITIO1 F3.4 0.24 2.21 0.95 0.62 0.395548 100 1 15.48 0.121697 0.098559 1.246577 0.0796559 1.246577 0.020 0.02 0.016 2.423998	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannings Roughness Coefficient, n V = 1.49r^(2/3)S^(1/2) / n Assumed velocity	C (ac) C (ac) C (ft) (%) (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4	RL2 0.24 1.88 0.95 0.484913 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4	RL 0. 0. 14 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015 6.171646	104 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 parabola 2 0.15 0.22 2.029607 0.098541 0.03 0.013 4.23201 4	0 - YEAR 3.5 0.33 0.77 3.581285 0.215007 0.05 7.96756	Bevent VDR1 0.24 2.88 0.95 1.19 0.447592 1200 1 16.95749 gutter 1.405188 0.162286 0.152029 1.453679 0.104582 0.011 0.016 2.06559 3	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498922 100 1 15.48 parabola 3 0.23 0.4638 0.150999 0.04 0.013 6.496035 2	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 0.5 0.5 0.5 0.23 0.46 3.0463802 0.1509989 0.04 3.0463802 0.1509989 0.013 6.4960355 4	Culvert 3.5 0.33 0.77 3.581285 0.215007 0.015 7.96756	RL6 0.24 1.73 0.95 0.25 0.329646 300 1 26.81215 parabola 2 0.266667 2.052121 0.266667 2.052121 0.29947 0.06 0.013 7.197808 4		VDR2 0.24 1.1 0.95 0.9 0.5595 120 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073 0.016 1.8577338 3	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 2 0.26666667 2.05212126 0.12994684 0.02 0.013 4.15565652 4		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 parabola 2 0.2 0.266667 2.052121 0.129947 0.02 0.013 4.155657 4	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D.E.F COMBINED WITH FLOW FROM F3.1	$\begin{array}{r} \hline VENT & - \\ \hline F3.2 \\ 0.24 \\ 1.01 \\ 0.95 \\ 0.48 \\ 0.468725 \\ 120 \\ 1 \\ 10.95749 \\ 16.95749 \\ 0.092431 \\ 0.092431 \\ 0.092431 \\ 0.065159 \\ 1.078595 \\ 0.060411 \\ 0.03 \\ 0.016 \\ 2.481032 \\ 3 \\ \end{array}$	DEVELO INFLOW TO REACH IJ FROM OUTFALL H COMBINED WITH FLOW FROM I	PED CO F3.3 0.24 2.85 0.955 0.72 0.383193 100 1 15.48 gutter 1.195117 0.117821 0.093873 1.225406 0.076606 0.03 0.016 2.906901 3	NDITION F3.4 0.24 2.21 0.955 0.62 0.395548 100 1 15.48 0.121607 0.098559 1.246577 0.079064 0.02 0.016 2.423998 3	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannings Roughness Coefficient, n V = 1.49r'(2/3)S^(1/2) / n Assumed velocity Reach	C (ac) C (ac) C (ft) (%) (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 90 1 14.68562 0.266667 2.052121 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4 RL1 - A	RL2 0.24 1.88 0.95 0.484913 900 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4 RL2 - B	RL 0.0 0.1 0.3 0.3 0.3 0.6 0.3 0.78182 2.0 0.9492 0.0 0.03 0.015 5.780831 6. A - B R	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 rabola 2 0.22 293333 062778 142203 0.04 0.013 241129 4 1 J - C	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015 6.171646 B - C	100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 9arabola 2 0.15 0.22 2.029607 0.098541 0.03 0.013 4.23201 4 RL5 - D	0 - YEAR 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015 7.96756 C - D	REVENT VDR1 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.162286 0.152029 1.453679 0.104582 0.01 0.016593 VDR1 - D	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 100 1 1.548 0.23 0.46638 0.150999 0.04 0.150999 0.04 0.013 6.496035 RL4 - E	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.	culvert 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015 7.96756 D - E	RL6 0.24 1.73 0.95 0.25 0.329646 3000 1 26.81215 parabola 2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0	G	VDR2 0.24 1.1 0.95 0.9 0.5595 120 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073 0.01 0.016 1.8577338 3 VDR2 - H	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 2.05212126 0.12994684 0.02 0.2666667 2.05212126 0.12994684 0.02 0.013 4.1556552 4 VDR4-Z		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 26.81215 26.81215 26.81215 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D.E.F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431 0.065159 1.078595 0.0604111 0.03 0.016 2.481032 3 F3.2-L	DEVELO INFLOW TO REACH IJ FROM OUTFALL H COMBINED WITH FLOW FROM I I IJ	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 1 15.48 gutter 1.195117 0.117821 0.093873 1.225406 0.076606 0.03 0.016 2.906901 3 F3.3.J	NDITION F3.4 0.24 2.21 0.955 0.62 0.395548 100 1 15.48 0.121697 0.098559 1.246577 0.079064 0.02 0.098559 1.246577 0.079064 3 F3.4.3	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannings Roughness Coefficient, n V = 1.49r^(2/3)S^(1/2) / n Assumed velocity Reach flow length, L Taret is L((601))	C (ac) C (ac) C (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 90 1 14.68562 2 0.266667 2.052121 0.129947 0.066 0.013 7.197808 4 RL1 - A 494	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 90 0.266667 2.052121 0.129947 0.06 0.013 7.197948 4 RL2 - B 494	RL 0. 0. 14 pat 3 0.3 0.6 0.1 3.078182 2. 0.19492 0. 0.3 0.015 5.780831 6. X - B R 2.83 2.02155	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 2 0.22 293333 0.02778 142203 0.02778 142203 0.04 0.013 241129 4 I.J. - C 10710	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015 6.171646 B - C 1111 2002	100 RL5 0.24 0.56 0.95 0.626748 30 1 8.478745 2 0.15 0.22 2.029607 0.098541 0.03 0.013 4.23201 4 RL5 - D 825 3.2000	0 - YEAR 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015 7.96756 C - D 150	REVENT VDR1 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.152029 1.435679 0.104582 0.01 0.015 2.06559 3 VDR1 - D 600	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 100 1 1.5.48 0.23 0.46 3.04633 0.150999 0.04 0.013 6.496035 2 RL4 - E 1010	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 9arabola 3 0.23 0.46 3.0463802 0.1509989 0.04 0.013 6.4960355 4 VDRJ-E 1010	Culvert 3.5 0.33 0.77 3.581285 0.215007 0.05 7.96756 D - E 150	RL6 0.24 1.73 0.25 0.25 0.329646 300 1 26.81215 parabola 2 0.2 0.2 0.266467 2.052121 0.129947 0.06 0.013 7.197808 4 F - G 300	G	VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073 0.01 0.016 1.8577338 3 VDR2 - H 320	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 2 0.26666667 0.02666667 0.02666667 0.02666667 0.0266667 0.0212126 0.12994684 0.02 0.013 4.15565652 4 VDR4-Z 3500		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 26.81215 26.81215 26.81215 20.266667 2.052121 0.129947 0.02 0.013 4.155657 4 F3.1-G 2000	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D.E.F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 0.092431 0.065159 1.078295 0.060411 0.03 0.016 2.481032 3 F3.2-I 500	DEVELO INFLOW TO REACH IJ FROM OUTFALL H COMBINED WITH FLOW FROM I I I J	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 15.48 gutter 1.195117 0.117821 0.093873 1.225406 0.003873 0.003606 0.033 0.003606 0.033 1.225406 0.003606 0.03 1.225406 0.03 0.016 2.906901 3 F3.3-J 940	NDITION F3.4 0.24 0.95 0.62 0.395548 100 1 15.48 0.121657 0.098559 1.246577 0.079064 0.02 0.016 2.42398 3 F3.4-J 360	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland Islope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannings Roughness Coefficient, n V = 1.49r^(2/3)S^(1/2) / n Assumed velocity Reach flow length, L Travel time L/(60V) Ourded 4 Torus Travel	C (ac) C (ac) C (ac) C (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4 RL1 - A 494 1.143867	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 2 0.266667 2.052121 0.266667 2.052121 0.129947 0.06 0.129947 0.06 0.129947 1.143867 4.143867 1.143867	RL 0.0 0.0 0.1 0.1 0.1 0.0 0.0 0.0 0.0 0.0	.3 0.24 2.08 0.95 1.41 526848 900 1 4.68562 2 0.22 293333 0.04 0.013 241129 4 L3 - C 1010 697161 4 (48252)	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015 6.171646 B - C 111 0.299758	100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 0.22 0.15 0.22 2.029607 0.098541 0.03 0.013 4.2301 4.2301 RL5 - D 825 3.249047 8.4787	0 - YEAR 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015 7.96756 C - D 150 0.313772	REVENT VDR1 0.24 2.88 0.955 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.162286 0.152029 0.104582 0.016 2.06559 3 VDR1 - D 6000 3.3333333	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 100 1 15.48 0.23 0.498925 100 1 0.498925 0.00 1 0.498925 0.013 0.46635 0.046635 2 RL4 - E 1010 2.591324	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 0.5 0.5 0.23 0.46 3.0463802 0.1509989 0.04 0.013 6.4960355 4 VDR3-E 1010 2.5913241	Culvert 3.5 0.33 0.77 0.05 0.215007 0.05 0.215007 0.05 0.015 7.96756 D - E 150 0.313772	RL6 0.24 1.73 0.95 0.25 0.329646 3000 1 26.81215 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.19208 4 F - G 3000 0.694556 2.05121 0.294556 3000 0.694556 3000 30	G	VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073 0.01 0.016 1.8577338 3 VDR2 - H 320 1.7777778	F3 0.36 10.37 0.95 0 0.33 300 3 15.9970661 2 0.2666667 2.05212126 0.12994684 0.02 0.013 4.15565652 4 VDR4-Z 350 1.40370921	2	100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 2.052121 0.266667 2.052121 0.129947 0.02 0.013 4.155657 4 F3.1-G 200 0.833333	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431 0.065159 1.078595 0.060411 0.03 0.016 2.481032 3 F3.2-I 500 2.777778	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW FROM 1	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 15.48 gutter 1.195117 0.17821 0.093873 1.225406 0.076606 0.03 0.016 2.90606 3.290603 F3.3-J 940 5.222222	NDITION F3.4 0.24 2.21 0.95 0.62 0.395548 100 1 15.48 8 0.121697 0.098559 1.246577 0.079064 0.02 0.016 2.42398 3 F3.4-J 360 2	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*LO.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannings Roughness Coefficient, n V = 1.49r^(2/3)S^(1/2) / n Assumed velocity Reach flow length, L Travel time L/(60V) Overland Travel Time	C (ac) C (ac) C (ac) C (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4 RL1 - A 494 1.143867 14.68562	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4 RL2 - B 494 1.143867 14.68562	RL 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 2 0.22 293333 0.62778 0.027 142203 0.04 0.013 241129 4 I.3.C 1010 697161 4.68562	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015 6.171646 B - C 1111 0.299758 0	100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 2 0.15 0.2 2 0.15 0.2 2 0.15 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	0 - YEAF 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015 7.96756 C - D 150 0.313772 0	REVENT VDR1 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.162286 0.162286 0.104582 0.016 2.06559 3 VDR1 - D 600 3.33333 16.95749	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 100 1 15.48 parabola 3 0.23 0.46 3.04638 0.150999 0.033 6.496035 2 RL4 - E 1010 2.591324 15.48	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.	Culvert 3.5 0.33 0.77 3.581285 0.015 7.96756 D - E 150 0.313772 0 0	RL6 0.24 1.73 0.95 0.25 0.329646 300 1 26.81215 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4 F - G 300 0.694656 26.81215 2.25 2.25 3.55 3.55	G	VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.1371456 0.0892073 0.01 0.016 1.8577338 VDR2 - H 320 1.7777778 16.95749	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 parabola 2 0.26666667 2.05212126 0.12994684 0.02 0.013 4.15565652 4 VDR4-Z 350 1.40370921 15.9970661		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 20266667 2.052121 0.129947 0.02 0.013 4.155657 F3.1-G 200 0.833333 26.81215	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431 0.065159 1.078595 0.060411 0.03 0.016 2.481032 3 F3.2-I 500 2.777778 16.95749	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW FROM 1 1	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 1 15.48 gutter 1.195117 0.117821 0.076606 0.03 0.0166 2.906901 3 F3.3-J 940 5.222222 15.48	NDITION F3.4 0.24 2.21 0.95 0.62 0.395548 100 1 15.48 0.121697 0.098559 1.246577 0.098559 1.246577 0.079064 0.079064 0.079064 0.02 0.079064 0.02 0.079064 0.02 0.079064 0.02 0.079064 0.02 0.079064 0.02 0.079064 0.02 0.079064 0.02 0.079064 0.02 0.02 0.02 0.02 0.02 0.02 0.02 0.0	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland Islope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannings Roughness Coefficient, n V = 1.49r'(2/3)S^(1/2) / n Assumed velocity Reach flow length, L Travel time L/(60V) Overland Travel Time Time of concentration Ivaneity	C (ac) C (ac) C (ac) C (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.066 0.013 7.197808 4 RL1 - A 494 1.143867 14.68562 15.83	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.026 0.013 7.197808 4 RL2 - B 494 1.143867 14.68562 15.83 2	RL 0.0 0.0 0.1 0.1 0.3 0.3 0.3 0.6 0.1 0.19492 0.0 0.015 5.780831 6. A - B R 283 0.815915 2. 0 1 0.82 2.07	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 2 0.22 293333 062778 142203 0.04 0.013 241129 4 i.JC 1010 697161 4.68562 17.38 3.97	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015 6.171646 B - C 1111 0.299758 0 0.30 0.30	100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 2.029607 0.098541 0.03 0.013 4.23201 4 RL5-D 8255 3.249047 8.478745 1.73	0 - YEAF 3.5 0.33 0.77 3.581285 0.215007 0.055 0.015 7.96756 C - D 150 0.313772 0 0.313772	REVENT VDR1 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.162286 0.162286 0.104582 0.016 2.06559 3 VDR1-D 600 3.333333 16.95749 20.29	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 1000 1 1.5.48 parabola 3 0.23 0.46 3.04638 0.150999 0.04 0.013 6.496035 2 RL4 - E 1010 2.591324 15.48	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.	culvert 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015 7.96756 D - E 150 0.313772 0 0.313772	RL6 0.24 1.73 0.95 0.25 0.329646 300 1 26.81215 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4 F - G 300 0.694656 26.81215 2.7.51 2.7.51	G	VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073 0.01 0.016 1.8577338 3 VDR2 - H 320 1.7777778 16.95749 18.74	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 parabola 2 0.26666677 2.05212126 0.12994684 0.02 0.021 0.0294684 0.02 0.013 4.15565652 4 VDR4-Z 350 1.40370921 15.9970661 17.400 2.07 2.07 2.07 15.9970661 17.400 2.07 15.9970661 17.400 2.07 15.9970661 17.400 2.07 15.9970661 15.907061 15.9070661		100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 26.81215 2.052121 0.129947 0.02 0.013 4.155657 4 F3.1-G 200 0.833333 26.81215 27.65	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D,E,F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431 0.065159 1.078595 0.060411 0.03 0.016 2.481032 3 F3.2-I 500 2.777778 16.95749 19.74	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FROM I I I I-J	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 1 15.48 gutter 1.195117 0.117821 0.093873 1.225406 0.076606 0.03 0.016 2.906901 3 F3.3-3 940 5.222222 15.48 20.70	NDITION F3.4 0.24 2.21 0.95 0.62 0.395548 100 1 15.48 0.121697 0.098559 1.246577 0.079064 0.02 0.016 2.423998 3 F3.4-J 360 2 15.48	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannings Roughness Coefficient, n V = 1.49r^(2/3)S^(1/2) / n Assumed velocity Reach flow length, L Travel time L/(60V) Overland Travel Time Time of concentration Intensity total area	C (ac) C (ac) C (f) C (f) (f) (f) (f) (f) (f) (f) (f) (f) (f)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 0.2 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4 RL1 - A 494 1.143867 14.68562 15.83 3.15 2.251	RL2 0.24 1.88 0.95 0.484913 90 1 14.68562 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.	RL 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 7 7 7 2 2 2 3 3 0 6 2 7 8 1 4 2 0.22 2 2 3 3 3 0 4 1 4 2 0.22 2 2 3 3 3 0 6 7 8 1 4 1 5 2 8 4 8 5 2 1 1 4 6 8 5 2 1 1 4 6 8 5 2 1 1 4 6 8 5 2 1 1 4 6 8 5 2 1 1 4 6 8 5 2 1 1 4 6 8 5 2 2 2 2 3 3 3 0 6 2 7 8 1 4 2 2 2 3 3 3 0 6 2 7 8 1 4 2 2 2 3 3 3 0 6 2 7 8 1 4 2 2 3 3 3 0 6 2 7 8 1 4 2 2 3 3 3 2 1 4 2 2 3 3 3 2 4 1 2 2 3 3 3 2 4 1 2 2 3 3 3 2 4 1 2 2 3 3 3 2 4 1 2 2 3 3 3 2 4 1 2 2 3 3 3 2 4 1 2 2 3 3 3 2 4 1 2 2 3 3 3 2 4 1 2 2 3 3 3 2 4 1 2 2 1 3 3 3 2 4 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015 6.171646 B - C 111 0.299758 0 0.30 0.30 2.99	100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 2 0.15 0.22 2.029607 0.098541 0.03 4.23201 4 RL5 - D 825 3.249047 8.478745 11.73 3.54 1.73 3.54	0 - YEAR 3.5 0.33 0.77 3.581285 0.215007 0.05 7.96756 C - D 150 0.313770 0.313770 0.313770 0.313.54	Bevent VDR1 0.24 2.88 0.95 1.19 0.447592 1200 1 16.95749 gutter 1.405188 0.162286 0.152029 1.453679 0.104582 0.01 0.016 2.06559 3 3.3333333 16.95749 20.29 2.84 4.92	- HIST	FORICAI RL4 0.24 0.95 0.95 1.9 0.498925 1000 1 15.48 3 0.23 0.4663 0.150999 0.04 0.013 6.496035 6.496035 2 RL4 - E 1010 2.591324 15.48 18.07 2.99	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.	Culvert culvert 3.5 0.33 0.77 3.581285 0.215007 0.05 7.96756 D - E 150 0.313770 0.313770 0.31299	RL6 0.24 1.73 0.955 0.255 0.329646 3000 1 26.81215 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	G	VDR2 0.24 1.1 0.95 0.9 0.5595 120 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073 0.016 1.3517338 3 VDR2 - H 320 1.777778 16.95749 18.74 2.91	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 parabola 2 0.26666667 2.05212126 0.12994684 0.02 0.212126 0.12994684 0.02 4 VDR4-Z 350 4 VDR4-Z 15.9970661 17.40 3.07 15.9970661	Z	100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 26.81215 20.2 0.26667 2.052121 0.129947 0.02 0.013 4.155657 4 F3.1-G 200 0.833333 26.81215 27.65 2.36 2.36	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D.E.F COMBINED WITH FLOW FROM F3.1	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 0.092431 0.065159 1.078595 0.060411 0.03 0.016 2.481032 3 F3.2-I 500 2.777778 16.95749 19.74 2.84 1.95749 19.74 2.84 1.95749 19.74 2.84 1.95749 19.74 2.84 1.95749 19.74 2.84 1.95749 19.74 2.84 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 19.74 1.95749 1.97748 1.95749 1.97748 1.95749 1.97748 1.95749 1.97748 1.977	DEVELO INFLOW TO REACH IJ FROM OUTFALL H COMBINED WITH FLOW FROM I I JJ	PED CO F3.3 0.24 2.85 0.955 0.72 0.383193 100 1 15.48 gutter 1.195117 0.117821 0.093873 1.225406 0.076606 0.03 0.016 2.906901 3 F3.3-J 940 5.222222 15.48 20.70 2.77	NDITION F3.4 0.24 2.21 0.955 0.62 0.395548 100 1 15.48 0.12164 0.02 0.098559 1.246577 0.079064 0.02 0.096559 1.246577 0.079064 3 F3.4-J 360 2 15.48 17.48 3.07 2.02 2.15.48	
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 Paved Surface Type 2 area Average Coffecient overland travel length overland Slope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannigs Roughness Coefficient, n V = 1.49r ² (2/3)S^(1/2) / n Assumed velocity Reach flow length, L Travel time L/(60V) Overland Travel Time Time of concentration Intensity total area Oev/a	C (ac) C (ac) C (ac) C (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.266667 0.052121 0.129947 0.06 0.013 7.197808 4 RL1 - A 494 1.143867 14.68562 15.83 3.15 2.94	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	RL 0.0 0.0 0.1 0.1 0.3 0.6 0. 3.078182 2. 0.19492 0. 0.03 0.015 5.780831 6. A - B R 2.83 0.815915 2. 0.82 0.82 0.82 0.82 0.82 0.82	.3 0.24 2.08 0.95 1.41 526848 90 1 4.68562 7 7 14.68562 2 0.22 293333 062778 142203 0.04 0.013 241129 241129 4 15.26 17.38 3.07 3.49 830721 ()	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015 6.171646 B - C 1111 0.299758 0 0.30 0.30 0.30 2.99 0 4.752167	100 RL5 0.24 0.56 0.95 0.626748 30 1 8.478745 9 arabola 2 0.15 0.22 2.029607 0.098541 0.03 0.013 4.23201 4 RL5 - D 825 3.249047 4.329047 1.73 3.54 1.23 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 3.54 0.84587 1.73 0.84587 1.73 1.73 1.73 1.73 1.73 1.73 1.73 1.73 1.73 1.73 1.73 1.73 1.73 1.73 1.75	0 - YEAR 3.5 0.33 0.77 3.581285 0.215007 0.05 7.96756 C - D 150 0.313772 0.313772 0.31354 0 0.31 3.54 0 0.31 0.54 0 0.31 0.54 0 0.31 0.55	REVENT VDR1 0.24 2.88 0.95 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.152029 1.453679 0.104582 0.01 0.015 3.33333 16.95749 20.29 2.84 4.07	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 100 1 1.548	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 9arabola 3 0.23 0.46 3.0463802 0.1509989 0.04 0.013 5.460355 4 VDR3-E 1010 2.5913241 0 0 5.00 4.95 0.13 0 2.5913241	Culvert culvert 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015 7.96756 D - E 150 0.313772 0.313772 0.313772 0.31 0.312 0.31 0.312 0.31 0.31 0.31 0.31 0.31 0.31 0.31 0.31 0.31 0.31 0.31 0.32 0.32 0.33 0.33 0.35 0.37 0.05 0.31 0.37 0.31 0.31 0.37 0.31	RL6 0.24 1.73 0.95 0.25 0.329646 3000 1 26.81215 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	G 21.92	VDR2 0.24 1.1 0.95 0.9 0.5595 120 16.95749 gutter 1.2933308 0.1377555 1.331456 0.187756 1.331456 0.0892073 0.01 0.016 1.8577338 3 VDR2 - H 320 1.7777778 16.95749 18.74 2.91 2 0.2002225	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 2.05212126 0.12994684 0.022 0.2666667 2.05212126 0.12994684 0.021 0.12994684 0.021 0.12994684 1.40570521 1.40370921 15.9970661 17.40 3.07 10.37	Z 34.29	100 F3.1 0.24 2.23 0.95 0.25 0.311573 3000 1 26.81215 26.81215 2.052121 0.129947 0.02 0.013 4.155657 4 F3.1-G 2.00 0.833333 26.81215 2.7.65 2.36 2.48	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D.E.F COMBINED WITH FLOW FROM F3.1 G G 24.4	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431 0.065159 1.078595 0.060411 0.03 0.016 2.481032 3 F3.2-I 500 2.777778 16.95749 19.74 2.84 1.49 0.164662	DEVELO INFLOW TO REACH IJ FROM OUTFALL H COMBINED WITH FLOW FROM I I I-J	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 15.48 gutter 1.195117 0.117821 0.093873 1.225406 0.093873 1.225406 0.093873 1.225406 0.003873 1.225406 0.003873 1.225406 0.003666 0.03 0.016 2.906901 3 F3.3-J 940 5.222222 15.48 20.70 2.77 3.57 0.77282	NDITION F3.4 0.24 0.25 0.62 0.395548 100 1 15.48 0.121697 0.098559 1.246577 0.079064 0.02 0.016 2.423998 3 F3.4-J 360 2 2.423998 3 F3.4-J 360 2.243998 3 F3.4-J 360 2.243998 3 F3.4-J 360 2.243998 3 F3.4-J 360 2.243998 3 F3.4-J 360 2.24398 3 F3.4-J 360 2.24398 3 F3.4-J 360 2.24398 3 F3.4-J 360 2.24398 7 F3.4-J7 F3.4-J 7 F3.4-J7	NS
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland Islope =1.8*(1.1-C)*L^0.5*S^-0.3333 Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannings Roughness Coefficient, n V = 1.49r^(2/3)S^(1/2) / n Assumed velocity Reach flow length, L Travel time L/(60V) Overland Travel Time Time of concentration Intensity total area Q=Va Overla	C (ac) C (ac) C (ac) C (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft)	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 90 parabola 2 0.266667 2.052121 0.129947 0.066 0.013 7.197808 4 RL1 - A 494 1.143867 14.68562 15.83 3.15 2.94 1.919416	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4 RL2 - B 494 1.143867 14.68562 15.83 3.15 2.87 1.919416 4.92954	RL 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.	.3 0.24 2.08 0.95 1.41 526848 900 1 4.68562 2 0.22 293333 0.04 0.013 241129 4 142203 0.04 0.013 241129 4 153-C 1010 697161 4.68562 17.38 3.07 3.49 830731 644890	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015 6.171646 B - C 111 0.299758 0 0.30 0 2.99 0 4.752167 14.29647	100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 2 0.15 0.2 2.029607 0.098541 0.03 0.013 4.23201 4.23204 RL5 - D 8.25 3.249047 8.478745 11.73 3.54 1.23 0.846402 2.73806	0 - YEAR 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015 7.96756 C - D 150 0.313772 0 0 0 0 0 0 0 0 0 0 0 0 0	REVENT VDR1 0.24 2.88 0.955 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.152029 1.435679 0.104582 0.01 0.016 2.05559 3 VDR1 - D 600 3.333333 16.95749 20.29 2.84 4.07 0.314029	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 100 1 1.5.48 0.23 0.498925 100 1 1.5.48 0.23 0.46 3.04633 0.150999 0.04 0.013 6.496035 2 RL4 - E 1010 2.591324 15.48 18.07 2.99 5.21 2.988176 7.77206	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 9arabola 3 0.23 0.46 3.0463802 0.1509989 0.04 0.013 6.4960355 4 VDRJ-E 1010 2.5913241 0 0 5.00 4.95 0.13 2.9881763 0.13 2.9881763	Culvert culvert 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015 7.96756 D - E 150 0.313772 0 0.313772 0 0.313772 0 0.313772 0 0.313772 0 0.313772 0 0.313772 0 0.313772 0 0.313772 0 0 3.51285 0.313772 0 0.313772 0 0 0 0 0 0 0 0 0 0 0 0 0	RL6 0.24 1.73 0.25 0.25 0.329646 300 1 26.81215 parabola 2 0.266667 0.2 0.266667 0.2052121 0.129947 0.06 0.013 7.197808 4 F - C 300 0.694656 26.81215 2.7511 2.36 1.98 1.919416 1.540373	G 21.92 32.12209	VDR2 0.24 1.1 0.95 0.9 0.5595 120 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073 0.01 0.016 1.8577338 3 VDR2 - H 320 1.777778 16.95749 18.74 2.91 0.2206535 3 25620	F3 0.36 10.37 0.95 0 0 0.36 300 3 15.9970661 2.05212126 0.2666667 0.025212126 0.12994684 0.02 0.013 4.15565652 4 VDR4-Z 350 1.40370921 15.9970661 17.40 3.07 1.0817507 1.10817507	Z 34.29 43 58301	100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 26.81215 2.052121 0.266667 2.052121 0.26947 0.02 0.013 4.155657 4 F3.1-G 200 0.833333 26.81215 2.7.65 2.36 2.48 1.108175 1.52572	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D.E.F COMBINED WITH FLOW FROM F3.1 G G 24.4	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 0.092431 0.065159 1.078595 0.060411 0.03 0.016 2.481032 5 00 2.777778 16.95749 19.74 2.84 1.49 0.161662	DEVELO INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW FROM I I I-J I-J 27.89	PED CO F3.3 0.24 2.85 0.75 0.75 0.72 0.383193 100 15.48 gutter 1.195117 0.17821 0.093873 0.093875 0.093875 0.093875 0.0056 0.005675 0.005675 0.005675 0.005755 0.0057555 0.0057555555555555	NDITION F3.4 0.24 0.55 0.622 0.395548 100 1 15.48 0.121697 0.079064 0.02 0.016 2.42393 F3.4-J 360 2 15.48 3.07 2.83 3.07 2.83 3.07 2.83 3.07 2.83 3.07 2.83 3.12555	Z 34.2*
Surface Type 1 - Landscaped Surface Type 1 area Surface Type 2 - Paved Surface Type 2 area Average Coffecient overland travel length overland Slope = $1.8^{+}(1.1-C)^{+}L^{0.5+}S^{-0.3333}$ Channel Shape width, T depth, d Cross-sectional flow area, a wetted perimeter, Pw Hydraulic Radius, r= a/Pw Channel slope, S Mannings Roughness Coefficient, n V = $1.49r^{+}(2/3)S^{-}(1/2)/n$ Assumed velocity Reach flow length, L Travel time L/(60V) Overland Travel Time Time of concentration Intensity total area Q=Va Q=Cia	C (ac) C (ac) C (ac) C (fi) (%) (fi) (fi) (fi) (fi) (fi) (fi) (fi) (fi	RL1 0.24 1.91 0.95 1.03 0.488741 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197808 4 RL1 - A 494 1.143867 14.68562 15.83 3.15 2.94 1.919416 4.526235.	RL2 0.24 1.88 0.95 0.99 0.484913 90 1 14.68562 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.197088 4 RL2 - B 4.94 1.143867 1.468562 1.5.83 3.15 2.87 1.919416 4.383855	RL 0.0 0.1 0.3 0.3 0.6 0.7 0.19492 0.19492 0.003 0.015 5.78081 6. 0.815915 2. 0.815915 2. 0.815915 2. 0. 0. 3.468498 1. 8.798754 5.	.3 0.24 2.08 0.95 1.41 526848 900 1 4.68562 2 0.22 293333 0.04 0.013 241129 4.68562 142203 0.04 0.013 241129 4.68562 17.38 3.07 3.49 830731 644809	3.5 0.33 0.77 3.581285 0.215007 0.03 0.015 6.171646 B - C 111 0.299758 0 0.30 0.30 0.30 0.30 0.2.99 0.4.752167 14.29647	100 RL5 0.24 0.56 0.95 0.67 0.626748 30 1 8.478745 2 0.15 0.2 2.029607 0.098541 0.03 0.013 4.23201 RL5 - D 825 3.249047 8.478745 1.173 3.54 1.23 0.846402 2.728986	0 - YEAR 3.5 0.33 0.77 3.581285 0.215007 0.05 0.015 7.96756 C - D 150 0.313772 0.0 0.313772 0.0 0.313772 0.0 0.313772 0.0 0.0 150 0.313772 0.0 0.0 150 0.313772 0.0 0.0 150 0.313772 0.0 150 1.0 1.54 1.0 1.0 1.0 1.54 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0	EVENT VDR1 0.24 2.88 0.955 1.19 0.447592 120 1 16.95749 gutter 1.405188 0.162286 0.152029 0.104582 0.01 0.016 2.05559 3 VDR1 - D 600 3.333333 16.95749 2.029 2.84 4.07 0.314029 5.173628	- HIST	FORICAI RL4 0.24 3.31 0.95 1.9 0.498925 100 1 1.5.48 parabola 3 0.23 0.46 3.04638 0.150999 0.04 0.013 6.496035 2 RL4 - E 1010 2.591324 15.48 18.07 2.99 5.21 2.988176 7.772206	CONDIT VDR3 0.24 0 0.95 0.13 0.95 375 0.5 0.5 0.5 0.5 0.5 0.5 0.2 0.1509989 0.04 0.013 6.4960355 4 VDR3-E 1010 2.5913241 0 0.5.00 4.95 0.13 2.9881763 0.611325	Culvert Culvert 3.5 0.33 0.77 0.35 0.215007 0.05 0.215007 0.05 0.015 7.96756 D - E 150 0.313772 0 0.313772 0 0.313272 0 0.31 2.19908	RL6 0.24 1.73 0.25 0.25 0.329646 300 1 26.81215 parabola 2 0.266667 2.052121 0.129947 0.06 0.013 7.19947 0.06 0.013 7.197808 F - G 300 0.694656 26.81215 2.7.51 2.36 1.919416 1.540372	G 21.92 32.12298	VDR2 0.24 1.1 0.95 0.9 0.5595 120 1 16.95749 gutter 1.2933308 0.1377555 0.1187756 1.331456 0.0892073 0.01 0.016 1.857738 3 VDR2 - H 320 1.7777778 16.95749 18.74 2.91 2 0.2206535 3.25629	F3 0.36 10.37 0.95 0 0.36 300 3 15.9970661 2 0.2666667 2.05212126 0.12994684 0.02 0.013 4.15565652 4 VDR4-Z 350 1.40370921 15.9970661 17.40 3.07 10.37 1.10817507 11.460924	Z 34.29 43.58391	100 F3.1 0.24 2.23 0.95 0.25 0.311573 300 1 26.81215 26.81215 2.052121 0.129947 0.02 0.013 4.155657 4 F3.1-G 200 0.833333 26.81215 27.65 2.36 2.48 1.108175 1.823572	0 - YEAR E INFLOW TO OUTFALL G FROM OUTFALLS D.E.F COMBINED WITH FLOW FROM F3.1 G G 24.4 33.946556	VENT - F3.2 0.24 1.01 0.95 0.48 0.468725 120 1 16.95749 gutter 1.057429 0.092431 0.065159 0.060411 0.03 0.016 2.481032 3 F3.2-I 500 2.777778 16.95749 19.74 2.84 1.49 0.161662 1.983456	DEVELOI INFLOW TO REACH I-J FROM OUTFALL H COMBINED WITH FLOW FROM I I J J J J J 39.186302	PED CO F3.3 0.24 2.85 0.95 0.72 0.383193 100 115.48 gutter 1.195117 0.17821 0.093873 1.225406 0.076606 0.03 0.016 2.90606 0.03 0.016 2.90609 3 F3.3-J 940 5.222222 15.48 20.70 2.77 3.57 0.272881 3.78936	NDITION F3.4 0.24 2.21 0.95 0.62 0.395548 100 1 15.48 8 0.121697 0.098559 1.246577 0.079064 0.02 0.016 2.42398 3 F3.4-J 360 2 15.48 3.07 2.83 0.238907 3.436558	NS

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19-Oct-95

Vista Del Rio		<u>, </u>				·····	· · · · · · · · · · · · · · · · · · ·						
Street flow depth at t	he gutter fo	or critical se	ections.	·····							······································		
Flow Through Stree Discharge quantity Q=0.56*(Z/n)*S^.5*d Where: Q = Disch Z = Invers	et, Curb & is calcula I^2.67 arge in CF e pavemer	Gutter ted by the S (Cubic Fo at cross slo	following eet per Sec pe	formula: cond)									
n = Manni S = Longit d = Depth	ng roughne tudinal slop of gutter fl	ess coeffici be of the str ow in feet	ent reet or gutte	er					Capacity F curb open Ponding Q	or Storm D ing length = .6 A (2gH)	prain Inlets = grate leng ^.5]	gth	
Solving for maximu	ım depth a	at gutter							Clogging	factors: gra	ate=0.5, box	x=0.0	
Manning Roughness C	oefficient=	0.016							H2 =	0.5 Ft.	H100 =	1.0 Ft.	
		Inverse	Min.	Required	2 year		100 Yr		_				
	Street	Pave.	Long.	2 Year	Water	100 Yr	Water	Grate	Open	Capacity	Required	Capacity	Required
Drainge Basin(s)	Locn.	x slope	Slope	Capacity	Depth	Capacity	Depth	Туре	Area	2 Yr	2 Yr	100 Yr	100 Yr
see Exhibit 2	ID	1/ft/ft	S ft/ft	QCFS	d Ft.	QCFS	d Ft.	NEENAH	Sq. Ft.	CFS	CFS	CFS	CFS
F3.2		66.67	0.005	0.70	0.13	1.98	0.19	R-3246 C	2.08	7.08	0.70	10.01	1.98
F3.3 & F3.4	J	66.67	0.005	2.53	0.21	7.23	0.31	R-3246 C	2.08	7.08	2.53	10.01	7.23

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VISTA DEL RIO SUBDIVISION

Alpine CM

CLIEN PROJI LOCA	T:			4	REPORT No. 1 DATE of TEST: 12-11-96 TEST BY: R\$W/MS LD JOB No.: 85853-1403			
TEST	YPE: Nuclear Backscatter Direct Trans. X	SPECIFICATIO)NS: Project:	Cit	y: <u>X</u>	County: State	a:	
Test No.	Location of Test		COMPAC. SPEC. %	MOISTURE	MOISTURE SPEC. %	PROCTOR	SOIL TYPE	
1	Sewer main 25' E of MH #5 @ -16' BSG	90	90	11.7	+-2	115.5 @ 13.6	С	
2	Sewer main 27' E of MH #5 @ -14' BSG	89*	90	11.9	+-2	115.5 @ 13.6	с	
3	Sewer main 30' E of MH #5 @ -12' BSG	99	90	8.1**	+-2	115.5 @ 13.6	с	
4	Sewer main 34' E of MH #5 @ -10' BSG	87*	90	8.8**	+-2	115.5 @ 13.6	с	
5	Sewer main 40' E of MH #5 @ -8' BSG	92	90	9.8**	+-2	115.5 @ 13.6	с	
6	Sewer main 80' E of MH #5 @ -6' BSG	98	90	11.6	+-2	115.5 @ 13.6	с	
7	Sewer main 85' E of MH #5 @ -4' BSG	100	90	11.6	+-2	115.5 @ 13.6	с	
2A	RETEST #2	91	90	13.0	+-2	115.5 @ 13.6	с	
3A	RETEST #3	92	90	12.3	+-2	115.5 @ 13.6	с	
4A	RETEST #4	93	90	15.4	+-2	115.5 @ 13.6	с	
5A	RETEST #5	93	90	15.3	+-2	115.5 @ 13.6	с	
8	SS, Lot 1 @ FG	100	95	13.2	+-2	115.5 @ 13.6	с	
	· · ·							
Distribu Clie Subc	KEY: * Fails Compaction SPEC. ** Fails Moisture SPEC. ** Fails Moisture SPEC. S = Standard Proctor M = Modified Proctor	. C = Cohesi NC = NonCo ABC = Aggreg PR = Pit Ru	ve hesive ate Base n	GRAND JL		NCOLN-DeVORE, Ir	nc.	
				FILL DEM	NSITY TE	ST DAILY REPO	ORT	
NOTE:	Results indicate in-place Soil densities at the locations and depth above. Grand Junction Lincoln-DeVore has relied on the contractor to pr uniform mix placement and compactive effort throughout the fill o	ns identified rovide orea.		BJ	GRAND LINCOL	JUNCTION N-DeVORE, I:	nc. sts	

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CLIEN PROJE LOCAT	T: <u>Alpine CM</u> CCT: <u>Vista Del Rio</u> NON:		· · · · · · · · · · · · · · · · · · ·				REPOR DATE C TEST B LD JOE	T No. 2 of TEST: <u>12–12–</u> IY: <u>RSW</u> B No.: <u>85853–</u>	96 140
TEST T	YPE: Nuclear Backscatter	Nuclear Direct Trans. X	_	SPECIFICATIO)NS: Proj e ct:	City	y: <u>X</u>	County: State	e:
Test No.	Location of	Test			COMPAC. SPEC. %	MOISTURE CONT %	MOISTURE SPEC. %	PROCTOR VALUE	1
9	SS, Lot 3 @ FSG			100	95	8.8	+-2	125.8 @ 9.5	1
10	SS, Lot 23 @ FSG			100	95	8.9	+-2	125.8 @ 9.5	
Distribut 2-Cli 1-LD/ 1-Sub 1-Ute	ion: ent CS div Env Water	KEY:	Fails Compaction SPEC. Fails Moisture SPEC. S = Standard Proctor M = Modified Proctor	C = Cohesi NC = NonCo ABC = Aggreg PR = Pit Rui	ve hesive ate Base n	GRAND JU BY:	NCTION LIN	NCOLN-DeVORE, Ir	nc. OF
NOTE:	Results indicate in-plac above. Grand Junction L uniform mix placement	e Soil densities incoln-DeVore has and compactive	ot the locations and depths relied on the contractor to pro- effort throughout the fill ar	identified vid e eo.		BJ.	GRAND LINCOL	JUNCTION N-DeVORE, I	n

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CLIEN PROJI LOCA	IT: <u>Alpine CM</u> ECT: Vista Del Rio TION:				REPOR DATE O TEST E LD JOE	T No. 3 of TEST: <u>12-13-</u> 3Y: <u>RSW</u> 3 No.: <u>85853-</u>	96 1403
TEST 1	TYPE: Nuclear Nuclear Backscatter Direct TransX	SPECIFICATI	ONS: Project:	City	v: <u> </u>	County: State	:
Test No.	Location of Test	. COMPACTION	COMPAC. N SPEC. %	OISTURE	MOISTURE SPEC. %	PROCTOR VALUE	SOIL TYPE
11	SS, Lot 2 @ FSG	95	95	10.1	+-2	125.8 @ 9.5	С
12	SS, Lot 4 @ FSG	100	95	9.0	+-2	125.8 @ 9.5	С
13	SS, Lot 22 @ FSG	100	95	8.3	+-2	125.8 @ 9.5	С
14	SS, Lot 5 @ FSG	95	95	12.7	+-2	116.9 @ 11.5	С
15.	SS, Lot 21 @ FSG	96	95	11.6	+-2	116.9 @ 11.5	с
16	SS, Lot 6 @ 1' BSG	98	95	9.6	+-2	116.9 @ 11.5	С
17	SS, Lot 🚺 @ 1' BSG	95	95	9.5	+-2	125.8 @ 9.5	с
18	SS, Lot 6 @ FSG	95	95	11.7	+-2	116.9 @ 11.5	с
19	SS, Lot 🕼 @ FSG	99	95	10.0	+-2	125.8 @ 9.5	C.
Distribu 2-Clier 1-LD/CS 1-Subdi 1-Ute W	KEY: • tion: •• nt S = S M = iv Env Vater	Fails Compaction SPEC. C = Cohes Fails Moisture SPEC. NC = NonCo Standard Proctor ABC = Aggrey Modified Proctor PR = Pit RL	ive G ohesive gate Base un B	GRAND JU	NCTION LI	NCOLN-DeVORE, In	DRT
NOTE:	Results indicate in—place Soil densities at above. Grand Junction Lincoln—DeVore has reli uniform mix placement and compactive eff	the locations and depths identified ed on the contractor to provide ort throughout the fill area.			GRAND LINCOL	JUNCTION	nc.

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CLIENT PROJE LOCAT	Alpine CM CT: Vista Del Rio				REPOR DATE C TEST B LD JOE	T No. 4 bi TEST: 12-19-9 by: RSW/LRS B No.: 85853-14	6 4 03
TEST T	PE: Nuclear Nuclear Backscatter Direct Trans. X	SPECIFICATIO)NS: Project:	Cit	y: <u>X</u> (County: State	::
Test No.	Location of Test		COMPAC. SPEC. %	MOISTURE	MOISTURE SPEC. %	PROCTOR VALUE	SOIL TYPE
20	Sewer main 60' NW of MH 3A @ FSG	97	95	12.5	+-2	122.0 @ 10.5	С
21	Sewer main 10' NW of MH 3A @ 8' BSG	97	95	14.7	+-2	115.5 @ 13.6	С
22	Sewer main 20' NW of MH 3A @ 6' BSG	98	95	12.7	+-2	115.5 @ 13.6	С
23	Sewer main 30' NW of MH 3A @ 4' BSG	96	95	14.3	+-2	115.5 @ 13.6	С
24	Sewer main 40' NW of MH 3A @ 2' BSG	96	95	12.4	+-2	115.5 @ 13.6	С
Distributi 2-Clien 1-LD/CS 1-Subdi 1-Ute w	on: t KEY: * Fails Compaction SPEC ** Fails Moisture SPEC. S = Standard Proctor M = Modified Proctor v Eng vater	C. C = Cohesi NC = NonCo ABC = Aggreg PR = Pit Ru	ve hesive ate Base n	GRAND JU BY:	NOTION LIN	NCOLN-DeVORE, In ST DAILY REP(DRT
NOTE:	Results indicate in-place Soil densities at the locations and dept above. Grand Junction Lincoln-DeVore has relied on the contractor to uniform mix placement and compactive effort throughout the fill	hs identified provide area.		E)	GRAND LINCOL	JUNCTION N-DeVORE, II	nc.

CLIE PRO LOC	NT:Alpine CM JECT:Vista Del Rio ATION:					REPORT DATE o TEST B LD JOB	r No. 5 f TEST: <u>1-8-97</u> Y: RSW No.: <u>85853-1</u>	403
TEST	TYPE: Nuclear Nuclear Backscatter Direct Trans.	<u>X</u>	SPECIFICATIO)NS: Proj e ct:	City	y: <u>X</u> C	county: State	:
Test No.	Location of Test	**************************************	COMPACTION	COMPAC. SPEC. 7	MOISTURE CONT %	MOISTURE SPEC. %	PROCTOR VALUE	SOIL TYPE
25	WS, Lots 12 & 13 @ 2' BSG		90*	95	10.8**	+-2	115.5 @ 13.6	С
26	WS, Lots 10 & 11 @ 2' BSG		98	95	15.5	+-2	115.5 @ 13.6	С
27	WS, Lots 10 & 11 @ FSG		95	95	14.1	+-2	115.5 @ 13.6	С
28	SS, Lot 10 @ 2' BSG		98	95	12.6	+-2	115.5 @ 13.6	С
29	SS, Lot 10 @ FSG		100	95	13.3	+-2	115.5 @ 13.6	С
30	SS, Lots 11 & 12 @ 2' BSG		96	95	10.0	+-2	121.8 @ 10.8	С
31	SS, Lots 11 & 12 @ FSG		98	95	9.6	+-2	121.8 @ 10.8	С
32	MH # 13 @ 2' BSG		99	95	9.6	+-2	121.8 @ 10.8	С
33	MH # 13 @ FSG		97	95	10.0	+-2	121.8 @ 10.8	С
34	Sewer main 20' SE of MH 13 @	2' BSG	97	95	9.5	+-2	121.8 @ 10.8	С
25A	RETEST, WS, Lots 12 & 13 @ 2	' BSG	99	95	10.7	+-2	116.9 @ 11.5	С
35	Water main corner of lots 9	& 10 @ FSG	99	95	14.4	+-2	115.5 @ 13.6	С
36	Water main corner of lots 9	& 10 @ 2' BSG	100	. 95	11.4	+-2	116.7 @ 11.5	С
37	WS. Lot 9 @ 2' BSG		·98	95	12.1	+-2	116.7 @ 11.5	C
38	WS, Lot 8 @ 2' BSG		99	95	11.1	+-2	121.8 @ 10.8	C
Distrit 2-Cli 1-LD/ 1-Sub 1-Ute	KEY: ent CS liv Env Water	 Fails Compaction SPEC. Fails Moisture SPEC. S = Standard Proctor M = Modified Proctor 	C = Cohesi NC = NonCo ABC = Aggreg PR = Pit Ru	ve hesive ate Base n	GRAND JU BY:	NCTION LIN	ICOLN-DeVORE, IN	c. DRT
NOTE	Results indicate in-place Soil densitien above. Grand Junction Lincoln-DeVore I uniform mix placement and compact	es at the locations and depth has relied on the contractor to pr ive effort throughout the fill c	s identified ovide orea.		5	GRAND LINCOL	JUNCTION N-DeVORE, II	nc.

LOCA		SPECIFICATIO	NS:			, no <u>-09099-1</u>	-
	Bockscatter Direct Trans		Project:	City	/: <u>X</u> C	County: State:	:_
Test No.	Location of Test	COMPACTION	COMPAC. SPEC. 7	MOISTURE	MOISTURE SPEC. %	PROCTOR VALUE	
39	WS, Lots 12 & 13 @ FSG	97	95	12.8	+-2	115.5 @ 13.6	
40	SS, Lot 13 @ 2' BSG	97	95	12.7	+-2	115.5 @ 13.6	
41	SS, Lot 13 @ FSG	97	95	13.6	+-2	115.5 @ 13.6	
42	SS, Lot 9 @ 2' BSG	100	95	12.2	+-2	115.5 @ 13.6	
43	SS, Lot 9 @ FSG	100	95	12.5	+-2	115.5 @ 13.6	
44	WS, Lot 9 @ FSG	100	95	12.1	+-2	116.9 @ 11.5	
45	WS, Lot 8 @ FSG	96	95	13.2	+-2	115.5 @ 13.6	
46	Sewer main 20' NW of MH 2B @ 2' BSG	99	95	10.7	+-2	116.9 @ 11.5	
47	Sewer main 20' NW OF MH 2B @ FSG	96	95	9.5	+-2	116.9 @ 11.5	
48	MH 2B @ 3' BSG	97	95	9.5	+-2	121.8 @ 10.8	
49.	MH 2B @ 1' BSG	97	95	8.7**	+-2	121.8 @ 10.8	
50	MH 2B @ FSG	100	95	10.0	+-2	121.8 @ 10.8	
51	SS, Lot 8 @ 4' BSG	99	. 95	10.2	+-2	121.8 @ 10.8	
52	SS, Lot 8 @ 2' BSG	.97	95	9.7	+-2	116.9 @ 11.5	
53	SS, Lot 8 @ FSG	100	95	13.9**	+-2	116.9 @ 11.5	
Page Distribu 2-Clie 1-LD/C	1 of 3KEY: • Fails Compactionation:•• Fails Moisture SantS = Standard ProctoSM = Modified Procto	n SPEC. C = Cohes PEC. NC = NonCo or ABC = Aggreg r PR = Pit Ru	ive hesive jate Base n	GRAND JU	INCTION LI	NCOLN-DeVORE, In	10
1-Subd 1-Ute	iy Env Water			FILL DE	NSITY TE	ST DAILY REP	O

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	CLIEN PROJE LOCAT	r: <u>Alpine CM</u> CT: <u>Vista Del Rio</u> ION:				REPOR DATE o TEST B LD JOB	No. 6 f TEST: 1-9-97 Y: <u>RSW</u> No.: <u>85853-1</u>	403
	TEST T	YPE: Nuclear Nuclear Backscatter Direct Trans	SPECIFICATIO)NS: Project:	City	y: <u>X</u> C	County: State	:
(Test No.	Location of Test	COMPACTION	COMPAC. SPEC. %	MOISTURE	MOISTURE SPEC. %	PROCTOR VALUE	SOIL TYPE
	54	WS, Lot 7 @ 2' BSG	96	95	14.2	+-2	115.5 @ 13.6	С
	55	WS, Lot 7 @ FSG	100	95	11.3	+-2	116.9 @ 11.5	С
	56	MH 3A @ 4' BSG	100	95	13.5	+-2	116.9 @ 11.5	с
	57	MH 3A @ 2' BSG	98	95	12.1	+-2	116.9 @ 11.5	с
_	58	MH 3A @ FSG	96	95	9.5	+-2	116.9 @ 11.5	С
•	59	Sewer main 15' SE of MH 3A @ 6' BSC	99	95	12.3	+-2	115.5 @ 13.6	с
	60	Sewer main 15' SE of MH 3A @ 4' BSG	97	95	14.4	+-2	115.5 @ 13.6	с
	61	Sewer main 15' SE of MH 3A @ 2' BSG	97	95	13.3	+-2	115.5 @ 13.6	с
	62	Sewer main 15' SE of MH 3A @ FSG	99	95	13.7	+-2	115.5 @ 13.6	с
	63	Sewer main 20' W of MH 3B @ 4' BSG	98	95	14.0	+-2	115.5 @ 13.6	с
	64	Sewer main 25' W of MH 3B @ 2' BSG	100	95	13.8	+-2	115.5 @ 13.6	с
	65	Sewer mian 25' W of MH 3B @ FSG	100	95	14.7	+-2	115.5 @ 13.6	С
	66	MH 3B @ 4' BSG	98	95	15.3	+-2	115.5 @ 13.6	с
	67	MH 3B @ 2' BSG	.99	95	14.9	+-2	115.5 @ 13.6	С
	68	MH 3B @ FSG	98	95	15.1	+-2	115.5 @ 13.6	Ċ
	Page Distribut 2-Clien 1-LD/CS 1-Subdi 1-Ute W	2 of 3 ion: t Set Standard Proctor y Env ater	C = Cohesi NC = NonCo ABC = Aggreg PR = Pit Ru	ve hesive ate Base n	GRAND JU BY:	NCTION LIN	NCOLN-DeVORE, IN	c.
	NOTE:	Results indicate in-place Soil densities at the locations and depth above. Grand Junction Lincoln-DeVore has relied on the contractor to pr uniform mix placement and compactive effort throughout the fill a	s identified ovide Irea.		BJ _{GE}	GRAND LINCOL dtechnical	JUNCTION N-DeVORE, In Engineers-geologis	nC. sts

	LIENT: <u>Alpine CM</u> ROJECT: <u>Vista Del Rio</u> OCATION:			REPORT No.7DATE of TEST:1-10TEST BY:RSWLD JOB No.:85853	-97 -1403			
a Tei	ST TYPE: Nuclear Nuclear X Backscatter Direct Trans	SPECIFICATIO	NS: Project: Ci	ity: X County: St	y: X County: State:			
Te	est Location of Test o.	COMPACTION	COMPAC. MOISTURE SPEC. % CONT %	MOISTURE PROCTOR SPEC. % VALUE	SOIL TYPE			
83 84	Water main @ corner of lots 5 & 6 @ 2' BSG Water main @ corner of lots 4 & 5 @ FSG	100 98	95 9.6 95 10.6	+-2 125.8 @ 10. +-2 16.7 @ 11.	8 C 5 C			
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					_			
Dist 2-C: 1-LJ 1-Su 1-U	KEY:Fails Compaction SPEC.tribution:** Fails Moisture SPEC.lientS = Standard ProctorD/CSM = Modified Proctorubdiy Envte Water	C = Cohesin NC = NonCoh ABC = Aggrega PR = Pit Rur	ve GRAND Ju nesive pte Base n BY:	NSITY TEST DAILY PE				
NC	DTE: Results indicate in-place Soil densities at the locations and depths above. Grand Junction Lincoln-DeVore has relied on the contractor to pro uniform mix placement and compactive effort throughout the fill a	s identified ovide irea.		GRAND JUNCTION LINCOLN-DeVORE, EOTECHNICAL ENGINEERS-GEOLO				

	CLIENT PROJE LOCAT	: <u>Alpine CM</u> CT: <u>Vista Del Rio</u> ION:				REPOR DATE C TEST B LD JOE	T No. 8 of TEST: <u>1-15-97</u> Y: <u>RSW</u> B No.: <u>85853-14</u>	403
	TEST T	1PE: Nuclear Nuclear X Backscatter Direct Trans	SPECIFICATIO	NS: Project:	City	y: <u>X</u>	County: State:	
	Test No.	Location of Test		COMPAC. SPEC. 7	MOISTURE	MOISTURE SPEC. %	PROCTOR VALUE	SOIL TYPE
	85	WS, Lots 4 & 5 @ 2' BSG	95	95	12.9	+-2	115.5 @ 13.6	С
	86	WS, Lots 4 & 5 @ FSG	96	95	12.6	+-2	115.5 @ 13.6	С
	87	WS, Lot 6 @ 2' BSG	100	95	9.4	+-2	125.8 @ 9.5	С
	88	WS, Lot 6 @ FSG	100	95	10.5	+-2	125.8 @ 9.5	С
,	<u>89</u>	FH corner of Lots 6 & 7 @ 2' BSG	96	95	9.5	+-2	122.0 @ 10.5	С
	90	FH corner of Lots 6 & 7 @ FSG	97	95	9.5	+-2	122.0 @ 10.5	С
	91	FH corner of Lots 8 & 9 @ 2' BSG	96	95	12.2	+-2	115.5 @ 13.6	С
	92	FH corner of Lots 8 & 9 @ FSG	100	95	12.9	+-2	115.5 @ 13.6	с
	93	Water main corner of Lots 17 & 18 @ 2' BSG	99	95	10.5	+-2	125.8 @ 9.5	с
	94	Water main corner of Lots 17 & 18 @ FSG	100	95	9.0	+-2	125.8 @ 9.5	С
	95	WS, Lots 17 & 18 @ 2' BSG	98	95	11.3	+-2	116.9 @ 11.5	с
	96	WS, Lots 17 & 18 @ FSG	95	95	12.7	+-2	116.9 @ 11.5	с
	97	WS, Lot 14 @ 2' BSG	99	95	11.5	+-2	116.9 @ 11.5	С
	98	WS. Lot 14 @ FSG	100	95	10.3	+-2	116.9 @ 11.5	С
	99	WS. Lots 15 & 16 @ 2' BSG	98	95	12.4	+-2	116.9 @ 11.5	С
	Page Distribut 2-Clier 1-LD/CS 1-Subd 1-Ute N	1 of 2KEY: • Fails Compaction SPEC.sion:•• Fails Moisture SPEC.atS = Standard ProctorSM = Modified Proctoriv EnvNater	C = Cohes NC = NonCo ABC = Aggreg PR = Pit Ru	ive chesive gate Base in	GRAND JU BY:	NSITY TE	NCOLN-DeVORE, In	c.
	NOTE:	Results indicate in-place Soil densities at the locations and depths above. Grand Junction Lincoln-DeVore has relied on the contractor to pro- uniform mix placement and compactive effort throughout the fill a	s identified ovide rea.		GJ.	GRAND LINCOI cotechnical	JUNCTION N-DeVORE, I encineers-geologi	NC. STS

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C 	CLIENT: <u>Alpine CM</u> PROJECT: <u>Vista Del Rio</u> OCATION:			REPOR DATE (TEST E LD JOE	T No. 8 of TEST: 1-15-97 BY: RSW B No.: 85853-1	403
TE	ST TYPE: Nuclear Nuclear Bockscatter Direct Trans	SPECIFICATIO	DNS: Project: C	ity: X	County: State	:
Te N	est Location of Test . Io.		COMPAC. MOISTUR SPEC. % CONT %	E MOISTURE SPEC. %	PROCTOR VALUE	SOIL TYPE
10	0 WS, Lots 15 & 16 @ FSG	100	95 10.6	+-2	116.9 @ 11.5	С
10	1 WS, Lot 22 @ 2' BSG	99	95 11.9	+-2	116.9 @ 11.5	с
10	2 WS, Lot 22 @ FSG	95	95 13.4	+-2	116.9 @ 11.5	С
10	3 WS, Lot 23 @ 2' BSG	95	95 12.7	+-2	116.9 @ 11.5	С
, 10	4 WS, Lot 23 @ FSG	97	95 12.6	+-2	116.9 @ 11.5	С
10	5 WS, Lot 3 @ 2' BSG	98	95 12.1	+-2	116.9 @ 11.5	С
10	6 WS, Lot 3 @ FSG	96	95 12.5	+-2	116.9 @ 11.5	С
10	7 WS, Lot 2 @ 2' BSG	97	95 12.7	+-2	116.9 @ 11.5	С
10	8 WS, Lot 2 @ FSG	98	95 12.6	+-2	116.9 @ 11.5	С
10	9 WS, Lot 1 @ 2' BSG	95	95 13.1	+-2	116.9 @ 11.5	С
P. Dis 2-C 1-L 1-S 1-U	age 2 of 2KEY: • Fails Compaction SPECstribution:•• Fails Moisture SPEC.lientS = Standard ProctorD/CSM = Modified Proctorubdiv Envte Water	. C = Cohesi NC = NonCo ABC = Aggreg PR = Pit Ru	ve GRAND hesive ate Base n BY:	INSITY TE	NCOLN-DeVORE, IN	c.
N	OTE: Results indicate in-place Soil densities at the locations and depth above. Grand Junction Lincoln-DeVore has relied on the contractor to p uniform mix placement and compactive effort throughout the fill	ns identified rovide area.	GJ.	GRAND LINCOL EOTECHNICAL	JUNCTION N- De VORE, II encineers-ceologis	nc. sts

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TEST	TYPE: Nuclear Nuclear X	SPECIFICATIONS: X Projec			v: X (County: State		
Test No.	Location of Test		COMPAC. SPEC. %	MOISTURE	MOISTURE SPEC. %	PROCTOR VALUE	S T	
110	Storm drain main line between MH4 & MH3 @ FSG	100	95	12.9	+-2	115.5 @ 13.6	c	
111	Storm drain main line MH4 & S side curb box @ FSG	100	95	9.9	+-2	121.8 @ 10.8	с	
112	Storm drain S side circle curb & MH5 @ FSG	100	95	12.2	+-2	115.5 @ 13.6	с	
113	Storm drain between MH 2 & 3 @ FSG	100	95	13.3	+-2	115.5 @ 13.6	с	
Distribu 2-Clie 1-LD/C 1-Subd 1-Ute	tion: nt S = Standard Proctor S Moisture SPEC. M = Modified Proctor Water	C = Cohesi NC = NonCo ABC = Aggreg PR = Pit Run	ve hesive ate Base n	GRAND JU BY:			ic.	

5	CLIENT:Alpine CM PROJECT:Vista Del Rio LOCATION:	REPORT No. 10 DATE of TEST: 2-7-97 TEST BY: Matt LD JOB No.: 85853-1403	
	TEST TYPE: Nuclear Nuclear Sockscatter Direct Trans.	SPECIFICATIONS: Project: Cil	y: X County: State:
	Test Location of Test . No.	COMPACTION COMPAC. MOISTURE % SPEC. % CONT %	MOISTURE PROCTOR SOIL SPEC. % VALUE TYPE
-	114 Storm drain, next to Lot 16 @ FSG	92* 95 14.6	+-2 115.5 @ 13.6 C
	115 Storm drain, next to Lot 7 @ FSG	100 95 11.2	+-2 122.0 @ 10.8 C
	116 Storm drain, next to Lot 14 @ FSG	100 95 9.2	+-2 122.0 @ 10.8 C
	117 Storm drain, next to Lot 8 @ FSG	94* 95 6.7	+-2 115.5 @ 13.6 C
,	118 Inlet box, next to Lot 10 @ FSG	97 95 10.2	+-2 122.0 @ 10.8 C
	119 Inlet box, next to Lot 6 @ FSG	96 95 9.0	+-2 122.0 @ 10.8 C
	Distribution:KEY: * Fails Compaction S2-Client** Fails Moisture SPEC1-LD/CSS = Standard Proctor1-Subdiv EnvM = Modified Proctor1-Ute Water1-Ute Water	SPEC. C = Cohesive GRAND JU C. NC = NonCohesive ABC = Aggregate Base PR = Pit Run BY:	UNCTION LINCOLN-DEVORE, Inc.
	NOTE: Results indicate in-place Soil densities at the locations and o above. Grand Junction Lincoln-DeVore has relied on the contractor uniform mix placement and compactive effort throughout the	depths identified to provide fill area.	GRAND JUNCTION LINCOLN-DeVORE, Inc. SOTECHNICAL ENGINEERS-GEOLOGISTS

TEST	rPE: Nuclear Nuclear	uclear SPECIFICATIONS:								
	Backscatter Direct Trans.	<u></u>		Project:	City:		Stati	=: 		
Test No.	Location of Test			COMPAC. SPEC. %	MOISTURE	MOISTURE SPEC. 7	PROCTOR VALUE	.		
120	Sidewalk sta 18+15 E side @ :	FSG	96	95	7.9	+-2	125.8 @ 9.5			
121	Sidewalk sta 18+15 W side @	FSG	100	95	10.7	+-2	125.8 @ 9.5			
122/	Street sta 18+00 Center line	@ FSG	96	95	9.4	+-2	125.8 @ 9.5			
123	Street sta 17+30 W lane @ FS	G ·	96	95	9.5	+-2	121.8 @ 10.8			
124⁄	Sidewalk 16+00 W side @ FSG		96	95	8.4	+-2	125.8 @ 9.5			
125-	Street sta 12+00, W lane @ F	SG	96	95	9.4	+-2	133.0 @ 7.5	-		
126-	Sidewalk sta 13+00 W side @	FSG	95	95	9.5	+-2	133.0 @ 7.5			
127-	Street sta 13+00 E side @ FS	G	100	95	9.6	+-2	121.8 @ 10.8			
128/	Sidewalk sta 13+00 E side @	FSG	96	95	9.0	+-2	121.8 @ 10.8			
129-	Street sta 14+00 W lane @ FS	G	100	95	8.8	+-2	133.0 @ 7.5			
130	Sidewalk sta 16+20 E side @	FSG	98	95	9.8	+-2	121.8 @ 10.8			
131⁄	Street sta 16+00 E side @ FS	Ģ	97	95	10.5	+-2	121.8 @ 10.8			
132⁄	Sidewalk sta 11+00 E side @	FSG	98	95	8.3	+-2	133.0 @ 7.5			
133~	Street sta 11+00 E side @ FS	G	.98	95	7.3	+-2	133.0 @ 7.5			
134⁄	Sidewalk sta 11+00 W side @	FSG	95	95	8.6	+-2	133.0 @ 7.5			
Distribu 2-Clie 1-LD/C	KEY	: • Fails Compaction SPEC •• Fails Moisture SPEC. S = Standard Proctor M = Modified Proctor	C = Cohes NC = NonCo ABC = Aggreg PR = Pit Ru	ive phesive gate Bose in	GRAND JU BY:	UNCTION LI	INCOLN-DeVORE, I	nc.		
1-Subd 1-Ute	Verv Tests 125, 129 & Jater	132 have a rock correct	tion.		FILL DE	NSITY TE	EST DAILY REP	POF		

	CLIENT: Alpine CM PROJECT: Vista Del Rio LOCATION:							REPORT No. 12 DATE of TEST: 2-24-97 TEST BY: RL/RSW LD JOB No.: 85853-1403				
								X County: State:				
	Test No.	Location of Test		COMPACTION	COMPAC. SPEC. %	MOISTURE	MOISTURE SPEC. %	PROCTOR VALUE	SOIL TYPE			
F	135/Sid	lewalk @ W end of cul-de-sac	@ FSG	100	95	13.0	+-2	115.5 @ 13.6	С			
	136 Str	eet sta 6+70 N lane @ FSG		100	95	13.6	+-2	115.5 @ 13.6	С			
	137 / Sid	lewalk S side sta 6+25 @ FSG		100	95	14.0	+-2	115.5 @ 13.6	С			
	138 Sid	lewalk /s side sta 6+25 @ FSG		100	95	15.6	+-2	115.5 @ 13.6	С			
,	139/Str	reet sta 6+00 S lane @ FSG		97	95	15.0	⊥ _2	115.5 @ 13.6	С			
	140 Str	reet sta 15+50 W lane @ FSG		100	95	13.2	2_ب	115.5 @ 13.6	С			
				•								
	Distribution: 2-Client 1-LD/CS 1-Subdiv Ed 1-Ute Wate:	KEY: nv r	 Fails Compaction SPEC. Fails Moisture SPEC. S = Standard Proctor M = Modified Proctor 	C = Cohesiv NC = NonCoh ABC = Aggrego PR = Pit Run	ve nesive nte Base	GRAND JU	NCTION LIN	ST DAILY REPO	c. DR T			
	NOTE: Re: abc uni	sults indicate in-place Soil densities ove. Grand Junction Lincoln-DeVore has iform mix placement and compactive	at the locations and depths a relied on the contractor to prov e effort throughout the fill are	identified ride eo.		BJ _{CEC}	GRAND LINCOLI DTECHNICAL	JUNCTION N- De VORE, Ir encineers-ceologis	1C. 375			

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	·						<u>.</u>		 		•		· · · · · · · · · · · · · · · · · · ·		·

	CLIENT: <u>Alpine CM</u> PROJECT: <u>Vista Del Rio</u> LOCATION:	EPOR DATE TEST & LD JOI	REPORT No. 14 DATE of TEST: 2-26-97 TEST BY: RSW LD JOB No.: 85853-1403			
	TEST TYPE: Nuclear Nuclear X Backscatter Direct Trans	SPECIFICATIO	DNS: Proj ec t:	City: X	County: Stat	e:
	Test Location of Test . No.	COMPACTION	COMPAC. MOISTI SPEC. % CONT	JRE MOISTURE	PROCTOR VALUE	SOIL TYP
	144 Sidewalk Casa Rio Ct E side sta 11+00 @ BCG	94	90M 7.0	+-2	136.8 @ 7.1	BC
	145 Sidewalk Casa Rio Ct E side sta 13+00 @ BCG	92	90M 5.4	+-2	136.8 @ 7.1	вс
	146 Sidewalk Casa Rio Ct E side sta 15+00 @ BCG	94	90M 7.9	+-2	136.8 @ 7.1	BC
	147 Sidewalk Casa Rio Ct E side sta 17+00 @ BCG	96	90M 6.1	+-2	136.8 @ 7.1	BC
	148 Sidewalk Casa Rio Ct W side sta 17+00 @ BCG	97	90M 6.8	+-2	136.8 @ 7.1	BC
2 1 1 1	Distribution: KEY: • Fails Compaction SPE -Client • Fails Moisture SPEC. -LD/CS S = Standard Proctor -Subdiv Env M = Modified Proctor -Ute Water -Ute Water	EC. C = Cohesin NC = NonCol ABC = Aggrega PR = Pit Rur	ve GRAND hesive ate Base h BY: FILL [JUNCTION LI	NCOLN-DEVORE, IN	ORT
	NOTE: Results indicate in-place Soil densities at the locations and dep above. Grand Junction Lincoln-DeVore has relied on the contractor to uniform mix placement and compactive effort throughout the fil	oths identified provide Il area.	G	GRAND LINCOL	JUNCTION N-DeVORE, I	NC.

County: Stat	State: _
PROCTOR	
VALUE	
136.8 @ 7.1	1
136.8 @ 7.1	1
136.8 @ 7.1	1
136.8 @ 7.1	1
INCOLN-DEVORE, I	
	NCOLN-DEVORE

TEST	ATION: TYPE: Nuclear Nuclear Backscatter Direct Trans	SPECIFICATIO)NS: Proj ec t:	City	<u>x x</u>	County: State	:
Test No.	Location of Test	COMPACTION	COMPAC. SPEC. %	MOISTURE	MOISTURE SPEC. %	PROCTOR VALUE	SOII TYP
49/	RETEST - MH 2B @ -1' BSG	100	95	8.2	+-2	125.8 @ 9.5	с
53/	RETEST - SS. Lot 8 @ FSG	98	95	8.6	+-2	125.8 @ 9.5	С
	A RETEST - Storm drain, Lot 8 @ FSG	100	95	8.0	+-2	125.8 @ 9.5	C
, 114	A RETEST - Storm drain, Lot 16 @ FSG	98	95	8.7	+-2	125.8 @ 9.5	С
		·					
Distri 2-C1 ± 1-LD/ 1-Sut 1-Ut e 1-N1	KEY: • Fails Compaction Fails Moisture SPE ent S = Standard Proctor GS M = Modified Proctor div Env Water Thols & Assoc. 1-City of GJ	SPEC. C = Cohes C. NC = NonCo ABC = Aggreg PR = Pit Ru	ive hesive gate Base in	GRAND JL BY:	NSITY TE	NCOLN-DeVORE, II	nc. ORT



Client	Alpine CM				Job No	8585	3-1403		
Project	Vista del	Rio			Test B Locati	on of Tes	t		
Concrete Truck No. Ticket No Date of T Mix, Prop 28-day Re	Supplier 2 2 1 est 3 ortions quired Strer	SJRM 27 3123 3-5-97 1gth 400	0 ps	— Ce — S1 — Ai — Te — Te	ment Type ump (ASTM r Content mperature st @ <u>ch</u> ter Added	C 143) (ASTM C (ASTM C ute 3 ga	651 CG 1 ¹ / ₄ 231) <u>5.4</u> 1064) 73 65 cu 110ns	inches % % F. yds.	
6" x 12" Cylinder No.	Avg. Cyl. Diameter (in.)	Cross- Sectional Area (in.')	Unit Weight (pcf)	Total Load (1bs.)	Unit Stress (psi)	Break Type	Break Date	Age (days)	
1	6.09	29.13	148	129,000	4430	СМ	3-12	7	
2	6.10	29.22	148	172,000	5890	СМ	4 – 2	28	
3	6.10	29.22	148	163,000	5580	СМ	4-2	28	
4	6.08	29.03						Reserve	
Remarks: Specimen o	or cap defec	ts:		Li 1	ncoln Dev working d	'ore requi lav's noti	ires a min ice to sch	imum of edule	
Distributi 1-Client 1-Subdiv E 1-Ute Wate 1-LD/CS 1-Nichols * Does not Break Type	on: Env er & Assoc. meet requis	l-City of G red strength (J if applica	pe ot te Fi cy 28 be able) of 1a	ersonnel f oservation est perfor nal repor linders, day brea responsi the test boratory	or any fi s. Compr med accor t will ir and will k. This ble for a results personnel	ield tests ressive st ding to A nclude dat be sent a laborator any interp by other	and rength STM C-39. a for all fter the y cannot retation than	
CM - Con CA - Con V - Shea Date Issue	ical Mortar ical Aggrega ar Break d: <u>4-2-97</u>	Break ate Break		LI By	NCOLN Dev	ORE, INC.	I AAA	la-	
	CONCRETE TEST REPORT DEVORE ENGINEERS GRAND JUNCTION, PUEBLO								

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Client	Alpine CM				Job No	85853-	-1403	
Project	Vista del	Rio	· · · · · · · · · · · · · · · · · · ·		Test B Locatio	on of Tes	t Sidewall	k on E side
Concrete S Truck No. Ticket No. Date of Te Mix, Propo 28-day Rec	Supplier est ortions quired Stren	GJRM 24 13219 3-10-97 gth 4000	ps	Cer Slu Ain Ter Tes si Wat	nent Type ump (ASTM Content nperature st @ cer Added	6 C 143) <u>1</u> (ASTM C (ASTM C chute	51 CG 2 231) <u>4</u> . 1064) <u>6 cu</u> 4 gallo	inches 5 % % F. yds. ons
6" x 12" Cylinder No.	Avg. Cyl. Diameter (in.)	Cross- Sectional Area (in.')	Unit Weight (pcf)	Total Load (lbs.)	Unit Stress (psi)	Break Type	Break Date	Age (days)
5	6.02	28.46	146	103,000	3620	СМ	3-17	7
6	6.03	28.56	146	134,500	4710	СМ	4-7	28
7	6.02	28.46	146	124,500	4380	СМ	4-7	28
8	6.02	28.46	146					Reserve
Pamarka								
Remarks:Specimen or cap defects:Lincoln DeVore requires a minimum of l working day's notice to schedule personnel for any field tests and observations. Compressive strength test performed according to ASTM C-39.1-Ute WaterFinal report will include data for all cylinders, and will be sent after the 28-day break. This laboratory cannot								
 Does not Break Types CM - Consi CA - Consi V - Sheat Date Issued 	meet requin s: ical Mortar ical Aggrega ar Break 1: <u>4-7-97</u>	red strength (Break ate Break	if applic	able) of la LI By	the test boratory NCOLN DeV	results personnel ORE, INC.	by other	than
	CONCRETE TES	ST REPORT			OLN co DRE GI EERS	DLORADO: (Rand Jung)	COLORADO	SPRINGS

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Client Project Concrete S Truck No. Ticket No. Date of Te Mix, Propo 28-day Rec	Alpine CM Vista del Supplier est prtions quired Stren	GJRM 24 14527 4-30-97 gth	ps	- Cen - Slu - Air - Ten - Tes i Wat	Job No Test By Locatio partia Rio Li ment Type ump (ASTM Content nperature st @ cer Added	85853- 	-1403 Valley p k/curve, & Rio Bor 01-B 3 3 231) 5.5 1064) 70 chute 0 gallons	an & corner of de /4 inches % % F. yds.
6" x 12" Cylinder No.	Avg. Cyl. Diameter (in.)	Cross- Sectional Area (in.')	Unit Weight (pcf)	Total Load (lbs.)	Unit Stress (psi)	Break Type	Break Date	Age (days)
13	6.05	28.75	146.6	85,500	2970	СМ	.5-7	7
14	6.05	28.75	146.4	119,900	4170	CM	5–28	28
15	6.05	28.75	146.4	121,000	4230	СМ	5–28	28
16	6.05	28.75	146.4					Reserve
Remarks: Specimen o	r cap defect	ts:		Li	ncoln DeV	ore requi	ires a min	imum of
Distributi	l working day's notice to schedule Distribution: personnel for any field tests and							

1-Client 1-Subdiv Env 1-Ute Water 1-LD/CS 1-GJRM 1-Nichols & Assoc. 1-City of GJ

* Does not meet required strength (if applicable)

Break Types: CM - Conical Mortar Break CA - Conical Aggregate Break V - Shear Break

Date Issued: 5-78-97 Januar

CONCRETE TEST REPORT

By: _ Z

observations. Compressive strength

test performed according to ASTM C-39.

Final report will include data for all

cylinders, and will be sent after the 28-day break. This laboratory cannot

be responsible for any interpretation of the test results by other than

LINCOLN DeVORE Engineers geologists COLORADO: COLORADO SPRINGS GRAND JUNCTION, PUEBLO

laboratory personnel.

LINCOLN DeVORE, INC.







Traffic Study VISTA DEL RIO SUBDIVISION MESA COUNTY, COLORADO

Prepared for:

ALPINE C.M., INC. GRAND JUNCTION, COLORADO

Prepared by:



RG CONSULTING ENGINEERS, INC. DENVER, COLORADO

August 1994



rg consulting engineers, inc.

August 1, 1994

Mr. V. Kevin Nourse Alpine C.M., Inc. 1111 South 12th Street Grand Junction, Colorado 81501

RE: Vista Del Rio Subdivision Mesa County, Colorado

Dear Mr. Nourse:

RG Consulting Engineers, Inc. is very pleased to submit this traffic study of the proposed Vista Del Rio Subdivision.

As you will note in our report, we find that traffic generated by the Vista Del Rio Subdivision will have minor effect on the area street network. We trust this report will be useful in implementing this project.

We have enjoyed working with you and the Mesa County traffic engineering staff during this study. If you have any questions or comments concerning our report, please do not hesitate to call.

Very truly yours,

RG CONSULTING ENGINEERS, INC.

Mark C. Schaefer, P.E. Transportation Engineering Manager

MCS:dm

Enclosure



MCS\VISTADEL.RIO

1331 17th street • suite 710 • denver, colorado 80202 • (303) 293-8107 fax (303) 293-8106



TRAFFIC STUDY

VISTA DEL RIO SUBDIVISION MESA COUNTY, COLORADO

Prepared for:

ALPINE C.M., INC. Grand Junction, Colorado

Prepared by:

.

RG CONSULTING ENGINEERS, INC. Denver, Colorado

August 1994

TRAFFIC STUDY

VISTA DEL RIO SUBDIVISION MESA COUNTY, COLORADO

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3.0	Conclusions	5

Appendix A: Year 2015 Traffic Forecasts
Appendix B: Intersection Level-of-Service Analysis
Appendix C: Traffic Signal Warrant Analysis
Appendix D: Intersection Sight Distance Analysis

RG Consulting Engineers, Inc.

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TRAFFIC STUDY

VISTA DEL RIO SUBDIVISION MESA COUNTY, COLORADO

1.0 INTRODUCTION

This report details an evaluation of the traffic impacts of the proposed development of the Vista Del Rio Subdivision. The proposed 50 dwelling unit subdivision is located northwest of the City of Grand Junction in Mesa County (see Figure 1). The primary access point to the development will be the intersection of Redlands Parkway and Vista Del Rio Drive.

This report was prepared in accordance with Article VII of the Mesa County Standard Specifications for Road and Bridge Construction, adopted March 1, 1994.

2.0 TRAFFIC ANALYSIS

2.1 <u>Traffic Data</u>

Peak hour intersection turning movement counts were collected by Alpine C.M., Inc. during the week of July 18, 1994 (see Figure 2). Counts were made at the intersections of Redlands Parkway/Vista Del Rio Drive and Redlands Parkway/S.H. 340 (Broadway).

Year 2015 daily traffic forecasts for the Redlands Parkway corridor were obtained from the Mesa County Department of Public Works. In the vicinity of Vista Del Rio Drive, <u>four</u>-lane Redlands Parkway is forecast to carry 29,850 vehicles/day (see Appendix A).

2.2 <u>Trip Generation</u>

Trip generation estimates were based on the latest trip generation rates compiled in the Institute of Transportation Engineers' report, <u>Trip Generation</u>, for "Single-Family Detached Housing" (Land Use Code 210). The resulting trip estimates are shown below:

	<u>A.M. Peak</u>	<u>P.M. Peak</u>	<u>Daily</u>
Trip Rate (trips/d.u.)	0.74	1.01	9.55
Entering	26%	64%	50%
Exiting	74%	36%	50%
Total Trips from 50 d.u. Development	37	51	478
Entering	10	33	239
Exiting	27	18	239
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d.u. = dwelling unit.

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VISTA DEL RIO SUBDIVISION TRAFFIC STUDY

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FIGURE 1 SITE PLAN AND VICINITY MAP

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EXISTING + SITE-GENERATED TRAFFIC

LEGEND: AM (PM) PEAK HOUR

VISTA DEL RIO SUBDIVISION TRAFFIC STUDY **RG** CONSULTING ENGINEERS, INC.

11 129 61 (60)(217)(46)

FIGURE 2 TRAFFIC DATA

2.3 Trip Assignment and Distribution

Site-generated intersection turning movements were assumed for this analysis to follow existing traffic patterns. The traffic patterns during the July 18 count period may be somewhat skewed from typical traffic patterns at this intersection due to construction on S.H. 340. We would expect that one effect of this skewed pattern would be greater proportion of eastbound left turns from Vista Del Rio Drive.

The traffic assignment for this analysis is shown in Figure 2.

2.4 <u>Level-of-Service Analysis</u>

Level-of-service (LOS) estimates for the intersections of Redlands Parkway/Vista Del Rio Drive and Redlands Parkway/S.H. 340 (Broadway) were made using the procedures described in the <u>Highway Capacity Manual</u>, 1985. Computer printouts of the analyses are attached as Appendix B to this report.

Level-of-service estimates for the unsignalized intersection of Redlands Parkway/Vista Del Rio Rive are summarized in the following table. As shown, there is no significant change in the level-of-service when site-generated traffic is added to existing volumes.

	Level-of-Service				
	A.M. Peak Hour		<u>P.M. P</u>	eak Hour	
Turning Movement	<u>Existing</u>	with Site	<u>Existing</u>	with Site	
Eastbound Left	D	D	Ε	Е	
Eastbound Right	Α	Α	Α	Α	
Northbound Left	Α	Α	Α	В	

Since LOS "E" operations were estimated for the Redlands Parkway/Vista Del Rio Drive intersection, it is appropriate to make some comment on the <u>Highway Capacity Manual</u> (HCM) analysis procedures for evaluating unsignalized intersection capacity. As noted in <u>Interim Materials on Unsignalized Intersection Capacity</u>, <u>Transportation Research Circular 373</u> (July 1991), the "reserve capacity" computation of the HCM has not gained extensive acceptance by the U.S. user community. Circular 373 indicates "Some research has indicated that the TWSC [two-way stop controlled] technique consistently underestimates Reserve Capacity and thus yields a poorer level of service than actually occurs" (p. 9). Further, Circular 373 states "The movements which yielded poorer levels of service than actually observed in the field were the through movements and left turns from the minor street" (p. 7). Referring to unpublished field surveys which attempted to relate calculated Reserved Capacity to observed average vehicle delay, Circular 373 reports "This limited data collection effort indicated that Levels of Service

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E or F (as determined using the analysis procedures contained in Chapter 10 of the 1985 HCM) correlated with average vehicle delays of between 15 and 35 seconds" (p. 7).

Specific to the Redlands Parkway/Vista Del Rio Drive intersection analysis, we believe the most appropriate conclusion that can be drawn is that there is no appreciable change in the level-of-service between the "existing" and "with site" scenarios.

Mesa County's traffic forecasts of the Redlands Parkway corridors shows a near tripling of daily traffic on Redlands Parkway by 2015. As expected, predicted service levels at the Redlands Parkway/Vista Del Rio Drive drop to LOS "F" levels (using the "reserve capacity" method) when existing trip distribution patterns are held constant (although it could be reasonably argued that local traffic patterns — specifically, the eastbound left turning movement, will change as delay levels increase). In addition to expected changes in traffic patterns, planning for the four-laning of Redlands Parkway should incorporate geometric design improvements (such as a median acceleration lane) to help facilitate the eastbound left-turn movement.

The signalized intersection of Redlands Parkway/S.H. 340 (Broadway) currently operates at LOS "B" during the weekday a.m. and p.m. peak hours. There is no change in the level-of-service when site-generated traffic is added to existing volumes.

Although a level-of-service analysis was not conducted for the signalized Redlands Parkway/I-70 Business Loop intersection, we would anticipate a conclusion consistent with the analysis of the previous two intersections — no change in level-of-service.

2.5 Traffic Signal Warrant Analysis

The need for a traffic signal at the Redlands Parkway/Vista Del Rio Drive intersection was reviewed against the traffic signal warrants described in the <u>Manual on Uniform Traffic Control</u> <u>Devices</u> (MUTCD) (see Appendix C). With peak hour approach volumes of less than 60 vehicles on Vista Del Rio Drive, intersection volumes <u>do not</u> meet the volume-based traffic signal warrants described in the <u>MUTCD</u>. Further, since the land uses served by Vista Del Rio Drive will have been substantially built-out with the Vista Del Rio Subdivision, we can anticipate that, barring unforeseen development, traffic volumes on Vista Del Rio Drive will never reach sufficient magnitude to meet the traffic volume-based warrants of the <u>MUTCD</u>.

Mesa County has not recorded any accidents at this intersection in the last five years; therefore, Warrant 6 "Accident Experience" is not presently met.

The <u>MUTCD</u> does consider several other conditions, besides traffic volume and accident history, under which a traffic signal installation may be warranted. As an example, locations with significant pedestrian traffic may warrant a signal, even if traffic volumes on the area street system are not high enough to meet the traffic volume-based warrants of the <u>MUTCD</u>. A pedestrian signal has been installed on Redlands Parkway one-third of a mile south of the Redlands Parkway/Vista Del Rio Drive intersection. There is not sufficient data within this present traffic study to judge whether pedestrian volumes presently, or will in the future, warrant

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a signal at the Redlands Parkway/Vista Del Rio Drive intersection, or whether pedestrian traffic in the Redlands Parkway corridor would be best served by either relocating the present pedestrian signal or providing some alternative means for pedestrian crossings of Redlands Parkway.

2.6 Intersection Sight Distance Analysis

- Using available topographic mapping of the Vista Del Rio Subdivision site and Mesa County design plans for Redlands Parkway, sight distance at the Redlands Parkway/Vista Del Rio Drive intersection was estimated in accordance with Article 4.7.5 of the Mesa County Standard Specifications for Road and Bridge Construction. Sight distance requirements were estimated for a 50 mph running speed (500 feet), adjusted by an additional 47 feet in consideration of the grade on Redlands Parkway.
- As shown on the graphic in Appendix D, roadside grades obstruct the sight line by approximately 0.5 foot. Proposed construction of the Vista Del Rio Subdivision will not create any additional impediment to intersection sight distance.

Consistent with the requirements of the Mesa County standards, the sight distance analysis was based on the "driver's eye" located 15 feet from the edge of pavement on Redlands Parkway. The intersection of Redlands Parkway/Vista Del Rio Drive, however, has right turn accel/decel lanes, and drivers exiting Vista Del Rio Drive can safely move further into the intersection to improve their line of sight prior to initiating a turn. The existing intersection sight distance was apparently deemed sufficient for Mesa County to increase the posted speed on Redlands Parkway to 50 mph. As noted previously, Mesa County has not reported any accidents at the Redlands Parkway/Vista Del Rio Drive intersection in the past five years.

The future four-laning of Redlands Parkway should improve the available sight distance at the Redlands Parkway/Vista Del Rio Drive intersection since the angle of the sight line will be increased.

2.7 Local Street Cross-Section Issues

Mesa County has adopted a number of standard street sections corresponding to the functional roadway classification system adopted by the County.

Vista Del Rio Drive, an existing street, was constructed with a 28 foot roadway width. Onstreet parking is not allowed. This cross-section corresponds to the "Urban Residential Subcollector" classification of the Mesa County standards. This street section typically serves traffic demand in the range of 250 to 1000 vehicles/day. Within the Mesa County roadway system, the next highest residential roadway cross-section which is designed for "no-parking" operation is the "Collector Street" which allows for a 44 roadway width which accommodates a center left turn lane.

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With full development of the Vista Del Rio Subdivision, traffic volumes on Vista Del Rio Drive are forecast at approximately 1150 vehicles/day. While this traffic level slightly exceeds the desirable range of the "Urban Residential Subcollector", the limited left turn demand to the culde-sac streets intersecting Vista Del Rio Drive would not warrant the reconstruction of Vista Del Rio Drive to the "Collector Street" standard.

3.0 <u>CONCLUSIONS</u>

It is evident from this analysis that the traffic generated by the Vista Del Rio Subdivision will have a very minor effect on the area street network. Specifically:

- Area intersection levels-of-service are not significantly affected by traffic from the proposed development.
- Traffic volume-based signal warrants are not met for the Redlands Parkway/Vista Del Rio Drive intersection.
- Sight distance at the Redlands Parkway/Vista Del Rio Drive intersection sight distance, while slightly below County standards, is not further impeded by the proposed development.
- The existing Vista Del Rio Drive roadway cross-section is adequate and appropriate for existing and proposed traffic volumes.

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Vista Del Rio Subdivision Traffic Study

APPENDIX A

YEAR 2015 TRAFFIC FORECASTS

RG Consulting Engineers, Inc.



JUL-19-94 TUE 16:20

MESA COUNTY SUPPORT SERV

FAX NO. 3032441639

P. 03

APPENDIX B

INTERSECTION LEVEL-OF-SERVICE ANALYSIS

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1985 HCM: UNSIGNALIZED INTERSECTIONS Page-1 *******

IDENTIFYING INFORMATION

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AVERAGE RUNNING SPEED, MAJOR STREET.. 50 AREA POPULATION..... 150000 NAME OF THE EAST/WEST STREET..... Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET..... Redlands Pkwy NAME OF THE ANALYST..... MC Schaefer DATE OF THE ANALYSIS (mm/dd/yy)..... 07-25-1994 TIME PERIOD ANALYZED..... AM Peak OTHER INFORMATION.... Existing INTERSECTION TYPE AND CONTROL

INTERSECTION TYPE: T-INTERSECTION MAJOR STREET DIRECTION: NORTH/SOUTH CONTROL TYPE EASTBOUND: STOP SIGN

TRAFFIC VOLUMES

______ ______

	EB	WB	NB	SB
LEFT	27		3	0
THRU	0		584	187
RIGHT	2		0	15

NUMBER OF LANES

	EB	WB	NB	SB	
LANES	2		1	1	

ADJUSTMENT FACTORS

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	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	90	20	Y
WESTBOUND			·	-
NORTHBOUND	0.00	90	20	N
SOUTHBOUND	0.00	90	20	N

VEHICLE COMPOSITION

 	 •																

	% SU TRUCKS AND RV'S	<pre>% COMBINATION VEHICLES</pre>	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND			
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAI (Table	R VALUES = 10-2)	ADJUSTED VALUE	SIGHT DIST. Adjustment	FINAL CRITICAL GAP			
MINOR RIGHT	S							
	EB 6	5.30	5.30	0.00	5.30			
MAJOR LEFTS	ND	- 40	E 40	0.00	F 40			
	ND .	5.40	5.40	0.00	5.40			
MINOR LEFTS	EB 7	7.70	7.70	0.00	7.70			
IDENTIFYING	INFORMATIC) N						
NAME OF THE EAST/WEST STREET Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET Redlands Pkwy DATE AND TIME OF THE ANALYSIS 07-25-1994 ; AM Peak								

OTHER INFORMATION.... Existing

CAPACITY AND LEVEL-OF-SERVICE

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POTEN-ACTUALFLOW-TIALMOVEMENTSHAREDRESERVERATECAPACITYCAPACITYCAPACITYCAPACITYv(pcph)c (pcph)c (pcph)c (pcph)c = c - v LOSpMSHR SH MOVEMENT _____ MINOR STREET LEFT33198198RIGHT2912912 165 D 910 A 198 EB LEFT 912 MAJOR STREET 4 885 885 885 881 A NB LEFT

IDENTIFYING INFORMATION

_____ NAME OF THE EAST/WEST STREET..... Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET.... Redlands Pkwy DATE AND TIME OF THE ANALYSIS..... 07-25-1994 ; AM Peak OTHER INFORMATION.... Existing

IDENTIFYING INFORMATION

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AVERAGE RUNNING SPEED, MAJOR STREET	50
PEAK HOUR FACTOR	. 9
AREA POPULATION	150000
NAME OF THE EAST/WEST STREET	Vista Del Rio Dr
NAME OF THE NORTH/SOUTH STREET	Redlands Pkwy
NAME OF THE ANALYST	MC Schaefer
DATE OF THE ANALYSIS (mm/dd/yy)	07-25-1994
TIME PERIOD ANALYZED	AM Peak
OTHER INFORMATION Existing + Site	
INTERSECTION TYPE AND CONTROL	

INTERSECTION TYPE: T-INTERSECTION MAJOR STREET DIRECTION: NORTH/SOUTH CONTROL TYPE EASTBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB	
LEFT	52			0	
THRU	o		584	187	
RIGHT	4		0	23	

NUMBER OF LANES

	EB	WB	NB	SB
LANES	2		1	1

ADJUSTMENT FACTORS

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	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	90	20	Ŷ
 WESTBOUND				-
NORTHBOUND	0.00	90	20	N
SOUTHBOUND	0.00	90	20	N

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	<pre>% COMBINATION VEHICLES </pre>	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND			
NORTHBOUND	ο	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR (Table	VALUES 10-2)	ADJUSTED VALUE	SIGHT DIST. Adjustment	FINAL CRITICAL GAP				
MINOR RIGHT:	S EB 6	.30	5.30	0.00	5.30				
MAJOR LEFTS	NB 5	.40	5.40	0.00	5.40				
MINOR LEFTS	E B 7	.70	7.70	0.00	7.70				
IDENTIFYING	INFORMATIO	N							
NAME OF THE EAST/WEST STREET Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET Redlands Pkwy DATE AND TIME OF THE ANALYSIS 07-25-1994 ; AM Peak OTHER INFORMATION Existing + Site									

CAPACITY AND LEVEL-OF-SERVICE

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MOVEMENT	FLOW- RATE V(pcph)	POTEN- TIAL CAPACITY C (pcph) P	ACTUAL MOVEMENT CAPACITY C (pcph) M	SHARED CAPACITY C (pcph) SH	RESERVE CAPACITY C = C - V R SH	LOS
MINOR STREET						
EB LEFT RIGHT	64 5	196 908	195 908	195 908	131 903	D A
MAJOR STREET						
NB LEFT	6	876	876	876	870	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET.... Redlands Pkwy DATE AND TIME OF THE ANALYSIS..... 07-25-1994 ; AM Peak OTHER INFORMATION.... Existing + Site

1985 HCM: UNSIGNALIZED INTERSECTIONS Page-1 IDENTIFYING INFORMATION ______ AVERAGE RUNNING SPEED, MAJOR STREET.. 50 PEAK HOUR FACTOR..... . 9 NAME OF THE EAST/WEST STREET..... Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET..... Redlands Pkwy NAME OF THE ANALYST..... MC Schaefer DATE OF THE ANALYSIS (mm/dd/yy)..... 07-25-1994 TIME PERIOD ANALYZED..... AM Peak OTHER INFORMATION.... 2015 + Site INTERSECTION TYPE AND CONTROL _____ **INTERSECTION TYPE: T-INTERSECTION** MAJOR STREET DIRECTION: NORTH/SOUTH CONTROL TYPE EASTBOUND: STOP SIGN TRAFFIC VOLUMES _____ EB WB NB SB ----____ _ _ _ _ ----LEFT 52 --5 0 -- 1709 547 THRU 0 RIGHT 4 -- 0 23

NUMBER OF LANES

	EB	WB	NB	SB
LANES	2		2	2

ADJUSTMENT FACTORS

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	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	90	20	Ŷ
WESTBOUND				-
NORTHBOUND	0.00	90	20	N
SOUTHBOUND	0.00	90	20	N

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND			
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR (Table	R VALUES 2 10-2)	ADJUSTED VALUE	SIGHT DIST. Adjustment	FINAL CRITICAL GAP
MINOR PICHT					
MINOK KIOMI.	EB 6	5.30	5.30	0.00	5.30
MAJOR LEFTS					
	NB 5	5.90	5.90	0.00	5.90
MINOR LEFTS					
	EB 8	3.20	8.20	0.00	8.20
IDENTIFYING INFORMATION					
NAME OF THE	EAST/WEST	STREET	Vista De	el Rio Dr	
NAME OF THE	NORTH/SOUT	'H STREET	Redlands	s Pkwy	
OTHER INFORM	AE OF THE A	2015 + Sit	0/-25-19 e	994 ; AM Peak	

CAPACITY AND LEVEL-OF-SERVICE

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MOVEMENTFLOW-
RATE
v(pcph)POTEN-
TIAL
CAPACITY
c(pcph)ACTUAL
MOVEMENT
CAPACITY
c(pcph)SHARED
CAPACITY
c(pcph)RESERVE
CAPACITY
c(pcph)MINOR STREETEB
RIGHT64
541
81541
815----
815----
815----
815----
815MAJOR STREETNB
LEFT6
6475475475469A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET.... Redlands Pkwy DATE AND TIME OF THE ANALYSIS..... 07-25-1994 ; AM Peak OTHER INFORMATION.... 2015 + Site

Page-3

1985 HCM: UNSIGNALIZED INTERSECTIONS Page-1 -----IDENTIFYING INFORMATION _____ AVERAGE RUNNING SPEED, MAJOR STREET.. 50 AREA POPULATION..... 150000 NAME OF THE EAST/WEST STREET..... Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET..... Redlands Pkwy NAME OF THE ANALYST..... MC Schaefer DATE OF THE ANALYSIS (mm/dd/yy)..... 07-25-1994 TIME PERIOD ANALYZED..... PM Peak OTHER INFORMATION.... Existing INTERSECTION TYPE AND CONTROL INTERSECTION TYPE: T-INTERSECTION

MAJOR STREET DIRECTION: NORTH/SOUTH CONTROL TYPE EASTBOUND: STOP SIGN

TRAFFIC VOLUMES

	EB	WB	NB	SB
LEFT	11		10	0
THRU	0		360	708
RIGHT	5		0	34

NUMBER OF LANES

	EB	WB	NB	SB
LANES	2		1	1

ADJUSTMENT FACTORS

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	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	90	20	Ŷ
WESTBOUND				-
NORTHBOUND	0.00	90	20	N
SOUTHBOUND	0.00	90	20	N

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	<pre>% COMBINATION VEHICLES</pre>	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND			
NORTHBOUND	0	0	0
SOUTHBOUND	о	0	0

CRITICAL GAPS

	TABULAR (Table	VALUES 10-2)	ADJUSTED VALUE	SIGHT DIST. Adjustment	FINAL CRITICAL GAP
MINOR RIGHTS	5 EB 6	. 30	5.30	0.00	5.30
MAJOR LEFTS	NB 5	. 40	5.40	0.00	5.40
MINOR LEFTS	EB 7	.70	7.70	0.00	7.70
IDENTIFYING INFORMATION					
NAME OF THE EAST/WEST STREET Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET Redlands Pkwy DATE AND TIME OF THE ANALYSIS 07-25-1994 ; PM Peak OTHER INFORMATION Existing					

CAPACITY AND LEVEL-OF-SERVICE

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MOVEMENT	FLOW- RATE v(pcph)	POTEN- TIAL CAPACITY c (pcph) p	ACTUAL MOVEMENT CAPACITY C (pcph) M	SHARED CAPACITY C (pcph) SH	RESERVE CAPACITY C = C - V R SH	LOS
MINOR STREET						
EB LEFT RIGHT	13 6	102 452	101 452	101 452	87 446	E A
MAJOR STREET						
NB LEFT	12	428	428	428	416	A

IDENTIFYING INFORMATION

NAME OF THE EAST/WEST STREET..... Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET.... Redlands Pkwy DATE AND TIME OF THE ANALYSIS..... 07-25-1994 ; PM Peak OTHER INFORMATION.... Existing

1985 HCM: UNSIGNALIZED INTERSECTIONS Page-1

IDENTIFYING INFORMATION

INTERSECTION TYPE: T-INTERSECTION MAJOR STREET DIRECTION: NORTH/SOUTH CONTROL TYPE EASTBOUND: STOP SIGN

TRAFFIC VOLUMES

_ _ _ _ _ _ _ _

	EB	WB	NB	SB
LEFT	23		18	0
THRU	0		360	708
RIGHT	11		0	60

NUMBER OF LANES

_ _ _ _ _

	EB	WB	NB	SB
LANES	2		1	1

ADJUSTMENT FACTORS

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	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	90	20	Ŷ
WESTBOUND				-
NORTHBOUND	0.00	90	20	N
SOUTHBOUND	0.00	90	20	N

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND			
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

	TABULAR (Table	VALUES A 10-2)	ADJUSTED A	SIGHT DIST. Adjustment	FINAL CRITICAL GAP
MINOR RIGHTS	5 EB 6	.30	5.30	0.00	5.30
MAJOR LEFTS	NB 5	.40	5.40	0.00	5.40
MINOR LEFTS	EB 7	.70	7.70	0.00	7.70
IDENTIFYING	INFORMATIO	N			
NAME OF THE NAME OF THE DATE AND TIM OTHER INFORM	EAST/WEST (NORTH/SOUT) IE OF THE A) IATION 1	STREET H STREET NALYSIS Existing +	. Vista De . Redlands . 07-25-199 Site	l Rio Dr Pkwy 94 ; PM Peak	

1985 HCM: UNSIGNALIZED INTERSECTIONS Page-1 ______ ****** * * * * * IDENTIFYING INFORMATION AVERAGE RUNNING SPEED, MAJOR STREET... 50 AREA POPULATION..... 150000 NAME OF THE EAST/WEST STREET..... Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET..... Redlands Pkwy NAME OF THE ANALYST..... MC Schaefer DATE OF THE ANALYSIS (mm/dd/yy)..... 07-25-1994 TIME PERIOD ANALYZED..... PM Peak OTHER INFORMATION.... 2015 + Site INTERSECTION TYPE AND CONTROL -------------INTERSECTION TYPE: T-INTERSECTION MAJOR STREET DIRECTION: NORTH/SOUTH CONTROL TYPE EASTBOUND: STOP SIGN

TRAFFIC VOLUMES

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	EB	WB	NB	SB
LEFT	23		18	0
THRU	0		1053	2071
RIGHT	11		0	60

NUMBER OF LANES

	EB	WB	NB	SB
LANES	2		2	2

ADJUSTMENT FACTORS

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	PERCENT GRADE	RIGHT TURN ANGLE	CURB RADIUS (ft) FOR RIGHT TURNS	ACCELERATION LANE FOR RIGHT TURNS
EASTBOUND	0.00	90	20	Ŷ
WESTBOUND				-
NORTHBOUND	0.00	90	20	N
SOUTHBOUND	0.00	90	20	N

VEHICLE COMPOSITION

	% SU TRUCKS AND RV'S	% COMBINATION VEHICLES	% MOTORCYCLES
EASTBOUND	0	0	0
WESTBOUND			
NORTHBOUND	0	0	0
SOUTHBOUND	0	0	0

CRITICAL GAPS

. •	TABULAF (Table	R VALUES 10-2)	ADJUSTED VALUE	SIGHT DIST. Adjustment	FINAL CRITICAL GAP
MINOR RIGHTS	5 EB 6	5.30	5.30	0.00	5.30
MAJOR LEFTS	NB 5	5.90	5.90	0.00	5.90
MINOR LEFTS	EB 8	3.20	8.20	0.00	8.20
IDENTIFYING	INFORMATIC) N			
NAME OF THE NAME OF THE DATE AND TIM OTHER INFORM	EAST/WEST NORTH/SOUT 1E OF THE A 1ATION	STREET TH STREET ANALYSIS 2015 + Sit	Vista De Redlands 07-25-19	el Rio Dr 5 Pkwy 994 ; PM Peak	

CAPACITY AND LEVEL-OF-SERVICE

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MOVEMENT	FLOW- RATE v(pcph)	POTEN- TIAL CAPACITY C (pcph) P	ACTUAL MOVEMENT CAPACITY C (pcph) M	SHARED CAPACITY c (pcph) SH	RESERVE CAPACITY c = c - v R SH	LOS
MINOR STREET						
EB LEFT RIGHT	28 13	41 283	36 283	36 283	8 270	E C
MAJOR STREET						
NB LEFT	22	121	121	121	99	E

IDENTIFYING INFORMATION

______ NAME OF THE EAST/WEST STREET..... Vista Del Rio Dr NAME OF THE NORTH/SOUTH STREET.... Redlands Pkwy DATE AND TIME OF THE ANALYSIS.... 07-25-1994 ; PM Peak OTHER INFORMATION.... 2015 + Site

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NB		0.0	0	2.00	N		0		0	0	.90		50		Y	19.8		3
SB		0.0	0	2.00	N		0		0	0	.90		50		Y	19.8		3
							SI	GN.	AL SET	 [T]	NGS				CYC	CLE LEN	 GTH =	90.
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	PD											PD						
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WB	LT TH RT PD	2	K								SB	LT TH RT PD			X X			
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Vista Del Rio Subdivision Traffic Study

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APPENDIX C TRAFFIC SIGNAL WARRANT ANALYSIS

RG Consulting Engineers, Inc.

#### **RG CONSULTING ENGINEERS, INC.** TRAFFIC SIGNAL VEHICLE VOLUME WARRANTS

PROJECT: VISTA DEL RIO SUBDIVISION	CALC_M2	DATE_	7/25/94
PROJECT NUMBER: 149001	СНК	DATE	
Major St: REDLANDS PKWY	·	_ Critical Approach Speed	SO mph
Minor St: VISTA DEL RIO DR		_ Critical Approach Speed	mph
Critical speed of major street traffic $\geq$ 40 r In built up area of isolated community of $\leq$	nph 10,000 pop	OR RURAL (R) URBAN (U)	

#### TRAFFIC VOLUMES

		<b>.</b>		Volu	mes	<u> </u>	
Approach	✓ = major street	ADT	AM <u>Peak</u>	PM <u>Peak</u>	8th Hr Factor*	8th Hr <u>Volume</u>	4th Hr <u>Volume**</u>
Eastbound		2:579	56	34	.0571	33	41
Westbound		579					
Northbound	~	5000	589	378	.0543	272	340
Southbound	~	5200	187	708	.0514	267	334
* Refer to atta	ached table **	Assume 125% of 8	th hour v	olume			

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Sec. .

		MINIMUM REQUIREMENTS (80% SHOWN IN BRACKETS)				
	U	R	U	R		
APPROACH LANES	•	1	2 or	more	8th Hour Volume	
Both Apprchs. Major Street	500 (400)	350 (280)	600 (480)	420 (336)	539	
Highest Apprch. Minor Street*	150 (120)	105 (84)	200 (160)	140 (112)	33	

* NOTE: Heavier left turn movement from Major Street included when LT-phasing is proposed  $\Box$ 

#### WARRANT 2 - Interruption of Continuous Traffic

WARRANT 1 - Minimum Vehicular Volume

		MINIMUM	REQU WN IN I	IREMENTS BRACKETS	S )	
	υ	R		U	R	
APPROACH LANES		1		2 or	more	8th Hour Volume
Both Apprchs. Major Street	750 (600)	525 (420)		900 (720)	630 (504)	539
Highest Apprch. Minor Street*	75 (60)	53 (42)		100 (80)	70 (56)	33

* NOTE: Heavier left turn movement from Major Street included when LT-phasing is proposed  $\Box$ 

100% SATISFIED	YES 🗆	NO 🛛
80% SATISFIED	YES 🗖	NO 🕱

(Continued)

#### 100% SATISFIED 80% SATISFIED

YES 🗆 NO 🎘 YES 🗆 NO 🕱

#### RG CONSULTING ENGINEERS, INC. TRAFFIC SIGNAL VEHICLE VOLUME WARRANTS (Continued)

#### WARRANT 8 - Combination of Warrants

SATISFIED YES 🗆 NO 🕱

REQUIREMENT	WARRANT	1	FULFILLED
TWO WARRANTS	1 - MINIMUM VEHICULAR VOLUME		
SATISFIED	2 - INTERRUPTION OF CONTINUOUS TRAFFIC		
80%			YES 🗆 NO 🛛

#### WARRANT 9 - Four Hour Volume

# SATISFIED YES D NO X

YES 🗆 NO 🗆

Approach Lanes	One	2 or more	4th Hour Volume
Both Approaches , Major Street			674
Highest Approaches , Minor Street			41

* Refer to attached graph to determine if this warrant is satisfied.

WARRANT 11 - Pea	ak Hour Volume		SATISFI	ED* YES	
	Approach Lanes	One	2 or more	AM Peak Hour Volume	РМ
	Both Approaches , Major Street			776	1086
	Highest Approaches , Minor Street		I	56	34

* Refer to attached graph to determine if this warrant is satisfied.

SUMMARY			
Warrant	Checked ?	Satisfied ?	Comments
1	Y	N	
2	Y	N	
8	Y	N	
9	4	N	
11	V	N	

YES IN NO

SIGNAL WARRANTED?

MCS\WARRANTS.FRM

# 8TH HOUR FACTORS

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% Peak Hour	Factor to Obtain 8th Hour
7	0.0614
8	0.0600
9	0.0586
10	0.0571
11	0.0557
12	0.0543
13	0.0529
14	0.0514
15	0.0500

Source: Missouri Highway Department



# APPENDIX D

#### INTERSECTION SIGHT DISTANCE ANALYSIS

RG Consulting Engineers, Inc.



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VISTA DEL RIO SUBDIVISION TRAFFIC STUDY RG CONSULTING ENGINEERS, INC. FIGURE D INTERSECTION SIGHT DISTANCE

# **REVIEW COMMENTS**

Page 1 of 3

FILE #FPP-95-182

TITLE HEADING:

Vista Del Rio Subdivision, Filing #3

Rio Linda Lane & Redlands Parkway **LOCATION:** 

**PETITIONER:** Alpine C.M., Inc.

**PETITIONER'S ADDRESS/TELEPHONE:** 

1111 South 12th Street Grand Junction, CO 81501 242-2505

**PETITIONER'S REPRESENTATIVE:** 

Nichols Associate

**STAFF REPRESENTATIVE:** Michael Drollinger

#### THE PETITIONER IS REQUIRED TO SUBMIT FOUR (4) COPIES OF WRITTEN NOTE: **RESPONSE AND REVISED DRAWINGS ADDRESSING ALL REVIEW COMMENTS ON OR BEFORE** 5:00 P.M., OCTOBER 26, 1995.

U.S. WEST	10/4/95
Max Ward	244-4721
New or additional telephone facilities neces	sitated by this project may result in a "contract" and up-
front monies required from developer, pr	ior to ordering or placing of said facilities. For more
information, please call 1-800-526-3557.	

GRAND JUNCTION FIRE DEPARTMENT	10/10/95
Hank Masterson	244-1414
The Fire Department has no problems with t	his proposal.
CITY DEVELOPMENT ENGINEER	10/11/95
Jody Kliska	244-1591

#### DRAINAGE REPORT

Needs to include sizing of storm drain inlets and check flows in the street. 1.

#### **COMPOSITE PLAN**

Storm sewer is shown outside of the easement. 1.

#### **GRADING AND STORMWATER MANAGEMENT PLAN**

- 1. Need to provide contours.
- Identify surface disturbance area if no Colorado Dept. of Health permit has previously been 2. acquired, one must be applied for. Section IX of SWMM contains the state regulations and phone numbers for contacts.

A Storm Drainage Plan and Profile is necessary - SSID IX-30. Please identify type and sizes 3. of storm drain inlets, detail of erosion protection at pipe inlet from wetlands, outlet at outfall.

#### **ROADWAY PLANS**

- 1. The street name signs can be mounted above the stop signs on the same post.
- 2. Identify the type and size of storm drain inlets on the plan view.

#### PLAT

1. The dedication language needs to contain a dedication for each specific easement, right of way or open space parcel shown on the plat. A copy of the City's Guide to Plat Dedications is attached.

PUBLIC SERVICE COMPANY	10/11/95
G. Lewis	244-2698

Existing Public Service Company easement across property will be quit claimed to match easement as shown on plat with petitioner supplying information on existing easement per prior agreement.

CITY PROPERTY AGENT	10/11/95	
Steve Pace	244-1452	

1. Should reference monuments be set along the south edge of the Colorado River?

2. The monumentation for the outer boundary of Filing 3 should be set in concrete.

3. The City format for addressing individual easements in the dedication should be followed.

4. The P.O.B. tie should show a N.W. direction to match the description.

- 5. This filing is not within the City yet but will be in the near future, so we may need City signature blocks.
- 6. What about the area south and east of the Redlands Parkway is it part of this Filing 3?

MESA COUNTY SCHOOL DISTRICT #51	10/16/95		
Lou Grasso	242-8500		
SCHOOL - ENROLLMENT / CAPACITY - IMPACT			
Broadway Elementary - 274 / 400 - 6			
Redlands Middle School - 552 / 650 - 3			
Fruita Monument High School - 1337 / 1100 - 4			
REDIANDS WATER & POWER	10/13/95		

REDLANDS WATER & POWER	10/13/95	
Gregg Strong	243-2173	

No	impact	to	our	faci	lities.
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UTE WATER	10/16/95
Gary R. Mathews	242-7491

1. Water mains are installed 2' from curb and gutter, in the oil.

- 2. An 8" C-900 water line is sufficient for El Quevin Court unless the Fire Department requires a 10" line.
- 3. Water mains shall be c-900, class 150. Installation of pipe fittings, valves and services including testing and disinfection shall be in accordance with Ute Water standard specifications and drawings.
- 4. Developer is responsible for installing meter pits and yokes. Ute Water will furnish the meter pits and yokes.
- 5. Policies and fees in effect at the time of application will apply.

COMMUNITY DEVELOPMENT DEPARTMENT	10/12/95	
Michael Drollinger	244-1439	

See attached comments.

#### FPP-95-182 / REVIEW COMMENTS / page 3 of 3

CITY UTILITY ENGINEER	10/18/95
Trent Prall	244-1590

#### <u>GENERAL</u>

1. Final plans have not been stamped by a Registered Professional Engineer as required. <u>SEWER - CITY OF GRAND JUNCTION</u>

- 1. Easement required for sewer line between MH4 and MH5 across lots 6, 7 & 24.
- 2. How are existing sewer to be abandoned?
- 3. Construction notes referring to City Standard Drawings and Specifications are missing.
- 4. Sewer service line to lot 2 appears to tap centerline rather than existing sewer line.
- 5. Long service line to Lot 20 should have a cleanout added.
- 6. Show on profile where water lines intersect sewer lines such as on sewer Line 3 between MH3A and MH14. Also show special construction if needed.
- 7. 0.2' drop is required across manholes rather than the proposed 0.1' drop.
- 8. Add existing profile as required in SSID manual IX-35.
- 9. Please ensure all items specified in SSID manual IX-35 are adhered to.
- 10. No "Exhibit I" was included in the packet for review.

<u>WATER - UTE</u>

1. Please provide bend information for waterline.

## **CITY PARKS & RECREATION DEPARTMENT**

<b>U</b>		•			
<u>Shawr</u>	Cooper	244-3869			
1.	Maintain the additional right-of-way on west	side of parkway	as possible trail	easement.	
	Provide 15' pedestrian easement between lot	s 10 & 11 or 9	& 10 for public	c access to	
				1 6 .1	

- designated open-space. The issue of bike and pedestrian access in the area and from the subdivision needs to be discussed.2. Areas to be owned and maintained by HOA in perpetuity need to be indicated on documents
- and plats.
- 3. Parks & Open Space Fees 23 @ \$225 = \$5,175.

#### CITY POLICE DEPARTMENT

Dave Stassen

10/17/95	
244-3587	

10/20/95

10/17/95

This development causes no problems for the Police Department. The use of cul-de-sacs follows currently accepted crime prevention techniques by limiting unwanted access.

#### MESA COUNTY PLANNING DEPT.

Matt Osborn 244-1724 As a Mesa County requirement of approval for the ODP and Filing 1 of Vista Del Rio, the petitioner was to design and construct a bike path along the west side of Redlands Parkway from Rio Linda Lane to Greenbelt Drive. The petitioner was waived the Development Impact Fee for Filings 1 & 2. If the cost of the bike path exceeded the total D.I.F. for the entire project, the petitioner would be reimbursed. Approval of the second filing reiterated the need for the bike path. If the bike path is not constructed, the D.I.F. for filings 1 & 2 should be paid to the County.

#### TO DATE, COMMENTS HAVE NOT BEEN RECEIVED FROM:

City Attorney Mesa County Surveyor Persigo Wastewater Treatment Plant TCI Cablevision
# Vista Del Rio - Filing 3: Response to Final Plat/Plan Comments

# Michael Drollinger

# Amended Preliminary Plan

1. Please label Filing #4 to indicate the number of units proposed with this filing.

To be evaluated at a future date. Please see comments in project narrative.

# Final Plan Filing #3

# General Note: Street names have been changed from El Quevin Ct. and El Quolony Ct. to Casa Rio Ct. and Vista Rio Ct. respectively.

1. Spelling error on cover sheet should be corrected to read Loma Rio "Subdivision", not "Subdivion".

Spelling error corrected

2. Composite Plan does not clearly identify easement locations, it appears that some existing/proposed utilities are outside of the proposed easements. This office will perform a complete review once these deficiencies have been corrected.

Existing Sanitary Sewer Easement Added Easements Annotated

3. Grading and Drainage Plan shows no grading information; please use the SSID manual checklist to ensure that complete information is provided for review. This office will perform a complete review once these deficiencies are adequately corrected.

See comment Jody Kliska, Item 1

4. Grammatical error on Grading and Drainage Plan: detail on lower left corner should read "Straw Bale Barriers", not "Bales Barriers".

Text revised to read "Straw Bale Barriers"

5. Please indicate on Roadway Plan and Profile whether handicapped ramps at corner of Rio Linda Lane and "El Quevin Court" are provided; are they existing?

Access ramps exist - notation has been added to plan sheet

6. A signed and sealed copy of the Drainage Report must be provided to this office; the unsigned/unsealed copy on file is unacceptable.

The certification sheet has been signed and sealed

7. The Building Setbacks sheet must be modified to include a table indicating the setbacks by lot (or group of lots if setbacks are the same).

Table has been added to sheet.

8. The legend on the Building Setbacks table is confusing - the lines are difficult to distinguish on the drawing. What is a "Multipurpose Utility and Easement"?

"Multipurpose Utility and Easement" has been changed to "Multi-purpose and Utility Easement." On all drawings we have and have looked at we have had no difficulty distinguishing setback

or easement lines.

9. Larger setbacks from Redlands Parkway will be required for lots 1-6; we recommend a minimum of 30

1

3184 response

#### Viate Del Rio - Filing 3: Response to Final Plat/Plan Comments

#### feet from property line.

Per agreement between Mike Drollinger and Steve Colony (Oct. 20, 1995), rear setbacks for lots 1 to 5 abutting Redlands Parkway are increased from 20 to 25 feet. Rear setback for lot 6 remains at 25 feet.

10.

Please correct plat cover sheet to include City signature blocks, not County signature blocks. Also, all setback and area summary information must be transferred to Building Setbacks sheet which will be recorded with the plat. Dedication language for the open space and the conservation easement appears to be missing; please verify and correct accordingly.

- 1. City signature blocks have been added. County blocks have been removed.
- 2. Setback Table added to Setbacks Sheet.
- 3. Area Summary moved to Setbacks Sheet.

#### 11.

A landscaping plan for the entrance feature, if proposed, must be prepared and submitted for review. Landscaping for the common open space must be guaranteed as part of the development improvements agreement for the project.

Landscape Plan sheet has been added to submittal package.

#### 12.

The easement between Lots 13 & 15 should be labeled as a "drainage and utility easement; please verify that all other easements are labeled appropriately to reflect the types of utilities or use anticipated.

Annotation has been added.

#### 13.

Please refer to SSID Manual regarding folding and securing of plans for resubmittal.

#### US West - Mex Ward

1. New or additional telephone facilities necessitated by this project may result in a "contract" and up-front monies required from developer, prior to ordering or placing of said facilities. For more information, please call 1-800-526-3557

Application for facilities will be made

#### Grand Junction Fire Department - Hank Masterson

1. The Fire Department has no problems with this proposal.

#### City Development Engineer - Jody Kliaka

#### Drainage Report

1. Needs to include sizing of storm drain inlets and check flows in the street.

A Storm Drainage Plan and Profile Sheet has been added to the construction plans.

#### Composite Plan

1. Storm sewer is shown outside of the easement.

#### 3184 response

# Vista Del Rio - Filing 3: Response to Final Plat/Plan Comments

Storm sewer and easement have been modified

#### Grading and Stormwater Management Plan

1. Need to provide contours.

Contours to be provided as per discussion with Jody Kliska 4:12 pm Oct. 20, 1995.

2. Identify surface disturbance area-if no Colorado Department of Health permit has previously been acquired, one must be applied for. Section IX of SWMM contains the state regulations and phone numbers for contacts.

Surface disturbance area is shown and labeled.

3. A Storm Drainage Plan and Profile is necessary - SSID IX-30. Please identify type and sizes of storm drain inlets, detail of erosion protection at pipe inlet from wetlands, outlet at outfall.

A Storm Drainage Plan and Profile Sheet has been added to the construction plans.

#### **Roadway Plans**

1. The street name signs can be mounted above the stop signs on the same post.

Plan sheet modified to include street name signs with stop signs.

2. Identify the type and size of storm drain inlets on the plan view.

Plan sheet modified to identify inlets.

#### Plat

1. The dedication language needs to contain a dedication for each specific easement, right of way or open space parcel shown on the plat. A copy of the City's Guide to Plat Dedications is attached.

See response to Michael Drollinger comment Item 10.

#### Public Service Company - G. Lewis

 Existing Public Service Company easement across property will be quit claimed to match easement as shown on plat with petitioner supplying information on existing easement per prior agreement.

The existing lines are on a revocable license. We will record a new easement.

#### City Property Agent - Steve Pace

1. Should reference monuments be set along the south edge of the Colorado River?

Lot corner monuments and plat dimensions suffice to locate the meander line along the Colorado River. Also, it is impractical to set monuments on the steep bank.

2. The monumentation for the outer boundary of Filing 3 should be set in concrete.

Filing 3 boundary monuments will be set in concrete as required by city and county. Once set, appropriate notations will be added to subdivision plat.

3. The City format for addressing individual easements in the dedication should be followed.

City easement dedications have been incorporated.

4. The P.O.B. tie should show a N.W. direction to match the description.

3184 response

#### Vista Del Rio - Filing 3: Response to Final Plat/Plan Commenta

Annotation has been changed

5. This filing is not within the City yet but will be in the near future, so we may need City signature blocks.

See response to Michael Drollinger's comment item 10.

6. What about the area south and east of the Redlands Parkway - is it part of this Filing 3?

Area south and east of Redlands Parkway is not part of Filing 3.

#### Mass County School District #51 - Lou Grasso

School - Enrollment/capacity - Impact

Broadway Elementary - 274/400 - 6 Redlands Middle School - 552/650 - 3 Fruita Monument High School - 1337/1100 - 4

#### **Ute Water District - Gery Matthews**

1. Water mains are installed 2' from curb and gutter, in the oil.

Water lines have been rerouted on plans. Sanitary sewer line encasements have been added appropriately.

2. A 8" C-900 water line is sufficient for El Quevin Court unless the fire Department requires a 10" inch line.

10-in water main required by City Fire Department.

3. Water mains shall be C-900, Class 150. Installation of pipe fittings, valves and services including testing and disinfection shall be in accordance with Ute Water standard specifications and drawings.

10-In water main required by City Fire Department. Notes added to plans to include Ute Water requirements and specifications.

4. Developer is responsible for installing meter pits and yokes. Ute Water will furnish the meter pits and yokes.

Notes added to plans to include Ute Water requirements and specifications.

5. Policies and fees in effect at the time of application will apply.

Policies will be adhered to and fees will be paid.

#### **City Utility Engineer - Trent Prall**

#### General

1. Final plans have not been stamped by a Registered Professional Engineer as required.

Plans submitted for approval and acceptance will be stamped by PE.

#### Sewer - City of Grand Junction

1. Easement required for sewer line between MH4 and MH5 across lots 6, 7 & 24.

Existing sewer easement (Book 2125, Pages 792-794) has been added to Composite Plan, Water and Sewer Plans, and Subdivision Plat.

3184 response

#### Vista Del Rio - Filing 3: Response to Final Plat/Plan Comments

2. How are existing sewer to be abandoned?

Sewer lines not in way of construction will be left in place.

3. Construction notes referring to City Standard Drawings and Specifications are missing.

Note added to Water and Sewer Plan sheets

4. Sewer service line to lot 2 appears to tap centerline rather than existing sewer line.

Corrected

5. Long service line to Lot 20 should have a cleanout added.

Cleanout added.

- 6. Show on profile where water lines intersect sewer lines such as on sewer line 3 between MH3A and MH14. Also show special construction if needed.
- 7. 0.2' drop is required across manholes rather than the proposed 0.1' drop.

Item discussed by M. Schumann and T. Prall 2:22pm Oct. 20, 1995. T. Prall said he'd allow 0.1 - foot drop.

8. Add existing profile as required in SSID manual IX-35.

Exiisting ground profile added

9. Please ensure all items specified in SSID manual IX-35 are adhered to.

10.

No "Exhibit I" was included in the packet for review.

City Details for Water and Sewer are on sheet 9 of 11 "Details - Water and Sewer".

#### Water - Ute

1. Please provide bend information for waterline.

#### City Parks and Recreation Department - Shawn Cooper

1. Maintain the additional right-of-way on west side of parkway as possible trail easement Provide 15' pedestrian easement between lots 10 & 11 or 9 & 10 for public access to designated open-space. The issue of bike and pedestrian access in the area and from the subdivision needs to be discussed.

After walking the site with City of Grand Junction Engineering, Community Development, and Parks and Recreation staff, it was decided trail and pedestrian easements were impractical due to topography, etc.

2. Areas to be owned and maintained by HOA in perpetuity need to be indicated on documents and plats.

Added to documents.

3. Parks & Open Space fees - 23 @ \$225 = \$5,175.

Fees will be paid.

3184 response

# Vista Del Rio - Filing 3: Response to Final Plat/Plan Comments

### Mess County Planning Dept. - Matt Osborn

1. As a Mesa County requirement of approval for Filing 1 of Vista Del Rio, the petitioner was to design and construct a bike path along the west side of Redlands Parkway from Rio Linda Lane to Greenbelt Drive. The petitioner was waived the Development Impact Fee for Fillings 1 & 2. If the cost of the bike path exceeded the total D.I.F. for the entire project, the petitioner would be reimbursed, Approval of the second filing reiterated the need for the bike path. If the bike path is not constructed, the D.I.f. for filings 1 & 2 should be paid to the County.

Refeer to the following excert from the Project Narrative which was prepaired by the petitioner:

#### **BICYCLE PATH**

At the County Commissioner hearing for Filing One approval, the subject of applying our Development Impact Fees (DIF) towards the construction of a bicycle path was discussed. The proposed path was to parallel the west side of the Redlands Parkway from Rio Linda Lane south to Greenbelt Drive. The purpose of the path was for pedestrians and cyclists to be able to travel south to a stop light and cross the Parkway to reach the main bike path on the east side of the Parkway. Nelghbors to the west of Vista Del Rio considered the Parkway too hard and dangerous to cross on foot or bicycle without a stop light. (The traffic study w commissioned said a stop light was not warranted.)

At the end of the discussion the Commissioners requested us to start cost estimates. At this time, everyone thought a bike path would be constructed.

At the County Commissioners hearing for the Filling Two approval, we presented the cost of the bike path, as well as some design constraints and safety concerns it posed. Discussions at the meeting then centered on reasons not build the path, as the money would be more effective if spent on other improvements. The bike path was almost nixed, when Commissioner Spehar noted, "We promised a bike path to the neighbors in an open meeting last month, and it would not be fair to those people to change what they are expecting, even though it doesn't make as much sense anymore".

At the end of the meeting we were wondering what we were supposed to do about the bike path. We listened to the tape transcripts of both meetings and still we were unclear as to the direction we should take. Meanwhile, County staff were telling us "Don't forget the bike path". When we asked them what we should provide, we were not given clear direction or designs.

During the course of the project, we were required to provide additional improvements. (See section above - Required Improvements) We felt our expenditure on these items more than satisfied our DIF requirement, but County staff still make occasional references, to the bike path.

3184 response

MTD COPY

Amended Preilin
Final

Vista Del Rio - Filing 3 Response to Comments

· Send set to tate

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prepared October 26, 1995



Add Allow comments: - stationing on sheet 425 is missing from plan view



October 26, 1995

Development Staff City of Grand Junction, Colorado

Ladies and Gentlemen:

The Review Comments for the Final Plat/Plan - Vista Del Rio, Filing 3 have been received and the Response to Comments are enclosed.

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Prepared by: Maurice L. Schumann State of Colorado, Number 15698 Registered Professional Engineer

Eric C. Marquez State of Colorado, Number 19097 Engineer in Training

P.O. BOX 60010 751 HORIZON CT S U I T E 1 0 2 GRAND JUNCTION COLORADO 81506 T E L E P H O N E 970-245-7101 F A C S I M I L E 970-245-3251

#### Michael Drollinger

Amended Preliminary Plan

1. Please label Filing #4 to indicate the number of units proposed with this filing.

Clarity

To be evaluated at a future date. Please see comments in project narrative.

Final Plan Filing #3

# General Note: Street names have been changed from El Quevin Ct. and El Quolony Ct. to Casa Rio Ct. and Vista Rio Ct. respectively.

- 1. Spelling error on cover sheet should be corrected to read Loma Rio "Subdivision", not "Subdivion".
- OK Spelling error corrected

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2. Composite Plan does not clearly identify easement locations, it appears that some existing/proposed utilities are outside of the proposed easements. This office will perform a complete review once these deficiencies have been corrected.

Existing Sanitary Sewer Easement Added **Easements Annotated** 

3. Grading and Drainage Plan shows no grading information; please use the SSID manual checklist to ensure that complete information is provided for review. This office will perform a complete review once these deficiencies are adequately corrected.

See comment Jody Kliska, Item 1

4. Grammatical error on Grading and Drainage Plan: detail on lower left corner should read "Straw Bale Barriers", not "Bales Barriers".

Text revised to read "Straw Bale Barriers"

5. Please indicate on Roadway Plan and Profile whether handicapped ramps at corner of Rio Linda Lane and "El Quevin Court" are provided; are they existing?

Access ramps exist - notation has been added to plan sheet

6. A signed and sealed copy of the Drainage Report must be provided to this office; the unsigned/unsealed copy on file is unacceptable.

The certification sheet has been signed and sealed

Table has been added to sheet.

7. The Building Setbacks sheet must be modified to include a table indicating the setbacks by lot (or group of lots if setbacks are the same).

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8. The legend on the Building Setbacks table is confusing - the lines are difficult to distinguish on the drawing. What is a "Multipurpose Utility and Easement"?

"Multipurpose Utility and Easement" has been changed to "Multi-purpose and Utility Easement."

On all drawings we have and have looked at we have had no difficulty distinguishing setback or easement lines.

9. Larger setbacks from Redlands Parkway will be required for lots 1-6; we recommend a minimum of 30

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3184 response

NOL

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#### Vista Dei __ວ - Filing 3: Response to Final Plat/Pla Jomments

feet from property line.

Michoe Per agreement betweer Drollinger and Steve Colony (Oct. 20, 1995), rear setbacks for lots 1 to 5 abutting Redlands Parkway are increased from 20 to 25 feet. Rear setback for lot 6 remains at 25 feet.

10.

Please correct plat cover sheet to include City signature blocks, not County signature blocks. Also, all setback and area summary information must be transferred to Building Setbacks sheet which will be recorded with the plat. Dedication language for the open space and the opense wation easement appears to be missing; please verify and correct accordingly. E == ED

 $oldsymbol{arepsilon}$ ity signature blocks have been removed. 2. Setback Table added to Setbacks Sheet. 3. Area Summary moved to Setbacks Sheet.

A landscaping plan for the entrance feature, if proposed, must be prepared and submitted for review. Landscaping for the common open space must be guaranteed as part of the development improvements agreement for the project.

Landscape Plan sheet has been added to submittal package.

12.

11.

that all other easements are labeled appropriately to reflect the types of utilities or use anticipated. 15 should be labeled as a "drainage and utility easement; please verifyAnnotation has been added.

Please refer to SSID Manual regarding folding and securing of plans for resubmittal.

# ÚS West - Max Ward

New or additional telephone facilities necessitated by this project may result in a "contract" and up-front 1. monies required from developer, prior to ordering or placing of said facilities. For more information, please call 1-800-526-3557

Application for facilities will be made.

#### **Grand Junction Fire Department - Hank Masterson**

1. The Fire Department has no problems with this proposal.

### **City Development Engineer - Jody Kliska**

Drainage Report

Needs to include sizing of storm drain inlets and check flows in the street. 1.

A Storm Drainage Plan and Profile Sheet has been added to the construction plans.

#### Composite/Plan

OL

Storm sewer is shown outside of the easement.

3184 response

# Vista De Lo - Filing 3: Response to Final Plat/Pl Comments

Storm sewer and easement have been modified.

Grading and Stormwater Management Plan

1. Need to provide contours.

Contours to be provided as per discussion with Jody Kliska 4:12 pm Oct. 20, 1995.

Surface disturbance area is shown and labeled.

A Storm Drainage Plan and Profile is necessary - SSID IX-30. Please identify type and sizes of storm drain inlets, detail of erosion protection at pipe inlet from wetlands, outlet at outfall.

A Storm Drainage Plan and Profile Sheet has been added to the construction plans.

**Roadway Plans** 

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except?

The street name signs can be mounted above the stop signs on the same post.

Plan sheet modified to include street name signs with stop signs.

Identify the type and size of storm drain inlets on the plan view.

Plan sheet modified to identify inlets.

Plat 1

store

Jo

The dedication language needs to contain a dedication for each specific easement, right of way or open pace parcel shown on the plat. A copy of the City's Guide to Plat Dedications is attached.

See response to Michael Drollinger comment Item 10.

#### Public Service Company - G. Lewis

Existing Public Service Company easement across property will be quit claimed to match easement as shown on plat with petitioner supplying information on existing easement per prior agreement.

The existing lines are on a revocable license. We will record a new easement.

# City Property Agent - Steve Pace

1. Should reference monuments be set along the south edge of the Colorado River?

Lot corner monuments and plat dimensions suffice to locate the meander line along the Colorado River. Also, it is impractical to set monuments on the steep bank.

2. The monumentation for the outer boundary of Filing 3 should be set in concrete.

Filing 3 boundary monuments will be set in concrete as required by city and county. Once set, appropriate notations will be added to subdivision plat.

3. The City format for addressing individual easements in the dedication should be followed.

City easement dedications have been incorporated.

4. The P.O.B. tie should show a N.W. direction to match the description.

3184 response

Annotation has been changed

5. This filing is not within the City yet but will be in the near future, so we may need City signature blocks.

See response to Michael Drollinger's comment Item 10.

6. What about the area south and east of the Redlands Parkway - is it part of this Filing 3?

Area south and east of Redlands Parkway is not part of Filing 3.

#### Mesa County School District #51 - Lou Grasso

School - Enrollment/capacity - Impact

Broadway Elementary - 274/400 - 6 Redlands Middle School - 552/650 - 3 Fruita Monument High School - 1337/1100 - 4

#### Ute Water District - Gary Matthews

. Water mains are installed 2' from curb and gutter, in the oil.

Water lines have been rerouted on plans. Sanitary sewer line encasements have been added appropriately.

2. A 8" C-900 water line is sufficient for El Quevin Court unless the fire Department requires a 10" inch line.

10-in water main required by City Fire Department.

3. Water mains shall be C-900, Class 150. Installation of pipe fittings, valves and services including testing and disinfection shall be in accordance with Ute Water standard specifications and drawings.

10-in water main required by City Fire Department. Notes added to plans to include Ute Water requirements and specifications.

4. Developer is responsible for installing meter pits and yokes. Ute Water will furnish the meter pits and yokes.

Notes added to plans to include Ute Water requirements and specifications.

5. Policies and fees in effect at the time of application will apply.

Policies will be adhered to and fees will be paid.

# **City Utility Engineer - Trent Prall**

General

Final plans have not been stamped by a Registered Professional Engineer as required.

Plans submitted for approval and acceptance will be stamped by PE.

# Sewer - Øity of Grand Junction

Easement required for sewer line between MH4 and MH5 across lots 6, 7 & 24.

Existing sewer easement (Book 2125, Pages 792-794) has been added to Composite Plan, Water and Sewer Plans, and Subdivision Plat.

3184 response

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Vista De. ຸວ - Filing 3: Response to Final Plat/Pla Jomments ÌS How are existing sewer to be abandoned? Sewer lines not in way of construction will be left in place. Construction notes referring to City Standard Drawings and Specifications are missing. Note added to Water and Sewer Plan sheets. Sewer service line to lot 2 appears to tap centerline rather than existing sewer line. Corrected. 5. Long service line to Lot 20 should have a cleanout added Cleanout added. Show on profile where water lines intersect sewer lines such as on sewer line 3 between MH3A and 6 MH14. Also show special construction if needed. No response? 7. 0.2' drop is required across manholes rather than the proposed 0.1' drop. Item discussed by M. Schumann and T. Prall 2:22pm Oct. 20, 1995. T. Prall said he'd allow 0.1 rent - foot drop. 8. Add existing profile as required in SSID manual IX-35. Exiisting ground profile added Please ensure all items specified in SSID manual IX-35 are adhered to. NO response 10. No "Exhibit I" was included in the packet for review. City Details for Water and Sewer are on sheet 9 of 11 "Details - Water and Sewer".

#### Water - Ute

Please provide bend information for waterline.

City Parks and Recreation Department - Shawn Cooper

. Maintain the additional right-of-way on west side of parkway as possible trail easement Provide 15' pedestrian easement between lots 10 & 11 or 9 & 10 for public access to designated open-space. The issue of bike and pedestrian access in the area and from the subdivision needs to be discussed.

After walking the site with City of Grand Junction Engineering, Community Development, and Parks and Recreation staff, it was decided trail and pedestrian easements were impractical due to topography, etc.

2. Areas to be owned and maintained by HOA in perpetuity need to be indicated on documents and plats.

Added to documents.

A. Parks & Open Space fees - 23 @ \$225 = \$5,175.

Fees will be paid.

3184 response

#### Vista L. Rio - Filing 3: Response to Final Plat/Fran Comments

#### Mesa County Planning Dept. - Matt Osborn

1. As a Mesa County requirement of approval for Filing 1 of Vista Del Rio, the petitioner was to design and construct a bike path along the west side of Redlands Parkway from Rio Linda Lane to Greenbelt Drive. The petitioner was waived the Development Impact Fee for Fillings 1 & 2. If the cost of the bike path exceeded the total D.I.F. for the entire project, the petitioner would be reimbursed, Approval of the second filing reiterated the need for the bike path. If the bike path is not constructed, the D.I.f. for filings 1 & 2 should be paid to the County..

Refeet to the following excert from the Project Narrative which was prepaired by the petitioner: **BICYCLE PATH** 

At the County Commissioner hearing for Filing One approval, the subject of applying our Development Impact Fees (DIF) towards the construction of a bicycle path was discussed. The proposed path was to parallel the west side of the Redlands Parkway from Rio Linda Lane south to Greenbelt Drive. The purpose of the path was for pedestrians and cyclists to be able to travel south to a stop light and cross the Parkway to reach the main bike path on the east side of the Parkway. Neighbors to the west of Vista Del Rio considered the Parkway too hard and dangerous to cross on foot or bicycle without a stop light. (The traffic study w commissioned said a stop light was not warranted.)

At the end of the discussion the Commissioners requested us to start cost estimates. At this time, everyone thought a bike path would be constructed.

At the County Commissioners hearing for the Filling Two approval, we presented the cost of the bike path, as well as some design constraints and safety concerns it posed. Discussions at the meeting then centered on reasons not build the path, as the money would be more effective if spent on other improvements. The bike path was almost nixed, when Commissioner Spehar noted, "We promised a bike path to the neighbors in an open meeting last month, and it would not be fair to those people to change what they are expecting, even though it doesn't make as much sense anymore".

At the end of the meeting we were wondering what we were supposed to do about the bike path. We listened to the tape transcripts of both meetings and still we were unclear as to the direction we should take. Meanwhile, County staff were telling us "Don't forget the bike path". When we asked them what we should provide, we were not given clear direction or designs.

During the course of the project, we were required to provide additional improvements. (See section above - Required Improvements) We felt our expenditure on these items more than satisfied our DIF requirement, but County staff still make occasional references, to the bike path.

3184 response

# STAFF REVIEW

FILE:	#FPP- 95-182
DATE:	October 12, 1995
STAFF:	Michael Drollinger
<b>REQUEST:</b>	Amended Preliminary Plan and Final Plan & Plat Filing #3 - Vista Del Rio
LOCATION:	Rio Linda Lane & Redlands Parkway

## STAFF COMMENTS:

#### Amended Preliminary Plan

1. Please label Filing #4 to indicate the number of units proposed with this filing.

# Final Plan Filing #3

- 1. Spelling error on cover sheet should be corrected to read Loma Rio "Subdivision", not "Subdivion".
- 2. Composite Plan does not clearly identify easement locations, it appears that some existing/proposed utilities are outside of the proposed easements. This office will perform a complete review once these deficiencies have been corrected.
- 3. Grading and Drainage Plan shows no grading information; please use the SSID manual checklist to ensure that complete information is provided for review. This office will perform a complete review once these deficiencies are adequately corrected.
- 4. Grammatical error on Grading and Drainage Plan: detail on lower left corner should read "Straw Bale Barriers", not "Bales Barriers".
- 5. Please indicate on Roadway Plan and Profile whether handicapped ramps at corner of Rio Linda Lane and "El Quevin Court" are provided; are they existing?
- 6. A signed and sealed copy of the Drainage Report must be provided to this office; the unsigned/unsealed copy on file is unacceptable.
- 7. The Building Setbacks sheet must be modified to include a table indicating the setbacks by lot (or group of lots if setbacks are the same).
- 8. The legend on the Building Setbacks table is confusing the lines are difficult to distinguish on the drawing. What is a "Multipurpose Utility and Easement"?
- 9. Larger setbacks from Redlands Parkway will be required for Lots 1-6; we recommend a

FPP-95-182 Vista del Rio Page 2

minimum of 30 feet from property line.

- 10. Please correct plat cover sheet to include City signature blocks, not County signature blocks. Also, all setback and area summary information must be transferred to Building Setbacks sheet which will be recorded with the plat. Dedication language for the open space and the conservation easement appears to be missing; please verify and correct accordingly.
- 11. A landscaping plan for the entrance feature, if proposed, must be prepared and submitted for review. Landscaping for the common open space must be guaranteed as part of the development improvements agreement for the project.
- 12. The easement between Lots 13 & 15 should be labeled as a "drainage and utility easement; please verify that all other easements are labeled appropriately to reflect the types of utilities or use anticipated.
- 13. Please refer to SSID Manual regarding folding and securing of plans for resubmittal.

You are urged to contact the Community Development Department should you have any questions or if you require further clarification of any of the above items. Resolution of the above comments/issues and those of all other review agencies is required by the response to comments deadline. Submittal of incomplete/incorrect information may require that the application be pulled from the Planning Commission hearing.

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### **STAFF REVIEW**

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FILE:	#FPP-95-182
DATE:	October 30, 1995
STAFF:	Michael T. Drollinger
REQUEST:	Final Major Subdivision Plan/Plat Filing #3 VISTA DEL RIO SUBDIVISION
LOCATION:	West of Redlands Parkway/N of Rio Linda Lane (Redlands)
APPLICANT:	B & P Development Inc. 702 Golfmore Drive

Grand Junction CO 81506

# EXECUTIVE SUMMARY:

A request for amended preliminary plan approval of Vista del Rio Subdivision and final plat/plan approval for Filing #3 of Vista del Rio Subdivision located west of Redlands Parkway and south of the Colorado River. The subdivision is presently being annexed as part of the Loma Rio Annexation. Filing #3 consists of 23 new building lots and the platting of a lot which contains an existing residence. All major technical issues regarding the development have been resolved and staff recommends approval of this application.

EXISTING LAND USE: Vacant

PROPOSED LAND USE: Single Family Residential

SURROUNDING LAND USE:

NORTH:	Colorado River
SOUTH:	Single Family Residential
EAST:	Single Family Residential
WEST:	Single Family Residential

EXISTING ZONING: PUD (1.86 units per acre)

PROPOSED ZONING: PR-1.86 (Proposed City Zoning)



Vista del Rio Amended Preliminary Plan & Final Plan/Plat Filing #3 AERIAL MAP

## SURROUNDING ZONING:

NORTH:	AFT & R-2 (County)
SOUTH:	R-2 & PR-2 (County)
EAST:	PR-2
WEST:	R-2 (County)

# RELATIONSHIP TO COMPREHENSIVE PLAN:

No comprehensive plan exists for this area.

#### STAFF ANALYSIS:

The Vista del Rio Subdivision is located on the west side of the Redlands Parkway and just north of the Colorado River. The project is presently being annexed by the City as part of the Loma Rio Annexation. Vista del Rio Subdivision has received County ODP approval and Filings #1 & #2 have been recorded in the County. As part this application, the petitioner is requesting an amended preliminary approval and a final plat/plan approval for Filing #3.

The number of units in each phase is summarized below:

Filing #1 (platted in County)	9 lots
Filing #2 (platted in County)	10 lots
Filing #3 (requesting final plat approval)	23 lots + platting of one existing residence
Filing #4 (future filing)	2 lots
TOTAL:	45 lots

Filing #4 is located in the east side of the Redlands Parkway with access from 23 Road. Each phase contains areas dedicated as Private Open Space, primarily along Rio Linda Lane, Redlands Parkway (including areas within Goat Wash), and along the Colorado River.

The Preliminary Plan has been amended to reflect the densities in the existing filings and the lot configuration in Filing #3. The location of proposed building lots in Filing #4 is also shown, although staff recommends that the map be modified to reflect the two (2) units proposed in the narrative for this filing.

For reference, copies of the proposed Plat, a Building Setbacks Plan and the Utility Composite Plan have been included for reference along with the attached aerial photograph.

All major technical issues with the petitioner have been resolved and revised final plans are presently being prepared by the petitioner's consultant. In their project narrative (attached to this staff report in the "materials supplied by petitioner" section), the petitioner documented a number of road improvements required by the County as part of the overall approval for the project. The petitioner is requesting that all or part of the cost of these improvements be credited to the required impact fees, in this case being the Transportation Capacity Payment (TCP). As per Zoning and Development Code Section 5-4-1H, the Public Works Director has the authority to determine which required improvements may be credited toward the TCP. Appeals of the Director's decisions may be made to the City Council. The Public Works Director has been advised of the petitioner's request.

Regarding the County's requirement for a bicycle path (actually a multi-use path) along Redlands Parkway from Rio Linda Lane to the existing traffic signal at Greenbelt Drive, City staff believes that this path would not be used by pedestrians/cyclists and others to access the multi-use path on the east side of the highway as was the intent since the distance involved is greater than the convenience to try to access the path directly across from Rio Linda Lane. In addition, while staff has not seen any formal plans for the path, there appear to be some physical constraints to construction of the path on the west side of the roadway, namely, significant cutting of slopes.

Staff recommends elimination of the multi-use path proposal as required by the County and that the following be required: connections from the existing multi-use path on the east side of the Redlands Parkway to the eastern edge of pavement of Redlands Parkway on both the north and south sides of Rio Linda Lane (aligning with the sidewalks and roadway edge). Presently there is a drainage swale which separates the existing multi-use path from the roadway. Since persons will attempt to make the crossing, this proposed improvement would expedite the crossing of the roadway and enhance user safety. Further improvements, such as striping or signing of the crossing, would in the opinion of the Public Works Department, offer little in the way of additional safety.

Construction of the proposed improvement would consist of piping of the swale, the provision of an adequate base course, and asphalt pavement (to a width of eight feet for each crossing). The developer would be eligible for a reduction in the required TCP for construction of the paths as allowed by Code.

Should the Planning Commission consider this item favorably, staff recommends approval with the following condition:

1. Elimination of the multi-use path proposal as required by the County replaced by the following: connections from the existing multi-use path on the east side of the Redlands Parkway to the eastern edge of pavement of Redlands Parkway on both the north and south sides of Rio Linda Lane (aligning with the sidewalks and roadway edge) as further described in the staff report.

2. Modification of the preliminary plan for the subdivision to indicate two proposed building lots in Filing #4.

STAFF RECOMMENDATION:

Staff recommends approval of the amended preliminary plan and the final plan for Filing #3.

# SUGGESTED PLANNING COMMISSION MOTION:

Mr. Chairman, on item #FPP-95-182, a request for amended preliminary plan and final plat/plan approval for Filing #3, I move that the plans be approved subject to staff conditions #1& #2 in the staff report dated October 30,1995.

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# STAFF REVIEW (City Council)

FILE:	FPP-95-182
DATE:	January 11, 1996
REQUEST:	Right-of-Way (ROW) Vacation (portion of Rio Linda Lane) Easement Vacation VISTA DEL RIO SUBDIVISION FILING #3
LOCATION:	Rio Linda Lane west of Redlands Parkway
STAFF:	Michael T. Drollinger
APPLICANT:	B&P Development, Inc. 702 Golfmore Drive Grand Junction, CO 81506

EXECUTIVE SUMMARY:

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A request to vacate a portion of Rio Linda Lane and an existing sanitary sewer easement as part of the development of Vista del Rio Filing #3, a 23 lot subdivision located west of the Redlands Parkway and south of the Colorado River.

STAFF ANALYSIS:

Vista del Rio Filing #3 is located west of the Redlands Parkway adjacent to Rio Linda Lane and consists of 23 building lots (see attached location maps). The property was recently annexed as part of the Loma Rio Annexation and is zoned PR-1.86. The Final Plat/Plan for Filing #3 was approved by the Planning Commission in November 1995. The platting of this filing will require a right-of-way (ROW) vacation for a small portion of Rio Linda Lane.

The easement vacation is requested to eliminate overlap and redundency which would occur with the platting of Filing #3. All sanitary lines for Filing #3 will be within either street ROW and/or easements which will be dedicated with the Filing #3 plat. The easement to be vacated presently contains a sanitary sewer line parts of which will be abandoned and relocated as part of Filing #3 construction.

# STAFF RECOMMENDATION:

Staff recommends approval of the ROW vacation of a portion of Rio Linda Lane and the easement vacation.

PLANNING COMMISSION RECOMMENDATION:

At their November 7, 1995 meeting, the Planning Commission approved the Final Plan/Plat for Vista del Rio Filing #3.

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# SKYLINE CONTRACTING, INC. 3189 MESA AVENUE GRAND JUNCTION, COLORADO 81504

Underground Utilities Excavation . Site Work

> Invoice Number 2995 Customer Number 1110

November 23, 1994

ALPINE CM, INC. 1111 South 12th Street Grand Junction, CO 81501

Billing for work performed at Vista Del Rio Phase I:

9401- PhuseI \$1,350.00 2100 Additional excavation - cut slopes Equipment

Road preparation Labor Equipment

TOTAL AMOUNT DUE

Respectfully Submitted, SKYLINE CONTRACTING, INC.

## SKYLINE CONTRACTING, INC. 3189 MESA AVENUE GRAND JUNCTION, COLORADO 81504

# Underground Utilities . Excavation . Site Work

Invoice Number 3014 Customer Number 1110

\$1,427.50

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December 19, 1994 ALPINE CM, INC. 1111 South 12th Street Grand Junction, CO 81501

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n han seren di kan sun di Kalan seren di kanan Terreta di kanan di kanan di kanan di kanan Terreta di kanan seren di kanan seren di kanan seren di kanan seren Billing for work performed at Vista Del Rio: PHASE I Excavate terraces and compact slopes per Mesa County request 11.5 hours excavator at \$65.00 \$747.50 3.0 hours labor at \$20.00 60.00 Spread and roll base in street 2.5 hours blade at \$60.00 150.00 2.0 hours roller at \$55.00 Site grading -Lot 5 110.00 6.0 hours blade at \$60.00 360.00 

TOTAL AMOUNT DUE 9401-2100

Respectfully Submitted, SKYLINE CONTRACTING, INC.



# ALPINE C.M., INC.



1111 S. 12TH ST. • GRAND JUNCTION, CO 81501 • 303/245-2505 • FAX 303/245-2591

February 27, 1995

RECEIVED FEB 2 7 1995 MESA COULTY PLANNING DEPARTMENT

Matt Osborne Mesa County Planning Department 750 Main Street Grand Junction, CO 81501

Dear Matt,

Please allow this letter to serve as confirmation of our telephone conversation this day and ultimately as an agreement to construct a bike path as an off-site improvement in lieu of Development Impact Fees for the Vista Del Rio Subdivision.

More specifically, B & P Development Co. shall design, engineer and construct a bike path from the south side of Rio Linda to the north side of South Rim Drive on the west side of the Redlands Parkway, paralleling the Parkway. The design and engineering shall follow the guidelines set forth by Ken Simms and Joe Bielman of Mesa County Planning & Engineering. Approval of the final design by Mesa County shall be given prior to work commencing.

Any costs exceeding the amount of the Development Impact Fees (between \$11,000 and \$12,000, pending the final number of developed lots in Phase III and IV), shall be paid by Mesa County by means not known to B & P Development Co. at this time.

It is further understood that the design and construction of the bike path and as witnessed by this letter of agreement will in no way prevent the developer from recording the first two filings for the Vista Del Rio Subdivision.

If you have any questions or require additional information, please contact me at 245-2505.

Sincerely,

V. Kevin Nourse Representative (P.O.A.)



1111 S. 12TH ST. • GRAND JUNCTION, CO 81501 • 303/245-2505 • FAX 303/245-2591

October 3, 1995

Mr. Michael Drollinger, Sr. Planner City of Grand Junction Community Development Department 250 N. 5th Street Grand Junction, CO 81501

RE: Vista Del Rio Subdivision, Filing Three

Dear Michael,

We would like to propose new building setbacks in Filing Three that are slightly different from Filings One and Two. The previously platted and constructed Filings One and Two had uniform 25' front and rear and 15' side yard setbacks. The attached plan shows our proposed new setbacks.

With the new plan, we have reduced the setbacks on most lots to 20' rear and 10' side yards. We have also increased the average lot size in Filing Three from the two previous. Given the constraints of topography and lot configurations, we feel the new setbacks will allow owners more options to fit their homes on their lots. Since the lots are larger and a home can be positioned close to one side, it will afford a larger open space on the other side, thus keeping home spacings on an average larger.

The setback document could be recorded with the plat, thus providing clear direction to planning staff and homeowners as to what the setbacks are on corner or other tricky shaped lots.

We request that the new setback proposal be considered with our Filing Three submittal, which is just beginning its way through the approval process.

We also inadvertently didn't change the names of the new streets in Filing Three from the rough working names they are shown with. We plan to correct this on the final plat to be recorded.

If you need additional information or clarification please feel free to call. Thank you for your consideration in these matters.

Steven P. Colony, Architect Project Manager

Attachment



# TCI Cablevision of Western Colorado, Inc.

November 1, 1995

Vista Del Rio Fil. 2 Alpine C.M., Inc. % Community Development Department 250 North 5th Street Grand Junction, CO 81501

Ref. No. TCICON.089

Dear Sir or Madame;

We are in receipt of the plat map for your new subdivision, Vista Del Rio Fil. 2. We will be working with the other utilities to provide service to this subdivision in a timely manner.

I would like to take this opportunity to bring to your attention a few details that will help both of us provide the services you wish available to the new home purchasers. These items are as follows:

- 1. We require the developers to provide, at no charge to TCI Cablevision, an open trench for cable service where underground service is needed. This trench may be the same one used by other utilities.
- 2. We require developers to provide, at no charge to TCI Cablevision, fill-in of the trench once cable has been installed in the trench.
- 3. We require developers to provide, at no charge to TCI Cablevision, a 4" PVC conduit at all utility road crossings where cable TV will be installed. This 4" conduit will be for the sole use of cable TV.
- 4. Should your subdivision contain cul-de-sac's the driveways and property lines (pins) must be clearly marked prior to the installation of underground cable. If this is not done, any need to relocate pedestals or lines will be billed directly back to your company.
- 5. TCI Cablevision will provide service to your subdivision so long as it is within the normal cable TV service area. Any subdivision that is out of the existing cable TV area may require a construction assist charge, paid by the developer, to TCI Cablevision in order to extend the cable TV service to that subdivision.
- 6. TCI will normally not activate cable service in a new subdivision until it is approximately 30% developed. Should you wish cable TV service to be available for the first home in your subdivision it will, in most cases, be necessary to have you provide a construction assist payment to cover the necessary electronics for that subdivision.

Should you have any other questions or concerns please feel free to contact me at any time. If I am out of the office when you call please leave your name and phone number with our office and I will get back in contact with you as soon as I can.

Sincerely,

Glen Vancil, Construction Supervisor 245-8777

RECEIVED GRAND JUNCTION PLANNING DEPARTMENT	
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<del>250</del>2 Foresight Circle Grand Junction, CO 81505 (303) 245-8750



Grand Junction Community Development Department Planning • Zoning • Code Enforcement 250 North Fifth Street Grand Junction, Colorado 81501-2668 (970) 244-1430 FAX (970) 244-1599

November 27, 1995

Steven Colony Alpine C.M., Inc. 1111 S. 12th Street Grand Junction, CO 81501

**RE:** Vista del Rio Filing #3 - Final Plat

Dear Mr. Colony:

As you know Vista del Rio Filing #3 received Final Plat/Plan approval on November 7th. The purposes of this letter are (1) to identify any remaining technical items to be addressed on the plans; and (2) to list the information required prior to approval of (a) construction drawings and (b) platting.

The remaining technical issues to be addressed are listed below and have for your convenience been organized by review agency:

City Community Development

We are still reviewing the Composite Plan and the Plat (including the dedication page) and will forward comments to you shortly under separate cover. All other plans are acceptable to this office.

# City Development Engineer

All concerns have been addressed. Included for your information is a construction phase submittal chart, a construction approval and progression form, and submittal requirements for final acceptance. A pre-construction notice as detailed in Section VII-3 of SSID is required.

City Utility Engineer

The petitioner needs to identify the water line crossing and sewer encasements between MH 3A and MH 3B on the plans.

Once the necessary corrections have been made, please submit six (6) complete sets of plans for final

To: Steven Colony Re: Vista del Rio Filing #3

approval. You will be issued two sets and the City will keep four sets.

The documentation required prior to approval of the project for construction is as follows:

- 1. Six (6) sets of plans which address all applicable review agency comments and Planning Commission conditions. All sets must be stamped as required and must be rolled and secured with staples. I would suggest that you delay printing the final plans until we have finished review of the plat and Composite Plan should there be any changes that require adjustments in some or all of the drawings.
- 2. A Development Improvements Agreement (DIA) which is completed in accordance to the enclosed instructions.
- 3. An acceptable form of improvements guarantee as detailed in the DIA.

Once these steps have been completed the City Development Engineer will schedule a Preconstruction meeting and will identify additional information to be provided.

The plat approval period is one (1) year from date of Planning Commission approval. Prior to recording of the plat, the following items must be provided to and/or approved by the City:

- 1. Mylar of plat with required property owner signatures
- 2. Plat must receive Utility Coordinating Committee (UCC) approval
- 3. County Surveyor's certificate of approval must be obtained
- 4. Subdivision covenants must be provided for City review
- 5. The petitioner must supply proof of incorporation of the Homeowner's Association for the subdivision filing.
- 6. Mylar of "Building Setbacks" sheet must be provided for recording with the plat.
- 7. A Transportation Capacity Payment credit request must be provided and approved by the City prior to recording.

As a reminder, the petitioner is responsible for all recording fees associated with the project.

To: Steven Colony Re: Vista del Rio Filing #3

If you require additional information or have any questions please do not hesitate to contact me.

Sincerely yours, Michael T. Drollin Senior Planner

Encls.

cc: Eric Marquez, Nichols Associates Jody Kliska, City Development Engineer Trenton Prall, City Utility Engineer File #FPP-95-182

h:\cityfil\1995\5-1824.wpd





29 November 1995

Michael Drollinger City of Grand Junction 250 North 5th Street Grand Junction, CO 81501

Dear Michael:

In your letter to Mr. Steven Colony dated November 27, 1995 regarding Vista del Rio Filing #3 - Final Plat, you listed a comment from the City Utility Engineer and requested changes be made to the plans to address the comment. The comment read "The petitioner needs to identify the water line crossing and sewer encasements between MH 3A and MH 3B on the plans." I spoke with Mr. Prall, the City Utility Engineer about the comment. I explained the crossing is shown in the profile and the vertical clear distance is greater than six feet therefore encasement would not be required. I asked Mr. Prall if we would need to make changes to the plan and he agreed that we would not need to make changes; that the plan was acceptable.

I am enclosing a reduction of the plan for your convenience.

Respectfully,

Eric C. Marquez

Encls.

Distribution:

Steven Colony, Alpine C.M. Trenton Prall, City Utility Engineer File

P.O. BOX 60010 751 HORIZON CT S U I T E 1 0 2 GRAND JUNCTION COLORADO 81506 T E L E P H O N E 970-245-7101 F A C S I M I L E 970-245-3251

NICHOLS ASSOCIATES

# NICHOLS

751 Horizon Court, Suite 102, Grand Junction, CO 81506 Phone 970-245-7101 + FAX 970-245-3251

CIVIL ENGINEERING . SURVEYING . PHOTOGRAMMETRY

Vista del Rio, Filing 3 Proj 3184

Maurice L. Schumann Jan 9 1996

# **FAX MEMO**

To: Michael Drollinger 244-1599 FAX; Maurice L Schumann From: Subject: Vista del Rio, Filing 3

#### Hello Michael,

This memo is in response to your comments on redlined blueprints and in letter dated Jan 6 1996, regarding the Vista del Rio, Filing 3 subdivision. It is our interpretation that you wish to eliminate Grand Junction street right-of-way overlap with existing Grand Junction sanitary sewer easement. Instead of vacating that part of the easement within the right-of-way(yellow highlighted), it is proposed that the entire easement (book 2125, page792) be entirely vacated immediately prior to recording of the subdivision plat. And on the plat, the easement as needed to satisfy requirements will be annotated and dimensioned accordingly. This will result in achieving two important objectives:

1. The plat will show explicity the easement as required. Confusion will be eliminated for

- anyone who may have to deal with the sewer easement in the future.
- 2. Work, time, and cost associated with making changes is reduced.

This is our proposed course of action. If there are concerns about handling the situation in this manner, please consult with John Shaver (244-1506) and then with owner's representative Steve Colony (245-2505).

Respectfully, umann

Maurice L Softumann

RECEIVED GRAND JUNCTION PLANNING DEPARTMENT	 , ;
JAN 0 9 RECT	

copy: Steve Colony Alpine CM



10 January 1996

Steve Colony Alpine C.M., Inc. 1111 South 12th Street Grand Junction, CO 81501

Steve,

In response to your phone call January 10th, regarding the sight analysis and design of the cut-bank on the Redlands Parkway, I have reviewed our billing records in order to estimate the amount invoiced for engineering services directly associated with the cut-bank. Though our office does not track cost by task, I was able to review logbooks and timecard memo fields to reach a reliable estimated cost of \$3,190. The tasks in preparing the cut-bank design included; a field survey, reducing survey data, numerous meetings and phone calls with the county engineer, drafting, engineers calculations, and construction staking.

I am enclosing 11x17 drawings of the plans accepted by the county engineer. If you have any questions please call.

Respectfully,

Eric C. Marquez

Encl.

P.O. BOX 60010 751 HORIZON CT S U I T E 1 0 2 GRAND JUNCTION COLORADO 81506 T E L E P H O N E 970-245-7101 F A C S I M I L E 970-245-3251



1111 S. 12TH ST. • GRAND JUNCTION, CO 81501 • 303/245-2505 • FAX 303/245-2591

January 12, 1996

Ms. Jodi Kliska, P.E., Development Engineer Public Works Department City of Grand Junction 250 N. Th Street Grand Junction, CO 81501

RE: Vista del Rio Subdivision

Dear Ms. Kliska:

As requested, we have completed a more thorough review of our records to determine the costs associated with cutting back of the west road bank along the Redlands Parkway from Rio Linda Lane to our south property line. The work was required by Mesa County during the review process for Filings One and Two. The bank cut study and construction was intended to increase the site distances from Rio Linda Lane up the Parkway.

The costs breakdown as follows:	
Engineering/Survey	\$3,190.00
See attached letter and road cut plans from Nichols Associates	
Project Management	\$500.00
Alpine C.M., Inc.	
Earthwork	\$2,777.50
See attached invoices from Skyline Contracting	
Bank Stabilization/Erosion Control	<u>\$979.50</u>
15% of attached invoice from Land Escapes	
Subtotal	\$7,447.00
Alpine C.M., Inc. Overhead & Profit @ 16%	1,192.00
Total	\$8,639.00

Per our previous discussions, we would like the costs of our work to be credited towards the City of Grand Junction fees for the project. Please advise us how this can be handled.

If you have any questions, please feel free to call.

Steven P. Colony, Architect Project Manager



# MESA COUNTY SURVEYOR'S OFFICE

(970) 244-1821

544 Rood Avenue P.O. Box 20000 Grand Junction, Colorado 81502-5026

March 15, 1996

Terry Nichols, President Nichols & Associates, Inc. 751 Horizon Ct.-Suite 137 Grand Junction, CO 81506

Re: Vista del Rio, Filing 3 Subdivision Final Plat Regulations

RECEIVED GRAND JUNCTION PLANNING DEPARTMENT MAR 1 1396

Dear Terry:

Yesterday, March 14, 1996, I met with the following representatives for the City of Grand Junction: Tim Woodmansee (Property Agent), Dan Wilson (City Attorney), Jim Shanks (Public Works Director) and Kathy Portner (Planner). We discussed the review issues pertaining to Vista del Rio Filing 3, and what should be done. Their decisions were as follows:

- 7.3.2.B Orientation--VARIANCE APPROVED
- 7.3.2.S Bearing and Distance--VARIANCE APPROVED
- 7.3.2.T Monumentation--VARIANCE DENIED
- 7.3.2.U(3)a Paved Surface--VARIANCE DENIED

Furthermore; all other review issues shall be complied with, except 7.3.2.J Addresses, and except 7.3.2.D Title Block (wherein it reads "In no case shall the name of a firm, association, or company be outstanding or larger than the name of the subdivision").

The City representatives authorized me to notify you of these decisions, which are final.

Thank you for your patience during this time consuming process. If there is anything this office can do to be of service, or if you have any additional questions, please call.

Sincerely,

head

Udell S. Williams, PLS Mesa County Surveyor

cc: Kathy Hall, Doralyn Genova, John Crouch, Tim Woodmansee, Dan Wilson, Jim Shanks, Kathy Portner
June 30, 1999

Steve Colony Alpine C.M. 1111 S. 12th Street Grand Junction, CO 81501

RE: Vista Del Rio Filing 3

Dear Mr. Colony:

"As Built" record drawings and required test results for the utilities and public streets in Vista Del Rio Filing 3 have been received from Nichols Associates. These documents have been reviewed and found to be acceptable. This completes the punch list items identified at the June 3, 1997 final inspection.

In light of the above, the public street and utility improvements within the public right-of-way are eligible to be accepted for future maintenance by the City of Grand Junction one year after the date of substantial completion. The date of substantial completion is June 1, 1999.

Your warranty obligation for all materials and workmanship for a period of one year beginning with the date of substantial completion will expire upon acceptance by the City.

If you are required to replace or correct any defects which are apparent during the period of the warranty, a new acceptance date and extended warranty period will be established by the City.

Thank you for your cooperation in the completion of the work on this project.

Sincerely,

Kerrie Ashbeck, P.E. City Development Engineer

> Don Newton Doug Cline Walt Hoyt

cc:

Sincerely,

Trent Prall, P.E. City Utility Engineer

Jerry-OBrien Community Development File #FPP-1995-182



City of Grand Junction, Colorado 250 North Fifth Street 81501-2668 FAX: (970)244-1599

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