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X		SITE PLAN			
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■ Industrial Pre-Treatment Sign-Off 1 1 1 8 NOTES: An asterisk in the item description column indicates that a form is supplied by the City.
■ Elcvation Drawins
Also Planning Clearance to Comm. Dev. 1 1

APRIL 1995

GENERAL PROJECT REPORT

FISHER RESUBDIVISION/SITE PLAN REVIEW

Project Description

The project proposes a resubdivision of Lots 3 and 4 of Fisher Subdivision. Lot 3 currently consists of 1.54 acres; Lot 4 consists of 2.25 acres. The subject parcels are bounded by 24-1/2 Road to the east and F Road (Patterson Road) to the southwest.

The resubdivision will reconfigure Lot 3 to 1.24 acres and Lot 4 to 2.55 acres. Proposed use of Lot 3 is a Bennett's BBQ restaurant (6,500 square feet, 300 seats). Lot 4 will contain Fisher's Liquor Barn (8,500 square feet) on the southern end of the parcel and a retail sales building (14,000 square feet) on the northern end of the parcel.

Project Compliance, Compatibility, and Impact

The parcel is currently zoned H.O. Highway Oriented. Adjacent uses include the Mall Car Wash to the south, vacant land to the west, Sticks and Stones landscaping business to the north, and vacant land to the east. All adjacent parcels are zoned H.O.

The proposal will have one access on F Road and two accesses on 24-1/2 Road. Two-way traffic will circulate throughout the site. Petitioner proposes a left-turn lane on F Road in lieu of a Traffic Impact Study.

Telephone service and a 1-1/4" MW gas line are currently available adjacent to the parcel in 24-1/2 Road. An 8" sanitary sewer line is currently available to the southern edge of the parcel in 24-1/2 Road and will be extended to service the proposal. An 8" Ute water line is located in F. Road. Fire hydrants will be located as shown on the accompanying utility plan. Underground electric power is available in F Road.

Utility providers to the parcel are as follows: Public Service Company, gas and electric; Ute Water Company, water; U.S. West, telephone; City of Grand Junction, sewer and drainage.

The restaurant will provide a two-compartment, 1000-gallon oil and grease separator to meet City pre-treatment requirements.

The Natural Conservation Service identifies soils on the parcel as Bc Sagers Silty Clay Loam. Slope on the parcel is 0.5% to the southwest. The project will have no adverse impact on site geology and no geological hazards or constraints have been identified.

Expected hours of operation for the restaurant are 10:00 a.m. to 10:00 p.m. Expected hours of operation for the liquor store are 8:00 a.m. to 10:00 p.m. Expected hours of operation for the retail sales units are 9:00 a.m. to 9:00 p.m. The liquor store will have two free-standing signs, one facing 24-1/2 Road and one facing F Road. The restaurant will have a free-standing

RP-96-200

Wayne Fisher 1041 24 Road Grand Junction, CO 81505 2945-043-00-051 Fourscored PO Box 654 Grand Junction, CO 81502

Cronk Construction 1129 24 Road Grand Junction, CO 81505 2945-043-01-002 Gertrude Fisher 667 -25 Road Grand Junction, CO 81505

City of Grand Junction Community Development Dept. 250 N 5th Street Gran d Junction, CO 81501 2945-043-01-012 C&A Enterprises c/o First South Bank PO Box 14099 Macon, GA 31203-0499

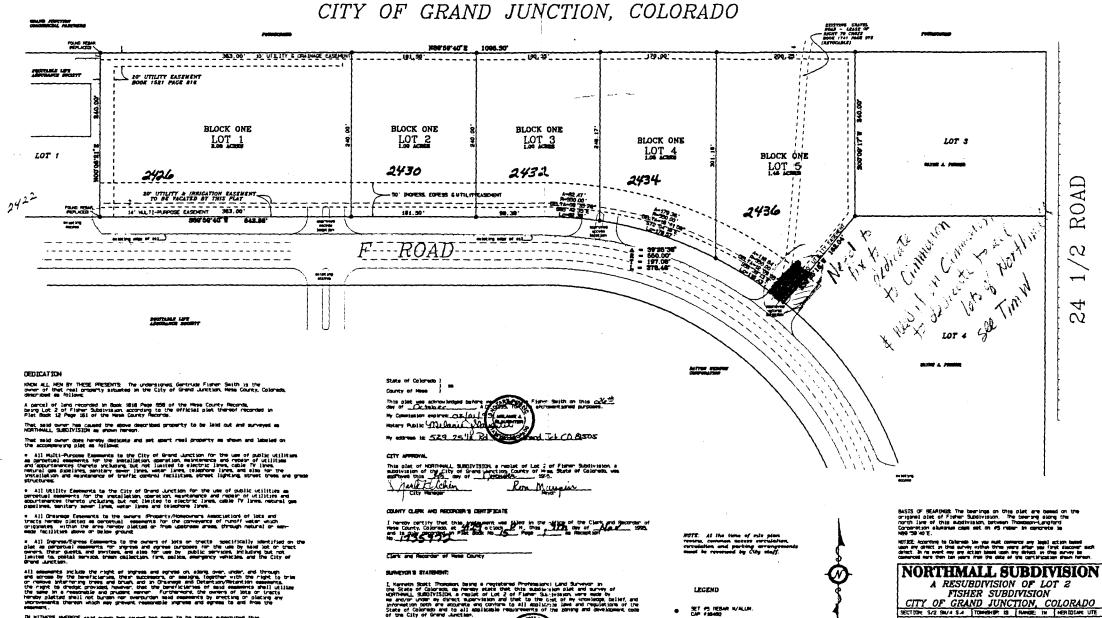
Marjean Moses 722 Hemlock Dr. Grand Junction, CO 81506 2945-043-01-018

2945-043-03-006
Dayton-Hudson Corp.
c/o Prop. Tax Dept. #93
777 Nicollet Mall
Minneapolis, MN 55402-2004
2945-044-00-058
Mustang Broadcasting Co.
715 Horizon Dr. Ste. 430
Grand Junction, CO 81506

2945-044-00-117 Denver G. Cherry Etal C/O Michael Bussey 2150 Shenandoah Dr. Grand Juction, CO 81503-1065

NORTHMALL SUBDIVISION A RESUBDIVISION OF LOT 2

A RESUBDIVISION OF LOT 2
FISHER SUBDIVISION
CITY OF GRAND JUNCTION, COLORADO



October 24, 1993

THOMPSON-LANGFORD CORPORATION

Grand Junction CO 81505 (303) 243-6067

529 25 1/2 ROAD - # 8-210

DRAINAGE PLAN

October 16, 1995 REVISED - December 19, 1995

FISHER SUBDIVISION 24-1/2 Road and F Road GRAND JUNCTION, CO 81505

Prepared For:
Wayne Fisher
Fisher's Liquor Barn
2448 -F- Road
Grand Junction, CO 81505

Prepared By: Cronk Construction Inc. 1129 -24- Road Grand Junction, CO 81505 303-245-0577

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Appen	dix E - Modified Rational Method Detention Basin Sizing Worksheet		

I. General Location and Description

The Fisher subdivision is located within the Grand Junction City limits northwest of the intersection of 24.5 Road and Patterson Road. The east boundary of the development fronts along approximately 556' of 24.5 Road. The property also fronts along approximately 240' of Patterson Road just north of Mesa Mall on the southwest boundary. Commercial property (a car wash and a landscape material supply) borders the subject property to the north and south. Vacant land borders the property to the west.

The development consists of 3.7 acres of tilled native soils. The site was formerly farmed but has been fallow for some time. The soil at the site is classified as SCS type "D" soil, being sandy clay and silty clay loam.

II. Existing Drainage Conditions

Historically drainage was directed to the southwest boundary of the property and entered the Ranchman's ditch which now runs under the parking lot of Mesa Mall as piped subsurface flow. The Ranchman's ditch drains west to 24 Road and then south under the Rio Grande Railroad tracks to the Colorado River located approximately 1000' to the south. The property has remained fallow for the past several years and all drainage has ponded on the property and evaporated or infiltrated. No existing drainage concerns are apparent.

III. Drainage Design Criteria

Drainage design criteria are taken from the *Stormwater Management Manual* (Public Works Department, City of Grand Junction, CO; June, 1994). Reference is also made to the Appendices in the *Stormwater Management Manual* for development of several constitutive design parameters. The Rational Method is used to develop Peak runoff estimate (cfs) for both pre- and post-development conditions. Peak runoff is developed for the 2 year and 100 year

precipitation events for the Mesa County urbanized area. The SCS Type II-A hydrograph (HEC-1, Corps of Engineers - U.S. Army) is used to develop the *time of critical storm duration*, T_d , for detention basin storage sizing. Orifices are used to control detention basin outflow for the 2 year design discharge while the 100 year design discharge is controlled by the size of the outflow piping diameter.

IV. Drainage Design (developed conditions)

The historic drainage outflow is located at the southwest corner of the property and will be changed by development. As shown on the Grading and Drainage Plan, post-development drainage will consist of channeling surface flows from the eastern 84% of the property to four detention basins located in the paved parking areas. Drainage from the remaining 16% of the property (consisting of the common access road with the adjoining property to the west) will be directed west along the northern barrow ditch of F Road. The western drainage is proposed to provide a favorable surface elevation transition across the common access between the subject property and the adjoining parcel to the west.

Each detention basin associated with the majority of drainage to the southeast will employ a single-stage outflow control orifice to limit the cumulative discharge from all detention areas to the design discharge rate. The City of Grand Junction Stormwater Management Manual (Public Works Department, City of Grand Junction, CO, June, 1994) allows use of two-stage outflow control with design discharge rates correlated to the 2 year and 100 year historic flows from the site. Two-stage outflow control is not utilized in detention design because existing downgradient drainage channels (12" dia. PVC) are of insufficient size to carry the larger second-stage outflows (e.g., corresponding to the 100 year historic flows for the drainage basin of concern).

The first-stage cumulative design discharge rate from the four detention areas (as limited by the down-gradient drainage channel capacity) is chosen as 1.0 cfs. Each of the four detention areas

will thus be limited to a design discharge of 0.25 cfs to facilitate a cumulative discharge rate from the four detention areas of 1.0 cfs. Orifice sizing for a design discharge rate of 0.25 cfs is developed in Appendix C. The design discharge rate is slightly more than the 2 year historic discharge rate of 0.87 cfs and substantially less than the 100 year historic discharge rate of 3.39 cfs (Appendix B). In accordance with the use of single stage outlet control, the detention basin is sized to retain the larger volumes of stormwater generated from the 100 year storm event under developed conditions (Appendix E).

Both historic and developed peak runoff flows are estimated using the *Rational Method*. Peak runoff flows for four site scenarios are calculated. The four scenarios investigated include both historic and developed peak runoff flow for precipitation event frequencies of 2 years and 100 years.

The time of concentration, T_c, worksheet for each of the 4 scenarios investigated is included for reference as Appendix A. The *Rational Method* worksheet used to calculate peak flow runoff is included for reference as Appendix B. Individual detention basin outflow design considerations (i.e., design outflow for each of the four detention areas taken as 1/4 of the cumulative design outflow) are addressed in Appendix C. The SCS Type II-A hydrograph for the area (HEC-1) is used to develop the time of critical storm duration, T_d, as shown in Appendix D. The detention basin sizing worksheet is included for reference as Appendix E.

V. Results and Conclusions

The historic peak flow runoff is estimated at 0.87 cfs (2 year event) and 3.39 cfs (100 year event). As shown in Appendix C, the single stage outlet control will limit developed peak outflow discharge from each detention area to 0.25 cfs (1.0 cfs cumulative total from the four detention areas). Under developed conditions, the 100 yr precipitation event will result in a maximum storage volume of approximately 14,575 cubic feet (Appendix E). A 12" PVC (C-900) pipe is proposed to channel storm flow from the detention areas to the existing

irrigation/stormwater drainage channel located at the southeast corner of the property. Under maximum free-flow conditions (i.e., full pipe flow at a slope of 0.4%), the maximum flow capacity of the outflow channel is 2.45 cfs. The design maximum flow capacity of the outflow piping (both existing and design) is thus in excess of the design peak discharge rate of 1.0 cfs under developed site conditions.

VI. Certification

I, Thomas A. Cronk, hereby certify this report was completed by myself or under my direct supervision and has been prepared in accordance with good engineering practices.

<u>Seal</u>

Thomas A. Cronk

Date

Thomas Resonate

December 12, 1995.

APPENDIX A

Time of Concentration, T_{c} , Worksheet

-

-

Time of Concentration, T_e , Worksheet

Project:

Fisher Subdivision

Site Condition:

Pre-development

Prepared by:

Tom A. Cronk

Date:

September 28, 1995

(The table below is an adaption of a worksheet provided in the SCS TR-55)

This table may be used in subbasin T_c calculations, or for travel time of subbasin runoff through a lower subbasin reach (T_c),

Use only channel flow for T_c calculations

STORM FREQUENCY		2 YEAR	100 YEAR
	AREA IDENTIFIER		
REACH	SEGMENT IDENTIFICATION		
	T, OR T, THROUGH BASIN REACH		
	SURFACE DESCRIPTION (TABLE E-1)	no till - no residue	no till - no residue
	"N" VALUE (TABLE E-I)	0.04 - 0.10 (assume 0.07)	0.04 - 0.10 (assume 0.07)
OVERLAND FLOW	FLOW LENGTH, L (TOTAL < 300 FT.) (ft.)	300	300
OVERLEAD FEET	LAND SLOPE, S (ft./ft.)	0.006	0.006
	To (min.) (TABLE E-2, OR FIGURE E-1)	28	17
	SURFACE DESCRIPTION (FIGURE E-3)	nearly bare/untilled	nearly bare/untilled
	FLOW LENGTH, L (ft.)	285	285
SHALLOW CONCENTRATED FLOW	FLOW SLOPE, S (ft./ft.)	0.006	0.006
7 LOW	FLOW VELOCITY, V (FIGURE E-3) (fps)	0.78	0.78
	TRAVEL TIME T, = L/(60V) (min.)	6.1	6.1
•	CROSS-SECTIONAL FLOW AREA, a (ft²)	no channel	no channel
,	WETTED PERIMETER, Pw (fi.)		
	HYDRAULIC RADIUS, r = a/Pw (ft.)		
	CHANNEL SLOPE, S (ñ./ñ.)	_	
CHANNEL FLOW	MANNINGS COEFFICIENT, 1 (APPENDIX F)		
	$V = 1.49 r^{20} S^{10}/n$ (fps)		
	ASSUMED VELOCITY (fps)		
	FLOW LENGTH, L (ft.)		
,	TRAVEL TIME $T_{dc} = L/(60V)$ (min.)		
T _c	$T_c = T_s + T_i + T_{ab} $ (min.)	34	23
T,	T _r =T _{ch} (min.)		
T,	T ₁ =0.6(T ₂) OR FROM FIGURE E-4		

NOTE - Table and all referenced tables, figures, and appendices from <u>Stormwater Management Manual, Public Works Department, City of Grand Junction, June, 1994</u>

Time of Concentration, T_c, Worksheet

Project:

Fisher Subdivision

Site Condition:

Post-development

Prepared by:

Tom A. Cronk

Date:

September 28, 1995

(The table below is an adaption of a worksheet provided in the SCS TR-55)

This table may be used in subbasin T_e calculations, or for travel time of subbasin runoff through a lower subbasin reach (T_e) .

Use only channel flow for T_e calculations

STORM FREQUENCY		2 YEAR	100 YEAR
	AREA IDENTIFIER		
REACH	SEGMENT IDENTIFICATION		
	T, OR T, THROUGH BASIN REACH		
	SURFACE DESCRIPTION (TABLE E-1)	pavement	pavement
	"N" VALUE (TABLE E-1)	0.05	0.05
OVERLAND FLOW	FLOW LENGTH, L (TOTAL < 300 FT.) (ft.)	100	100
O VERCENE I EUV	LAND SLOPE, S (ft./ft.)	0.01	0.01
· · · · · · · · · · · · · · · · · · ·	To (min.) (TABLE E-2, OR FIGURE E-1)	8	5
	SURFACE DESCRIPTION (FIGURE E-3)	paved area	paved area
	FLOW LENGTH, L (fl.)	100	100
SHALLOW CONCENTRATED FLOW	FLOW SLOPE, S (ft./ft.)	0.005	0.005
1	FLOW VELOCITY, V (FIGURE E-3) (fps)	1.4	1.4
	TRAVEL TIME T, = L/(60V) (min.)	1.2	1.2
	CROSS-SECTIONAL FLOW AREA, a (ft²)	0.0569	0.1745
	WETTED PERIMETER, Pw (ft.)	0.6509	1.047
	HYDRAULIC RADIUS, r = a/Pw (ft.)	0.0875	0.1667
	CHANNEL SLOPE, S (ñ./ñ.)	0.004	0.004
CHANNEL FLOW	MANNINGS COEFFICIENT, 11 (APPENDIX F)	0.012	0.012
	$V = 1.49r^{26}S^{1/2}/n$ (fps)	1.55	2.38
	ASSUMED VELOCITY (fps)	1.6	2.4
	FLOW LENGTH, L (ft.)	500	500
	TRAVEL TIME T _a = L/(60V) (min.)	5.2	3.5
T _c	$T_c = T_c + T_s + T_{ck} $ (min.)	14.4	9.7
т,	T _r =T _{ab} (min.)		
T _i	T,=0.6(T,) OR FROM FIGURE E-4		

NOTE - Table and all referenced tables, figures, and appendices from <u>Stormwater Management</u> <u>Manual, Public Works Department, City of Grand Junction, June, 1994</u>

APPENDIX B RATIONAL METHOD PEAK FLOW RUNOFF WORKSHEET

Rational Method Peak Flow Runoff Worksheet

Project:

Fisher Subdivision

Prepared by:

Tom A. Cronk

Date:

September 28, 1995

SITE CON	DITION: PRE-DEVELOPME	NT									
BASIN		AREA			NOFF CIENT', C						
	SURFACE TYPE	SCS GROUP	ACREAGE, A	C _{to}	C ₁₀₀						
All	bare ground	D	3.79	0.28	0.34						
<u></u>	- 		TOTAL ACREAGE, A _T	RU	GHTED NOFF CIENT, C _w	CONCENT TIME ² , T			NSITY', i L/hr.)	RUNO	EAK FF iA, (cfs)
				C ₆₂	C _{ioo}	Tona	T _{C100}	i _{n2}	i _{loo} Q _{a2}		Q _{ioo}
			3.79	0.28	0.34	34	23	0.82	2.63	0.87	3.39

- ¹ Rational Method runoff coefficients taken from Table B-1, Stormwater Management Manual, Public Works Department, City of Grand Junction, June, 1994
- ² Time of Concentration as derived in attached Appendix A worksheet
- ³ Intensity taken from Table A-1, <u>Stormwater Management Manual</u>, <u>Public Works Department</u>, <u>City of Grand Junction</u>, <u>June</u>, <u>1994</u>

Rational Method Peak Flow Runoff Worksheet

Project:

Fisher Subdivision

Prepared by:

Tom A. Cronk

Date:

September 28, 1995

SITE CON	DITION: POST-DEVELOPMEN	Т									
BASIN	A	REA			OFF CIENT', C						
	SURFACE TYPE	SCS GROUP	ACREAGE, A	C ₁₀₂	C _{ioo}						
	pavement/roof	D	3.19	0.93	0.95						
	landscape	D	0.60	0.28	0.34						
			TOTAL ACREAGE, A ₇	RUN COEFF	HTED NOFF ICIENT,	CONCENT TIME², T			STTY', i /br.)		RUNOFF ,iA _t (cfs)
				C ₀₂	C ₁₀₀	T _{cm2}	T _{C100}	i _{oz}	i ₁₀₀	Q ₁₀₂	Q _{i∞}
			3.79	0.83	0.85	14.4	9.7	1.32	3,80	4.15	12.24

- Rational Method runoff coefficients taken from Table B-1, Stormwater Management Manual,
 Public Works Department, City of Grand Junction, June, 1994
- ² Time of Concentration as derived in attached Appendix A worksheet
- ³ Intensity taken from Table A-1, <u>Stormwater Management Manual</u>, <u>Public Works Department</u>, <u>City of Grand Junction</u>, <u>June</u>, <u>1994</u>

APPENDIX C
DETENTION BASIN OUTFLOW DESIGN WORKSHEET

DETENTION BASIN OUTFLOW DESIGN WORKSHEET DISCHARGE PIPING ORIFICE CONTROL

Project:
Prepared by:

Fisher Subdivision

Date:

Tom A. Cronk September 28, 1995

	D	etention Basin	A		Detention Basin B (cumulative discharge from Basin A)								
head difference, h ¹ , (ft.)	design discharge, Q ² , (cfs)	design orifice diameter ³ (in.)	actual orifice diameter ⁴ (in.)	actual discharge, Q _a ⁵ , (cfs)	head difference, h ¹ , (ft.)	design discharge, Q ² , (cfs)	design orifice diameter ³ (in.)	actual orifice diameter⁴ (in.)	actual discharge, Q _a ⁵ , (cfs)				
1.7	0.25	2.67	2.50	0.22	2.1	0.50	3.57	3.50	0.48				

¹ Difference in inlet and outlet waterlevel elevation at maximum detention capacity (ft.)

 $Q = C_d A \sqrt{2gh}$, where,

Q = design discharge, (cfs)

 C_d = coefficient of discharge = 0.62 for sharp edge transition

A = cross-sectional area of pipe (ft²)

 $g = gravitational acceleration = 32 ft/sec^2$

h = head difference, (ft)

² Design discharge = 1/4 of cumulative design discharge, Q_h (cfs) less other discharge sources (i.e., lower stage discharge and/or sheetflows)

³ Design diameter (assuming submerged inlet and outlet, full orifice flow, negligible head loss across orifice) calculated from:

⁴ Actual orifice diameter based on construction feasibility not exceed design diameter

⁵ Actual discharge as based on actual orifice diameter, to be used in determining average discharge rate Q_r for detention basin sizing

DETENTION BASIN OUTFLOW DESIGN WORKSHEET DISCHARGE PIPING ORIFICE CONTROL

Project: Fisher Subdivision
Prepared by: Tom A. Cronk
Date: September 28, 1995

Detention	Basin C (cum	ulative dischar	ge from Basin	s A and B)	Detention Basin D									
head difference, h ¹ , (ft.)	design discharge, Q ² , (cfs)	design orifice diameter ³ (in.)	actual orifice diameter ⁴ (in.)	actual discharge, Q _a ⁵ , (cfs)	head difference, h ¹ , (ft.)	design discharge, Q ² , (cfs)	design orifice diameter ³ (in.)	actual orifice diameter⁴ (in.)	actual discharge, Q _a ⁵ , (cfs)					
1.32	0.75	4.90	5.0	0.78	1.59	0.25	2.70	2.50	0.21					

¹ Difference in inlet and outlet waterlevel elevation at maximum detention capacity (ft.)

$$Q = C_d A \sqrt{2gh}$$
, where,

Q = design discharge, (cfs)

 C_d = coefficient of discharge = 0.62 for sharp edge transition

A = cross-sectional area of pipe (ft²)

 $g = gravitational\ acceleration = 32\ ft/sec^2$

h = head difference, (ft)

² Design discharge = 1/4 of cumulative design discharge, Q_h (cfs) less other discharge sources (i.e., lower stage discharge and/or sheetflows)

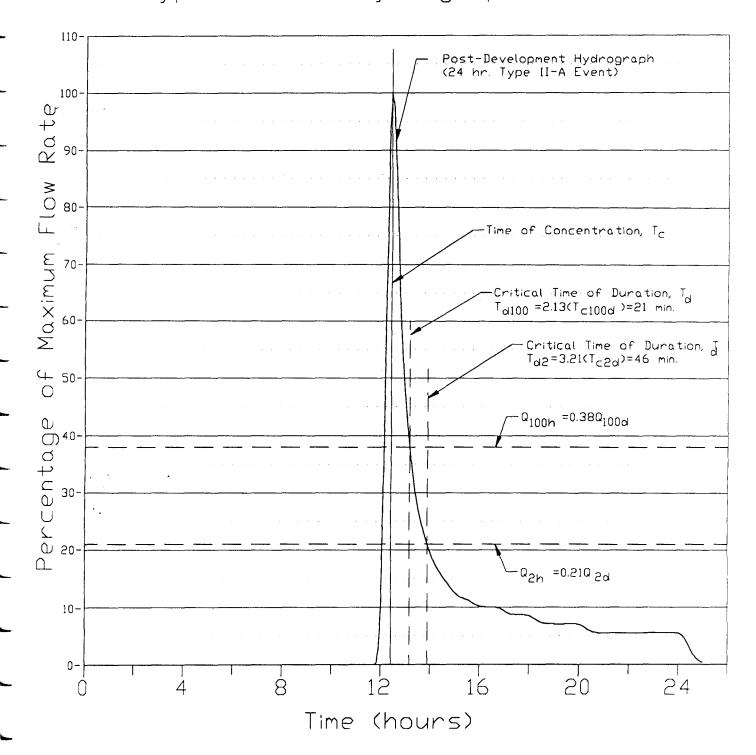
³ Design diameter (assuming submerged inlet and outlet, full orifice flow, negligible head loss across orifice) calculated from:

⁴ Actual orifice diameter based on construction feasibility not exceed design diameter

⁵ Actual discharge as based on actual orifice diameter, to be used in determining average discharge rate Q, for detention basin sizing

 $\label{eq:appendix} \textbf{APPENDIX D}$ TIME OF CRITICAL DURATION, $\textbf{T}_{\text{d}},$ WORKSHEET

Runoff Hydrograph Post-Construction (Fisher Subdivision) SCS Type II-A Unit Hydrograph (24 hr. event)



Page D-2 of D-2

APPENDIX E

MODIFIED RATIONAL METHOD DETENTION BASIN SIZING WORKSHEET

MODIFIED RATIONAL METHOD DETENTION BASIN SIZING WORKSHEET

Project:

Fisher Subdivision

Prepared by: Tom A. Cronk

Date:

September 28, 1995

				Site Hydr	ology	Detention Basin Sizing									
Basin	Site Cond		2 year event 100 year event 2 year				2 year	event	nt 100 year event						
			C ₂₄	T _{c24} (min.)	Q _{2d} (cfs)	C _{100d}	T _{e100d} (min.)	Q ₁₀₀₄ (cfs)	T ₄₂ 1 (min.)	Q ₂₂ ² (cfs)	Storage Volume, V ₂ ³ , (ft ³)	T _{d100} 1 (min.)	Q _{r100} ² (cfs)	Storage Volume, V ₁₀₀ ³ , (ft ³)	
	Pre-devel	oped	0.28	34	0.87	0.34	23	3.39							
	Post-deve	loped	0.83	14.4	4.15	0.85	9.7	12.24	46	0.75	9,560	21	0.75	14,575	
All	Development	quantity			+3.28			+8.85							
	Impact	percent			+377%			+261%		<u> </u>					

¹ Time of critical duration, T_d, from Appendix D worksheet

$$V = 60 \left[Q_d T_d - Q_r T_d - Q_r T_{cd} + \frac{KQ_r T_{cd}}{2} + \frac{Q_r^2 T_{cd}}{2Q_d} \right], \text{ where,}$$

 $K = Ratio \ of \ pre- \ and \ post-development \ T_{cd}$

² Average rate of discharge, Q_r, = 55% of actual discharge, Q_a, taken from Appendix C plus other discharge sources (i.e., lower stage discharge and/or sheetflows)

³ Storage volume required, V (ft³), calculated from:

REVIEW COMMENTS

Page 1 of

FILE #RP-95-200

TITLE HEADING:

Replat & Site Plan Review -

Cimmaron North Minor Subdivision

LOCATION:

24 1/2 & F Roads

PETITIONER:

Wayne Fisher

PETITIONER'S ADDRESS/TELEPHONE:

1041 24 Road

Grand Junction, CO 81505

242-0999 / 242-4226

PETITIONER'S REPRESENTATIVE:

Cronk Construction

STAFF REPRESENTATIVE:

Kristen Ashbeck

NOTE: THE PETITIONER IS REQUIRED TO SUBMIT FOUR (4) COPIES OF WRITTEN RESPONSE AND REVISED DRAWINGS ADDRESSING ALL REVIEW COMMENTS.

CITY ATTORNEY

Dan Wilson

11/11/95

244-1505

- 1. What happened to the S 100' of Lot 4? This part of lot needs to be shown and dealt with as part of this replat, until applicant explains/shows how it is that such 100' has already been property split off.
- 2. Need consent of the holder of the Deed of Trust, or evidence that a release has been recorded.
- 3. My packet did not contain adequate title information to evaluate. This needs to be done to make the application complete
- 4. The plat note suggests a drainage easement; applicant needs to provide a copy of the recorded instrument for review.

U.S. WEST

11/13/95

Max Ward

244-4721

Okay.

CITY UTILITY ENGINEER

<u>Trent Prall</u> SEWER - CITY 11/13/95

<u>244-1590</u>

1. Sewer extension will require an engineered plan and profile drawing stamped by a Registered Civil Engineer. City review of plans required prior to construction. An improvements agreement will be required to cover the cost of construction and inspection for the sewer line.

2. Please contact Utility Billing (244-1580) for more information regarding plant investment fees for sewer. The following information will be requested by Utility Billing: 1) hours of operation, 2) number of employees, 3) what food will be served on (paper plates or washable plates), 4) seating capacity (differentiate between lounge and dining).

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3. Grease interceptor O.K. per letter from Dan Tonello dated 10/9/95.

WATER -UTE

Please contact Gary Mathews at 242-7491 for Ute requirements for this proposal.

CITY PROPERTY AGENT

11/13/95

Steve Pace

256-4003

- 1. The description as written seems confusing since the property being replatted is referenced by Book & Page in the caption, then the metes and bounds portion should describe the entire parcel being re-platted.
- 2. I the east 10' exception for road right-of-way shown?
- 3. All easements should be addressed in the dedication using City Standards for Dedications. If easements are existing then they should be labeled existing with only new easements or rights-of-way being addressed in the dedication.
- 4. No interior lot corner monumentation is shown.
- 5. Should the outer monumentation be reset in concrete?

MESA COUNTY BUILDING DEPARTMENT

11/13/95

Bob Lee

244-1656

No comments.

CITY FIRE DEPARTMENT

11/15/95

244-1414

Hank Masterson

- 1. The minimum number of fire hydrants required for Phase One is two. Locate one at the Patterson Road entrance. The second hydrant must be located along 24 1/2 Road at the south entrance. It will be acceptable to the Fire Department to locate this hydrant along the east side of 24 1/2 Road.
- 2. For Phase Two, one additional fire hydrant is required and it must be located along 24 1/2 Road at the north entrance. Location along the east side of 24 1/2 Road is acceptable.
- 3. For both hydrants along 24 1/2 Road, the existing 12" line which ends about 100' north of Patterson must be extended north along 24 1/2 Road to serve these hydrants. Contact Ute Water for details regarding the extension of this line. The Fire Department requires this line to be a minimum of 8" in size.
- 4. Fire Department access is adequate as shown.

CITY DEVELOPMENT ENGINEER

11/16/95

Jody Kliska

244-1591

- 1. Use the City's Guide to Plat Dedications for the plat. Each easement needs to be labeled on the plat and have a corresponding dedication in the dedication language. It appears the plat lacks easements and dedications for the common ingress/egress, parking, drainage.
- 2. How does the restaurant get deliveries? It appears surrounded by parking.
- 3. In lieu of the traffic study requirement, off-site improvements to 24 1/2 Road in the form of a continuous two-way left turn lane was the option discussed. Judging from a quick look at potential traffic distribution, it seems the 24 1/2 Road driveways will attract the majority of the traffic to the site. Improvements to Patterson Road would certainly be welcomed but may not be required of this project. Determination of the need for improvements was the reason for requirements of a traffic study originally.

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- 4. Off-site improvements need to be detailed on a separate plan sheet. A traffic plan checklist was added to the most recent edition of the SSID Manual and should be used to prepare plans. This plan should include the extent of the improvements, a typical cross-section with a pavement design, any relocation of drainage facilities such as the roadside ditch, striping, lane widths, tapers.
- 5. The drainage study and plan proposes to tie into an existing irrigation/drainage structure. Please provide additional information on the existing structure, who it belongs to, and provide a detail on the plans showing how the proposed system ties into it. The plan show this structure on an adjacent property. Is it located in an easement? If not, one may need to be obtained in order to tie in and use it.

COMMUNITY DEVELOPMENT DEPARTMENT

11/16/95

Kristen Ashbeck

244-1437

See attached comments and attachments.

GRAND JUNCTION DRAINAGE DISTRICT

11/16/95

<u>Iohn L. Ballagh</u>

242-4343

The drainage ditches described in the report are not GJDD facilities. The conditions and/or capacities of the drains to which this site will discharge surface runoff are not know to the District. The Grand Valley Irrigation Company does have some authority over the "pipe under the mall".

TO DATE, COMMENTS NOT RECEIVED FROM:

City Police Department Mesa County Surveyor Grand Valley Irrigation Ute Water Public Service Company Persigo Sewer Treatment Facility RP 95-200 REPLAT & SITE PLAN REVIEW - CIMMARON NORTH (24.5 & F RDS) Community Development - Kristen Ashbeck 244-1437 11/15/95

FINAL PLAT

Common access easement with North Mall not shown
Blanket drainage easement statement not adequate
Common easement for parking not indicated
Show building setbacks on plat
See attached standard dedication language & Final Plat checklist

SITE PLAN

A separate Planning Clearance is required for each building. Once this project is approved and Community Development is ready to issue a Planning Clearance for any one of the buildings, the petitioner is advised that Planning Clearances for all of the buildings must be obtained within 6 months of the approval. A Site Plan Review (re-review) will be required for those not issued a Planning Clearance by that date.

Building setbacks are 15 feet from property line along F Road, 25 feet from property line along 24-1/2 Road, and 15 feet from side and rear property lines. Need to check setback of liquor store from 24-1/2 -- it appears to be just short of 25 feet. Also need to indicate centerlines of rights-of-way.

Dimensions of liquor store footprint on site plan do not match dimensions shown on elevations. Need to correct-this may affect landscape areas around the building.

Provide details of screening to be provided around each of the dumpster sites.

Label streets.

PARKING/CIRCULATION/LIGHTING

Narrative states that there will be two access points on 24-1/2 Road and two-way circulation on the site, yet plan indicates 3 access points, one of which is one-way?

Amount of parking is adequate as shown and as phased; however, changes may need to be made per comments below for Landscape Plan.

Lighting has several gaps that could be filled if landscaped/parking islands were added in the large vacant triangular areas and lights were placed on the islands (see red-lined plan). Also, a building light on the northwest facade of the restaurant may help fill the gap in lighting on that side of the building. Such islands would also help facilitate better traffic flow in these areas that are otherwise a "free-for-all".

LANDSCAPE PLAN

The Highway Oriented (HO) zone requires that a minimum of seventy-five percent (75%) of the required front yard setback be landscaped. This has not been met in the 25-foot front yard setback along 24-1/2.

The landscape plan must distinguish which species of trees will be placed where rather than lumping all trees under one symbol -- this is primarily to be able to review placement of coniferous trees so as not block sight distance.

Minimum planting size of coniferous trees is 6 feet rather than the 1-1/2" caliper indicated on the plan.

The intent of the Highway Oriented (HO) zone is to achieve higher quality, more aesthetically pleasing commercial and business development. In this regard, gravel is not acceptable to meet the requirement for landscaping in the right-of-way. At a minimum, it should be living groundcover or grass as an extension of the grassed areas to be proposed on site.

Provide a detail showing parking island to meet standards of section 5-5-1 F.2.c.(2) of the Zoning and Development Code regarding pavement for door swing and overhangs. Detail should also indicate that all landscaped areas shall have curbing (scale of drawing is too difficult to tell what is proposed).

GENERAL

Provide narrative information regarding percent of gross sales receipts anticipated to be from the sale of alcoholic beverages at the proposed Bennett's Restaurant and Saloon. If gross sales receipts are anticipated to exceed 25%, a Conditional Use Permit for the use is required.

Narrative and plan indicates that the liquor store will have a freestanding sign along 24-1/2 Road and that a freestanding sign for the retail outlets is not proposed at this time. Be advised that a second freestanding sign along 24-1/2 Road is not allowed by code.

For your information, total sign allowance for Lot 1 is 248 square feet, of which a maximum of 134 square feet may be on the facade(s) of the proposed building. Total sign allowance for Lot 2 is based on street and building frontages and must be distributed throughout the site (both liquor store and retail outlets). Exact allowance can be calculated once signs are proposed.

RP 95-200 REPLAT & SITE PLAN REVIEW - CIMMARON NORTH (24.5 & F RDS) Community Development - Kristen Ashbeck 244-1437 1/8/96

Per our phone conversation of 1/8/96, staff has the following comments regarding the landscape/site plan (refer to attached drawing for clarification):

- 1. Do <u>not</u> need to provide a physical triangle at the northern 24-1/2 Road entrance.
- 2. For circulation purposes, <u>do</u> need to provide the a physical triangle (no striping) at the southerly 24-1/2 Road entrance. The southern curb of the triangle should align with the northerly curb of the southern entrance.
- 3. In lieu of meeting the landscape requirement for 24-1/2 Road setback (75% of 15'), the Administrator has determined that the requirement may be varied if the perimeter trees provided are larger at planting than the minimum required planting size. Please provide 2-1/2" caliper trees along the perimter rather than 1-1/2" caliper trees. Please revise landscape plan to indicate this requirement.

