# **Table of Contents**

\_\_\_\_\_SPR-1995-035

Date\_\_\_\_\_7/12/99\_\_\_

P	S	A few items are denoted with a (*) are to be scanned for permanent record on the ISYS retrieval system. In some
r	c	instances, not all entries designated to be scanned, are present in the file. There are also documents specific to certain
e	a	files, not found on the standard list. For this reason, a checklist has been included.
s e	n n	Remaining items, (not selected for scanning), will be marked present on the checklist. This index can serve as a quick
n	e	guide for the contents of each file.
t	d	Files denoted with (**) are to be located using the ISYS Query System. Planning Clearance will need to be typed in full,
		as well as other entries such as Ordinances, Resolutions, Board of Appeals, and etc.
X	X	
		Application form
X		Receipts for fees paid for anything
X	Х	*Submittal checklist – Change of Use Review
X	Х	*General project report
		Reduced copy of final plans or drawings
		Reduction of assessor's map
		Evidence of title, deeds
		*Mailing list to adjacent property owners
		Public notice cards
		Record of certified mail
		Legal description
		Appraisal of raw land
		Reduction of any maps – final copy
		*Final reports for drainage and soils (geotechnical reports)
		Other bound or nonbound reports
x		Traffic studies
	X	Individual review comments from agencies *Consolidated review comments list
⊢^	<b>A</b>	*Petitioner's response to comments
x	X	
L.		*Planning Commission staff report and exhibits
		*City Council staff report and exhibits
	-	*Summary sheet of final conditions
		*Letters and correspondence dated after the date of final approval (pertaining to change in conditions or expiration
		date)
		DOCUMENTS SPECIFIC TO THIS DEVELOPMENT FILE:
X	X	Flood Plain Permit
	X	Certification of elevation of lowest floor
	X	Preliminary drainage report
X		Fax to Mike Davis from Kathy Portner
X		Release of Improvements Agreement & Guarantee
X		Elevation certificate
X		Letter from Bill Nebeker to Darryl Fleming – 3/22/96 X Owner's policy of title insurance
X		E-mail from Jody Klisa to Bill Nebeker – 3/22/96 X Development Improvement Agreement
X		Certificate of occupancy
X		Planning Clearance - **
X		Letter to Don Newton from Nalow – 10/27/95
. ~1		E-mail from Don Newton to Bill Nebeker – 10/27/95
X		E-mail to Don Newton from Bill Nebeker – 10/25/95
		Memo to Bill Dunn from Bill Nebeker – 9/21/95
X		Summary of revisions to plans reviewed on 3/6/95 (dated 9/18/95)

Location: Subclivision F	SIT Rennie X 24			_				Hentieren		-								_		-	_			_		-						Ú	- /~	
ITEMS		Ċ													-			DIS							-					_		<u>×</u>	1	
DESCRIPTION M-95-35	SSID REFERENCE	<ul> <li>City Community Development</li> </ul>	<ul> <li>City Dev. Eng.</li> </ul>	<ul> <li>City Utility Eng.</li> </ul>	<ul> <li>City Property Agent</li> </ul>	<ul> <li>City Parks/Recreation</li> </ul>	<ul> <li>City Fire Department</li> </ul>	<ul> <li>City Attorney</li> </ul>	O City Downtown Dev. Auth.	County Planning	<ul> <li>County Bldg, Dept.</li> </ul>	O Irrigation District	1 1	O Water District	O Sewer District	O U.S. West	O Public Service			O Corres of Environments	O Wolker Field	MAINE	004		3 - 20.			Reit	5		26			TOTAL REQ'D.
Application Fee     Submittal Checklist*     Review Agency Cover Sheet*     Planning Clearance*	VII-1 VII-3 VII-3 VII-3	1 1 1 1			1	1	1		1	1		1	1			1													+++++++++++++++++++++++++++++++++++++++					
11"x17" Reduction of Assessor's Map.     Evidence of Title     Appraisal of Raw Land     Deeds	VII-2 VII-1 VII-1	1 1 1 1			1 1 1	1		1																										
O Easements O Avigation Easement O ROW O Improvements Agreement/Guarantee	VII-2 VII-1 VII-3 VII-2	i	1	1				1 1 1											╞										+++++++++++++++++++++++++++++++++++++++					
O CDOT Access Permit O Industrial Pretreatment Sign-off General Project Report (normalized) O Elevation Drawing Site Plan	VII-3 VII-4 X-7 IX-13 IX-29	1	1					1	1			1	1				1																+++++++++++++++++++++++++++++++++++++++	
<ul> <li>Site Plan</li> <li>11*x17" Reduction of Site Plan</li> <li>O Grading and Drainage Plan</li> <li>O Storm Drainage Plan and Profile</li> <li>O Water and Sewer Plan and Profile</li> </ul>	IX-29 IX-29 IX-16 IX-30 IX-34	1	2				1	1		1			1	1		1	1	1	ŀ		-												+++++++++++++++++++++++++++++++++++++++	
Acadway Plan and Profile     Road Cross-Sections     Detail Sheet     Landscape Plan	IX-28 IX-27 IX-12 IX-20	1 1 1	2										1																				+ +	
O Geotechnical Report O Final Drainage Report O Stormwater Management Plan O Phase I and II Environmental Report	X-8 X-5,6 X-14 X-10,11	1 1 1	1 2 2								1		1							1														
O Traffic Impact Study	X-15	1																	1															
NOTES: 1) An asterisk in the item d																																		

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# Francis Constructors, Inc.

P.O. Box 1767 Grand Junction, Colorado 81502-1767 (303) 434-9093 FAX (303) 434-7583

**DATE:** FEBRUARY 15, 1995

TO: CITY OF GRAND JUNCTION COMMUNITY DEVELOPMENT DEPARTMENT 250 NORTH 5TH STREET GRAND JUNCTION, COLORADO 81501

FLP-95-35

Removi From Office

#### **PROJECT:** GRAND JUNCTION STORAGE UNITS GRAND JUNCTION, COLORADO

Project to be developed on Lot 1, Appleton Kennels Subdivision, on F and 24 Roads.

The proposed scope of work for this project will consist of extensive site work including drainage shaping, flood berms and drainage retention pond. Installation of 8" fire water line with six (6) each new fire hydrants. There will be a total of nine new buildings for storage units with one building for office/residence. The total of all buildings will be approximately 68,000 square feet. Foundations will be slab on grade with thickened edges. Structures will be steel framed with metal siding and roofing. Access roads will be paved and sloped for drainage. Site will be landscaped on F Road frontage. There will be a curb cut and installation of new ingress, egress area approximately 25' x 50' off of F Road at the edge of Lots 1 and 2.

RMC:sc

wp\prop\gjstrg

### **REVIEW COMMENTS**

Page 1 of 2

FILE #FLP-95-35

TITLE HEADING:

Flood Plain Permit / Site Plan Review - Grand Junction Storage Units

**LOCATION:** Lots 1, Appleton Kennels Subdivision, F & 24 Roads

**PETITIONER:** Colorado Self Storage

**PETITIONER'S ADDRESS/TELEPHONE:** 

2745 N Locust Avenue Rialto, CA 92377 909-875-8529

**PETITIONER'S REPRESENTATIVE:** Francis Constructors

**STAFF REPRESENTATIVE:** Tom Dixon

NOTE: THE PETITIONER IS REQUIRED TO SUBMIT FOUR (4) COPIES OF WRITTEN RESPONSE AND REVISED DRAWINGS ADDRESSING ALL REVIEW COMMENTS ON OR BEFORE 5:00 P.M., , 1995.

GRAND JUNCTION FIRE DEPARTMENT	2/23/95
Hank Masterson	244-1414
1 The existing 9" water line shown on	site plan is fed by a 2" line and dees not prov

- 1. The existing 8" water line shown on site plan is fed by a 3" line and does not provide adequate fire flows. This line must be extended east along F Road to connect to the 8" line that ends just east of Leach Creek.
- 2. The fire flow requirements for structures as shown on site plan are 3,200 gallons per minute. The available water based on fire flow tests conducted on 2/22/95 for the 8" line east of Leach Creek is 3,400 gallons per minute. The extension of this water line to the existing water line serving Appleton Kennels Subdivision will result in several thousand feet of dead end line. Petitioner must supply documentation from a licensed engineer showing that minimum require fire flows will be available.
- 3. Locations of on-site fire hydrants and the public hydrant at the entrance to the storage units is acceptable to the Fire Department.

CITY	Y DEVELOPMENT ENGINEER	2/23/95
<u>lody</u>	Kliska Transportation Capacity Payment = \$5,916 based of Please provide a Drainage Report detailing the calc A Floodplain Application and Engineering Report a	244-1591
1.	Transportation Capacity Payment = \$5,916 base	d on 68,000 s.f.
2.	Please provide a Drainage Report detailing the ca	alculations.
3.	A Floodplain Application and Engineering Report	t are required.
MES	SA COUNTY BUILDING DEPARTMENT	2/21/95
Bob	lee	244-1656

Building plans submitted to our office for plan review must be sealed by an architect or engineer. No other comments.

FILE #FLP-95-35 / REVIEW COMMENTS / page 2 of 2

CITY PARKS & RECREATION DEPARTMENT	2/22/95
Don Hobbs	244-1542

Does this action require an Open Space Fee? If it does, we need an appraisal of the site.

GRAND JUNCTION DRAINAGE DISTRICT	2/23/95
John L. Ballagh	242-4343

The ditch this site will drain into is not a facility of the Grand Junction Drainage District. The proposed pipe from the detention site will not be a facility of the Drainage District. The detention site will not be a drainage district facility.

MESA COUNTY PLANNING DEPARTMENT	2/24/95
Debbie	244-1634

Refer to the file on the following concerns: flooding, building permit hold, access and drainage. Development impact fees are due at the time of building so have not been paid.

CITY UTILITY ENGINEER Bill Cheney	2/27/95 244-1590	
No comment.		
COMMUNITY DEVELOPMENT DEPARTMENT Tom Dixon	3/6/95 244-1447	

See attached.

#### ADMINISTRATIVE REVIEW

FILE: #FLP-95-35

DATE: March 6, 1995

STAFF: Tom Dixon, AICP

REQUEST: Floodplain and Site Plan Review for mini-storage units and one office/residence

LOCATION: F Road east of Highway 6 & 50

APPLICANT: Bill Dunn, Francis Construction EXISTING LAND USE: Undeveloped

PROPOSED LAND USE: Commercial

EXISTING ZONING: C (Mesa County)

RELATIONSHIP TO COMPREHENSIVE PLAN: No such plans have been adopted in this part of the City.

**STAFF ANALYSIS:** The proposal is to create 68,000 square feet of mini-warehouse floor area to be contained in nine separate structures. The development also includes an office/residence for the manager of the facility.

The site is located in the 100-year flood plain of Leach Creek. Therefore, a floodplain review and approval is necessary prior to the issuance of a planning clearance. This proposal is also subject to the requirements of site plan review.

As proposed, the site would be developed with a perimeter flood berm which would rise some two feet above the finished grade of the site. This berm is intended to protect all structures from potential flood damage should a 100-year storm event occur. The berm will have a finished top elevation of 4,546 feet. The 100-year flood elevation on this site is right at 4,546 feet. The finished floor elevations of the buildings will range from elevations of 4,544 to 4,545 feet. The berm will therefore floodproof the buildings as required by FEMA regulations and the City's Zoning and Development Code.

**ADMINISTRATIVE DECISION:** Approval is subject to satisfaction of the following issues:

1) A Drainage Report from a registered engineer is necessary and shall be reviewed and approved by the City prior to the issuance of a planning clearance.

2) The petitioners may be required to pay a Transportation Capacity Payment (TCP), a Plant Investment Fee (PIF), and other service fees prior to the issuance of a planning clearance.

3) The petitioner shall provide a certificate from a registered engineer that the proposed berm will adequately satisfy floodproofing requirements.

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# CITY OF GRAND JUNCTION FLOODPLAIN PERMIT

APPLICANT:	Francis Constructors Inc.	Do NOT Remote
MAILING ADDRESS	PO BOX 1767	From
	Grand Junction, CO 81502	FLP-95-35
TELEPHONE:	Home () Work ( 303 )	434-9093
OWNER (If different	han applicant):Colorado Self Storage	
MAILING ADDRESS	2745 North Locust Ave.	
	Rialto, California 92376	
TELEPHONE:	Home () Work ( <sup>909</sup> ) 8	875-8529
COMMON LOCATIO	N OF PROJECT SITE: Lot 1, Appleton Kennels S	Subdivision F & 24 Roads
	(STREET ADD	DRESS)
MESA COUNTY ASSI	<b>SSOR'S TAX PARCEL NO:</b> 2945-054-05-00	01
DRIFE DESCRIPTION		orage Units
DRIEF DESCRIPTION	OF THE PROPOSED USE OF THE SITE:	
RIVER, STATION:	Leach Creek	
	<b>100 YEAR FLOOD EVENT:</b> 4546.0	
DETERMINED FROM:	( ) CORPS OF ENGINEERS, FLOOD HAZARD STUDY, N	
	( ) HUD FLOOD INSURANCE STUDY, JANUARY 1978	OVEMBER 1970
ENGINEER:	(x) 1992 FEMA Study TED V. HERMANNS/ MESA F	ENGINEERING
MAILING ADDRESS	PO BOX 1287	
	Montrose, CO 81402	
TELEPHONE:	Work ( <sup>303</sup> ) <sup>249–7771</sup>	
TO BE COMPLETED	BY STAFF:	
DATE REC'D	RECEIPT NO	
FILE NO	FEE	·
REQUIRED DOCUMENTS	·	
_		

3.	CERTIFICAT	E OF ELEVATION OF LOWEST FLOOR	FLP-95-35
Name:	Francis (	Constructors Inc.	FLF
Address	PO BOX 17	267	
	Grand Junc	ction, CO 81502	OT Remove
Date Sub	omitted:	1-12-95	From Office
Property	/ Location:	Lot 1, Appleton Kennells Subdivision	
		F & 24 Roads Grand Junction, CO	· ·
Structur	re Type:	Storage Units on concrete slabs	
		Steel framed with metal siding.	

This document is submitted to the Mesa County Flood Plain Administrator to certify that the elevation of the lowest floor of the above described structure is 4543.90This elevation was established from a survey conducted by <u>Ted Hermanns</u>, a registered land surveyor in the State of Colorado, on <u>1/12</u>, 19.95.

Submitted By: \_ (Owner) Surveyed By: 1 201 (Surveyor)

Grand Junction Storage Units Lot 1, Appleton Kennels Subdivision Grand Junction, Colorado

Preliminary Drainage Plan Report

Mesa Engineering

P.O. Box 1287

Montrose, Colorado,81401

- I. Location and Description of Property
  - A. Property Location
    - 1. The property occupies the area northwest of the intersection of F and 24 with frontage on both roads.
    - 2. SE 1/4 of Section 5, T 1S, R 1W, Ute PM
    - 3. The proposed development is adjacent to Cummińs
      Power on the west and Apppleton Kennels occupies
      Lot 2 of the subdivision. Across F-road lies the
      Ryder warehouse and Pipe Traders Education Center,
      and directly north is a storage yard for 2387
      Leland Ave.
    - 4. Parcel ID # 2945-054-05-001
  - B. Description of Property
    - 1. The area of the property is 3.72 acres.
    - 2. The ground cover consists of a dense patch of willows and sparsely located elm trees.
    - 3. The dominant soil type has been classified as a sandy clay.
    - 4. There are no existing irrigation facilities.
- II. Drainage Basins and Sub-Basins Locations and Evaluation.
  - A. Major Basin Description
    - 1. The existing major drainageway is the Colorado River.
    - 2. The 100-year floodplain is shown on the attached plot.

- 3. The major basin drainageway characteristics are consistent with those established for the Colorado River.
- There are no nearby and contributing irrigation facilities within 100 feet of the property boundary.
- B. Sub-basin Description-Leach Creek Sub-basin
  - 1. The property historically drains from north to south and due to the configuration of Pattterson Road, the drainage is directed to the southwest corner where it is routed into an existing drainage CMP. From this CMP, it is then routed into the existing drainage route south of the highway.
  - Off-site drainage flow patterns that will impact the development result from the configuration of Patterson Road and the drainage patterns of Lots
     2,3, & 4 of Appleton Kennels Subdivision. These lots also drain from north to south and thus are routed in the same manner as the lot being developed.

#### III. Drainage Design Criteria

- A. No optional criteria or deviation from the manual has resulted.
- B. Development Criteria Reference and Constraints
  - Previous drainage studies that will be influenced by this drainage design are the studies that led

to the design conclusions concerning the existing drainage plan by Western Engineering. In best case scenario, the current drainage plan should not be affected due to the on-site storage and the design assumption that the CMP will not be required to convey any larger flows than predevelopment.

- 2. The drainage impact due to development should be held to a minimum due to the type of development and materials chosen. The layout of these storage buildings provides for simple routing and the majority of the property is covered by materials that allow for optimum stormwater runoff management.
- C. Hydrological Criteria, Discussion
  - The design rainfall for the property in question has been established as 1.6 in for the 10 yr. flood and 2.2 in for 100 yr.
  - 2. The SCS Tabular Hydrograph Method has been chosen as the runoff calculation criteria.
  - 3. The detention discharge and storage calculations were a result of the sizing of a single stage
    - riser.
  - The 10 yr and 100 yr floods are the chosen design storm recurrence intervals.
  - No calculations or other criteria was used that was not presented in the manual.

- D. Hydraulic Criteria, Discussion and Method Reference
  - The storage basin is designed to hold and convey 9450 ft<sup>3</sup> for the 100 yr flood and 6400 ft<sup>3</sup> for the 10 yr flood.
  - 2. The detention outlet type will be a properly sized pipe at a calculated elevation with the design assumption that it will convey the estimated amount of flow difference between the 10 yr flood and 100 yr flood.
  - 3. The method of energy dissipation will be a rip-rapped channel at the outlet facility. At such small grades, the energy dissipation will be minimal.
  - 4. No other drainage criteria has been used that was not in the manual.
- IV. Drainage Facility Design
  - A. General Concept, Discussion
    - 1. The concept behind this design is that this facility will be able to hold the roof and street drainage with typical drainage patterns resulting from slab and road grades.
    - 2. The off-site drainage patterns have been complied with due to the fact that the resulting drainage will not be routed any differently than predevelopment.
    - For existing and proposed drainage patterns, please see sections
       IIB and IVA respectively.
    - For tables, charts, figures, or drawings presented in this report, please attached appendices.

- B. Specific Details, Discussion
  - 1. The most influential drainage problem encountered would be the elevation and current capacity of the existing drainage CMP at the southwest corner of the property. The site is relatively flat, thus resulting in the utilization of minimum flow line grades which lead into this CMP.

There really is no proposed solution other than keeping the site as close to existing grade as possible and utilizing minimum grades.

- For detention storage and outlet configuration, please see enclosed plan.
- 3. The storage facility will be accessible through the proposed development
- 4. Maintenance responsibility is implied by ownership.
- V. Conclusions
  - A. The standards presented in the Mesa County Storm Drainage Criteria Manual and referenced in Hydrologic Analysis and Design By Richard H. McCuen have been complied with.
  - B. Drainage Concept
    - The effectiveness of the drainage design results from the fact that the detention area will only discharge at a rate which reflects the difference between the major and minor floods. This implies that the facility will store a 10-yr flood and convey a 100-yr flood.
    - 2. For post development peak discharge control, please see section section VB.

C. Hydrologic Impact of the Proposed Project

The result of the proposed development will be that more of the surface drainage will be routed into existing conductors rather than entering the groundwater.

D. Impact Mitigation Plans

The proposed development impact should be minimal due to the fact that all surface drainage will be routed and stored. This may result in standing water at times, but this consequence can be lessened with effective landscaping and construction techniques.

#### VI. References

- 1. Mesa County Storm Drainage Criteria Manual
- 2. Hydrologic Analysis and Design by Richard McCuen
- 3. Western Engineering Analysis of current drainage

#### VII. Appendices

- A. Hydrologic Computation Summary
  - 1. Assume:
    - A. Uniform spatial distribution over watershed.
    - B. Rainfall excess rate constant with time.
    - C. Specific duration of rainfall excess.
  - 2. Major and Minor Storm Runoff at Design Point
    - A. The 100 yr storm runoff predevelopment is 2.54 CFS.
       Postdevelopment is 6.44 CFS.
    - B. The 10 yr storm runoff predevelopment is 1.46 CFS.
       Postdevelopment is 4.68 CFS.
  - 3. See attached flood worksheets for runoff computations.
  - 4. See attached hydrographs.
  - 5. For example calculations, see hydraulic computations summary.
- B. Hydraulic Computations Summary
  - 1. Culvert capacity using DW ADS,

18" dia.

 $q = (1.49/0.012)(1.77 \text{ ft}^2)(1.77 \text{ ft}^2/4.71 \text{ ft})^{2/3}(0.005)^{1/2}$ 

=  $8.1 \text{ ft}^3/\text{s}$  will carry 100 yr flood

- 2. Storm sewer is not utilized.
- 3. Using 0.3' drop in 25',

 $q = (1.49/0.013)(3.75 \text{ ft}^2)^{2/3}(3.75 \text{ ft}^2/25 \text{ ft})^{1/2}$ 

 $= 8.58 \text{ ft}^3/\text{s}$  OK

### 4. Storm inlets are not utilized.

5. Energy Grade Line (EGL) =  $V_o^2/2g + Z$ 

Z(ft)	V <sub>o</sub> ²/2g	EGL (ft)
4541.4	0	0
4542.9	0	1.5
4540.07	0.33	2.9

6. An open channel 2 ft wide, 1.5 ft deep will carry,

 $q = (1.49/0.022)(3 ft^2)(3 ft^2/5 ft)^{2/3}(0.005)^{1/2}$ 

 $= 10.22 \text{ ft}^3/\text{s}$  OK

- Channel drop will consist of an evenly spaced rip-rapped entrance or exit utilizing 6" to 10" dia. material set in concrete.
- 8. Erosion control will consist of evenly spaced 6" to 10" dia. material used for rip-rap.
- 9. Detention Area/Volume

10 yr flood yields 6400 ft<sup>3</sup>

@ 1.5 ft deep, use 100x85 triangular area

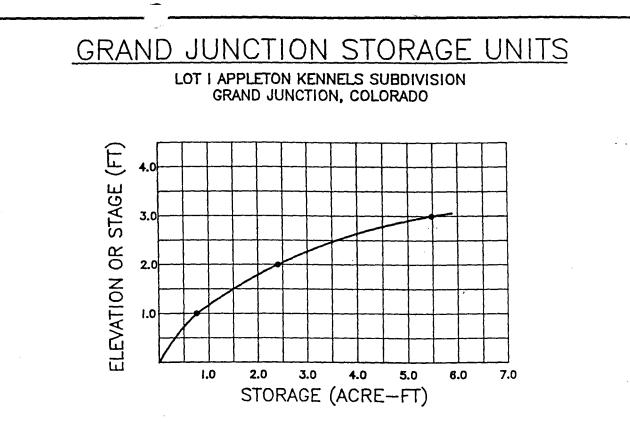
Volume will detain 10 yr flood

100 yr flood yields a difference of 4 ft<sup>3</sup>/s in flow rates

thus a 14" dia. pipe will conduct

 $q = (1.49/0.010)(1.07ft^2)(1.07 ft^2/3.66 ft)^{2/3}(0.005)^{1/2}$ 

= 4.96 ft<sup>3</sup>/s OK



STAGE-STORAGE

CONTOUR ELEVATION (FT)	AREA (ACRES)	AVERAGE AREA (ACRES)	CONTOUR INTERVAL (FT)	DEPTH h (FT)	CHANGE IN STORAGE (ACRE-FT)	STORAGE (ACRE-FT)
4542.60	0	0.38		0	0.76	0
4543.60	0.76			1		0.76
4544.60	2.53	1.65	 	2	1.65	2.41
4545.60	3.60	3.07	L	3	3.07	5.48

 $\Delta S = 1/2 (A_1 + A_{HH}) \Delta h$ 

### STAGE-STORAGE DISCHARGE

ELEVATION h (FT)	STORAGE (FT <sup>3</sup> ) X 10 <sup>5</sup>	DISCHARGE (FT <sup>3</sup> /S)
4542.90	0	0
4544.30	0.06316	0
4545.30	0.106	4.8

ENERGY GRADE LINE (EGL) EGL =  $Z + (V_0^2 / 2g)$ 

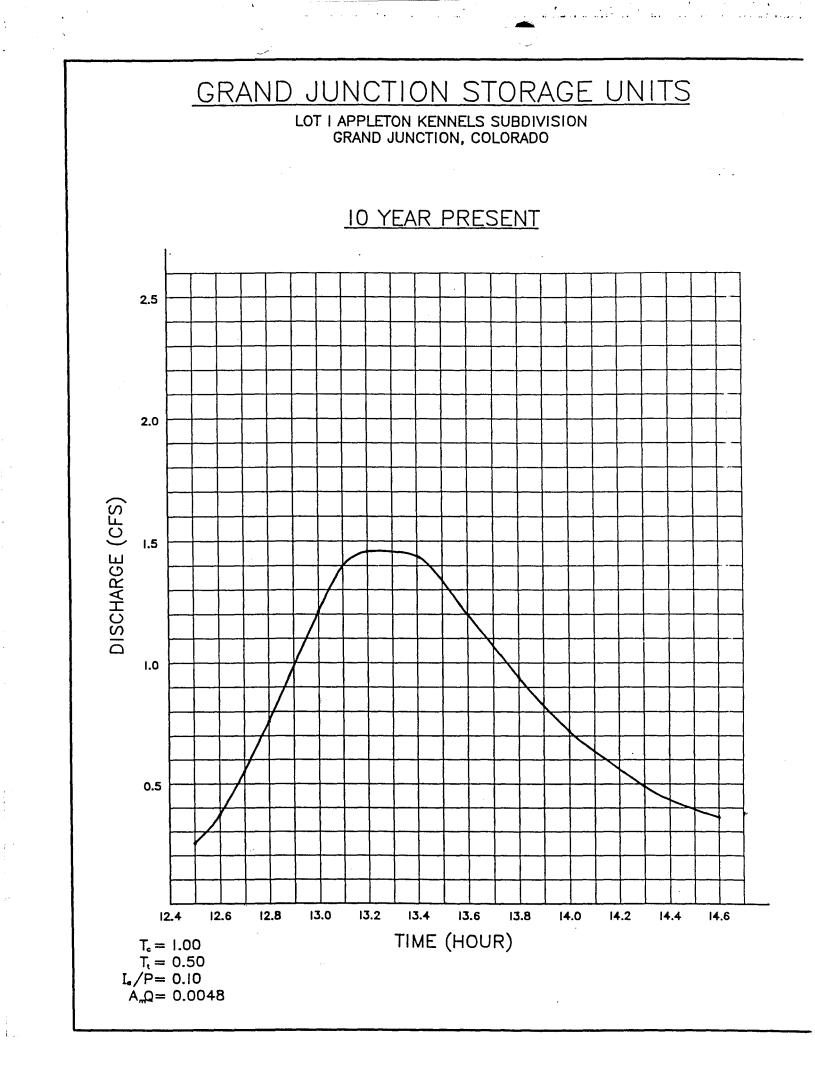
Z (FT)	V <mark>0</mark> /2g (FT)	EGL (FT)	
4542.90	0	0	]
4544.30	0	1.5	1
4545.30	0.33	2.9	1

Q = 0when h≤1.70

r

1

 $00.30 < h \le 1.37$ Q = VAwhen



Project Grand Junction Storage Units	Lot 1, Appleton Kennels Subdivision Location Grand Junction, Colorado By DT	T/DC Date 12/21/94
Circle one: Present Developed	Frequency (yr) 10 Checked	Date

)

Worksheet 5a: Basic watershed data

1

Subarea name	Drainage area	Time of concen- tration	Travel time through subarea	Downstream subarea names	Travel time summation to outlet	24-hr Rain- fall	Runoff curve number	Run- off		Initial abstrac- tion	
	A <sub>m</sub>	т <sub>с</sub>	T <sub>t</sub>		۲ <sub>t</sub>	P	СN	Q	A <sub>m</sub> Q	I	1_/1
	(m1 <sup>2</sup> )	(hr)	(hr)		(hr)	(in)		(in)	(m1 <sup>2</sup> -111)	(1n)	
1	0.0058	1.09	0.5			1.6	91	0.822	0.0048	0.198	0.124
		1									

(210-VI-TR-55, Second Ed., June 1986)

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#### Worksneet 2: Runoff curve number and runoff

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Project	Grand Junction Storage Units	₿₩	DT/DC	Cate	12/21/94
Location	Lot 1, Appleton Kennels Subdivision Grand Junction, Colorado	Chi	ecked	Date	
Circle o	ne: (Present.) Developed				

#### 1. Runoff curve number (CN)

• '

Soil name and hydrologic group	Cover description (cover type, treatment, and hydrologic condition; percent impervious;	2-2	<u>CN</u> <u>1</u> /	2-4	Area	Product of CN x atea
(appendix A)	unconnected/connected impervious area ratio)	Table	F1g.	Flg.		
c	Developing Urban Area	91			0.0058	0.5278
1/ Use only	one CN source per line.	Tota	als =			0.5278
CN (weighted)	total product	Use	CN =	[	91	
2. Runoff		Stor	m #1		Storm #2	Storm #3
Frequency	уг	10				
Rainfall, P (	24-hour) in	1.6				

Runoff, Q ..... in (Use P and CN with table 2-1, fig. 2-1, or eqs. 2-3 and 2-4.) CEE + Cr. 413 = Fq. 4-8

1.6 0.822

1.2

(210-VI-TR-55, Second Ed., June 1986)

# Worksheet 5b: Tabular hydrograph discharge summary

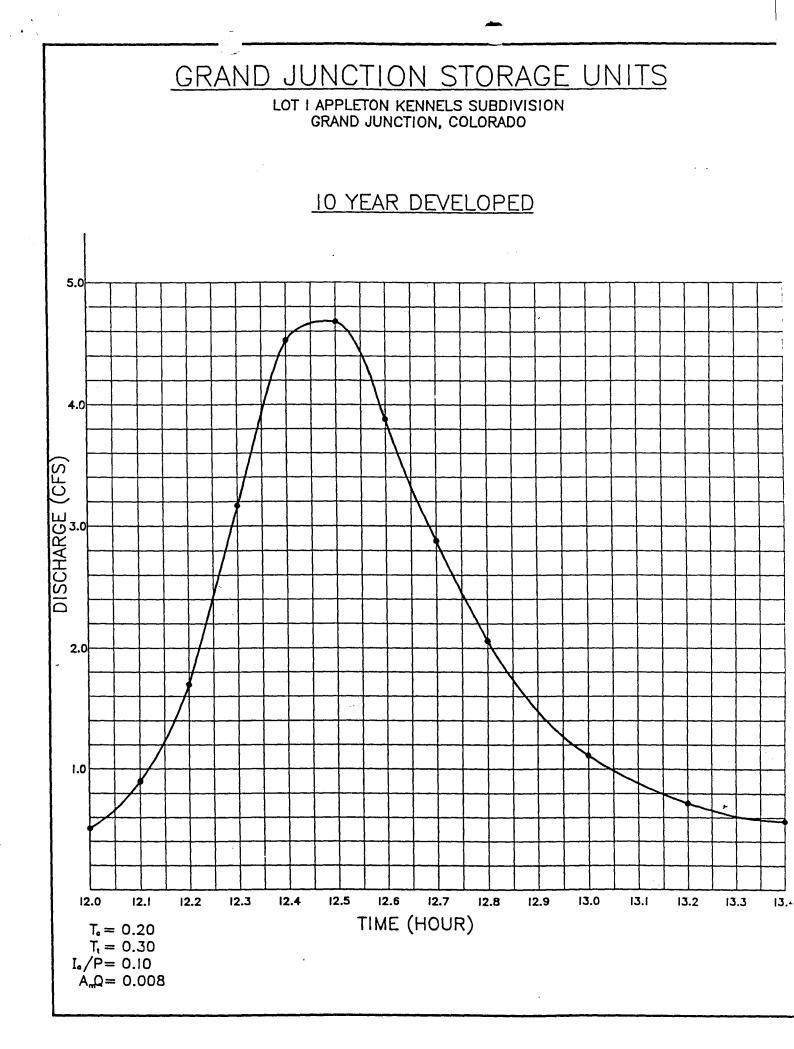
Project (	Grand Ju	unction St	orage Ur	nits		Locati	Lot on <u>Gran</u>	1, Apple d Juncti	ton Kenn on, Colo	els Subdi rado	vision	By	DT/DC		$Date \frac{12}{-}$	2/21/94
Circle o	ne: Pi	resent D	)evelope	ed					Freq	uency (	yr)	<u> </u>	hecked .		Dute _	
	Bast	c waters	hed dat	a used 1/		Selec	t and c	nter hy	drograp	h times	In hou	rs from	exuibi	<u>.</u> <u>.</u> <u>.</u>	2/	
Subarea name	Sub- area	ET <sub>L</sub> to	I <sub>a</sub> /P	۸ <sub>m</sub> Q	12.5	12.6	12.7	12.8	13.0	13.2	13.4	13.6	13.8	14.0	14.3	14.6
	T (hr)			(mi <sup>2</sup> -in)			D		es at "	elected	hydrogi	aph th	lies 37			
	1.09	0.5	0.124	0.0048	0.25	0.37	0.55	0.76	1.21	1.46	1.44	1.2	0.94	0.72	0.49	0.36
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l-	l	l	L								·	·`				
omposite	hydrog	raph at (	outlet							1						

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 $\frac{1}{2!}$  Worksheet 5a. Rounded as needed for use with exhibit 5.  $\frac{1}{2!}$  Enter rainfall distribution type used. Hydrograph discharge for selected times is A Q multiplied by tabular discharge from appropriate exhibit 5.

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Project Grand Junction Storage Units	ot 1, Appleton Kennels Subdivision rand Junction, Colorado	By DT/DC	Date 12/21/94
Circle one: Present Developed	 Frequency (yr) 10	Checked	Dute

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Worksheet 5a: Basic watershed data

Subarea name	Drainage area	Time of concen- tration	Travel time through subarea	Downstream subarea names	Travel time summation to outlet	24-hr Rain- fall	Runoff curve number	Run- off		Initial abstrac- tion		
.4	A <sub>m</sub>	т <sub>с</sub>	Т <sub>t</sub>		۲۲ <sub>t</sub>	P	CN	Q	۸ <sub>m</sub> Q	I	I_P	
	(m1 <sup>2</sup> )	(hr)	(hr)		<u>(hr)</u>	(in)		(in)	(m1 <sup>2</sup> -111)	<u>(1n)</u>		
1	0.0058	0.163	0.3			1.6	98	1.38	0.008	0.041	0.026	j
										i		
											<b></b>	
												:
I		t t t t t From works		l			t t t t t From work:		I Fro	↑ † † † m table 5-	<b>J</b>	

(210-VI-TR-55, Second Ed., June 1986)

Worksheet	2:	Runoff	curve	number	and	runoff
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Project	Grand Junction Storage Units	Bv DT/DC	Cate 12/21/94	
Location	Lot 1, Appleton Kennels Subdivision Grand Junction, Colorado	Checked	Date	
	ne: Present Developed			•

#### 1. Runoff curva number (CN)

Cover description	1 T	<u>1/ 17</u>	,	Area	Product of CN x atea
hydrologic condition; percent impervious; unconnected/connected impervious	e 2-	lg. 2-3	lg. 2-4	□acycs □mi- □%	on a dica
area racio)	<u>ц</u>	<u>c</u>	<u>.</u>		
Asphalt, Roof, Concrete	98			0.0058	0.5684
one CN source per line.	Tota	als •	•		0.5684
total product	Use	C11 ·	- [	98	
	Stor	m #1		Storm #2	Storm #3
уг	10				
(24-hour) in	1.6				
CN with table 2-1, fig. 2-1, 3 and $2-4$ .)	1.3	8			
	<pre>(cover type, trtatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio) Asphalt, Roof, Concrete one CN source per line. - total product total area ; (24-hour)</pre>	(cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)       """"""""""""""""""""""""""""""""""""	(cover type, trtatment, and hydrologic condition; percent impervious; area ratio)       (a)         Asphalt, Roof, Concrete       98         Asphalt, Roof, Concrete       98         one CN source per line.       Image: CN - Could area         total product       Image: CN - CN	(cover type, treatment, and hydrologic condition; percent impervious; area ratio)       (1)       (2)       (3)       (4)       (5)       (5)       (6) <td< td=""><td>(cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected Lapervious area ratio)</td></td<>	(cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected Lapervious area ratio)

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(210-VI-TR-55, Second Ed., June 1986)

# Worksheet 5b: Tabular hydrograph discharge summary

Project <u>(</u>	Grand Ju	unction St	orage U	nits		Locatio	Lot 1, on <u>Grand</u>	Appleto Junction	n Kennel 1, Colora	s Subdiv do	ision	Ву	DT/DC	_	Date 12	/21/94
Circle or	ne: Pr	esent	evelop	ed _					- Freq	uency (	y <b>r)</b> 10	CI	neckad _		Dite _	
	Bast	c waters	hed dat	a used $\frac{1}{}$		Selec	t and e	nd enter hydrograph times in hours from exhibit 5- 2/								
Subarea name	Sub- area	ET <sub>t</sub> to	I <sub>a</sub> /P	۸ <sub>m</sub> Q	12.0	12.1				12.5			12.8	13.0	13.2	13.4
114146	T (hr)	outlet (hr)		(m1 <sup>2</sup> -1n)			D	Ischarg		elected			ies <u>37</u>		·	
1	0.163	.3	0.026	0.008	0.51	0.90	1.70	3.17	4.53	4.68	3.88	2.88	2.06	1.11	.72	.57
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amposite	hydrog	raph at	outlet													

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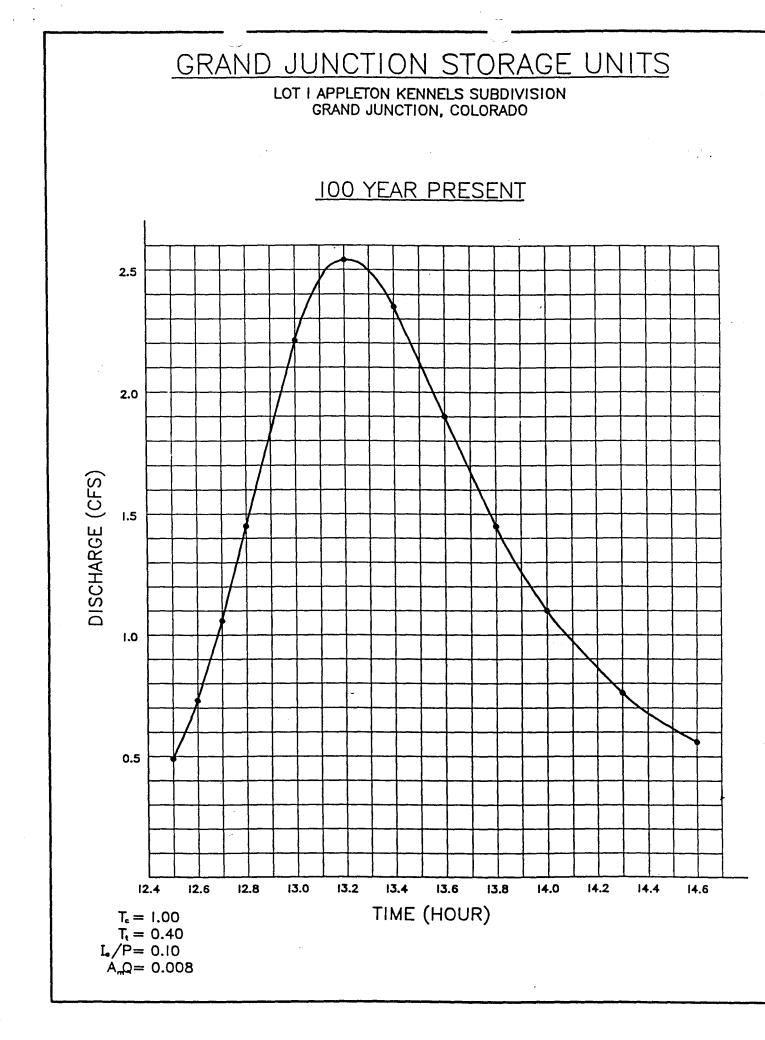
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 $\frac{1}{2}$  Worksheet 5a. Rounded as needed for use with exhibit 5. Enter rainfall distribution type used. Hydrograph discharge for selected times is A Q multiplied by tabula. discharge from appropriate exhibit 5.



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Project Grand Junction Storage Units	Location	Lot 1, Appleton Kennels Subdivision Grand Junction, Colorado	By DT/DC	Date 12/21/94
Circle one: Present Developed		Frequency (yr) 100 C	Checked	Date

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Worksheet 5a: Basic watershed data

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Subarea name	Drainage area	Time of concen- tration	Travel time through subarea	Downstream subarea names	Travel time summation to outlet	24-hr Rain- fall	Runoff curve number	Run- off		Initial alistrac- tion	
	۸ <sub>m</sub>	т <sub>с</sub>	T <sub>t</sub>		٤Tt	P	CN	Q	∧ <sub>m</sub> Q	Ia	I_/P
	(m1 <sup>2</sup> )	(hr)	(hr)		(hr)	(in)		(in)	(m1 <sup>2</sup> -111)	<u>(jn)</u>	
	0.0058	1.09	0.43			2.2	91	1.34	0.008	0.198	0.09
									· · · · · · · · · · · · · · · · · · ·		
		t + t t t From works					t t t t t From work:		Fro	<pre>↑ t t t n table 5-1</pre>	

(210-VI-TR-55, Second Ed., June 1986)

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# Worksneet 2: Runoff curve number and runoff

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Project Grand Junction Storage Units		Cate 12/21/94
Lot 1, Appleton Kennels Subdivision Location <u>Grand Junction</u> , Colorado	Checked	Date
Circle one: Present Developed		

#### 1. Runoff curve number (CN)

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Soil name and hydrologic group (appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	Table 2-2	Fig. 2-3 2	F18. 2-4	Area actus mi C %	Product of CN x alea
С	Developing Urban Area	91			0.0058	0.5278
		-				
1/ Use only	one CN source per line.	Tota	als	•		0.5278
CN (weighted)	total product	Use	CN •	- [	91	
2. Runoff		Stor	m #1		Storm #2	Storm #3
Frequency	у <del>г</del>	100	0			
Rainfall, P (	24-hour) in	2.3	2			
	CN with table 2-1, fig. 2-1,	1.34	4			

or eqs. 2-3 and 2-4.) SEE 16. 413 Eq. 4-3

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(210-VI-TR-55, Second Ed., June 1986)

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### Worksheet 5b: Tabular hydrograph discharge summary

ircle on	ie: Fr	esent)D	evelope	ed					Freq	uency (	yr) <u>100</u>	Cł	neckad _		Dute _		
	Basi	c waters	hed dat	a used 1/		Select and enter hydrograph t						times in hours from exhibit 5- 2/					
Subarea name	Sub- area	ET <sub>t</sub> to	I <sub>a</sub> /P	۸ <sub>m</sub> Q	12.5	<b>1</b>	12.7	12.8	13.0	13.2	13.4	13.6	13.8		1	14.	
	T <sub>c</sub> (hr)	outlet (hr)		(m1 <sup>2</sup> -1n)				Ischarg	es at %	elected <u>(cfs</u>		aph th.	ies <u>)/</u>				
	1.09	0.43	0.09	0.008	0.49	0.73	1.06	1.45	2.21	2.54	2.35	1.90	1.45	1.10	0.76	0.56	
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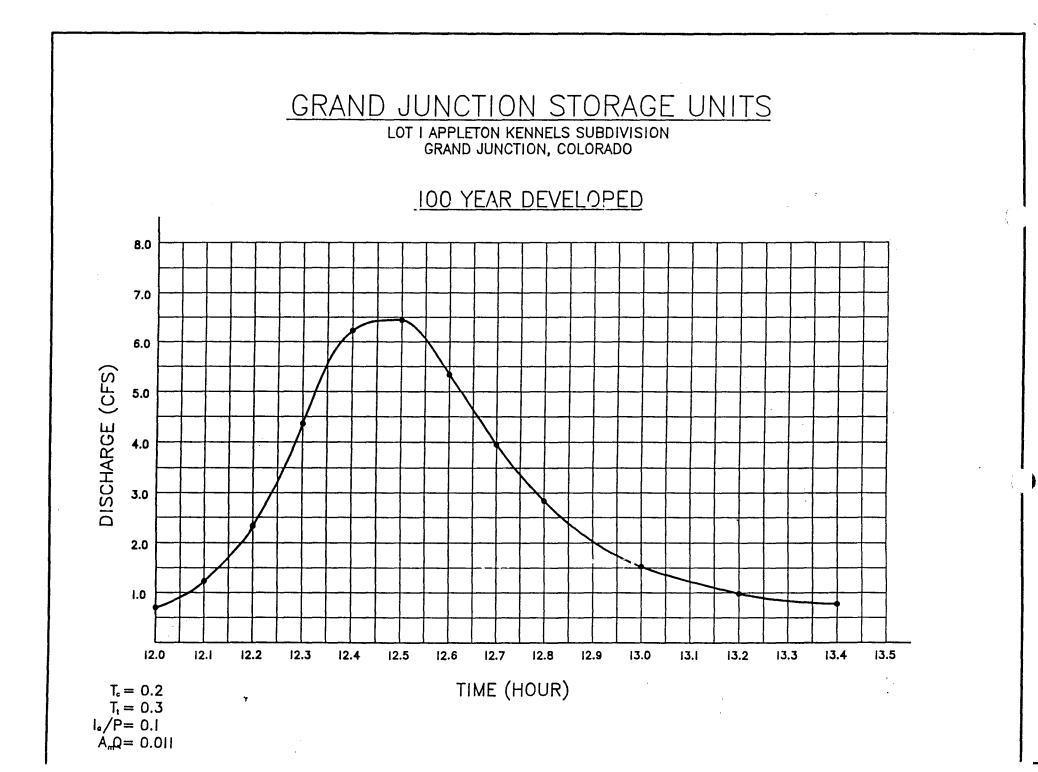
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Worksheet Sa. Rounded as needed for use with exhibit 5. Enter rainfall distribution type used. Hydrograph discharge for selected times is A Q multiplied by tabula: discharge from appropriate exhibit 5.

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Circle on	e: Presen	t Develop	ped		Fi	requency	(yr) <u>100</u>	Cł	ecked	Date	
Subarea name	Drainage area	Time of concen- tration	Travel time through subarea	Downstream subarea names	Travel time summation to outlet	24-hr Rain- fall	Runoff curve number	Run- off		Initial abstrac- tion	
		T <sub>c</sub>	Tt		٤Tt	Р	CN	Q	۸ <sub>m</sub> Q	I <sub>a</sub>	1 <sub>a</sub> /1
	^ <sub>m</sub> (mi <sup>2</sup> )	c (hr)	τ (hr)		(hr)	(in)		(in)	(m1 <sup>2</sup> -1n)	<u>(11)</u>	
	0.0058	0,163	0.26			2.2	98	1.97	0.011	0.041	0.019
	0.0058										
								· .			
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Worksheet 5a: Basic watershed data

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Project	Grand J	unction Si	torage U	nits		Locatio	Lot 1 Dn <u>Gran</u> c	Apple Junctio	eton Kenn on, Color	nels Subdi rado	ivision	Ву	DT/DC		Date 12	2/21/94
Circle o	ne: Pi	resent ([	evelop	ed _					_ Freq	luency (	yr) 100	C	heicked _		Dute	
	Bast	c waters	hed dat	ta used $\frac{1}{2}$		Selec	t and e	nter hy	drograp	h times	In hou	rs from	exhibit	<u> </u>	2/	
Subarea name	Sub- area	ET <sub>t</sub> to	I <sub>a</sub> /P	۸ <sup>۳</sup> Ó	12.0	12.1	12.2	12.3	12.4	12.5	12.6	12.7	12.8		3	13.4
	$\frac{T_{c}}{(hr)} \frac{\text{outlet}}{(hr)} \frac{(mi^{2}-in)}{(mi^{2}-in)}$									elected <u>(of</u> e	<u>;)</u>					
	0.163	0.26	0.019	0.011	0.70	1.23	2.33	4.36	6.23	6.44	5.34	3.96	2.84	1.53	0.99	0.78
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Composite	hydrog	raph at	outlet				[									

# Worksheet 5b: Tabular hydrograph discharge summary

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1/2/1 Worksheet 5a. Rounded as needed for use with exhibit 5. Enter rainfall distribution type used. Hydrograph discharge for selected times is A Q multiplied by tabular discharge from appropriate exhibit 5.

# Worksheet 2: Runoff curve number and runoff

Project Grand Junction Storage Units	BV DT/DC	Cate 12/21/94	
Lot 1, Appleton Kennels Subdivision Location Grand Junction, Colorado	Checked	Date	
Circle one: Present Developed			

#### 1. Runoff curva number (CN)

Soil name and hydrologic group (appendix A)	Cover description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	Table 2-2	Fig. 2-3 2	F18. 2-4	Area acțes mi C %	Product of CN x atea
	Asphalt, Roof, Concrete	98			0.0058	0.5684
				T		
1/ Use only	one CN source per line.	Tota	als •	4		0.5684
CN (weighted)	total product;	Use	C11 •	• [	98	

2. Runoff

Frequency	ут
Rainfall, P (24-hour)	(n
Runoff, Q (Use P and CN with table 2-1, fig. 2-1, or eqs. 2-3 and 2-4.) $C_{EE}$ 10 413 Eq. 4-8	in

Second S

Storm #1	Storm #2	Storm #3
100		
2.2		
1.97		

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# Worksheet 9-3 Single-stage and two-stage riser design

#### WATERSHED CHARACTERISTICS

#### OUTLET FACILITY CHARACTERISTICS

Var.	Units	Before	After	Comments
A	acres	3.72	3.72	
CN	-	91	98	from Table 7-6
tc	hr	1.09	0.163	
Р	in.		1.6	low stage/high stage
I <sub>a</sub> /P				low stage
I <sub>a</sub> /P			1	high stage

п	-	0.013	
L	ft	100	
٥	ft	1.0	Initial estimate
Kp		0.031	
С.		0.58	
E <sub>0</sub>	ft	1.0	
Ec	ft	· 0	

Step	Var.	Units	Low Stage	High Stage	Comments
1	Q <sub>b</sub> Q <sub>a</sub>	in. in.		0.82	$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \qquad S = \frac{1000}{CN} - 10$
2	q <sub>ub</sub> q <sub>ua</sub> q <sub>pb</sub>	ft <sup>3</sup> /sec/mi <sup>2</sup> /in. ft <sup>3</sup> /sec/mi <sup>2</sup> /in. ft <sup>3</sup> /sec ft <sup>3</sup> /sec		1.46	See Fig. 7-10 $q_p = q_u AQ$ with $A [=] mi^2$
3	q <sub>pa</sub> R <sub>q</sub>	-		0.36	$R_q = q_{pb}/q_{pa}$
4	R <sub>s</sub>	-		0.34	From Fig. 9-6
5	V <sub>s</sub>	in.		0.47	$V_{\rm s} = Q_{\rm a} R_{\rm s}$
		acre-ft		0.145	$V_{\rm s} = V_{\rm s} A / 12$ with A [=] acres
6	V <sub>d</sub>	acre-ft	0.76	-	From elevation-storage curve
7	V <sub>t</sub>	acre-ft		0.91	$V_t = V_d + V_s$
8	E	ft		1.15	From elevation-storage curve
9	D	ft		0.70	$D = C_{*} q_{\rho b 2}^{0.5} (E_{1} - E_{c})^{-0.25}$
10	Wo	ft		-	Try 0.75D
	A	ft <sup>2</sup>		-	$A_0 = 0.2283 q_{pb1} / \sqrt{E_1 - E_0}$
	H <sub>0</sub>	ft		-	$H_0 = A_0 / W_0$
	902	ft <sup>3</sup> /sec		-	$q_{02} = 4.82A_0(E_2 - E_1)^{0.5}$
11	L <sub>W1</sub> 9 <sub>02</sub>	ft ft <sup>3</sup> /sec	-		$L_{W1} = q_{pb1} / [3.1(E_1 - E_0)^{1.5}]$ $q_{02} = 3.1 L_{W1} (E_2 - E_0)^{1.5}$
12	Lwa	ft	-		$L_{W2} = (q_{pb2} - q_{02})/[3.1(E_2 - E_1)^{1.5}]$
13	E,	ft		-	$E_i = E_c - 0.5D$

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#### Worksheet 9-3 Single-stage and two-stage riser design

#### WATERSHED CHARACTERISTICS

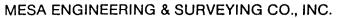
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#### OUTLET FACILITY CHARACTERISTICS

Var.	Units	Before	After	Comments
A	acres	3.72	3.72	
CN	-	91	98	from Table 7-6
tc	hr	1.09	0.163	
Ρ	in.		2.2	low stage/high stage
I <sub>a</sub> /P				low stage
I <sub>a</sub> /P				high stage

n	-	0.013	
L	ft	100	
D	ft	1	Initial estimate
Kp		0.31 ·	
С.		0.58	
E <sub>0</sub>	ft	2.2	
Ec	ft	00.20	

Step	Var.	Units	Low Stage	High Stage	Comments
1	Q <sub>b</sub> Q <sub>a</sub>	in. in.		1.34 1.97	$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \qquad S = \frac{1000}{CN} - 10$
2	q <sub>ub</sub> q <sub>ua</sub> q <sub>pb</sub> q <sub>pa</sub>	ft <sup>3</sup> /sec/mi <sup>2</sup> /in. ft <sup>3</sup> /sec/mi <sup>2</sup> /in. ft <sup>3</sup> /sec ft <sup>3</sup> /sec		2.54	See Fig. 7-10 $q_p = q_u AQ$ with A [=] mi <sup>2</sup>
3	Rq			0.39	$R_q = q_{pb}/q_{pa}$
4	R <sub>s</sub>			0.32	From Fig. 9-6
5	V <sub>s</sub>	in. acre-ft		0.63	$V_s = Q_a R_s$ $V_s = V_s A/12$ with A [=] acres
6	Vd	acre-ft	1.95	-	From elevation-storage curve
7	V <sub>t</sub>	acre-ft	2.145		$V_t = V_d + V_s$
8	E	ft		1.96	From elevation-storage curve
9	٥	ft		0.77	$D = C_* q_{\rho b 2}^{0.5} (E_1 - E_c)^{-0.25}$
10	$W_0$ $A_0$ $H_0$ $q_{02}$	ft ft <sup>2</sup> ft ft <sup>3</sup> /sec			Try 0.75D $A_0 = 0.2283q_{pb1}/\sqrt{E_1 - E_0}$ $H_0 = A_0/W_0$ $q_{02} = 4.82A_0(E_2 - E_1)^{0.5}$
11	L <sub>W1</sub> 902	ft ft <sup>3</sup> /sec	- '		$L_{W1} = q_{pb1} / [3.1(E_1 - E_0)^{1.5}]$ $q_{02} = 3.1 L_{W1} (E_2 - E_0)^{1.5}$
12	Lw2	ft	-		$L_{W2} = (q_{pb2} - q_{02})/[3.1(E_2 - E_1)^{1.5}]$
13	Ei	ft		-	$E_i = E_c - 0.5D$



P.O. Box 1287 — 330 South 5th Street Montrose, Colorado 81401 303-249-7771 FAX 303-249-7773

January 12, 1995

Francis Constructors, Inc. P.O. Box 1767 Grand Junction, CO 81502

"I hereby certify that this report (plan) for the preliminary drainage design of \_\_\_\_\_\_ was prepared by me, (or under my direct supervision)."

mann Registered Professional Engineer,

State of Colorado, Number <u>5720</u>





# Deep Creek Inc.

LANDSCAPING & IRRIGATION

DESIGN • CONSTRUCTION • MAINTENANCE

#### Tierra Corporation Re; Storage Unit Landscaping Grand Junction, CO

#### Proposal #9577 11/10/95

Quan.	Size	Price	Description	Cost
			Planting Per Landscape Plan	
17	1 1/2"	140.00	Russian Olive	2,380.00
5	2 "	230.00	Cottonwoods	1,150.00
45	#5	22.00	Yucca	990.00
51	#5	24.00	Potentilla	1,224.00
				5,744.00
			Planting Substitutes:	
			Pinion Pine 5' - 6' \$170.00/ea	
			Austrian Pine 5' - 6' \$180.00/ea	
			Juniper Evergreen #5 \$28.0 <b>4</b> ea	
			Steel Edging, Bark Mulch, Cobble Mulch	
			and Landscape Boulders	2,151.00
			Native Seed and Mulch	1,080.00
			Drip Irrigation System	595.00
	Total Amou	nt		\$9,570.00

All grades to be 2/10ths of finish grade. If the backhoe, dozer and/or dump truck is required for additional work, it will be charged at a rate of \$50.00 per hour.

OWNER/OWNER'S REP. CONTRACTOR'S REP Darol Stumpf AM IN Print Name 9S Signature Signature Date

P.O. BOX 40268

#### GRAND JUNCTION, CO 81504

(970) 244-8768

