		Table of Contents
Fil	le	SPR-1995-070
D -		P/10/00
Da	ite	8/18/99
r r	S c	A few items are denoted with an asterisk (*), which means they are to be scanned for permanent record on the ISVS netriced system. In some instances, not all antries designated to be seened are present in the file. These
e	a	is realized to be scanned, are present in the files not found on the standard list. For this reason, a checklist has been
S P	n' n	included.
n	e	Remaining items, (not selected for scanning), will be marked present on the checklist. This index can serve as a
t	d	quick guide for the contents of each file.
		Files denoted with (**) are to be located using the ISYS Query System. Planning Clearance will need to be typed
v	v	in full, as well as other entries such as Ordinances, Resolutions, Board of Appeals, and etc.
Λ	Λ	*Summary Sheet – Table of Contents
		Receipts for fees paid for anything
v	v	Accepts for rees paid for anyuning
A X	A X	*Submittal checklist - Change of Use Review
-		Reduced conv of final plans or drawings
		Reduction of assessor's map
		Evidence of title, deeds
		*Mailing list
		Public notice cards
		Record of certified mail
		Legal description
		Appraisal of raw land
×	v	Reduction of any maps – final copy
÷	^	"Final reports for drainage and sons (geotechnical reports)
		Other bound or nonbound reports
		Traffic studies
X	v	Individual review comments from agencies
	X	*Consolidated review comments list
^	Λ	*Petitioner's response to comments
		*Planning Commission staff report and exhibits
<u> </u>		*City Council staff report and exhibits
		*Summary sheet of final conditions
		*Letters and correspondence dated after the date of final approval (pertaining to change in conditions or
		expiration date)
		DOCUMENTS SPECIFIC TO THIS DEVELOPMENT FILE:
		Warrants Dood **
$\frac{\Lambda}{X}$		Letter from Michael Drollinger to Steve McCallum – 5/10/95
X		Policy of Title Insurance from Ticor Title Insurance
X		Notes to file Warehouse Storage
	x	Walchouse Storage Wells Commercial Development Site Plan - approved
X	X	Drainage Plan
	X	Site Plan, Landscape, Utilities - approved Drainage and Grading Map
X		Drainage Construction Details
X		Handwritten note marked AP – 4/25/95
Η Χ	X	DEV. IMP AGREEMENT - **
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5PR-95-70

GENERAL PROJECT REPORT

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Project Location: 554 25 Road Grand Junction, CO 81505

Project Name: Wells Enterprises Commercial Project

Date of Report: April 3, 1995

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Project is located at 554 25 Road in the City of Grand Junction. The site contains one acre and is to be used for retail and commercial rental spaces of approximately 2,000 square feet each.

Public benefit shall be additional space for service related businesses.

The project will not require re-zones or conditional use permits.

All surrounding properties will be compatible as they are now zoned for and involved in similar use.

Site will access from 25 Road. Traffic patterns will remain the same.

Utilities are presently in 25 Road. Fire hydrant locations are tentatively acceptable with the GJFD. However, any changes requested will be complied with.

No unusual demands on existing utilities or sewage are anticipated. The effects on Public facilities should be limited to items covered by the TCP.

There will be no adverse effects on site geology nor will there be any geological hazards created.

Hours of operation will be in compliance will historically accepted days and times.

A sign permit will be requested if required.

Construction will be in one phase from April 1, 1995 through June 30, 1995.



- Trash ,۴ Fenced yards UNIT-8 3 additional Parting Spaces 50'-4" ACCEPTED Datt 3-11-95 ANY CHANGE OF SETBACKS MUST BE APPROVED BY THE CITY PLANNING DEPT. IT IS THE APPLICANTS RESPONSIBILITY TO FROPERLY LOCATE AND IDENTIFY EASEMENTS AND PROPERTY LINES. 25'

 $\frac{1}{16}$ = 1

Wells COMMercial DeU, 554 25 Rd.

ATTACHMENT TO SITE PLAN - WELLS COMMERCIAL

1. FIRE HYDRANT SHALL BE LOCATED SOUTH TO WITHIN 10 FEET OF SITE DRIVEWAY AS PER FIRE DEPARTMENT

Signature of Petitioner/Representative

plall

DRAINAGE PLAN April 4, 1995

WELLS COMMERCIAL DEVELOPMENT 554 25 ROAD GRAND JUNCTION, CO 81505

Prepared For: TPI 552 25 Road #D Grand Junction, CO 81505

Prepared By: Cronk Construction Inc. 1129 -24- Road Grand Junction, CO 81505 303-245-0577

Table of Contents

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			Page
	I.	General Location and Description	1
	II.	Existing Drainage Conditions	1
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	IV.	Drainage Design (developed conditions)	2
	v.	Results and Conclusions	3
	VI.	Certification	3
	Appen	dix A - Time of Concentration, T _e , Worksheets	
	Appen	dix B - Rational Method Peak Flow Runoff Worksheets	
	Appen	lix C - Retention Basin Outflow Design Worksheets	
	Appen	dix D - Time of Critical Duration, T_d , Worksheet	
	Appen	lix E - Modified Rational Method Retention Basin Sizing Worksheet	

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Appendix F - Culvert Sizing Worksheet

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I. General Location and Description

The Wells Commercial Development is located approximately 450 ft north of Hwy 6&50 on the east side of 25 Road in Grand Junction, CO. The development fronts along approximately 90' of the east side of 25 Road. The southern boundary of the property lies approximately 400' north of Hwy 6&50. Other commercially developed lots lie to the north and south of the subject property and Ute Water has an open land storage area to the east of the property.

The development is on 1.3 acres of uncultivated native soils. The site is currently bare ground. The soil at the site is classified as SCS type "D" soil, being clay and silty clay. No soil evaluation test pits were excavated at the site; however, the storm sewer located on the west side of 25 Road is open in the bottom of a manhole and exhibits no evidence of a high seasonal water table or standing groundwater to a depth of 12' below ground surface.

II. Existing Drainage Conditions

There is an irrigation delivery ditch along the east side of 25 road which carries water to the south and also serves to conduct excess runoff. There is no irrigation on the property. A twenty four inch storm sewer (Grand Junction Drainage District) lines along the west side of 25 Road and drains south to the Colorado River. Historic drainage from the site is directed to the southwest corner of the parcel and hence south in the irrigation ditch which drains into Blue Heron Lake south of the City Market Warehouse. Historically, the site has not discharged to the storm sewer. No existing drainage concerns are apparent.

III. Drainage Design Criteria

Drainage design criteria are taken from the *Stormwater Management Manual* (Public Works Department, City of Grand Junction, CO; June, 1994) for development of several constitutive design parameters. The Rational Method is used to develop Peak runoff estimate (cfs) for both pre- and post-development conditions. Peak runoff is developed for both the 2 year and 100 year precipitation events for the Mesa City urbanized area. The SCS Type II-A hydrograph (HEC-1, Corps of Engineers - U.S. Army) is used

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to develop the *time of critical storm duration*, T_d , for retention basin storage sizing. Two-stage retention basin outflow control is sized using an orifice for first stage discharge and a weir for second stage discharge.

IV. Drainage Design (developed conditions)

Post-development drainage will include construction of a retention basin located in the parking area south of the building. The retention basin will run from west to east across the building parking access area and will contain a maximum volume of 3,100 cubic feet. The basin will discharge through a two stage outlet control structure into a 12" reinforced concrete pipe culvert under 25 Rd discharging into the 24 inch storm sewer. The irrigation ditch that has provided historic drainage for the site will be piped across the parcel from north to south. Although the developed drainage scenario does not include discharge to the irrigation ditch, a curb cut at the southwest corner of the property will direct any unforeseen retention basin overflow into the irrigation ditch (open to the south of the property) for discharge to Blue Heron Lake.

Both historic and developed peak runoff flows are estimated using the *Rational Method*. Peak runoff flows for four site scenarios are calculated. The four scenarios investigated include both historic and developed peak runoff flow for precipitation event frequencies of 2 years and 100 years.

The time of concentration, T_{e} , worksheets for each of the 4 scenarios investigated are included for reference as Appendix A. The *Rational Method* worksheets used to calculate peak flow runoff for the four scenarios investigated are included for reference as Appendix B. The two-stage retention basin outflow design considerations are addressed in Appendix C. The SCS Type II-A hydrograph for the area (HEC-1) is used to develop the time of critical storm duration, T_d , as shown in Appendix D. The retention basin sizing worksheet is included for reference as Appendix E. Appendix F address culvert sizing considerations.

V. **Results and Conclusions**

The historic peak flow runoff is estimated at 0.32 cfs (2 year event) and 0.99 cfs (100 year event). Postdevelopment peak flow runoff is estimated at 1.03 cfs (2 year event) and 3.34 cfs (100 year event). Development will result in an increased discharge of 0.71 cfs (221%) for the 2 year event and 2.34 cfs (236%) for the 100 year event. If historic drainage flows are to remain unaffected by development, retention basin volumes of 2,891 cubic feet and 6,588 cubic feet are required for the 2 year and 100 year design storms respectively. Because of: 1) the small size of the area, 2) the nearness of the property to the primary drainage outfall (i.e., the Colorado River), and 3) the adequacy of the storm sewer to carry developed runoff flows; it is proposed that the site be granted an exemption from peak discharge control. The proposed developed drainage design for the property will incorporate a partial retention basin with a two stage outflow control structure. The first stage outflow orifice is sized to discharge at the 2 year developed peak discharge rate of 1.0 cfs. Discharge from higher intensity storms (e.g., 10, 25, and 100 year events) will also be held to the 2 year developed rate (1.0 cfs) with excess runoff being ponded in the retention basin. Upon filling the retention basin (maximum capacity of 3,100 cubic feet), the second stage outflow weir will increase design runoff to 3.34 fps (the 100 year developed peak discharge rate). The drainage design as presented effectively limits peak runoff flows from all but the 100 year event to 1.0 cfs (equal to both the 100 year historic and 2 year developed rates). For the 100 year event under developed conditions, the retention basin will retain approximately one-half of the increased development runoff with the remaining unretained volume being discharged at the developed 100 year peak runoff rate of 3.34 cfs. It is felt additional retention will have a detrimental impact on the major drainage course peak discharge and capacity because of the close proximity of the project to the Colorado River.

VI. Certification

I, Thomas A. Cronk, hereby certify this report was completed by myself or under my direct supervision and has been prepared in accordance with good engineering practices.

<u>Seal</u>



Thomas A. Cro <u>Thoms J.</u> Con Date <u>April 10,1995</u>.

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 $\label{eq:APPENDIX A} \label{eq:APPENDIX A}$ Time of Concentration, T_c, Worksheet

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Time of Concentration, T_c, Worksheet

Project: Site Condition: Prepared by: Date:

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Wells Commercial Development Pre-development Tom A. Cronk April 4, 1995

(The table below is an adaption of a worksheet provided in the SCS TR-55) This table may be used in subbasin T_c calculations, or for travel time of subbasin runoff through a lower subbasin reach (T_r) , Use only channel flow for T, calculations

STORM FREQUENCY		2 YEAR	100 YEAR
	AREA IDENTIFIER	no stream	no stream
REACH	SEGMENT IDENTIFICATION		
	T, OR T, THROUGH BASIN REACH		
	SURFACE DESCRIPTION (TABLE E-1)	poor grass on bare surface	poor grass on bare surface
	"N" VALUE (TABLE E-1)	0.3	0.3
	FLOW LENGTH, L (TOTAL < 300 FT.) (ft.)	150	150
OVERLAND FLOW	LAND SLOPE, S (ft./ft.)	.012	.012
	To (min.) (TABLE E-2, OR FIGURE E-1)	35	24
	SURFACE DESCRIPTION (FIGURE E-3)	nearly bare and untilled	nearly bare and untilled
	FLOW LENGTH, L (fl.)	150	150
SHALLOW CONCENTRATED	FLOW SLOPE, S (ft./ft.)	.012	.012
FLOW	FLOW VELOCITY, V (FIGURE E-3) (fps)	1.6	1.6
	TRAVEL TIME T, = $L/(60V)$ (min.)	1.56	1.56
	CROSS-SECTIONAL FLOW AREA, a (ft ²)	1.5	1.5
	WEITED PERIMETER, Pw (ft.)	3.24	3.24
·	HYDRAULIC RADIUS, $r = a/Pw$ (fl.)	0.46	0.46
	CHANNEL SLOPE, S (ft./ft.)	0.01	0.01
CHANNEL FLOW	MANNINGS COEFFICIENT, n (APPENDIX F)	0.025	0.025
	$V = 1.49r^{2/3}S^{1/2}/n$ (fps)	3.54	3.54
	ASSUMED VELOCITY (fps)	3.5	3.5
	FLOW LENGTH, L (fl.)	400	400
TRAVEL TIME $T_{ab} = L/(60V)$ (min.)		1.9	1.9
T.	$T_{e} = T_{o} + T_{s} + T_{ab} (\min.)$	38	27
Т,	$T_r = T_{ak}$ (min.)	1.9	1.9
Т,	T ₁ =0.6(T ₆) OR FROM FIGURE E-4	23	16

NOTE - Table and all referenced tables, figures, and appendices from Stormwater Management Manual, Public Works Department, City of Grand Junction, June, 1994

Page A-2 of A-3

Time of Concentration, T_e, Worksheet

Project: Site Condition: Prepared by: Date:

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Wells Commercial Development Post-development Tom A. Cronk April 4, 1995

(The table below is an adaption of a worksheet provided in the SCS TR-55) This table may be used in subbasin T_c calculations, or for travel time of subbasin runoff through a lower subbasin reach (T_c), Use only channel flow for T_c calculations

STORM FREQUENCY	n Mara (Pa rana	2 YEAR	100 YEAR
	AREA IDENTIFIER	no stream	no stream
REACH	SEGMENT IDENTIFICATION		
	T, OR T, THROUGH BASIN REACH		
	SURFACE DESCRIPTION (TABLE E-1)	unpacked gravel	unpacked gravel
	"N" VALUE (TABLE E-1)	.15	.15
	FLOW LENGTH, L (TOTAL < 300 FT.) (fl.)	150	150
OVERLAND FLOW	LAND SLOPE, S (ft./ft.)	0.01	0.01
	To (min.) (TABLE E-2, OR FIGURE E-1)	24	15
	SURFACE DESCRIPTION (FIGURE E-3)	paved area/roof	paved area/roof
	FLOW LENGTH, L (ft.)	150	150
SHALLOW CONCENTRATED	FLOW SLOPE, S (ft./ft.)	0.01	0.01
FLOW	FLOW VELOCITY, V (FIGURE E-3) (fps)	2	2
	TRAVEL TIME T, $\approx L/(60V)$ (min.)	1.25	1.25
	CROSS-SECTIONAL FLOW AREA, a (ft ²)	37.5	37.5
	WETTED PERIMETER, Pw (ft.)	51.0	51.0
-	HYDRAULIC RADIUS, $r = a/Pw$ (ft.)	.74	.74
	CHANNEL SLOPE, S (ft./ft.)	.005	.005
CHANNEL FLOW	MANNINGS COEFFICIENT, 1 (APPENDIX F)	.016	.016
	$V = 1.49r^{20}S^{1/2}/n$ (fps)	5.4	5.4
	ASSUMED VELOCITY (fps)	5.5	5.5
	FLOW LENGTH, L (fl.)	400	400
	TRAVEL TIME $T_{a} = L/(60V)$ (min.)	1.2	1.2
Τ _ε	$T_{c} = T_{o} + T_{i} + T_{di} (min.)$	26.5	17.5
T,	$T_r = T_{a}$ (min.)	1.2	1.2
T,	T ₁ =0.6(T ₂) OR FROM FIGURE E-4	16	11

NOTE - Table and all referenced tables, figures, and appendices from Stormwater Management Manual, Public Works Department, City of Grand Junction, June, 1994

Page A-3 of A-3

APPENDIX B RATIONAL METHOD PEAK FLOW RUNOFF FLOW WORKSHEET

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Rational Method Peak Flow Runoff Worksheet

Project: Prepared by: Date:

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Wells Commercial Development TOM A. CRONK April 4, 1995

SITE CON	DITION: PRE-DEVELOPMENT										
BASIN			RUI COEFFI	NOFF CIENT ¹ , C							
	SURFACE TYPE	SCS GROUP	ACREAGE, A	C _{c2}	C ₁₀₀						
All	bare ground	D	1.3 -	.32	.38						
			TOTAL ACREAGE, A _T	WEIC RUN COEFFIC	HTED NOFF CIENT, C _w	CONCEN TIME ² ,	TRATION T _c (min.)	INTEN (in	/SITY [*] , i ./br.)	PEAK Q=C _v	RUNOFF "iA ₇ (cfs)
				C _{tt}	C ₁₀₀	Tcm	T _{cico}	i _{o2}	i ₁₀₀	Q ₀₂	Q _{im}
			1.3	.32	.38	38	27	.78	2.00	.32	.99

¹ - Rational Method runoff coefficients taken from Table B-1, <u>Stormwater Management Manual</u>, <u>Public Works Department</u>, City of Grand Junction, June, 1994

² - Time of Concentration as derived in attached Appendix A worksheet

³- Intensity taken from Table A-1, <u>Stormwater Management Manual, Public Works</u> <u>Department, City of Grand Junction, June, 1994</u>

Page B-2 of B-3

Rational Method Peak Flow Runoff Worksheet

Project: Prepared by: Date:

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Wells Commercial Development TOM A. CRONK April 4, 1995

SITE CON	DITION: POST-DEVELOPMEN	т									
BASIN		AREA		RUN COEFFIC							
	SURFACE TYPE	SCS GROUP	ACREAGE, A	C ₀₂	C ₁₀₀						
	pavement/roof	D	1.06 .	.93	.95						
	gravel - unpacked	D	.21	.48	.50						
	Landscape	D	.031	.29	.35			-			
			TOTAL ACREAGE, A _t	WEIG RUN COEFFIC	HTED NOFF CIENT, C _w	CONCEN TIME ² ,	TRATION T _c (min.)	INTEN (in.	ISITY ⁹ , i ./hr.)	PEAK Q=C _u	RUNOFF _v iA _T (cfs)
				C ₀₂	C ₁₀₀	Tana	T _{c1∞}	i ₀₂	i,00	Q ₀₂	Q ₁₀₀
			1.30	.84	.86	26.5	17.5	.94	2.99	1.03	3.34

¹ - *Rational Method* runoff coefficients taken from Table B-1, <u>Stormwater Management Manual</u>, <u>Public Works Department, City of Grand Junction, June, 1994</u>

² - Time of Concentration as derived in attached Appendix A worksheet

³ - Intensity taken from Table A-1, <u>Stormwater Management Manual, Public Works</u> <u>Department, City of Grand Junction, June, 1994</u>

Page B-3 of B-3

APPENDIX C RETENTION BASIN OUTFLOW DESIGN WORKSHEET

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RETENTION BASIN OUTFLOW DESIGN WORKSHEET ORIFICE CONTROL

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Project:Wells Commercial DevelopmentPrepared by:Tom A. CronkDate:April 4, 1995

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	2 year	r event			100 yea	ar event	
head difference, h ¹ , (ft.)	design discharge, Q ² , (cfs)	discharge coefficient, C ³	orifice area, A ⁴ , (ft ²)	head difference, h ¹ , (ft.)	design discharge, Q ² , (cfs)	discharge coefficient, C ³	orifice area, A ⁴ , (ft ²)
				0.6	0.99	0.595	0.27

¹ Water depth (ft.), Figure K-6, Stormwater Management Manual, Public Works Department, City of Grand Junction, June, 1994

² Design discharge = maximum historic discharge, Q_h (cfs) less other discharge sources (i.e., lower stage discharge and/or sheetflows)

³ Table K-4, Stormwater Management Manual, Public Works Department, City of Grand Junction, June, 1994

⁴ Area of orifice calculated as, $A = \frac{Q}{C\sqrt{2gh}}$, where $g = \text{gravitational constant (32.2 ft/sec^2)}$

Page C-2 of C-3

RETENTION BASIN OUTFLOW DESIGN WORKSHEET WEIR HYDRAULIC CONTROL (broad crest weir equation)

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Project:Wells Commercial DevelopmentPrepared by:Tom A. CronkDate:April 4, 1995

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	2 year	r event		100 year event								
head difference, h ¹ , (ft.)	design discharge, Q ² , (cfs)	coefficient of discharge, C ³	weir width, L ⁴ , (ft.)	head difference, h ¹ , (ft.)	design discharge, Q ² , (cfs)	coefficient of discharge, C ³	weir width, L⁴, (ft.)					
				0.4	2.1	2.6	2.0					

1 1

¹ Water depth measured from weir crest to flow depth upstream (ft.)

1 1

² Design discharge = maximum historic discharge, Q_h (cfs) less other discharge sources (i.e., lower stage discharge and/or sheetflows)

³ Figure L-5, Stormwater Management Manual, Public Works Department, City of Grand Junction, June, 1994

⁴ Width of weir calculated from the broad crested weir equation as, Q = CLh

Page C-3 of C-3

APPENDIX D TIME OF CRITICAL DURATION, T_d , WORKSHEET

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Page D-2 of D-2

APPENDIX E MODIFIED RATIONAL METHOD RETENTION BASIN SIZING WORKSHEET

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MODIFIED RATIONAL METHOD RETENTION BASIN SIZING WORKSHEET

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Project:Wells Commercial DevelopmentPrepared by:Tom A. CronkDate:April 4, 1995

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	Site Hydrology									Retention Basin Sizing										
Basin	Site Condition		Site Condition 2 year event			100 year event				2 yea	r event	100 year event								
			C ₂₄	T _{e24} (min.)	Q ₂₄ (cfs)	C _{100d}	T _{c100d} (min.)	Q _{100d} (cfs)	T ₄₂ ¹ (min.)	Q ₂ ² (cfs)	Storage Volume, V_2^3 , (ft^3)	T _{d100} 1 (min.)	Q _{r100} ² (cfs)	Storage Volume, V_{100}^{3} , (ft ³)						
	Pre-devel	oped	.32	38	.32	.38	27	.99												
	Post-devel	oped	.84	26.5	1.03	.86	17.5	3.34	64	.26	2891	44	.81	6588						
	Development	quantity			+.71			+2.34												
	Impact	percent			221%			236%												

¹ Time of critical duration, T_d , from Appendix D worksheet

² Average rate of discharge, Q_r , = 82% of actual discharge, Q_a , taken from Appendix C plus other discharge sources (i.e., lower stage discharge and/or sheetflows)

³ Storage volume required, V (ft³), calculated from:

$$V = 60 \left[Q_d T_d - Q_r T_d - Q_r T_{cd} + \frac{KQ_r T_{cd}}{2} + \frac{Q_r^2 T_{cd}}{2Q_d} \right], \text{ where,}$$

 $K = Ratio of pre- and post-development T_{cd}$



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APPENDIX F CULVERT SIZING WORKSHEET

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PROJECT: WELLS COMMERCIAL DEVELOPMENT	STATION: -25- ROAD STORM SEWER	CULVERT DESIGN FORM
<u>554 -25- RDAD</u>	SHEET DF	DESIGNER/DATE: TOM A. CRONK / 04/10/95
GRAND JUNCTION, CO 81505		REVIEWER/ DATE://
HYDROLOGICAL DATA	24° STORM SEWER EL. INV = 50.75 (APPROX.)	12" RCP EL. INV = 59.50
DESIGN FLOWS BASIN: WELLS MEHTOD: RATIONAL R. I. (years) FLOWS (cfs) 2 (DEVEL.) 1.00 100 (DEVEL.) 3.34 TAIL WATER CHANNEL FLOW CHANNEL TYPE STORM SEWER (24*) CHANNEL SHAPE ROUND	4565.00- 4565.00- 4565.00- 4560.00- 4660.00- 4560.0	GROUND SURFACE GROUND SURFACE GROUND SURFACE -456000 -456000 -456000 -456000 -456000 -456000 -456000 -455000
FLOW VELOCITY <u>2.0 - 5.0 fps</u> <u>R. I. (years) FLOWS (cfs TAIL WATER (ft)</u>		
CULVERT_SIZING TYPE OF FLOW: PARTIALLY FULL, NO HEAD		
MATERIAL: <u>RCP (ASTM C-76) CLASS 5</u> SHAPE: <u>ROUND</u> FLOW EQUATION: $Q = A_f \frac{149}{n} (r_h)^3 \sqrt{S}$ CULVERT SIZE: <u>12" I.D.</u> DESIGN MAXIMUM DISCHARGE (cfs) <u>4.1 (ND HEAD)</u> ENTRANCE: <u>SHARP EDGE</u>	Q=discharge (cfs) A _f =cross sectional area in flow (ft) n=manning roughness coefficient (0.016 for concrete pipe/rough joints) r _h =hydraulic radius S=slope	CULVERT BARREL SELECTED SHAPE:ROUND SIZE:12*_I.D. CLASS_5 MATERIAL:_REINFORCED_CONCRETE_ASTM_C-76 ENTRANCE:_SHARP_EDGE

REVIEW COMMENTS

Page 1 of 3

FILE # SPR-95-70

TITLE HEADING: Site Plan Review - Office Warehouse

LOCATION: 554 25 Road

PETITIONER: Bill Wells

PETITIONER'S ADDRESS/TELEPHONE:

2156 Buffalo Drive Grand Junction, CO 81503 243-2337

PETITIONER'S REPRESENTATIVE:

Steve McCallum

STAFF REPRESENTATIVE: Kristen Ashbeck

NOTE: WRITTEN RESPONSE (4 COPIES) BY THE PETITIONER TO THE REVIEW COMMENTS IS REQUIRED. A PLANNING CLEARANCE WILL NOT BE ISSUED UNTIL <u>ALL</u> ISSUES HAVE BEEN RESOLVED.

CITY POLICE DEPARTMENT	04/11/95
Dave Stassen	244-3587

I would suggest that there be adequate lighting between the businesses (low pressure sodium light would work well here). In addition, the fencing around the storage area (East end) should not be "slated chain" link. The slats would defeat the benefits of "transparent" fencing.

MESA COUNTY BUILDING DEPT.	04/12/95
Bob Lee	244-1656

Fire walls must be provided as required by the Building Code. Plans submitted for plan review must be scaled. North wall of building will require a parapet.

CITY ATTORNEY	04/11/95	
Dan Wilson	244-1501	
No Comments.		
PUBLIC SERVICE	04/15/95	
Dale Clawson	244-2695	

I find no provisions made for utilities on this project. Petitioner needs to contact Public Service Company about service locations and easements.

FILE # SPR-95-70 / REVIEW COMMENTS / PAGE 2 OF 3

GRAND JUNCTION FIRE DEPARTMENT	04/13/95
Hank Masterson	244-1414

A Fire Flow Survey is required-submit complete building plans to the Fire Department. Hydrants and water lines are not shown on the Site Plan. Minimum water line size is 8" and must be looped. Because of the 400' length of this building, a hydrant will need to be located along 25 Road and no more than 70' from the southwest corner of the property. This will allow for a maximum distance of 400' from hydrant to a location on the south side of the building that is within 150' of the northeast corner of the building.

A more serious problem for this site plan is the lack of Fire Department access along the north side of the building. Emergency vehicle access is required to withing 150' of all perimeter portions of the building. The submitted site plan does not indicate any dedicated access along the 400' length of the north side. The Fire Code allows for modification of access requirements if the building is protected by an approved fire sprinkler system.

GRAND JUNCTION DRAINAGE DISTRICT	04/18/95
John Ballagh	242-4343

The subsurface drain line in the western side of 25 Road is a GJDD facility, it is not a storm drain. It is not "normally dry". Underground, seep water, return flow irrigation water and surface runoff all flow in the pipe. There is not a great deal of unused capacity in the <u>18"</u> RCP line. The 25 Road drain line empties into the Buthorn Drain just south of Hwy 6 & 50 on the Coldwell Banker property.

The site is definitely in the "lower 1/3" of the basin. While the addition of waters to the 25 Road tile is not desireable, the site location does argue to get rid of surface runoff as quick as possible. Thus the 12" RCP line will be approved. However there must be a 48" manhole installed at the point where the 12" RCP line will discharge to the existing 18" RCP 25 Road tile.

U. S. WEST	04/18/95
Max Ward	244-4721

Okay. Building entrance cable.

New or additional telephone facilities necessitated by this project may result in a "contract" and up-front monies required from developer, prior to ordering or placing of said facilities. For more information, please call 1-800-526-3557.

CITY DEVELOPMENT ENGINEER	04/25/95
Jody Kliska	244-1591

Dedication of 10' of additional right-of-way along 25 Road is required.

Transportation Capacity Payment - 16,000 sq.ft. X 4.88 trips/1000 sq.ft. X 1/10 X \$500 = \$3,904 based on warehouse-type use.

Where does the new pipe (12"RCP) tie into the existing storm drain? Is a manhole required? It appears a single unit vehicle will have difficulty accessing Unit 5 without encroaching on the parking space.

FILE #SPR-95-70 / REVIEW COMMENTS / PAGE 3 OF 3

COMMUNITY DEVELOPMENT DEPARTMENT	04/25/95	
Kristen Ashbeck	244-1437	

- 1. Total sign allowance for site based on building frontage along 25 Road is 80 square feet. This must be divided among all businesses in the building.
- 2. PARKING: Difficult to determine proposed use. Narrative refers to retail/commercial, yet parking calculations on plan refer to wholesale/warehouse. Worst case scenario for parking requirement should be used in order to accommodate the greatest variety of uses say, 50% retail sales area = 40 spaces, 2 of which must be handicap accessible. Since only 20 are shown, at a minimum, the gravelled storage area must be paved and striped in the likely event that it is needed for parking. It appears this area could accommodate at least 16 more spaces (show on revised plan).

Show with a turning template how a large truck can back into a garage door of one of the bays without encroaching on an adjacent parking space, or on neighboring property (especially in the bays across from the proposed retaining wall).

Move bicycle rack to a more visible/accessible location e.g. along front or 25 Road end of building instead of east end.

- 3. Fire access along back of building may be required or the building must be properly sprinkled. See Fire Department comments.
- 4. Landscaping meets requirements, including square footage for gravelled area to be paved.
- 5. Transportation Capacity Payment (TCP) and drainage fee may be required. Refer to Development Engineer comments.

ĊITY UTILITY ENGINEER	04/26/95
Bill Cheney	244-1590

Water: Ute Water.

Sewer:

Show proposed connection to sewer.

There may be a payback on the sewer line in 25 Road. Contact City Utility Billing (244-1513).

Provide information for calculation of sewer Plant Investment Fee.

UTE WATER DISTRICT	04/25/95
Gary Mathews	242-7491

Ute Water has a 8" main line on the West side of 25 Road.

Developer needs to contact Ute Water for options in water metering.

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POLICIES AND FEES IN EFFECT AT THE TIME OF APPLICATION WILL APPLY.

RESPONSE TO REVIEW COMMENTS

FILE #SPR95-70

Location: 554 25 Road
 Petitioner: Bill Wells
 Petitioner's Address/Telephone: 2156 Buffalo Drive
 Grand Junction, CO 81503
 243-2337
 Petitioner's Representative: Steve McCallum
 243-4642
 Staff Representative: Kristen Ashbeck
 Response Submitted: April 25, 1995

****City Police Department, Dave Stassen**** Any fencing used will be of the open chain link design to create transparent fencing as per your request. The building will be adequately lighted for security purposes.

****Mesa County Building Department, Bob Lee**** Fire walls and fire protection will be provided. A full set of scaled plans will be provided.

****Public Service, Dale Clawson**** Public Service will be provided with plans and request for service.

****Grand Junction Fire Department, Hank Masterson**** Complete plans will be provided, as well as a request for the fire flow survey. Hydrants required will be installed along with access to the proposed structure.

****Grand Junction Drainage Dist., John Ballagh**** The 48" manhole requested on 25 Road will be provided as per your request.

Sincerely.

Steve McCallum



RESPONSE TO REVIEW COMMENT

FILE #SPR95-70

.

Location: 554 25 Road Petitioner: Bill Wells Petitioner's Address/Telephone:

Petitioner's Representative:

RECEIVED CRAND JUNCTION 2156 Buffal Drive Grand Junction 243-2337 Steve McCallum

243-4642 Staff Representative: Michael Drollinger Response Submitted: May 2, 1995

****City Police Department, Dave Stassen**** Any fencing used will be of the open chain link design to create transparent fencing as per your request. The building will be adequately lighted for security purposes.

****Mesa County Building Department, Bob Lee**** Fire walls and fire protection will be provided. A full set of scaled plans will be provided.

****Public Service, Dale Clawson**** Public Service will be provided with plans and request for service.

****Grand Junction fire department, Hank Masterson**** Complete plans will be provided, as well as a request for the fire flow survey. Hydrants required will be installed along with access to the proposed structure.

****Grand Junction Drainage Dist.., John Ballagh**** The 48" manhole requested on 25 Road will be provided as per your request.

******City Development Engineer, Jody Kliska****** Additional ROW as required will be dedicated. We will pay TCP as 12" RCP will be shown on revised drawing. Landscape computed. area will be re-designed at unit #8, to allow accessing by a single unit vehicle.

****Community Development Department, Michael Drollinger**** Total sign allowance of 80 square feet will be complied with. The proposed use was correct as per the engineer and was improperly termed retail/commercial in the project narrative. The additional graveled area will be for storage only. The bicycle rack will be relocated as per your request. The TCP has been completed by Jody Kliska and will be paid. SPE 95-70 Webrs Commercial Building - 554 25 Road AP Community Development Dept 4/25/95

- 1. Total sign allowance for site based on building frontage along 25 Road is 80 square feet. This must be divided among all businesses in the building.
- 2. PARKING

-Difficult to determine proposed use. Narrative refers to retail /commercial, yet parking is calculation on plan refer to wholesale/warehouse. Worst case scenario for parking requirement should be used in order to accommodate the greatest variety of uses -- say, 50% retail sales area = 40 spaces, 2 of which must be accessible. Since only 20 are shown, at a minimum, the gravelled storage area must be paved and striped in the likely event that it is needed for parking. It appears this area could accommodate at least 16 more spaces (show on revised plan).

- Show with a turning template how a large truck can back into a ganage door of one of the bays without encroaching on an adjacent parking space, or on neighboring property (especially in the bays across from the proposed retaining walk).

- Move bicycle rack to a more visible/accessible location e.g. along front or 25 Road end of building instead of east end.

Fire access along. back of building may be required or the building must be properly sprinkled. See Fire Department comments.
Landscaping meets requirements, including square footage for gravelled area to be paved.
Transportation Eapacity Eayment (TCP) and drainage fee may be required. Refer to Development Engineer comments.



City of Grand Junction, Colorado 250 North Fifth Street 81501-2668 FAX: (303) 244-1599

May 10, 1995

Steve McCallum 552 25 Road Grand Junction, CO 81505

RE: Wells Commercial Development (Our File # SPR 95-70)

Dear Mr. McCallum,

At this time the City is approving your proposal with the use mix and provided parking which is detailed on the site plan. As you are aware, sufficient parking will not be available for the development should the approved uses change to uses requiring parking above the limited number of spaces which have been provided exceeding the existing parking requirement. The City will approve use change requests which can provide for sufficient parking on a "first come" basis. Future tenants may run the risk of not having sufficient parking available for their proposal.

If you have any questions or require further information please do not hesitate to call.

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Sincerely yours Michael T. Drollinger Senior Planner

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