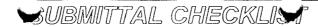
Table of Contents

File __SPR-1996-190 Name: Barnes and Noble Bookstore / Café - 2451 F Road A few items are denoted with an asterisk (*), which means they are to be scanned for permanent record on the ISYS retrieval system. In some instances, items are found on the list but are not present in the scanned electronic development file because they are already scanned elsewhere on the system. These scanned documents are denoted with (**) and will be found on the ISYS query system in their designated categories. e n Documents specific to certain files, not found in the standard checklist materials, are listed at the bottom of the page. t d Remaining items, (not selected for scanning), will be listed and marked present. This index can serve as a quick guide for the contents of each file. **Table of Contents** *Review Sheet Summary *Application form Review Sheets Receipts for fees paid for anything *Submittal checklist X *General project report Reduced copy of final plans or drawings Reduction of assessor's map. Evidence of title, deeds, easements *Mailing list to adjacent property owners Public notice cards Record of certified mail Legal description Appraisal of raw land Reduction of any maps – final copy *Final reports for drainage and soils (geotechnical reports) Other bound or non-bound reports Traffic studies X X *Review Comments X *Petitioner's response to comments *Staff Reports *Planning Commission staff report and exhibits *City Council staff report and exhibits *Summary sheet of final conditions **DOCUMENT DESCRIPTION:** X X Correspondence General Legend and Construction Notes X X Cost Estimate Sheet - 8/21/96, 9/10/96, 9/27/96 X Standard concrete Details - Exhibit E, I, X X | X | Final Drainage Report - 8/14/96 Standard Storm Drain Details $\overline{\mathbf{x}}$ Standard Water Line Details Commitment for Title Ins. - Chicago Title Ins. Company X X Development Improvements Agrmt/Initial Acceptance – no X Accessible Ramp and Parking Stall Details release - not recorded 10/23/96 - delivered to City Clerk for retention /scanning - ** Certificate of Occupancy - 5/22/97 Miscellaneous Site Specific Details Map Planning Clearance - ** - issued 10/23/96 X X Landscape Plan – not signed, but will be scanned X X X | X | Planting Plan – not signed, but will be scanned Elevation Maps Monument Sign Diagram Two Easements - to be delivered to the City Clerk for retention/scanning - do not have the original-** X X Site Plan Reciprocal Easement Agreement – not recorded/not signed X X | X | Site Improvement Plans – to be scanned Sanitary Sewer Plan and Profile



SITE PLAN REVIEW

Location: F # 24 1/2 /d Project Name: 3PR - While Stand

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PRE-APPLICATION CONFERENCE Date: 51 Hema, Dick Francis, Jem Langford Conference Attendance Proposal: Location: Tax Parcel Number: Review Fee: (Fee is due at the time of submittal. Make check payable to the City of Grand Junction.) Adjacent road improvements/required? <u>\(\frac{1}{2} \)</u> Area identified as a need in the Master Plan of Parks and Recreation? Parks and Open Space fees required? Estimated Amount: Recording fees required? Estimated Amount: __ Half street improvement fees/TCP required? Estimated Amount: Revocable Permit required? In landocasus in State Highway Access Permit required? On-site detention/retention or Drainage fee required? [1] -514 Applicable Plans, Policies and Guidelines Located in other geohazard area? Located in established Airport Zone? Clear Zone, Critical Zone, Area of Influence? Avigation Easement required? While all factors in a development proposal require careful thought, preparation and design, the following "checked" items are brought to the petitioner's attention as needing special attention or consideration. Other items of special concern may be identified during the review process. O Access/Parking O Screening/Buffering O Land Use Compatibility O Drainage O Landscaping O Traffic Generation O Floodplain/Wetlands Mitigation O Availability of Utilities O Geologic Hazards/Soils O Other Related Files: It is recommended that the applicant inform the neighboring property owners and tenants of the proposal prior to the public hearing and preferably prior to submittal to the City. PRE-APPLICATION CONFERENCE WE RECOGNIZE that we, ourselves, or our representative(s) must be present at all hearings relative to this proposal and it is our responsibility to know when and where those hearings are. In the event that the petitioner is not represented, the proposed item will be dropped from the agenda, and an additional fee shall be charged to cover rescheduling expenses. Such fee must be paid before the proposed item can again be placed on the agenda. Any changes to the approved plan will require a re-review and approval by the Community Development Department prior to those changes being accepted. WE UNDERSTAND that incomplete submittals will not be accepted and submittals with insufficient information, identified in the review process, which has not been addressed by the applicant, may be withdrawn from the agenda. WE FURTHER UNDERSTAND that failure to meet any deadlines as identified by the Community Development Department for the review process may result in the project not being scheduled for hearing or being pulled from the

agenda.

Signature(s) of Petitioner(s)

Barnes & Noble Retail Development

Site Plan Review Submittal

General Project Report

A. Project Description

The property is located at the corner of 24-1/2 Road and Patterson Road. The lot is 1.62 acres to be developed as a retail outlet for the sale of books, software, and other informational media. The store also includes a coffee shop. The remainder of the site will be used for the required parking, storm water management, and landscaping.

As per agreement with the adjacent land owner to the east, the entry is to be shared, and the access to the subject property is to be by easement. This is to mutually benefit both properties by reducing curb cuts on Patterson Road, allow the subject property to expand parking, and allow the adjacent property to avoid the expense of crossing the irrigation ditch to the north.

B. Public Benefit

The proposed development will provide infill in an area surrounded by commercial uses. The architecture and surrounding landscape will provide an attractive location for a quality retail outlet on a highly visible site. The proposed development will pay its commensurate share of fees and taxes to support the existing infrastructure and operating expenses for the services required.

C. Project Compliance, Compatibility, and Impact

The development meets setbacks, access, parking, and landscape requirements of the existing C-2 zoning. The retail sales area of 13,690 s.f. requires 69 parking spaces. The 38 seats in the restaurant requires 13 spaces, totaling a gross parking requirement of 82 spaces. The proposal shows 91 spaces. The landscape requirements with the proposed tree counts and square footage's of landscaping are shown on the landscape plan.

The surrounding land uses are commercial, with the property to the east remaining undeveloped at this time

Infill Development: The proposed project meets the intent of the promoting the development of infill development, allowing for the efficient use of existing utilities and roadways.

Northwest Area: The Northwest Area Plan promotes the development of the portion of the City due to it's proximity to existing growth areas, accessibility, and existing commercial uses.

Patterson Road Guidelines: "commercial development is appropriate on the south side of Patterson Road from Highway 6&50 to 25-1/2 Road"; "Access points should be designed to serve more than one lot" (entry is designed to be shared with the adjacent lot to the east).

D. Development Schedule and Phasing

construction is scheduled to begin immediately following site plan approval and issuance of a building permit.



August 12, 1996

City of Grand Junction 250 North 5th Street Grand Junction, CO 81501

Re: 24 1/2 and F Road

Dear Sir:

This letter serves as The Sterling Co.'s authorization to allow SB Advisors, Inc., or their respective contractors, Ciavonne & Associates, or Thompson-Langford, to pursue site plan approvals on the southeast corner of 24 1/2 and F Roads. Attached is a legal description for the property.

If you have any questions regarding this authorization or the extent of this approval, please contact me at your earliest convenience.

Sincerely,

Richard Scariano The Sterling Co.

RS/fah

Final Drainage Report

Barnes and Noble Book Store

August 14, 1996

Prepared for:

Tim Sittema Sittema - Bullock 5445 DTC Parkway, Penthouse Four Englewood, CO 80111

Prepared by:

THOMPSON-LANGFORD ©ORPORATION 529 251/2 RD., SUITE B-210 Grand Junction, CO 81505 PH. 243-6067

Job No. 0293-001

Engineer's Certification

I hereby certify that this report was prepared by me or under my direct supervision for the Owner's hereof.

James E. Langford, PE & LS Reg. No. 14847

Introduction

I. GENERAL LOCATION AND DESCRIPTION:

A. Site and Major Basin Location:

The site of the future Barnes and Noble Book Store is located in Northwest 1/4 of the Northeast 1/4 of Section 9, Township 1 South, Range 1 West of the Ute Meridian. In more local terms, it is located immediately north of the Carmike theater at the intersection of 24 1/2 Road and Patterson Road.

B. Site and Major Basin Description:

The site, located at the southeast corner of the intersection of 24 1/2 Road and Patterson Road, is a parcel approximately 400 feet long by 200 feet wide. The property is hydraulically separated from Patterson Road by the Independent Ranchmen's Ditch; the portion of the ditch to the west running beneath Patterson Road and into the Mall area. The site is presently unoccupied and devoid of vegetation.

II. EXISTING DRAINAGE CONDITIONS:

A. Major Basin:

The site as it presently exists is nearly flat, sloping at slightly less than 0.5% to the west. Due to this lack of gradient, ponding does occur in the center of the site. Stormwater impacting the site either drains out the west end of the parcel onto 24 1/2 Road, or leaches into the ground.

B. Site:

Given the hydraulic isolation of the property, the major drainage basin and the site are essentially the same.

III PROPOSED DRAINAGE CONDITIONS:

A. Changes in Drainage Patterns:

Site stormwater drainage either ponds on-site or overflows into the curb and gutter and runs south along 24 1/4 Road to inlets near the intersection of 24 1/4 Road with State Highway 6 & 50. We are proposing to collect the site stormwater in a detention facility near the northwest corner of the site and regulate the flow to historic levels

directly into the buried portion of the Independent Ranchmen's Ditch.

B. Maintenance Issues:

The on-site collection and detention facilities will be the responsibility of the lot owner.

IV DESIGN CRITERIA AND APPROACH:

A. GENERAL CONSIDERATIONS:

Natural conditions such as the tight adobe soils, high ground water and the extremely flat slopes have historically made this site difficult to drain.

As mentioned earlier in this report, the site is very flat and does not drain well. When a storm does occur that is large enough to cause runoff, the site drains down 24 1/2 Road. With development of this site, drainage down 24 1/2 Road is no longer possible because of the inability of the storm sewer near Highway 6 & 50 to handle the increased flow.

The site is within the 100-year floodplain of the Independent Ranchmen's Ditch. We will be elevating the building to place it at least 1-foot above the identified maximum water surface elevation.

B. Hydrology:

Pre-development Runoff coefficients used in the Rational equation were based on the hydrologic soil group index for the soil type found within the project. For post-development conditions we used the range of coefficients for the various proposed surface covers found in Appendix "B" of the above referenced manual.

According to the Soil Conservation Service soil survey for the Grand Junction Area, the dominant soil type is the Billings soil group having a hydrologic soil group index of "C". The soils on the site are hard packed which would suggest that the site has been used for some commercial or light industrial purpose. This would have suggested classifying the area as a traffic area. To do so would have made the historic runoff too high, closing the gap between the historic and the developed condition thus limiting the required detention. It was felt that using runoff coefficients based on HSG "A" versus HSG "C" would better represent the runoff potential and yield a more realistic detention volume.

Times of Concentration were calculated using the procedures outlined in Appendix "E" of the Storm Water Management Manual.

Given the small size of the project, the site was considered as one basin and analyzed using the Rational Method as described in Section VI. Hydrology, City of Grand Junction Storm Water Management Manual. Stormwater runoff for the 2-year and 100-year events were quantified and routed through a detention facility located in the northwest corner of the site.

C. Hydraulics:

The flow capacity of concrete pans, curb and gutter or underground conduits was calculated using Manning's Equation and the requisite coefficients all found in Appendices "G" & "H".

The detention facility was designed to detain both the 2-year and 100-year events, discharging through a two stage outlet only at the historic rates. Discharge calculations are included as an appendix to this report assuring that during the 2-year event, only the historic 2-year flow is released from the facility, and during the 100-year event the combinations of the outlets will discharge only the historic 100 year flow.

The two stage outlet control structure was sized using the procedures outlined in Appendix "K".

IV Results and Conclusions

Runoff Results:

2-year historic runoff rate = 0.82 CFS

2-year developed runoff rate = 2.56 CFS

100-year historic runoff rate = 3.10 CFS

100-year developed runoff rate = 6.65 CFS

Detention Facility:

Storage volume for 2-year event = 2242.79 cu-ft.

Storage volume for 100-year event = 3513.87 cu-ft.

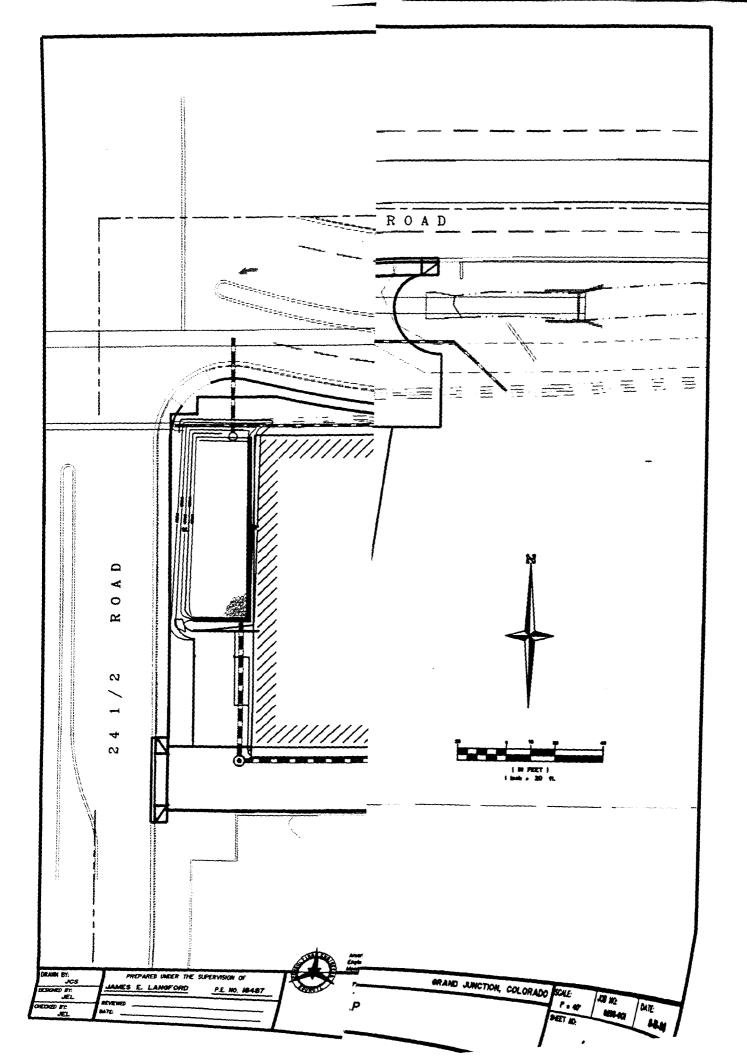
The outlet works will be constructed using a modified CDOT Type "C" inlet with a close mesh grate. An orifice will be constructed in the front face of the box at an invert elevation of 4553.00 which also coincides with the bottom of the pond. A weir will be constructed at the top, just below the close mesh grate. The orifice, having a diameter of 0.50 feet, has been sized to pass the 2-year historic flow when the water surface of the pond is up to the 2-year storage level of 4554.33. The second stage outlet ,or weir, which has been set at the 2-year water surface elevation, and given a width of 1.08 feet in length, will in combination with the orifice, pass the 100-year event when the water surface elevation is up to the 100-year storage level of 4554.9.

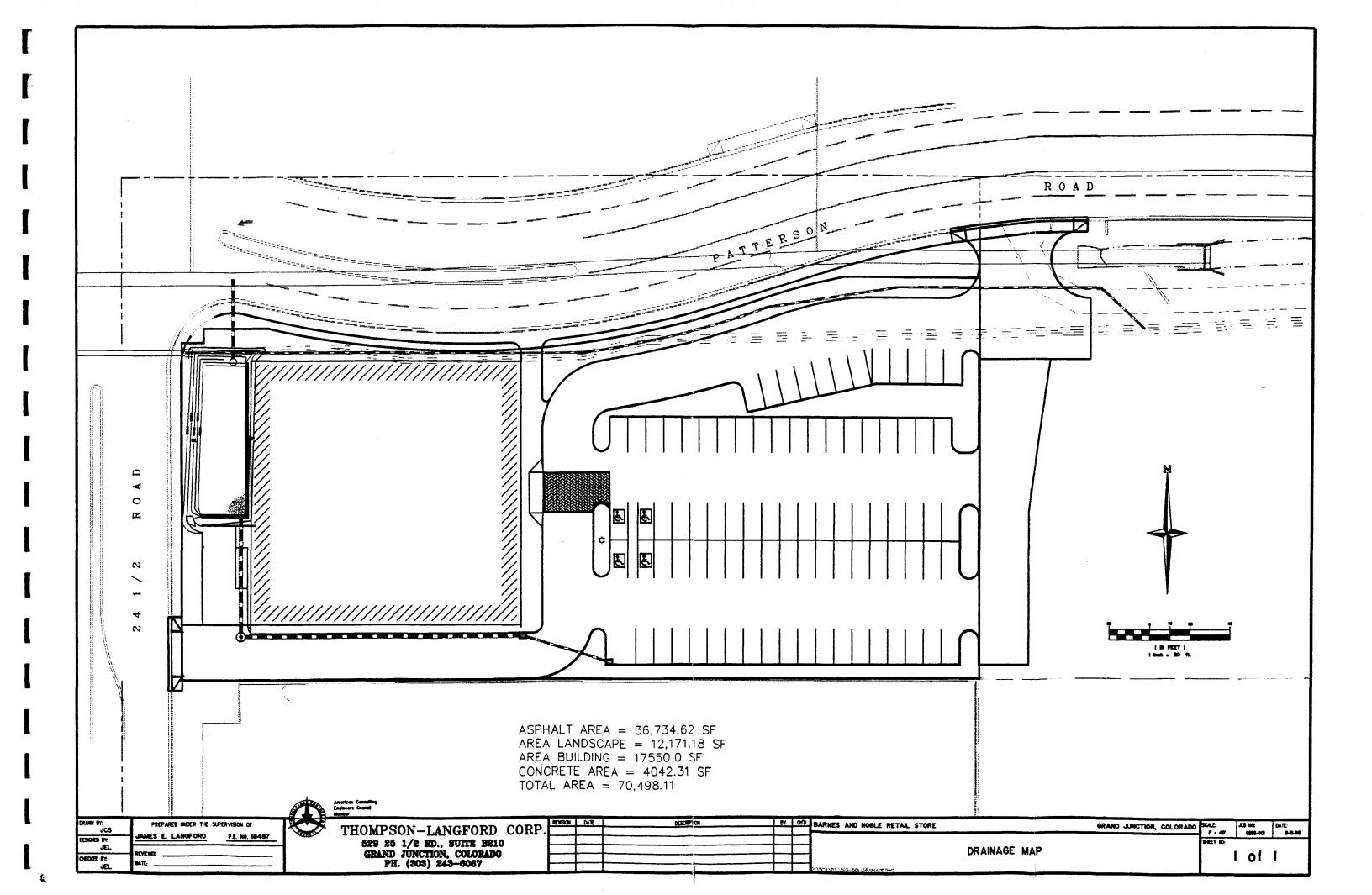
The top of the box is to be set at the 100-year water surface elevation and is to be covered with a standard CDOT close mesh steel grating. The steel grating will serve as an emergency overflow in the event of a storm in excess of the maximum design event. The outfall from the outlet works is to be 18-inch PVC pipe.

References

"STORMWATER MANAGEMENT MANUAL (SWMM)", City of Grand Junction, June 1994

"MESA COUNTY STORM DRAINAGE CRITERIA MANUAL", Mesa County, Colorado, Final Draft March 1992.





BARNES & NOBLE, 0293-001 TWO STAGE OUTFALL CALCULATION

Procedure as described in the City of Grand Junction's Storm Water Management Manual See Page N-5

NOTE:

- * Enter data from Drainage Study
- ** Vary this number until the desired result is obtained
- X Calculated by spreadsheet (no entry required)

Orifice Flow (2-year event)

- * Water Surf. El. 4554.33 Ft. * Orifice Invert 4553.00 Ft.
- ** Orifice Dia. (d) 0.50 Ft. **Vary orifice diameter until

 * Discharge (Qr) 0.82 CFS areas match
- * "Co" Coef. 0.60
- X Area = $(3.1416)d^2/4$ = 0.20 sF x = $Qr/0.82C(2gh)^0.5$ = 0.20 sF

Combined Wier Flow and Orifice Flow (100-year event)

* Water Surf. El. 4554.90 X Wier Invert El. 4554.33

The 100-year storage elevation is dictated by pond configuration. The elevation of the invert of the wier is set equal to the 2-year storage elevation. The wier width will be calculated such that the discharge when added to the orifice discharge equals the 100-year discharge.

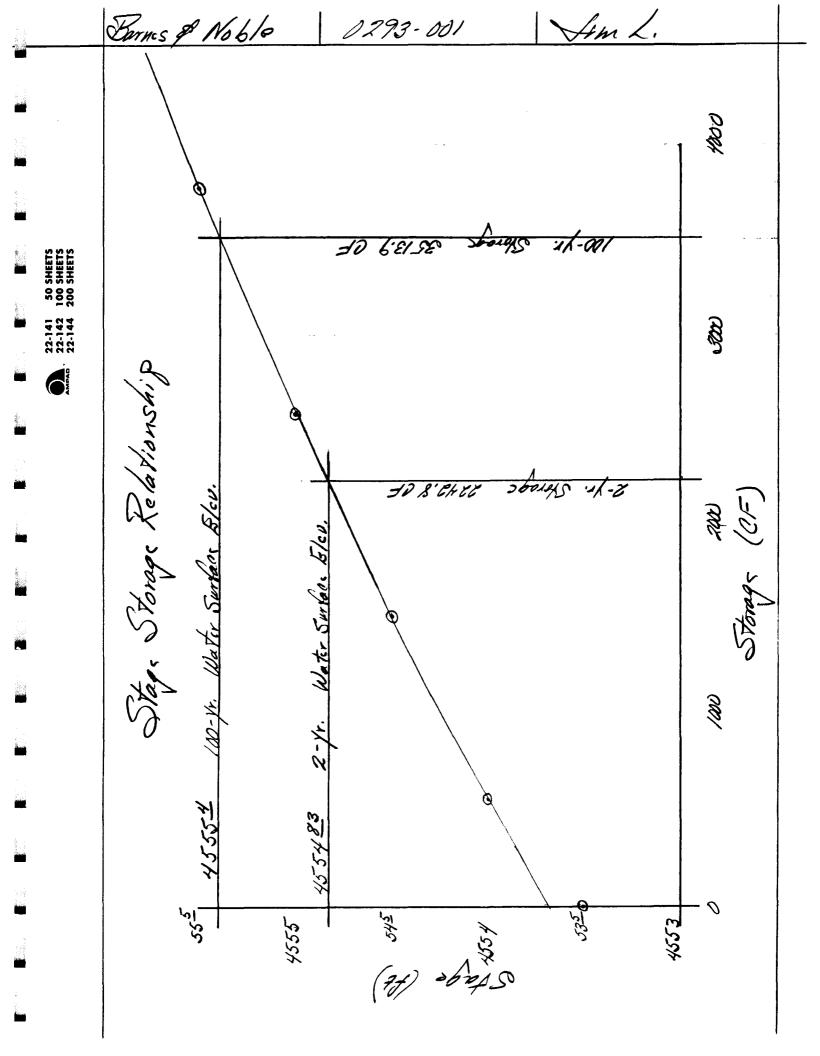
- * Q100 discharge = 2.56 CFS
 - Q (orifice) = $0.82CoA(2gh)^0.5 = 1.01 CFS$

Wier Flow Equasion

 $Q = CwLH^1.5$

- X Wier discharge = 1.55 CFS
 * "Cw" Coef. 3.33
 X Flow Depth (H) = 0.57 Ft.
- ** Wier Length (L) 1.08 Ft. **Vary unit1 "Q" = Q100
 - Q= Wier Flow + Orifice Flow

2.56 CFS **If this calculated flow equals the historic 100-year flow then the wier length is correct.



AUGUST 7, 96 BARIXS É NOBLE RETAIL STORE (JOB # 0293-00) PETENTION POND STAYED STORME VOLUME COLCULATION TOTAL POND VOL = 3,780 cf (790 and Rep 1004) 45549 455435 45530 * COULLATIONS PCZ DCD BIRTHWILL Morene Cumulative AREA SE VOL CE ELEV VOLUME 1555 3,780 °F 2479sf 455° 2240 2,592 4 1053 2049 1,539 0 15542 972 45540 567-6= 19570 50 45535

B&N Detention Volume

DETENTION VOLUME

For: Barnes and Noble

USING

METHOD OUTLINED ON PAGE N-4 SWMM

```
Td = Time of critical storm duration, minutes
                                                                                  0.81
C2
      = Runoff coefficient (2-Year Event)
                                                                                  0.83
Cl00 = Runoff coefficient (100-Year Event)
A = Area in acres (developed condition)
                                                                                  1.62
Or2 = Detention pond average release rate, cfs (Note that this will
                                                                                  0.82
     not likely be the historic rate Qh, nor even Qmax)
                                                                                   3.10
Qr100 = Detention pond average release rate, cfs (Note that this will
     not likely be the historic rate Qh, nor even Qmax)
                                                                                 28.20
Tch2 = Time of concentration (historic), minutes (2-year event)
                                                                                 18.50
Tch100 = Time of concentration (historic), minutes (100-year event)
Tcd2 = Time of concentration (developed), minutes (2-year event)
                                                                                   5.00
                                                                                   5.00
Tcd100 = Time of concentration (developed), minutes (100-year event)
                                                                                  1.95
Id2 = Intensity at Td, inches per hour (2-year event)
                                                                                   4.95
Id100 = Intensity at Td, inches per hour (100-year event)
Od = Runoff rate at Td, cfs
K = Ratio of pre-and post-development Tc
V2 = Storage volume (2-year event) cu. ft.
V100 = Storage volume (100-year event) cu. ft.
        = (((633.4*Cd2*A)/(Qr2-(Qr2^2*Tcd2)/(81.2*Cd2*A)))^0.5)-15.6
Td2
               16.85 Min.
Td100
        = (((1832*Cd*A)/(Qr100-(Qr100^2*Tcd)/(213*Cd*A)))^0.5)-17.2
               11.77 Min.
```

B&N Detention Volume

Qd2 = Cd*A*Id2

= 2.56 cfs

Qd100 = Cd*A*Id100

= 6.65 cfs

K2 = Tch2/Tcd2

5.64

K100 = Tch100/Tcd100

3.70

 $V2 = 60[Qd2*Td2-Qr2*Td2-Qr2*Tcd2+K2*Qr2*Tcd2/2+Qr2^2*Tcd2/(2Qd2)]$

= 2,242.79 cu-ft.

 $V100 = 60[Qd100*Td100-Qr100*Td100-Qr100*Tcd100+K100*Qr100*Tcd100/2+Qr100^2*Tcd100/($

= 3,513.87 cu-ft.

RUNOFF VOLUME

For: Barnes and Noble

USING

RATIONAL METHOD Q=CxCfxIxA

BASIN	Q Volume	C Composite Coefficient	Cf Antecedent Precip. Fac.	I* Rainfall Intensity	A Basin Area
	cfs	n/a	n/a	in/hr	acres
Historic (2-Yr)	0.82	0.55	1	0.92	1.6184
Historic (100-Yr)	3.10	0.65	1	2.95	1.6184
Developed (2-Yr)	2.56	0.81	1	1.95	1.6184
Developed (100-Yr)	6.65	0.83	1	4.95	1.6184

^{*}Rainfall intensity was picked from Table A-1, the Intensity/Duration curves for the City of Grand Junction, based on Time of Concentration

TABLE - 2

TIME OF C	ONCENTRATIO	ON and	RAINF	ALL INTE	INSITI	ES					
For: Bar	nes and Nol	ble								2-Year	100-Year
BASIN		L	S	N*	V*	Tt2	Tt100	Tc2	Tc100	i	i
	Descrip. of Flow	Length	Slope	Mannings	Vel.	Travel Time	Travel Time	Time Concen	of tration	Intensity Grd. Jctn.	Intensity Grd. Jctn.
		ft.	8	coef.	fps	min.	min.	min.	min.	Curves	Curves
"Full Site"	•										
Historic	overland*	300	0.50%	0.030				28.2	18.5	0.92	2.95
	Nat. Ch.***	170	0.50%	n/a	0.70	4.05	4.05				
	C&G**	0	800.0	0.000	0.00	0.00	0.00				
"Full Site"	•										
Developed	Landscape*	0	1.00%	0.400				4.0	4.0	1.95	4.95
As	ph. Swale***		1.00%	n/a	1.75	2.62	2.62	** Used 5	minutes as	a minimum	
C	Conc. Swale**	170	0.50%	0.000	2.50	1.13	1.13				

0.000 5.00 0.27 0.27

Pipe Flow 80 0.50%

^{*} Overland "To" based on SCS formula pg. E-2 Storm Water Management Manual

^{**}Mannings Equa. was used to determine gutter and natural swale velocities.

Mannings n=0.016 was used for curb and gutter, and n=0.030 was used for natural swales.

^{***}Figure "E-3", Pg. E-9, Storm Water Management Manual was used for shallow flows.

TABLE - 1a

COMPOSITE RUNOFF COEFICIENTS

For: Barnes and Noble

USING

GRAND JUNCTION RECOMMENDED RUNOFF COEFICIENTS

	Hydro.	Slope <2%			BASIN Histori	c	BASIN Develope	ed
Description	Soils	Runoff	Sel.		Unit	Wt'd	Unit	Wt'd
Surface Area	Group	Coeff.'s	Coeff.		Area	Value	Area	Value
Pavement and Roofs	В*	0.93	0.93	2-Yr.			1.34	1.25
	B*	0.95	0.95	100-Yr.			1.34	1.27
Green landscaping	B*	0.14 to 0.22	0.22	2-Yr.			0.28	0.06
lawns and parks	B*	0.20 to 0.28	0.28	100-Yr.			0.28	0.08
Traffic Area	A	0.55 to 0.65	0.55	2-Yr.	1.62	0.89		
Soil and Gravel	A**	0.65 to 0.70	0.65	100-Yr.	1.62	1.05		
Total Basin Area:					1.62		1.62	
COMPOSITE "C" VALUE	(2-year))			•	0.55		0.81
COMPOSITE "C" VALUE	(100-yea	ar)				0.65		0.83

^{*} The natural soils are in HSG "C", but because we are filling the site with pit run material to get it above the 100-year floodplain, we have used HSG "B".

^{**} The surface soils at the site are hard packed silts and clays devoid of any vegetation. The area's runoff potential is somewhere between bare ground and a traffic area. It was felt that the lowest classification of traffic area (HSG "A") was most appropriate.

LAND USE OR		SCS	HYDROI	LOGICS	OIL GRO	UP (SEE	APPENI	DIX "C" I	FOR DES	CRIPTIC	ONS)	
SURFACE CHARACTERISTICS		A			В			С			D	
	0-2%	2-6%	6%+	0-2%	2-6%	6%÷	0-2%	2-6%	6%+	0-2%	2-6%	6%+
UNDEVELOPED AREAS Bue ground	.1020	.1626	.2535	.1422	.2230	.3038	.2028	.2836	.3644	,2432	.3038	.4048
	.1424	.2232	.3040	.2028	.2836	.3745	.2634	.3543	.4048	.3038	.4048	.5058
Cultivated/Agricultural	.0818	.1323	.1626	.1119	.1523	.2129	6.1422	.1927	.2634	.1826	.2331	.3139
	.1424	.1828	.2232	.1624	.2129	.2836	.2028	.2533	.3442	.2432	.2937	.4149
Pasture	.1222	.2030	.3040	.1826	.2836	.3745	.2432	.3442	.4452	.3038	.4048	.5058
	.1525	.2535	.3747	.2331	.3442	.4553	:.3038	.4250	.5260	.3745	.5058	.6270
Meadow	.1020	.1626	.2535	.1422	.2230	.3038	.20 - 28	.2836	.3644	.2432	.3038	.4048
	.1424	.2232	.3040	.2028	.2836	.3745	.2634	.3543	.4452	.3038	.4048	.5058
Forest	.0515 .0818	.0818 .1121	.1121 .1424	.0816	.1119 .1422	.1422 .1826	.1018 .1220	.1321 .1624	.1624 .2028	.1220 .1523	.1624 .2028	.2028 .2533
RESIDENTIAL AREAS 1/8 acre per unit	.4050	.4353	.4656	.4250	.4553	.5058	.4553	.4856	.5361	.4856	.5159	.5765
	.4858	.5262	.5565	.5058	.5462	.5967	.5361	.5765	.6472	.5664	.6068	.6977
1/4 acre per unit	.2737	.3141	.3444	.2937	.3442	.3846	.3240	.3644	.4149	.35 • .43	.3947	.4553
	.3545	.3949	.4252	.3846	.4250	.4755	.41 - 49	.4553	.5260	.43 • .51	.4755	.5765
1/3 acre per unit	.2232	.2636	.2939	.2533	.2937	.3341	.28 - 36	3240	.3745	.3139	.3543	.4250
	.3141	.3545	.3848	.3341	.3846	.4250	.3644	4149	.4856	.3947	.4351	.5361
1/2 acre per unit	.1626	.2030	.2434	.1927	.23 r .31	.2836	2230	.2735	.3240	.2634	.3038	.3745
	.2535	.2939	.3242	.2836	.3240	.3644	3139	.3543	.4250	.3442	.3846	.4856
l acre per unit	.1424	.1929	.2232	.1725	2129	.2634	.2028	.2533	.3139	.2432	.2937	.3543
	.2232	.2636	.2939	.2432 -	28 - 36	.3442	.2836	.3240	.4048	.3139	.3543	.4654
MISC. SURFACES Pavement and roofs	.93	.94	.95	93	94	.95	.93	.94	.95	.93	.94	.95
	.95	.96	.97	95	.96	.97	.95	.96	.97	.95	.96	.97
Traffic areas (soil and gravel)	.5565	.60 -).70	.6474	.6068	64 - 72	.6775	.6472	.6775	.6977	.7280	.7583	.7785
	.6570	.7075	.7479	.6876	72 - 80	.7583	.72 - 80	.7583	.7785	.7987	.8290	.8492
Green landscaping (lawns, parks)	10 - 20	.1626	.2535	.1422	.2230	.3038	.20 - 28	.2836	.3644	.2432	.3038	.4048
	.14 - 24	.2232	.3040	.2028	.2836	.3745	.2634	.3543	.4252	.3038	.4048	.5058
Non-green and gravel landscaping	.3040	.3646	.4555	.4555	42 - 50	.5058	.4048	.4856	.5664	.4452	.5058	.6068
	.3444	.4252	.5060	.5060	48 - 56	.5765	.4654	.5563	.6472	.5058	.6068	.7078
Cemeteries, playgrounds	.2030	.2636	.3545	.3545	.3240	.4048	.3038	.3844	.4654	.3442	.4048	.50 - .58
	.2434	.3242	.4050	.4050	.3846	.4755	.3644	.4553	.5462	.4048	.5058	.60 - .68

NOTES: 1.

RATIONAL METHOD RUNOFF COEFFICIENTS (Modified from Table 4, UC-Davis, which appears to be a modification of work done by Rawls)

Values above and below pertain to the 2-year and 100-year storms, respectively.

The range of values provided allows for engineering judgement of site conditions such as basic shape, homogeneity of surface type, surface depression storage, and storm duration. In general, during shorter duration storms (Tc ≤ 10 minutes), infiltration capacity is higher, allowing use of a "C" value in the low range. Conversely, for longer duration storms (Tc ≥ 30 minutes), use a ""C value in the higher range.

For residential development at less than 1/8 acre per unit or greater than 1 acre per unit, and also for commercial and industrial areas, use values under MISC SURFACES to estimate "C" value ranges for use.

^{3.}

	TABLE "A-1" INTENSITY-DURATION-FREQUENCY (IDF) TABLE											
Time (min)	2-Year Intensity (in/hr)	100-Year Intensity (in/hr)	Time (min)	2-Year Intensity (in/hr)	100-Year Intensity (in/hr)							
5	1.95	(4.95)	33	0.83	2.15							
6	1.83	4.65	34	0.82	2.12							
7	1.74	4.40	35	0.81	2.09							
8	1.66	4.19	36	0.80	2.06							
9	1.59	3.99	37	0.79	2.03							
10	1.52	3.80	38	0.78	2.00							
11	1.46	3.66	÷39	0.77	1.97							
12	1.41	3.54	40	0.76	1.94							
13	1.36	3.43	41	0.75	1.91							
14	1.32	3.33	42	0.74	1.88							
15	1.28	3.24	43	0.73	1.85							
16	1.24	3.15	44	0.72	1.82							
17	1.21	3.07	45	0.71	1.79							
18	1.17	299	46	0.70	1.76							
19	1.14	2.91	47	0.69	1.73							
20	1.11	2.84	48	0.68	1.70							
21	1.08	2.77	49	0.67	1.67							
22	1.05	2.70	50	0.66	1.64							
23	1.02	2.63	51	0.65	1.61							
24	1.00	2.57	52	0.64	1.59							
25	0.98	2.51	53	0.63	1.57							
26	0.96	2.46	54	0.62	1.55							
27	0.94	2.41	55	0.61	1.53							
28	0.92	2.36	56	0.60	1.51							
29	0.90	2.31	57	0.59	1.49							
30	0.88	2.27	58	0.58	1.47							
31	0.86	2.23	59	0.57	1.45							
32	0.84	2.19	60	0.56	1.43							
Source: Mesa	County 1991											

A-2 JUNE 1994

REPRODUCED FROM TABLE 2, LECTURE 2, DAY 2, ACOE 1990

SURFACE	N VALUE	SOURCE	
ASPHALT/CONCRETE	0.05 - 0.15	Developed Use D. OTB Histor	
BARE PACKED SOIL FREE OF STONE	0.10	1 000 E	
FALLOW - NO RESIDUE	0.008 - 0.012	1 , AB // /	
CONVENTIONAL TILLAGE - NO RESIDUE	0.06 - 0.12	Use O.Ul B MisYor	i e
CONVENTIONAL TILLAGE - WITH RESIDUE	0.16-0.22	В	
CHISEL PLOW - NO RESIDUE	0.06 - 0.12	В	
CHISEL PLOW - WITH RESIDUE	0.10 - 0.16	В	
FALL DISKING - WITH RESIDUE	0.30 - 0.50	В	
NO TILL - NO RESIDUE	0.04 - 0.10	В	
NO TILL (20-40 PERCENT RESIDUE COVER)	0.07 - 0.17	В	
NO TILL (60-100 PERCENT RESIDUE COVER)	0.17 - 0.47	В	
SPARSE RANGELAND WITH DEBRIS:			
O PERCENT COVER	0.09 - 0.34	В	
20 PERCENT COVER	0.05 - 0.25	В	
SPARSE VEGETATION	0.053 - 0.13	F	
SHORT GRASS PRAIRIE	0.10 - 0.20	F	
POOR GRASS COVER ON MODERATELY	0.30	С	
ROUGH BARE SURFACE			
LIGHT TURF	0.20	A	
AVERAGE GRASS COVER	0.4	С	
DENSE TURF	0.17 - 0.80	A, C,E, F	
DENSE GRASS	0.17 - 0.30	D	
BERMUDA GRASS	0.30 - 0.48	D	
DENSE SHRUBBERY AND FOREST LITTER	0.4	A	

"N" values provided in this table pertain to both the SCS TR-55 "To" and FHWA 1984 HEC-12 "To" methods

A) CRAWFORD AND LINSLEY (1966).

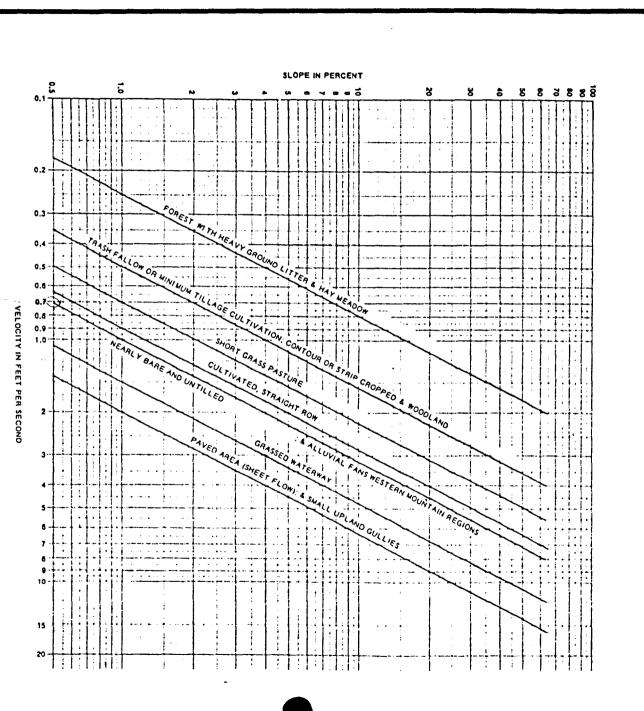
B) ENGMAN (1986).

C) HATHAWAY (1945).

D) PALMER (1946).

E) RAGAN AND DURU (1972).

F) WOOLHISER (1975).



DETERMINATION OF

"SI"

JUNE 1994

FIGURE "E-3"

E-9

United States Department of Agriculture

Soil Conservation Service

Engineering Division

Technical Release 55

June 1986



Urban Hydrology for Small Watersheds



Appendix A: Hydrologic soil groups

Soils are classified into hydrologic soil groups (HSG's) to indicate the minimum rate of infiltration obtained for bare soil after prolonged wetting. The HSG's, which are A, B, C, and D, are one element used in determining runoff curve numbers (see chapter 2). For the convenience of TR-55 users, exhibit A-1 lists the HSG classification of United States soils.

The infiltration rate is the rate at which water enters the soil at the soil surface. It is controlled by surface conditions. HSG also indicates the transmission rate—the rate at which the water moves within the soil. This rate is controlled by the soil profile. Approximate numerical ranges for transmission rates shown in the HSG definitions were first published by Musgrave (USDA 1955). The four groups are defined by SCS soil scientists as follows:

Group A soils have low runoff potential and high infiltration rates even when thoroughly wetted. They consist chiefly of deep, well to excessively drained sands or gravels and have a high rate of water transmission (greater than 0.30 in/hr).

Group B soils have moderate infiltration rates when thoroughly wetted and consist chiefly of moderately deep to deep, moderately well to well drained soils with moderately fine to moderately coarse textures. These soils have a moderate rate of water transmission (0.15-0.30 in/hr).

Group C soils have low infiltration rates when thoroughly wetted and consist chiefly of soils with a layer that impedes downward movement of water and soils with moderately fine to fine texture. These soils have a low rate of water transmission (0.05-0.15 in/hr).

Group D soils have high runoff potential. They have very low infiltration rates when thoroughly wetted and consist chiefly of clay soils with a high swelling potential, soils with a permanent high water table, soils with a claypan or clay layer at or near the surface, and shallow soils over nearly impervious material. These soils have a very low rate of water transmission (0-0.05 in/hr).

In exhibit A-1, some of the listed soils have an added modifier; for example, "Abrazo, gravelly." This refers to a gravelly phase of the Abrazo series that is found in SCS soil map legends.

Disturbed soil profiles

As a result of urbanization, the soil profile may be considerably altered and the listed group classification may no longer apply. In these circumstances, use the following to determine HSG according to the texture of the new surface soil, provided that significant compaction has not occurred (Brakensiek and Rawls 1983):

HSG Soil textures

- A Sand, loamy sand, or sandy loam
- B Silt loam or loam
- C Sandy clay loam
- D Clay loam, silty clay loam, sandy clay, silty clay, or clay

Drainage and group D soils -

Some soils in the list are in group D because of a high water table that creates a drainage problem. Once these soils are effectively drained, they are placed in a different group. For example, Ackerman, soil is classified as A/D. This indicates that the drained Ackerman soil is in group A and the undrained soil is in group D.

Exhibit A-1, continued: Hydrologic soil groups for United States soils

BELMONT		BERTRAM		BILLINGS.		BLACKNOLL		BLUE LAKE	•
BELMORE		BERTRAND	8 1			BLACKOAR		BLUE STAR	8
BELPRE	C 1	BERVILLE	0/01			BLACKPIPE		BLUEBELL	c
HELSAC	8	BERVOLF	8	BILLYCREEK	c 1			BLUECHIEF	C
BELTED	0 1	BERYL	8 1	BILLYHAW	DI		a !		D
BELTON	C 1	BERZATIC	0 1	BILTPORE	A I	BLACKSAN	8		c
BELTRAMI	8 1	BESEMAN	A/D	BINNEP	D [PLACKSPAR	0 1	BLUEFLAT	c
BELTSVILLE	C 1	BESHERM	c !	PINCO	D	BLACKSPOT	0 !	BLUEGROVE	c
BELUGA		BESNER	8 1	BIFOLE	e 1	BLACKSTON	8 I	BLUEGULCH	Ð
BELUGA. DRAINED.		BESSEHER	č i	-	a i	BLACKTHORN	e 1	BLUEHILL	c
	, ,		0 1			BLACKTOP	0 1	BLUEHON	c
SLOPING			- ,	BINGHAM	6 I		ōi		8
BELVOIR		BESTROM						BLUENOSE	6
BELZAR		BETHANY		BINGHAMPTON		PLACKVELL			
BEMIDJI	A		B	-:		BLADEN	- •	BLUEPOINT	A .
BEN LOHOND	8 I	BETHERA	0 1		0 1	= -	0 1	-	c
BENCHLEY	c !	BETHESDA	c 1	BINNSYILLE	0 1	ELAGO /		BLUESLIDE	D
BENCLARE	c I	BETHLEHEM	8	BINS	8 1	BLAINE !	C 1	= -	c
BENCO	в 1	BETIS	A 1	BINTON	c 1	BLAIR	C I	BLUESTONE	0
BENDER	e 1	BETONNIE	8 1	BINTON. RECLAIMED	B 1	6LAIRTON	c I	BLUEWING	A
BENDIRE		BETHA	c i	BIOYA	6 I	BLAKABIN	c !	BLUFF	0
BENEVOLA		PETIERAVIA	c i	BIPPUS	8 1	PLAKE	8 1	BLUFFDALE	C
BENEVAH	o i			BIRCHBAY	-	BLAKELAND	A I	BLUFFION	C/0
		=	В		ò i		c i		c
BENFIELD		BEULAH		BIRCHWOOD	ć 1			BLUM	č
BENGAL		BEVENT							ò
BENGE	9 1		0 1		B			BLY	
BENHAM	8 1	BEVERLY	8 !			BLAMER	c I		Ð
BENIN	D	BEVERLY. GRAVELLY	A !	BIRDSALL	DΙ	BLANCA	8 1		0
BENITO	0 1	8E¥	C 1	ORDB20RI B	F	BLANCHARD	A 1	BOARDHAN	D
MIMALMER	0 1	BEWLEYVILLE	0 1	BIRDSLEY	D 1	BLANCHE	8 t	BOARDTREE	Ç
BENKL IN	c i		0	BIRDSVIE	A 1	BLANCHESTER	8/01	BOASH	٥
BENMAN		BEZO	0 1		8 j		8 1	BOAZ	c
BENNDALE		BEZZANT	8		8 1		C 1	8088177	c
			c		8 1			BOBILLO	A
BENNINGTON		8 188					-	608M808	Ĉ
BENRIDGE		HIBLESPRINGS		BIROME		BLANEY			٥
BENSLEY	- •	BICE	9 1		A			608 S	
BENSON		BICKERDYKE	0 1		D I		A 1		c
BENTEEN	C 1	BICKETT	0 1	BISCAY	e/01	BLANTON.	8 1		в
BENYY	8 1	BICKLETON	8 1	BISGANI.	9]	HODERATELY VET	ı		8/0
BENZ	D 1	BICKHORE	c	MODERATELY WET	1	BLANYON	C I	BOCA - DEPRESSIONAL	. Р
BEOR	D 1	BICONDOA	0 1	BISGANI. FLOODED	c 1	BLAPPERT	0 1	BOCA. TIDAL	O
BEOSKA	8 j	BICONDOA. DRAINED	c 1	BISHOP	0 1	BLAQUIERE	c i	BOCK	8
BEOTIA		BIDDEFURD		BISMARCK		BLASDELL	ĀÌ	BOCKER	D
		BIDDLEMAN	9 1			BLASE		BOCKSTON	8
REDATAE	-		-			BLASINGAME	è i		5
BEQUINN	-	BIDMAN		BISPING				BODECKER	Ā
BENCUMB	8 1		8 1		e 1				
BERDA		BIEBER	-	BISSONNET	0		0 1		0
BEREA		BIEDELL		811	c i		D		c
BERENICETON	в :	PIEDSAN		BITTER	D		C 1		8
BERGHOLZ	c 1	BIENVILLE	A]	BITTER SPRING	B	6LEDSOE	C I	BODINE	
BERGLAND	D 1	BIG BLUE	0 1	# ITTERROOT	C 1	BLEIBLERVILLE	0 1	8000RUMPE	C
9ERGOUIST	8 1	BIG HORN	8 1	BITTERWATER	B]	6LENCOE	0 1	60001	C
BERGS TROM	8 1	RIG TIMBER	0 1	BITTON	8 1	BLEND	0 1	BOEL	4
BERGSVIK	0 1	BIGARM	9 1	BIVANS	0 1	BLENDON	B 1	BOEL . OVERWASH	C
BERINO	. B i	BIGBEE	A 1	n ixe y	6 1	BLETHEN	e i	BOELUS	A
BERIT		BIGBEND	8 j	BIXLER	ci	BLEVINS	8 1	BOERNE	8
BERKS		BIGBROWN		BJORK	- :	BLEVINTON	B 1		c
BERKSHIRE			a i		6 1		D 1		ō
	8 1		8 1		8 1		o i	BOETTCHER	č
SERLAKE	-	BIGFLAT	D 1		0 1		8 1		č
BERLIN					- ,				
BERMESA	c i		c 1			86140	8 1		-
BERHUDIAN	8 1		CI			BLIMSTER	¢ !		c
BERNAL	0 1			PLACK RIDGE		BLINN		BOCCA	C
BERNALDO		BIGHILL		BLACKA		BLISS		BOGRAP	8
BERNARD		BIGLAKE		BLACKSJRN		BLITZEN		BOGUE	0
BERNAROINO	c i	BIGMEADOW		BLACKDRAW	0 1	BLOCKHOUSE		80GUS	c
BERNARDSION	ς 1	BIGNELL .	c 1	ELACKETT	8 1	BLO⊭FORD	8/01	BOHANNON	C
BERNHILL	8 1	BIGRIVER	8	BLACKFOOT	(I	PLOOM	D 1	HAIMSHOE	8
BERNICE		BIGSHEEP	8 1	BLACKFOOT. DRAINED	0 1	BLOOMFIELD	A 1	BOHICKET	0
BERNING		BIGSPRING		BLACKHALL		ELOONING	a i	BOHNA	B
BERNOV		BIGHTN		BLACKHALL. VARM		BLOOMSDALE		BOHNL Y	D
BERRYLAND		BIGWINDER		BLACKHAMMER		BLOOR		BUHNSACK	8
BERRYHAN		AL ROLL B		BLACKHAWK		BLOOR. GRAVELLY		BOISTFORT	8
BERSON				BLACKHOOF					8
		81,00				SUBSTRATUM		BOJAC	
BERTAG		811.80		eLACKHORSE		BLOUNT		8CJ0	0
BERTELSON		BILGER		BLACKLEED		BLOVERS		BOLAN	8
BERTHOUD	-	BILLETT		BLACKLEG		BLUCHER		POLAR	C
8E911E		BILLINGS		BLACKLOCK		BLUE EARTH		BOLD	8
BERTO	0 1			BENCKMAN		BLUE EARTH.		BOLENT	A
6ERTCLOTTI	8 1		1	BLACKMOUNT	B 1	SLOPING	- 1	BOLES	Ç

MOTES: TWO HYDROLOGIC SOIL GROUPS SUCH AS B/C INDICATES THE DRAINED/UNDRAINED SITUATION.
MODIFIERS SHOWN. E.G.. BEDROCK SUBSTRATUM. REFER TO A SPECIFIC SOIL SERIES PHASE FOUND IN SOIL MAP LEGEND.

REPORT CHECKLIST AND OUTLII

PRELIMINARY DRAINAGE REPORT

_		
CHECKLIST	ОК	NA
Typed text		
8½ x 11" format		
Bound: Use bar or spiral binder or staple. Do not use a notebook.		
Title Page: Name of report and preparer, date of preparation and revision (if any)		
Exhibits: Maximum 11" high and 32" wide, bound in report and folded as required to 8½ x 11" size		
Maps attached to or contained in the report: Vicinity Map and Preliminary Major Basin Drainago Map		

OUTLINE

- GENERAL LOCATION AND DESCRIPTION
 - Site and Major Basin Location
 - 1. Streets in the vicinity
 - 2. Development in the vicinity
 - B. Site and Major Basin Description
 - 1. Acreage

 - Ground cover types
 Hydrologic soil types
- **EXISTING DRAINAGE CONDITIONS**
 - A. Major Basin
 - 1. General topography, drainage patterns and features, canals, ditches, wetlands
 - 2. Previously determined 100-year floodplains
 - B. Site
 - 1. Historic drainage patterns
 - 2. Inflow characteristics from upstream
 - 3. Discharge characteristics to downstream sub-basins
- III. PROPOSED DRAINAGE CONDITIONS
 - A. Changes in Drainage Patterns
 - Major basin 1.
 - 2. Site
 - B. Maintenance Issues
 - 1. Access
 - 2. Ownership and responsibility
- IV. DESIGN CRITERIA & APPROACH
 - A. General Considerations
 - Previous drainages studies performed for the area
 - Master planning issues (large scale considerations)
 - Constraints imposed by site and other proposed development
 - B. Hydrology
 - 1. Design storms and precipitation
 - 2. Runoff calculation method
 - 3. Detention/retention basin design method
 - 4. Parameter selection procedures
 - 5. Analysis and design procedures
 - 6. Justification of proposed methods not presented or referenced in SWMM
 - B. Hydraulics
 - 1. Hydraulic calculation methods
 - 2. Parameter selection procedures
 - 3. Analysis and design procedures
 - Justification of proposed methods not presented or referenced in SWMM

COMMENTS

- No calculations are required for the Preliminary Drainage Report.
- It may not be necessary to cover all of the above topics, but the report should address all concerns applicable to the proposed project, even issues not identified above.

SOIL SURVEY

Grand Junction Area, Colorado



Series 1940, No. 19

Issued November, 1955

UNITED STATES DEPARTMENT OF AGRICULTURE

Soil Conservation Service

In cooperation with the

COLORADO AGRICULTURAL EXPERIMENT STATION

Billings silty clay loam, 0 to 2 percent slopes $(B\varepsilon)$.—This soil, locally called adobe, is one of the most important and extensive in the Grand Valley. It covers nearly one-fifth of the Grand Junction Area. The areas occur on the broad flood plains and very gently sloping coalescing alluvial fans along streams. Many large areas are north of the Colorado River.

The soil is derived from deep alluvial deposits that came mainly from Mancos shale but in a few places from fine-grained sandstone materials. The deposits ordinarily range from 4 to 40 feet deep but in places exceed 40 feet. The deposits have been built up from thin sediments brought in by the streams that have formed the coalescing alluvial fans or have been dropped by the broad washes that have no drainage channel. The thickest deposit, near Grand Junction, was built up by Indian Wash.

The color and texture of the soil profile vary from place to place. The S- to 10-inch surface soil normally consists of gray, light-gray, light olive-gray, or light brownish-gray silty clay loam. This layer grades into material of similar color and texture that extends to depths of 3 or 4 feet. Below this depth the successive depositional layers show more variation. Although the dominant texture is silty clay loam, the profile may have a loam, clay loam, fine sandy loam, or a very fine sandy loam texture.

Where there are fairly uniform beds of Mancos shale and where the soil is not influenced by materials deposited by adjoining drainage courses, the profile varies only slightly within the upper 3 or 4 feet. In areas bordering drainage courses, however, the soil varies more in

texture and color from the surface downward.

One small area about 112 miles southeast of Loma consists of light gravish-brown or pale-brown heavy silty clay loam that shows only slight variation in texture to depths of 4 to 6 feet. The underlying soil material is more variable. Below depths of 6 to 10 feet the layers generally are somewhat thicker and have a higher percentage of coarse soil material.

Also included with this soil are several small areas totaling about 3 square miles that are dominantly pale yellow. These are located 2½ to 3½ miles northeast of Fruita, 5 miles north of Fruita, 2½ miles northeast of Loma, 3 to 5 miles north of Loma, 11/2 miles northwest of Loma, and 4 miles northwest of Mack. In these areas the 8- or 10-inch surface soil is pale-yellow silty clay loam, and the subsoil is a relatively uniform pale-yellow silty clay loam to depths of 4 to 8 feet. The accumulated alluvial layers are difficult to distinguish, but in a few places transitional to Fruita soils there are small areas having a pale-brown to light-yellowish brown color. These transitional areas are included with Billings silty clay loam because they have a finer textured subsoil than is characteristic of the Ravola soils.

Although moderately fine textured, this Billings soil permits successful growth of deep-rooted crops such as alfalfa and tree fruits. Its permeability is normally not so favorable as that of the Mesa, Fruita, and Ravola soils. Its tilth and workability are fair, but it puddles so quickly when wet and bakes so hard when dry that good tilth can be maintained only by proper irrigation and special cultural practices. Runoff is slow and internal drainage is very slow.

Like all other soils in the area, this one has a low organic-matter content. Under natural conditions it contains a moderate concentration of salts derived from the parent rock (Mancos shale). In places, however, it contains so much salt that good yields cannot be obtained. Some large areas are so strongly saline they cannot be used for crops. Generally, this soil is without visible lime, but it is calcareous. In many places small white flecks or indistinct lightcolored streaks or seams indicate that lime, gypsum, or salts are

Use and management.—About 80 percent of this soil is cultivated. The chief irrigated crops are alfalfa, corn, dry beans, sugar beets, small grains, and tomatoes and other truck crops. Where the soil is

located so as to avoid frost damage, tree fruits are grown.

Most of the field crops are grown in the central and western parts of the valley, or from Grand Junction westward. The entire acreage in tree fruits-approximately 3 square miles-lies between Grand Junction and Palisade. Because the climate is more favorable near Palisade, the acreage in orchard fruits is greater there. A few small orchards are located northeast of Grand Junction in the direction of Clifton. The main fruit acreage is between Clifton and Palisade. Peach orchards predominate, but a considerable acreage is in pears, especially near Clifton. Yields depend on the age of the trees and other factors, including management, but the estimated potential yield is somewhat less on this soil than on Mesa soils. This takes into account the slower internal drainage of this soil and its susceptibility to salinity if overirrigated. Yields of other crops vary according to the length of time the land has been irrigated, internal drainage or subdrainage, salt content of the soil, management practices, and local climate.

The uncultivated areas of this soil are mostly inaccessible places adjoining the larger washes, which occur mainly in the western part of the area, and those places that cannot be cropped profitably because they have inadequate drainage and a harmful concentration of salts. The uncultivated land supports a sparse growth of greasewood, saltbush, shadscale, rabbitbrush, rvegrass, peppergrass, and saltgrass. From 70 to 90 acres are required to pasture one animal

during a season.

A number of places shown on the map by small marsh symbols are low and seepy. They could be ditched, but their acreage is likely too small to justify the expense. Left as they are, their salt content

makes them worthless for any use except pasture.

Sizeable acreages of this soil apparently were overirrigated in the past. Irrigation water applied at higher levels to the north seeps upward in this soil where it occurs in low areas toward the river. Even now, new saline areas are appearing, and existing areas are getting larger. The total acreage affected by salts has remained more or less the same for the last two decades, but affected areas will continue to change in size and shape because of seepage.

Most fields are ditched where necessary. Some uncultivated areas require both leveling and ditching. In places subdrainage is inadequate because irregularities in the underlying shale tend to create pockets and prevent underground water from flowing into the drainage ditches. Also, in some areas where the alluvial mantle is 30 to 40 feet thick, the ditches are not always deep enough to drain the soil. Some areas are seepy because there are no ditches running in an east-west direction to intercept lateral flow of ground water from the overirrigated, permeable, medium-textured, stratified soils on the upper parts of the fan to the north. After being leveled, uncultivated areas would have to be cropped for 3 years before their salt content would be reduced enough to permit good yields.

Farmers can increase the organic-matter content of this soil by applying manure liberally and by growing alfalfa or clovers at least part of the time. A combination field crop and livestock type of farming favors improvement of this soil. Many of the small imperfectly drained areas may be kept in pasture. Strawberry clover and sweetclover are well suited, and mixtures of pasture grasses grow well.

Billings silty clay loam, 2 to 5 percent slopes (BD).—This soil covers a relatively small acreage in the Grand Valley. The areas are widely scattered. Except for its stronger slope, the soil is almost the same as Billings silty clay loam, 0 to 2 percent slopes. In a few places, notably north of Loma, there are areas having a pale-yellow color rather than the gray typical of the Billings soils.

Use and management.—Only about 15 percent of this soil is cultivated. Many of the areas lie along large drainageways or washes where they are difficult to reach. Even a larger number have such an uneven surface that considerable leveling would have to be done before they could be cropped. The cost of leveling, together with the expense of controlling crosion and gullying, discourages farmers from using them.

Many of the uncultivated areas have moderate concentrations of salts, but they are not particularly difficult to reclaim because they border natural ditches or washes which afford free disposal of irrigation water. Furthermore, for the most part, they have a porous substratum.

About the same crops are grown on this soil as on Billings silty clay loam, 0 to 2 percent slopes. The average yields are approximately the same.

Billings silty clay, 0 to 2 percent slopes (Ba).—This soil, locally called heavy adobe, occurs well toward the Colorado River. It is on alluvial materials—4 to about 40 feet thick—that largely came from Mancos shale. Most of this soil lies east and southeast of Grand Junction and along the railroad between Grand Junction and Fruita.

The 8- or 10-inch surface soil consists of light brownish-gray, gray, or olive-gray silty clay. The layer is similar to the surface layer of Billings silty clay loam soils but it is harder and, in many places, darker. The subsoil consists of similarly colored layers of silty clay loam, silt loam, and silty clay. In places the soil is silty clay to depths exceeding 4 feet.

The entire profile is firm when moist and has a massive structure. The subsoil has many small irregularly shaped light-gray specks or indistinct mottles. Poorly defined light-colored streaks indicate the presence of lime, gypsum, or salts. The surface soil and subsoil are calcareous, the lime being well distributed. The fine texture of the soil greatly retards penetration of roots, moisture, and air.

Surface runoff is very slow to slow where the slope is less than 1 percent. Internal drainage is very slow because the subsoil is massive and very slowly permeable. Even with ample drainage ditches, the discharge of irrigation water is slow.

Tilth and workability are not good, because the soil has a fine texture and a low content of organic matter. Moreover, some fields contain areas 20 to 60 feet across that have excessive amounts of salts. Slick spots also occur. These salty areas and slick spots produce low or negligible yields of most crops and are extremely difficult to climinate.

Use and management.—About 75 percent of this soil is cultivated. Most of the rest is affected by salts. Small grains, beans, sugar beets, and alfalfa are the chief crops. They yield less than on Billings silty clay loam, 0 to 2 percent slopes. Ordinarily, newly broken fields are cropped to oats or other small grains the first few seasons so that excess salts can be removed. Afterwards, if drainage is adequate, they may be planted to pinto beans, sugar beets, corn, or alfalfa. The very slow permeability of this soil makes it unsuitable for orchard crops. Also, it is located mainly in areas where the frost hazard is great. Probably the greater part of the irrigable acreage is used for sugar beets. Small grains, alfalfa, and pinto beans usually follow in the order named.

Billings silty clay, 2 to 5 percent slopes (BB).—This soil is similar to Billings silty clay, 0 to 2 percent slopes. It differs mainly in having greater slopes and a slightly finer textured and darker gray surface soil. In places, below depths of 3 or 4 feet, the silty clay or clay material is light olive gray.

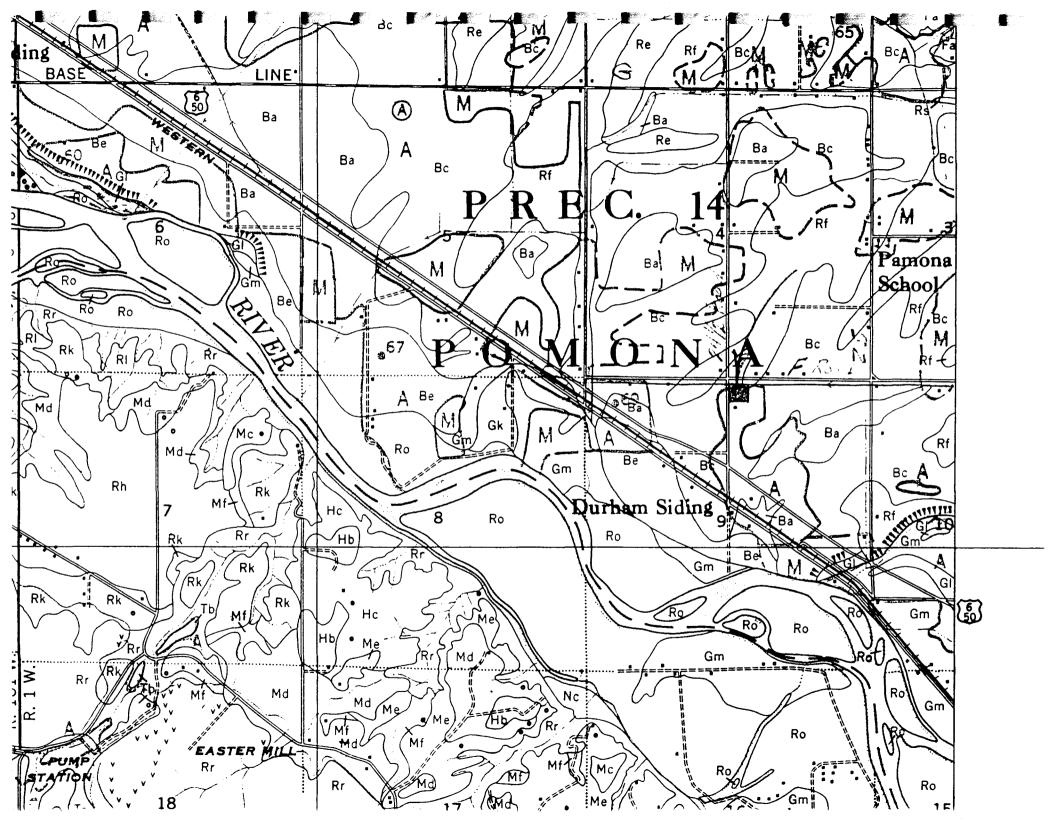
The tilth and workability are poor. Surface runoff is medium, and internal drainage is very slow. The soil is better suited to irrigation than most of the larger nearly level areas of Billings silty clay, 0 to 2 percent slopes, many of which are affected by salts. Approximately 12 acres of this soil is in peach orchards. All the rest is normally used for cultivated crops, principally corn, pinto beans, and alfalfa. This soil is suited to about the same crops as Billings silty clay, 0 to 2 percent slopes, but it generally produces better yields.

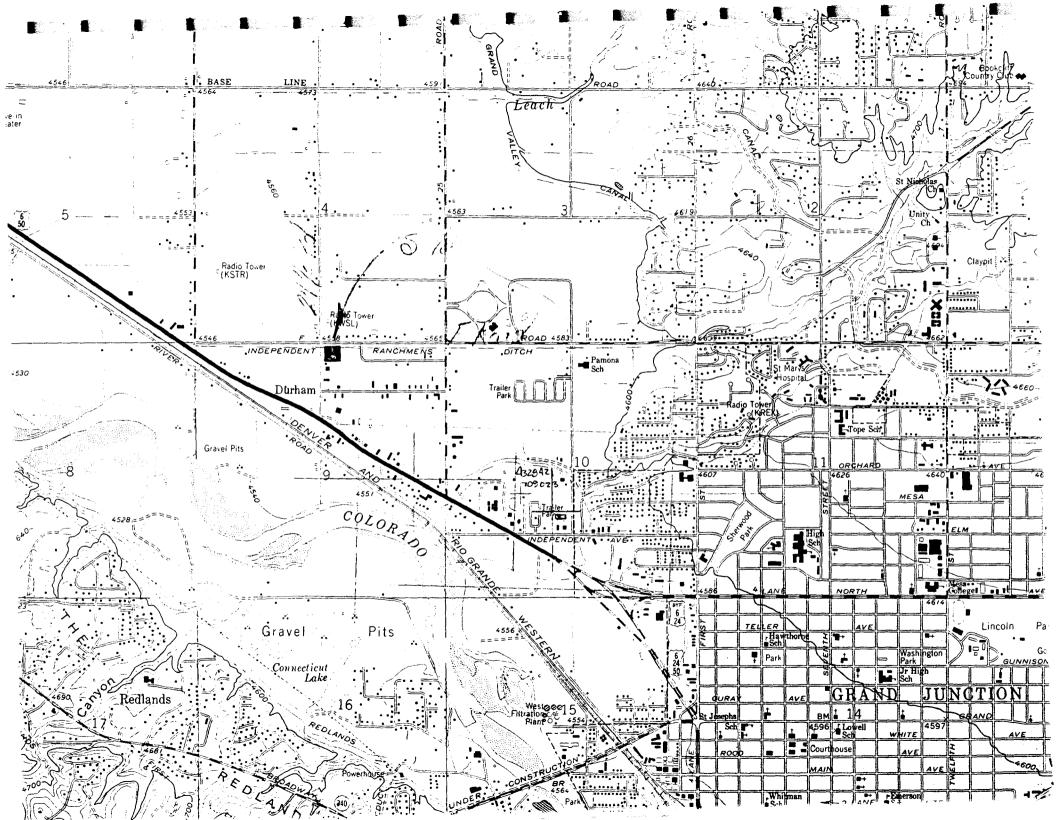
Billings silty clay, moderately deep over Green River soil material, 0 to 2 percent slopes (BE).—This soil occurs on the outer margin of coalescing alluvial fans where 1 to 4½ feet of fine-textured deposits derived from shale overlies Green River soil materials.

Except for a few strips only a few rods wide that adjoin low-lying areas of Green River soils, this soil has not been altered by high overflows from the Colorado River. It is not likely that the main part of the soil will be covered by floodwaters from the Colorado River, as it lies well above the level of normal overflow.

Use and management.—About 85 percent of this soil is cultivated. The principal crops are alfalfa, corn, sugar beets, and pinto beans. A few peach orchards are on this soil near Clifton. Because the underlying strata are coarser, crops produce better on this soil than on most areas of the other Billings silty clay soils. Drainage and saline conditions have to be corrected before the soil will produce well

Uncultivated acreages of this soil northwest of Grand Junction are saline, imperfectly drained, or both. Their tilth and workability are poor because they have a fine texture and a low content of organic matter.





BARNES AND NOBLE RETAIL STORE

Engineers Opinion of Cost

DATE:

8/20/96

NAME OF DEVELOPMENT:

BARNES AND NOBLE RETAIL STORE

LOCATION

NW/4 NE/4 SEC 9, T.1 S., R.1 W., U.P.M

PRINTED NAME OF PERSON PREPARING: JOHN C. SMITH

CONST	RUCTION COST ESTIMATE:			Unit	Total
,	Water system:	Units	Quant	Price	Price
1	8" Line Connect	EA	1	175.00	175
2	8" Fittings w/TB's	EA	5	180.00	900
3	6" Fittings w/TB's	EA	3	150.00	450
4	8" G.V.'s and box's	EA	2	500.00	1,000
5	6" G.V.'s and box's	EA	2	400.00	800
6	Fire Hydrant Assymblies	EA	2	1,200.00	2,400
7	8" PVC Waterline	$_{ m LF}$	543	12.00	6,516
8	6" PVC Waterline	$_{ m LF}$	110	10.50	1 , 155
9	2 1/2" Fittings	EA	2	100.00	200
10	2 1/2" Serv. Line	$_{ m LF}$	26	9.50	247
			Sub-total 1	Potable Water:	13,843
				Unit	Total
	Sewer system:	Units	Quant	Price	Price
1	8" Line Connect	EA	1	220.00	220
2	8" PVC Sewer Main	$_{ m LF}$	473	15.00	7,095
3	San. Sewer Manhole	EA	3	1,150.00	3,450
4	Drop Manhole Connection	EA	1	350.00	350
5	San. Sewer Serv. Tap	EA	1	50.00	50
6	6" Service Line	LF	90	10.00	900
7	Service Cleanout	EA	1	650.00	650
			Sub-total S	anitary Sewer:	12,715
				Unit	Total
	Site grading and paving	Units	Quant	Price	Price
1	Unclassified Excavation	CY	230.00	1.50	345
2	Unclassified Embankment	CY	2100.00	9.00	18,900
3	Asphalt Removal	SY	85.00	3.00	255
4	Asphalt Patch	SY	85.00	30.00	2,550
5	Class-6 A.B.C.	CY	1015.00	16.50	16,748
6	3" Asphalt	SY	4736.00	5.50	26,048
7	6" Barrier Curb (1'Height)	LF	895.00	11.00	9,845
8	2' Standard Curb/Gutter	$_{ m LF}$	202.00	11.00	2,222
9	2.5' Drive Over Curb/Gutter	$_{ m LF}$	110.00	12.50	1,375
10	2.5' Door Walk	LF	162.00	10.00	1,620
11	5' Conc Walk	$_{ m LF}$	516.00	15.00	7,740
12	8' Conc Walk	LF	118.00	24.00	2,832
13	1' Drive Over Curb	LF	350.00	11.50	4,025
14	3' Conc. V-Pan	LF	280.00	16.00	4,480

15	6"Barrier Curb	FSF	400.00	11.50	4,600
	(Variable Height)			•	
16	Landscape Wall	FSF	350.00	17.50	6,125
17	8" Conc Driveway	SY	87.00	60.00	5,220
18	6" Conc Loading Ramp	SY	50.00	55.00	2,750
19	Conc Filled Bollard	EA	2.00	100.00	200
20	Conc Deco X-Walk	SY	72.00	35.00	2,520
21	Stop Sign	EA	2.00	200.00	400
22	Pavement Striping	GAL	6.00	90.00	540
23	6"Conc Entrance Ramp	SY	28.00	55.00	1,540
24	8"Conc Trash/Loading Pad	SY	104.00	60.00	6,240
25	PSCO Relocates	LS	1.00	4000.00	4,000
		Sub-t	otal Site gradir	ng and paving:	133,120
				Unit	Total
1	Orainage	Units	Quant	Price	Price
1	Connect to Outfall pipe	LS	1.00	800.00	800
	(with Grout Collar)				
2	18" PVC Storm Sewer	LF	41	20.00	820
3	12" PVC Storm Sewer	LF	10	25.00	250
4	Rip-Rap	SY	20.00	50.00	1,000
5	Orifice Control Outlet Struc	t EA	1.00	1,500.00	1,500
6	Curb Inlet	EA	1.00	1,500.00	1,500
			Sub-1	otal Drainage:	5,870
				Unit	Total
]	rrigation	Units	Quant	Price	Price
1	Connect to Existing	EA	1	500.00	500
3	12" PVC Irrigation	LF	500.00	12.00	6,000
7	12" Fittings	EA	6.00	250.00	1,500
25	Headwall/Frame/Grate	EA	1	1,500.00	1,500
25	Remove Concrete Lined Ditch	LF .	500	3.00	1,500
			Sub-t	otal Irrigation:	11,000
		Total S	ite Construc	tion Costs:	176,548

EXHIBIT "E'



27,092

BARNES AND NOBLE RETAIL STORE Engineers Opinion of Cost

DATE: NAME OF DEVELOPMENT: LOCATION 9/10/96

BARNES AND NOBLE RETAIL STORE

NW/4 NE/4, SEC 9, T.1 S., R.1 W., U.P.

PRINTED NAME OF PERSON PREPARING:

JOHN C. SMITH

cons	TRUCTION COST ESTIMATE:			Unlt	Total
				Unit	Total
	Site grading and paving	.Units	Quant	Price	Price
1	Unclassified Excavation	CY	65.00	2.00	1,30
2	Unclassified Embankment	CY	30.00	3.00	90
3	Class-6 A.B.C.	CY	48.00	16.50	792
4	5' Condrete Walk	LF	516.00	15.00	7,740
5	8" Concrete Driveway	SY	87.00	60.00	5,220
6	PSCO Relocates	LS	2	1,000.00	4,000
		Sub-ti	otal Site gradin	ig and paving:	17,972
		•		Unlt	Total,
	Drainage	Units	Quant	Price	Price
1		EA	1.00	00.00	800
2		\mathtt{LF}	41.00	20.00	820
3		SY	20,00	50.00	1,000
4	Retention Pond	LS	1.00	5,000.00	5,000
5		EA	1.00	1,500.00	1,500
	Dolacare		Sub-t	otal Drainage:	9,120

5, + Bul, LLC
By: TB Manage 9-24-96
ISNATURE OF DEVELOPER DATE

Total Off Site Construction Costs:

I have reviewed the estimated costs and time schedule shown above and, bas on the plan layouts submitted to date and the current costs of constructio take no exception to the above.

CITY ENGINEER	DATE
COMMUNITY DEVELOPMENT	DATE

EXRIBIT "5"

BARNES AND NOBLE RETAIL STORE Engineers Opinion of Cost

DATE:

9/27/96

NAME OF DEVELOPMENT:

BARNES AND NOBLE RETAIL STORE

LOCATION

NW/4 NE/4, SEC 9, T.1 S., R.1 W., U.P.M

PRINTED NAME OF PERSON PREPARING:

JOHN C. SMITE

CONSTRUCTION COST ESTIMATE:

RUCTION COST ESTIMATE:				_
•			Unit	Total
Site grading and paving	Units	Quant	Price	Price
Unclassified Excavation	CX	€5.00	2.00	130
Unclassified Embankment	CX	30.00	3.00	90
Class-6 A.B.C.	CA	46.00		792
5' Concrete Walk	LF	149.00		2,100
6' Concrete Walk	LF	384.00	17.00	6,528
8" Concrete Driveway	SY	95.00	60.00	5,700
PSCO Relocates	ĿS	1	4,000.00	4,000
	Sub-to	otal Site gradi	ng and paving:	19,340
Sewer System	Units	Quant	Price	Price
8" Line Connect	EA	1.00	220.00	220
8" PVC Sewer Main	LF	458.00	15.00	6, 87C
San Sewer Manhole	EA	2.00	1,150.00	2,300
Drop Manhole Connection	EA	1.00	350.00	350
San Sewer Serv Tap	EA `	1.00	50.00	50
6" PVC Sewer Service	LF.	90.00	10.00	9.00
Service Cleanout	ea	1.00	650,00	650
		Sub-t	otal Drainage:	11,340
			Unit	Total
Drainage	Unita	Quant	Price	Price
Connect to Outfall Pipe (with Grout Collar)	£Ά	1.00	500.00	800
	LF	41.00	20.00	820
-	SY	20.00	50.00	1,000
Retention Pond	LS	1.00	5,000.00	5,000
Orifice Controlled Outlet	EA	1.00	1,500.00	1,500
Structure		Sub-t	otal Drainage:	9,120
	Unclassified Excavation Unclassified Embankment Class-6 A.B.C. 5' Concrete Walk 6' Concrete Walk 8" Concrete Driveway PSCO Relocates Sewer System 8" Line Connect 8" PVC Sewer Main San Sewer Manhole Drop Manhole Connection San Sewer Serv Tap 6" PVC Sewer Bervice Service Cleanout Drainage Connect to Outfall Pipe (with Grout Collar) T8" PVC Storm Sewer Pipe Rip-Rap Retention Pond	Site grading and paving Unclassified Excavation Unclassified Embankment CY Class-6 A.B.C. 5' Concrete Walk 6' Concrete Walk 8" Concrete Driveway PSCO Relocates Sewer System Units 8" Line Connect 8" PVC Sewer Main San Sewer Manhole Drop Manhole Connection San Sewer Serv Tap 6" PVC Sewer Bervice LF Service Cleanout Drainage Connect to Outfall Pipe (with Grout Collar) T8" PVC Storm Sewer Pipe Rip-Rap Retention Pond Orifice Controlled Outlet EA	Site grading and paving Unclassified Excavation CY 65.00 Unclassified Embankment CY 30.00 Class-6 A.B.C. CY 48.00 5' Concrete Walk LF 140.00 6' Concrete Walk LF 384.00 8" Concrete Driveway SY 95.00 PSCO Relocates LS 1 Sub-total Site gradin Sewer System Units Quant 8" Line Connect EA 1.00 San Sewer Main LF 458.00 San Sewer Manhole EA 2.00 Drop Manhole Connection EA 1.00 San Sewer Serv Tap EA 1.00 Service Cleanout EA 1.00 Sub-total Site gradin Drainage Units Quant Connect to Outfall Pipe EA 1.00 (with Grout Collar) T8" PVC Storm Sewer Pipe LF 41.00 Rip-Rap Retention Fond LS 1.00 Orifice Controlled Outlet EA 1.00 Structure	Unit Site grading and paving

SIGNATURE OF DEVELOPER

DATE

39,800

I have reviewed the estimated costs and time schedule shown above and, bas on the plan layouts submitted to date and the current costs of constructio take no exception to the above.

CITY	ENGINEER	DATE
COMMI	INITY DEVELORMENT	Date

Total Off Site Construction Costs:

October 04, 1996

To whom it may concern:

The site for the proposed Barnes & Nobel Bookstore, located at 2450 Patterson Rd. (tax schedule #2945-091-00-118), is zoned C-2 (heavy commercial). According to Section 4-3-4, Use/Zone Matrix, of the Zoning and Development Code, Retail Business is an allowed use in the C-2 zone.

If you have any further questions regarding this information, please do not hesitate to call the City of Grand Junction Community Development Department at (970)244-1430.

Sincerely,

Senta Costello Planning Technician

REVIEW COMMENTS

Page 1 of 2

FILE #SPR-96-190

TITLE HEADING: Barnes & Noble

LOCATION:

SE corner 24 1/2 & Patterson Roads

PETITIONER:

Sittema-Bullock

PETITIONER'S ADDRESS/TELEPHONE:

5445 DTC Parkway, #4 Englewood, CO 80110

303-770-7275

PETITIONER'S REPRESENTATIVE:

Ciavonne & Associates

STAFF REPRESENTATIVE:

Michael Drollinger

NOTE: THE PETITIONER IS REQUIRED TO SUBMIT FOUR (4) COPIES OF WRITTEN RESPONSE AND REVISED DRAWINGS ADDRESSING ALL REVIEW COMMENTS.

CITY COMMUNITY DEVELOPMENT

Michael Drollinger

244-1439

- 1. Lighting Plan for parking lot is required as per Code.
- 2. Bicycle parking is required see Code; also provide bicycle rack detail.
- 3. Landscaping Plan is acceptable but will need to be resubmitted if site plan revisions are required.
- 4. "Stamped" concrete pedestrian decorative crosswalk detail not provided with Landscape Plan; please provide for review.
- 5. Please contact project planner (244-1439) regarding Comment #4 of the Grand Valley Irrigation Company BEFORE revising plans.

CITY DEVELOPMENT ENGINEER

9/4/96

Jody Kliska

244-1591

- 1. The city TEDS manual requires minimum on-site stacking distance at the driveway of 40'. It appears possible to reconfigure the parking near the driveway to meet this minimum stacking distance.
- 2. Please provide a copy of the common access agreement with the adjacent parcel.
- 3. Does the improvements agreement include the curb, gutter and sidewalk improvements on 24 ½ Road and on Patterson? Typically the improvements agreement covers the public improvements and drainage improvements.
- 4. Drainage report is acceptable.

CITY UTILITY ENGINEER

9/4/96

Trent Prall

244-1590

- 1. Please contact Jodi Romero of the City Customer Service Division at 244-1520 for information regarding sewer plant investment fees.
- 2. Please contact Dan Tonello with the Industrial Pretreatment section (244-1489) at the Persigo Sewer Treatment Plant for industrial waste review.
- 3. Public easement and sewer profile required for sewer extension.

SPR-96-190 / REVIEW COMMENTS / page 2 of 2

4. 6" sewer service line from Barnes and Noble building should terminate in a manhole at the 8" line.

CITY POLICE DEPARTMENT

9/3/96

Dave Stassen_

244-3587

Is there a lighting plan. Further comments can hold until a lighting plan is provided.

CITY FIRE DEPARTMENT

9/3/96

Hank Masterson

244-1414

The Fire Department has no problems with this proposal.

A complete set of sealed building plans must be submitted to the Fire Department for our review. Also, a complete set of plans, specifications, and calculations for the fire sprinkler system and fire alarm system must be submitted. Allow 10 working days for completion of plan reviews.

MESA COUNTY BUILDING DEPARTMENT

8/28/96

Bob Lee

244-1656

Need to submit 2 sets of sealed plans and allow 10-14 days for plan review and permit issuance. City licensed contractors are required to perform work.

GRAND VALLEY IRRIGATION

9/3/96

Phil Bertrand

242-2762

- 1. Need clear understanding of storm water into Independent Ranchman ditch (not sure it will be allowed).
- 2. A canal break and a unique storm on the Bookcliffs in the last 20 years has caused large amounts of run-off water to cross over this property.
- 3. Must show detailed design of Patterson Road access (this is not to interfere with canal access road).
- 4. NO permanent building/structures or plants within 30 feet of Independent Ranchman ditch or pipe.
- 5. Need clear documentation for crossing Independent Ranchman ditch.

UTE WATER

9/3/96

Gary Mathews

242-7491

- 1. Contact with Ute Water is needed to discuss the water line locations and back flow prevention for fire protection.
- 2. Water mains shall be c-900, class 150. Installation of pipe fittings, valves and services including testing and disinfection shall be in accordance with Ute Water standard specifications and drawings.
- 3. Construction plans required 48 hours before development begins.
- 4. Policies and fees in effect at the time of application will apply.

TO DATE, NO COMMENTS RECEIVED FROM:

Grand Junction Drainage District

August 20, 1996

Mr. Michael Drolinger Community Development Department City of Grand Junction 250 North 5th St. Grand Junction, CO. 81501

Dear Mr. Drolinger,

The following is a response to the review Agency Comments concerning the Barnes & Noble Site Plan Submittal, File #SPR-96-190.

Community Development Staff

- 1. Lighting Plan has been included. The isolux diagram as well as the electrical plan showing the locations has been included.
- 2. Bicycle parking has been shown on the Landscape Plan, included in this submittal, with a detail.
- 3. Landscaping has been adjusted to accommodate the entrance reconfiguration.
- 4. Detail for stamped concrete are shown on the site plan.
- 5. Grand Valley Irrigation has been contacted, and once the drainage system was explained, had no problems with the proposed system.

Community Development Engineer

- 1. Plans have been changed to reflect the stacking distance requirement.
- 2. Access agreement has been included with this submittal.
- 3. Improvement agreement has been included with this submittal.

Grand Junction City Utility Engineer

- 1. Thank you for direction concerning fees. City has been contacted concerning sewer investment fees.
- 2. Industrial pre-treatment has been addressed.
- 3. Easement is shown on the Site Plan. Sewer Profile has been included on Utilities composite.
- 4. Manhole has been added.

Grand Junction Police Department

1. Lighting Plan has been included.

Grand Valley Irrigation

- 1. Grand Valley Irrigation has been contacted, and once the drainage system was explained, had no problems with the proposed system.
- 3. Design of Patterson Road access an ditch crossing is shown.

Ute Water

1. Applicant has no problem with Ute requirements.

File #214-94

Sincerely,

Craig Roberts | Secretary/Treasurer Ciavonne & Associates, Inc.



September 24, 1996

Mr. Michael T. Drollinger City of Grand Junction 250 North 5th Street Grand Junction, CO 81501-2668

Re: Barnes & Noble Bookstore

Southeast Corner of 24 1/2 Road and F Road

Dear Michael:

I have enclosed two fully executed copies of the Development Improvements Agreement for the above referenced project. Thank you for sending me the form for this document.

I have attached, as Exhibit B, the cost estimate prepared by our civil engineer for the improvements anticipated by this Agreement. If the form and content of this document meets with your approval, I would appreciate your processing it for signature and returning one fully executed copy to me.

As a follow-up to a discussion we had last week, I would appreciate receiving from you a letter indicating that our proposed use of this site as a retail bookstore complies with the current zoning on this property. As I mentioned, our lender is requiring this "zoning compliance" letter.

Thank you again for all of your efforts on this project. I look forward to finalizing the planning process and beginning construction very shortly.

Sincerely,

SITBUL, LLC

A COLORADO LIMITED LIABILITY COMPANY

Timothy B. Sittema

Manager

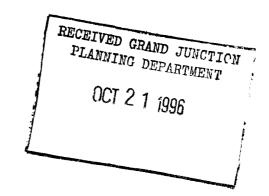
/dsm

Enclosures



October 18, 1996

Mr. Michael T. Drollinger City of Grand Junction 250 North 5th Street Grand Junction, CO 81501-2668



Re:

Barnes & Noble Bookstore

Dear Michael:

I have enclosed three (3) executed copies of Exhibit B to the Development Improvements Agreement which we had discussed. It is my understanding that you will insert this revised Exhibit B into the documents that I have previously signed and forwarded to you. It is also my understanding that as this work is constructed by our General Contractor, he will be invoicing the City for this work and you will use these proceeds for payment to him. If this is not the case let me know.

I will forward a check in this amount next week. We look forward to getting going on our construction and sincerely appreciate all of your efforts on this project.

Sincerely,

SITBUL, LLC,

A COLORADO LIMITED LIABILITY COMPANY

Timothy B. Sittema

Manager

/tas

October 29, 1996

Bob Lee Mesa County Building Department P.O. Box 20000 Grand Junction, CO 81502

Dear Bob,

After reviewing the <u>Restaurant Industrial Pretreatment Permit Application</u> submitted by Gordon Stanfield of Mac Gregor/Wathen Construction, for the Barnes & Noble Book Store to be built at 2450 Patterson Road, Grand Junction, it has been determined that this facility will not be required to install a grease interceptor. If you need more information, please call at 244-1489.

Sincerely,

Catherine Crabb Assistant Coordinator Industrial Pretreatment Program

cc: Trent Prall, Utility Engineer, City of Grand Junction Marcia Rabideaux, Community Development

CERTIFICATE O. OCCUPANCY

BUILDING DI PARTMENT CITY OF GRAND JUNCTION (OR MESA DOUNTY)

PERMIT #58257			' DATE	5-22-97
PERMISSION IS HEREBY GRANTED TO _	Barnes &	Noble Bookst	ore	TO OCCUPY THE
BUILDING SITUATED AT	2451 Pa	tterson		
LOTBLOCKFILING	GSUBl	IVISION		
TAX SCHEDULE NUMBER 29	45-091-00-	118		
FOR THE FOLLOWING PURPOSE:	Bookstore	with coffee s	hop	
			t.	
THIS CERTIFICATE ISSUED IN CONFORM	MITY TO SECTIO.	307, UNIFORM BUIL	DING CODE	
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	<i>IN</i> S	PECTOR Horol	d Ones	<u> </u>
	City Pl	anning Hall	en M.	Portru

To: Bobbie Paulson Cc: Kathy Portner

From: Michael Drollinger Subject: Barnes and Noble Date: 5/26/97 Time: 12:53PM

Bobbie:

If you get a C.O. on this while I'm gone please give it to Kathy for signature. I have inspected the site and the improvements have been installed satisfactorily.

Thanks.

mtd





To: Mike Drollinger

Company: City of Grand Junction Planning Dept.

Phone: 970.244.1439 Fax: 970.244.1599

From: Michael Lewis

Company: Runyon Architects

Phone: 972.233.7705 Fax: 972.387.2553

Project: Grand Junction

RA+A No.: 9642

Date: 6.4.97

Pages incl. 2

cover:

Comments:

Mike,

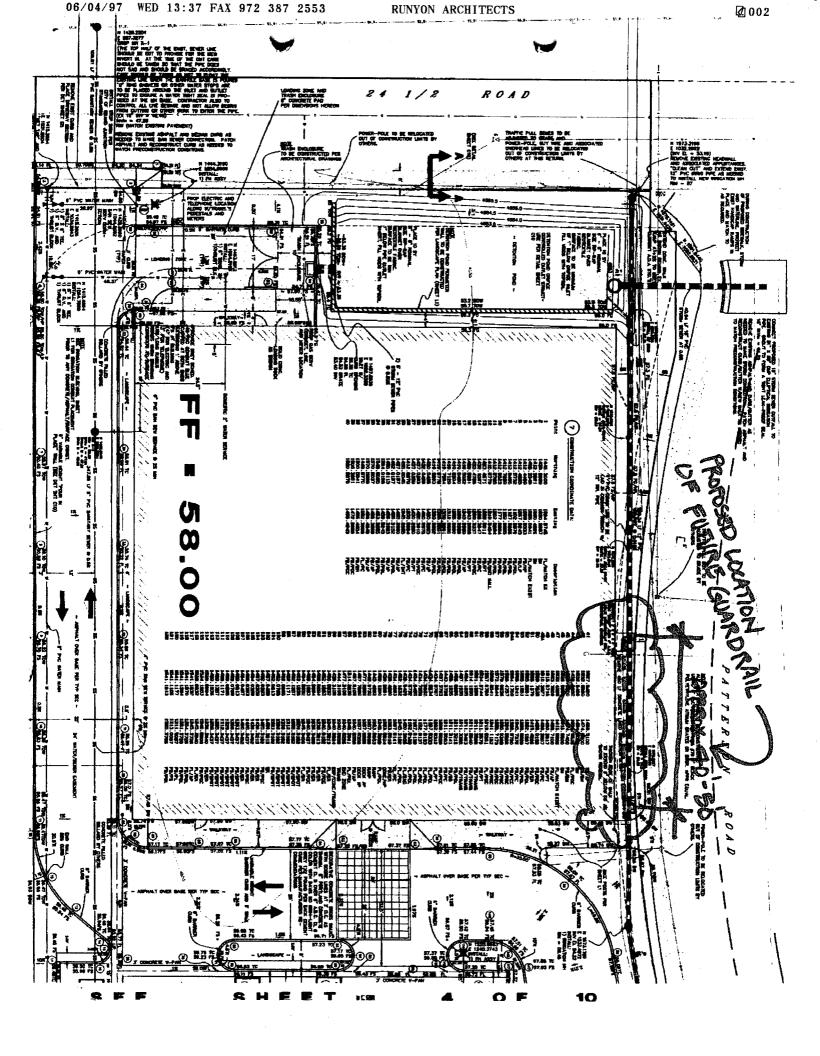
Per our discussion with you this morning, I have attached a site plan indicting the location for the proposed guardrail. The guardrail would be approx. 30" above finish grade and approx. 40' to 50' long. Please review the attached site plan and

Please contact me with questions and comments.

Respectfully, Michael Lewis

Copies: file 9642

2033 CHENAULT DRIVE, SUITE 150 CARROLLTON, TEXAS 75006 Ph(214) 233-7705 Fax(214) 387-2553



Facsimile Cover Sheet



To: Mike Drollinger

Company: City of Grand Junction Planning Dept.

Phone: 970.244.1439 Fax: 970.244.1599

From: Michael Lewis

Company: Runyon Architects

Phone: 972.233.7705 Fax: 972.387.2553

Project: Grand Junction

RA+A No.: 9642

Date: 7.7.97

Pages incl. 4

cover:

Comments:

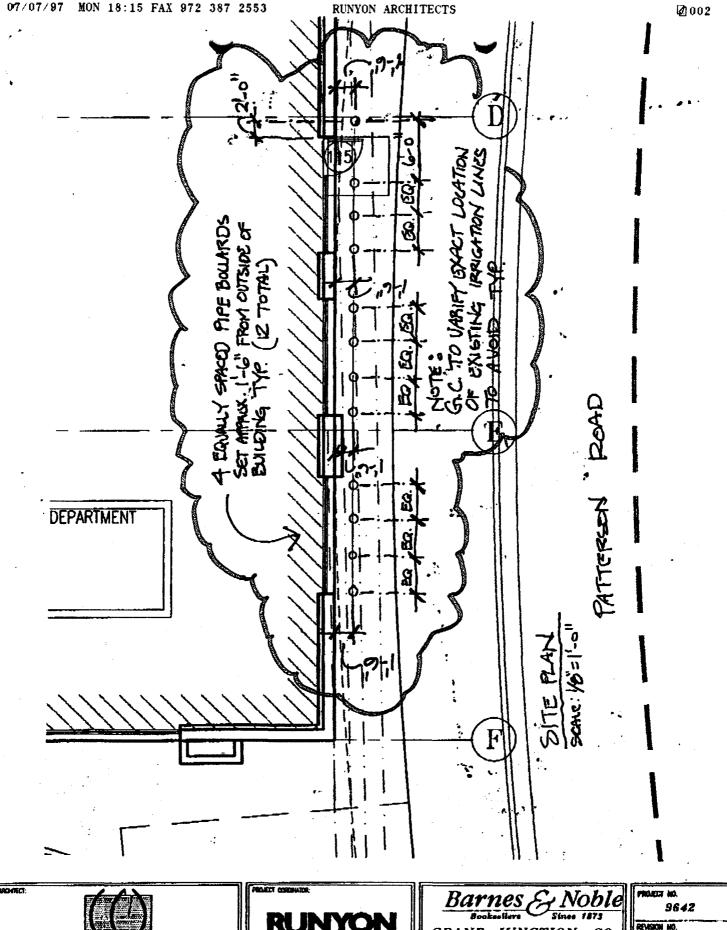
Mike,

Please review the attached sketches illustrating the pipe bollards proposed for the Grand Junction Barnes & Noble. If you have do not have any comments or revisions, we would like to issue this for construction as soon as possible.

Feel free to contact me with questions or comments.

Respectfully, Michael Lewis





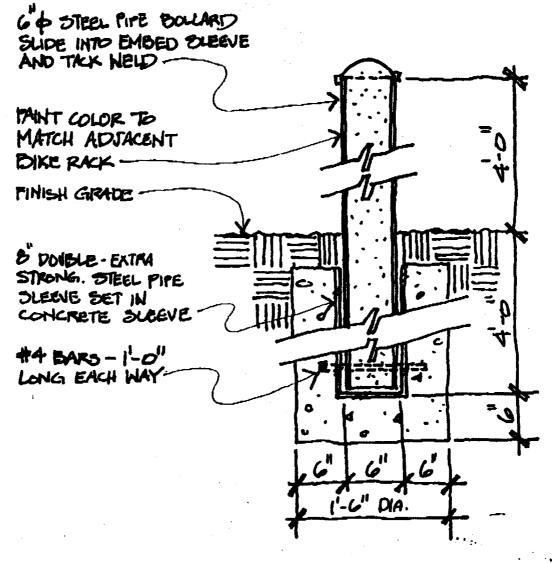






24.6 & F ROAD
GRAND JUNCTION, COLORADO

9642
REVISION NO.
BATC:
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PIPE BOLLARD.

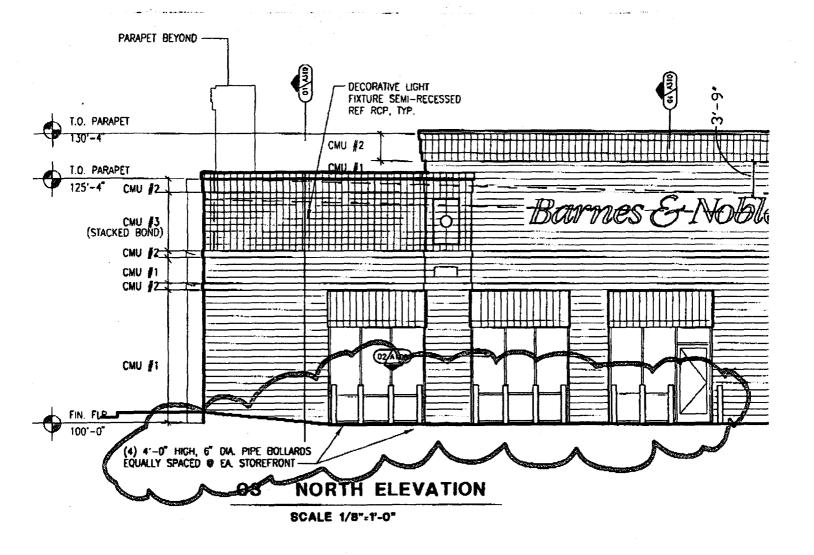






24.5 & F ROAD CRAND JUNCTION, COLORADO

PROJECT NO.
9642
NEVISION NO.
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DATE









24.6 & F ROAD GRAND JUNCTION, COLORADO

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CHANDLER SIGNS

3201 Manor Way Dallas, TX 75235-5909

FAX COVER SHEET

DATE:

March 14, 1997

TIME:

11:01 AM

TO:

Michael Drollinger

PHONE:

970-244-1439

City Planning

anning

fax

970-244-1599

FROM:

Patty Pryor

PHONE:

214-902-2000

Chandler Signs

FAX:

214-902-2044

RE: BARNES & NOBLE BOOKSELLERS

Number of pages including cover sheet 1

MESSAGE:

Michael, I am sorry I missed you again. I got your voice mail this morning and found you will be out until Tuesday.

I Need to speak to you in regard to permitting information for

the Barnes & Noble Booksellers at 24-1/2 Road & "F" Road there in Grand Junction.

Please call me when you return.

Thanks

copy: Shannon Frost

CHANDLER SIGNS

3201 Manor Way Dallas, TX 75235-5909

FAX COVER SHEET

8,30

DATE:

March 14, 1997

TIME:

TO:

Michael Drollinger

PHONE: fax

970-244-1439 970-244-1599

FROM:

Patty Pryor

City Planning

PHONE:

214-902-2000

Chandler Signs

FAX:

214-902-2044

RE: BARNES & NOBLE BOOKSELLERS

Number of pages including cover sheet 1

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I Need to speak to you in regard to permitting information for

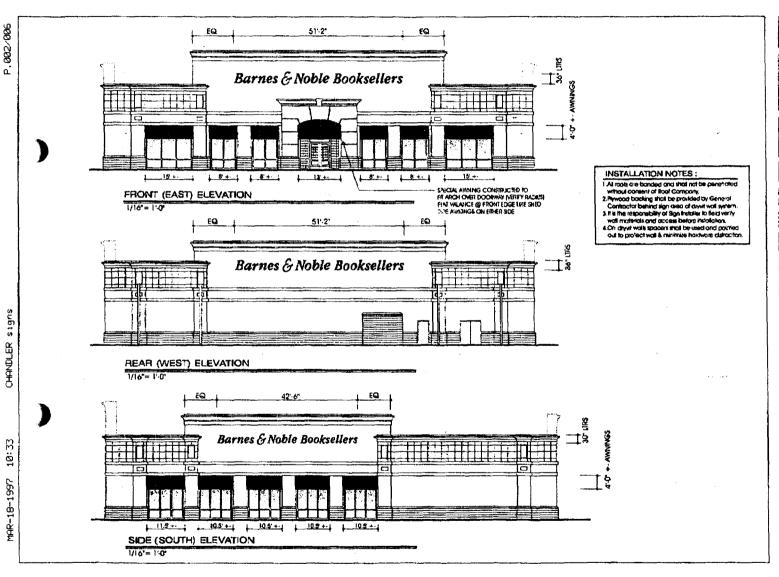
Barnes & Noble Booksellers at 24-1/2 Road & "F" Road there in Grand Junction.

Please call me when you return.

Thanks

copy: Shannon Frost

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CHANDLER

(A) ELEVATION - LETTER LAYOUT BN-SL-36

TWO (2) SIGNS REQUIRED

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ELECTRICAL MOCK-UP TO BE PERFORMED BY CUSTOMER

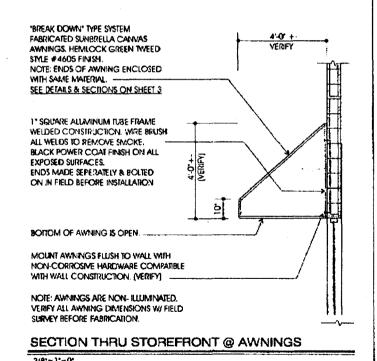
■ Barnes & Noble Booksellers

B ELEVATION - LETTER LAYOUT BN-SL-30

ONE (1) SIGN REQUIRED

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FACE LIT LETTER SECTION



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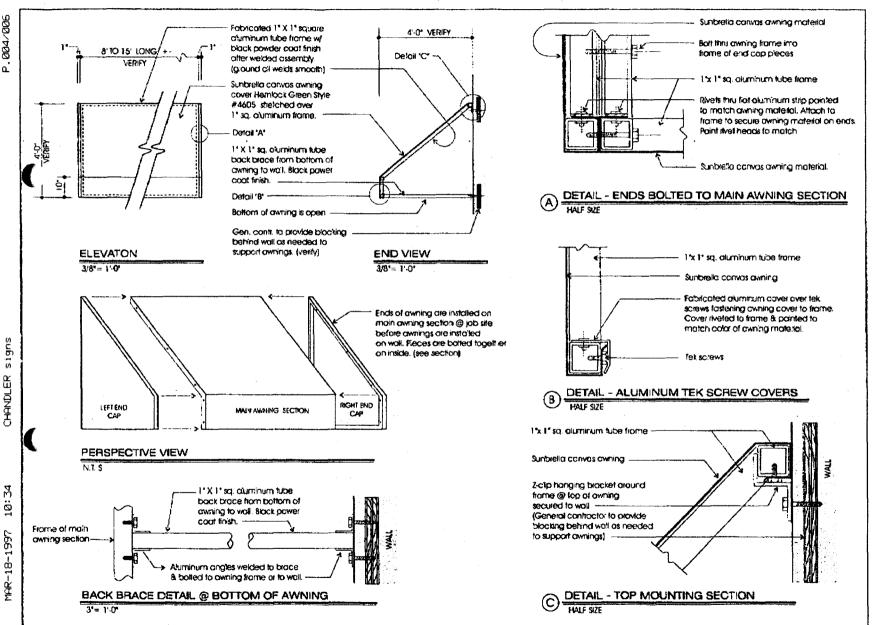
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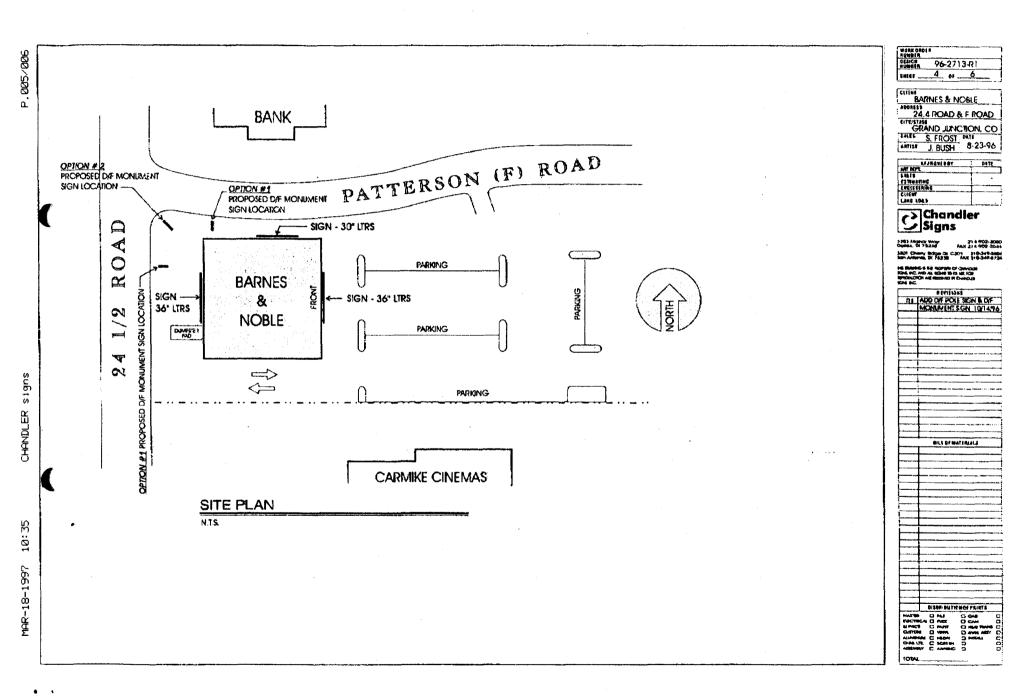


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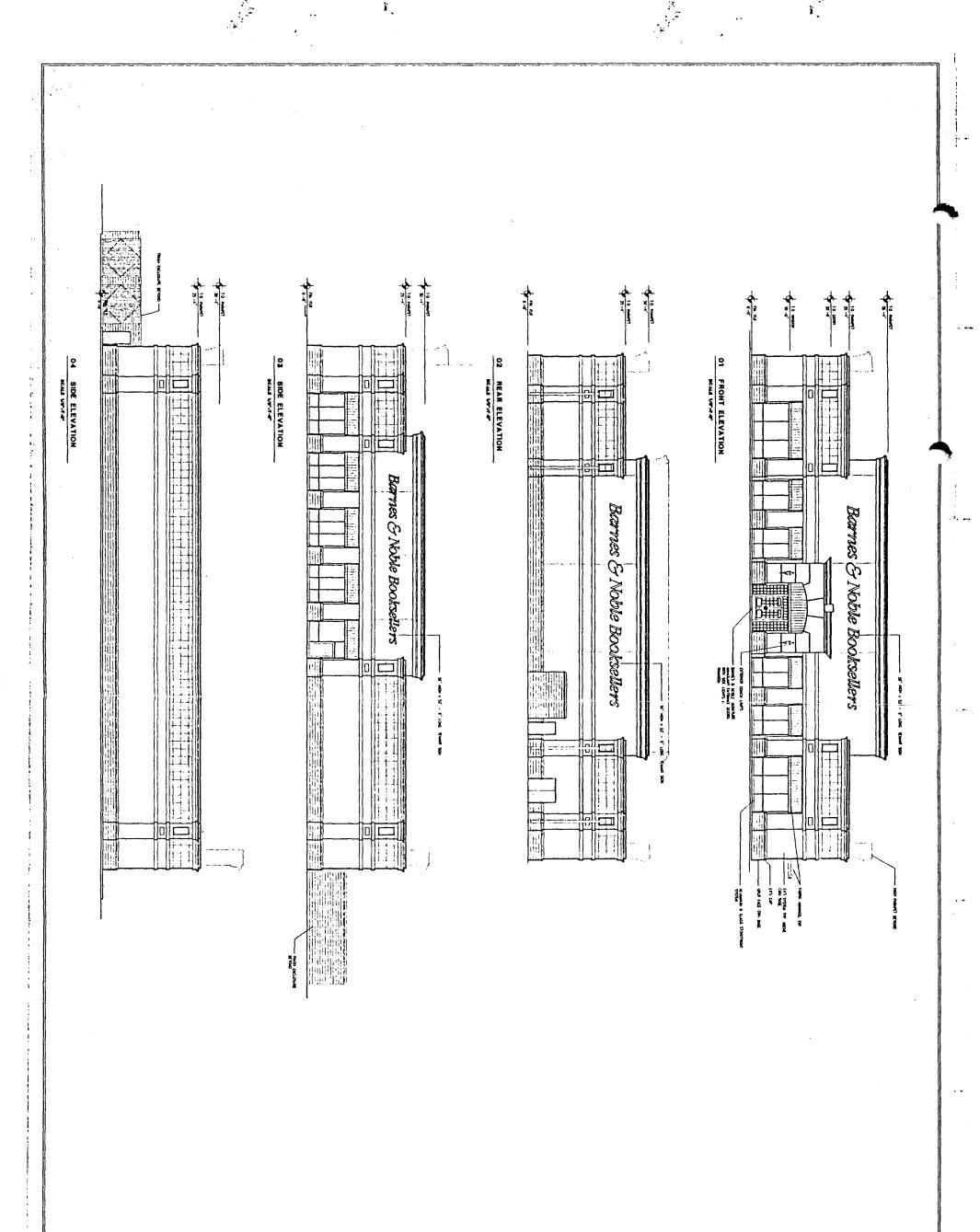
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A300 ELEVATION

RUNYONAIrchitects 2033 CHENAULT DR. SUITE 150

CARROLLTON, TX 75006 (214) 233-7705

Barnes & Noble

GRAND JUNCTION, CO.

Store Number

9642 XXXXXX

SPR-1996-110

City of Grand Junction

Community Development Department
Planning ● Zoning ● Code Enforcement
250 North 5th Street
Grand Junction, CO 81501-2668

Phone: (970) 244-1430 FAX: (970) 244-1599

August 14, 1997

Deborah Alexus
The Canada Life Assurance Company
c/o Terrix Financial Corporation
1777 South Harrison Street #507
Denver CO 80210

Dear Ms. Alexus:

As per your request I am forwarding you this letter to confirm that the Barnes and Noble Bookstore located at 2451 Patterson Road is an allowed use in the C-2 (Heavy Commercial) zoning district, and that as of the issuance of the Certificate of Occupancy in June 1997 the store was in compliance with City zoning regulations.

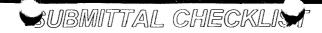
I trust that this letter contains the information you requested. Please do not hesitate to contact me if you have any questions or require additional information.

Sincerely yours

Michael T. Drollinger

Senior Planner

cc: File



SITE PLAN REVIEW

Location:	F #24/2 Rd	Project Name: 3f	R- Retail Starl

		DISTRIBUTION																												
Date Received 8-21-94 Receipt # 4415 File # SPR-96-190 DESCRIPTION	SSID REFERENCE	City Community Development				O City Parks/Recreation City Fire Department	Gity Attorney	O City Downtown Dev. Auth.	l	ot.	• Irrigation District G.V. (. C.	• Drainage District $oldsymbol{arphi}.\mathcal{T}_{i}$	■ Water District 1140	O Sewer District	O U.S. West	O Public Service	O GVRP	о свот	O Corps of Engineers	O Walker Field	O Persigo WWT	O Mesa County Health	O State Environ. Health	O City Sanitation	O School Dist #51					G.CTB. AFCT
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No IES: * An asterisk in the item description column indicates that a form is supplied by the City.

PRE-APPLICATION CONFERENCE

	Date: 5/28/96 Conference Attendance: Ky Hyy f. Jym Si Hema, Dick Scanano, Jym Langing, Proposal: 5/K - Krail Bark Syry, Location: 5/K - Krail Bark Syry, Location: 5/K - Krail Bark Syry, Review Fee:
	(Fee is due at the time of submittal. Make check payable to the City of Grand Junction.) Additional ROW required?
	Half street improvement fees/TCP required? TCP - 4700/1,000 5-6. Estimated Amount: Revocable Permit required? In Macalague in Macalagu
	Located in identified floodplain? FIRM panel #
	Located in established Airport Zone? Clear Zone, Critical Zone, Area of Influence? Avigation Easement required?
	While all factors in a development proposal require careful thought, preparation and design, the following "checked" items are brought to the petitioner's attention as needing special attention or consideration. Other items of special concern may be identified during the review process.
	O Access/Parking O Drainage O Land Use Compatibility O Drainage O Floodplain/Wetlands Mitigation O O Availability of Utilities O Geologic Hazards/Soils O Other Related Files:
	It is recommended that the applicant inform the neighboring property owners and tenants of the proposal prior to the public hearing and preferably prior to submittal to the City.
	PRE-APPLICATION CONFERENCE
	WE RECOGNIZE that we, ourselves, or our representative(s) must be present at all hearings relative to this proposal and it is our responsibility to know when and where those hearings are.
	In the event that the petitioner is not represented, the proposed item will be dropped from the agenda, and an additional fee shall be charged to cover rescheduling expenses. Such fee must be paid before the proposed item can again be placed on the agenda. Any changes to the approved plan will require a re-review and approval by the Community Development Department prior to those changes being accepted.
	WE UNDERSTAND that incomplete submittals will not be accepted and submittals with insufficient information, identified in the review process, which has not been addressed by the applicant, may be withdrawn from the agenda.
	WE FURTHER UNDERSTAND that failure to meet any deadlines as identified by the Community Development Department for the review process may result in the project not being scheduled for hearing or being pulled from the agenda.
K	h
`	Signature(s) of Petitioner(s) Signature(s) of Representative(s)

Barnes & Noble Retail Development

Site Plan Review Submittal

General Project Report

A. Project Description

The property is located at the corner of 24-1/2 Road and Patterson Road. The lot is 1.62 acres to be developed as a retail outlet for the sale of books, software, and other informational media. The store also includes a coffee shop. The remainder of the site will be used for the required parking, storm water management, and landscaping.

As per agreement with the adjacent land owner to the east, the entry is to be shared, and the access to the subject property is to be by easement. This is to mutually benefit both properties by reducing curb cuts on Patterson Road, allow the subject property to expand parking, and allow the adjacent property to avoid the expense of crossing the irrigation ditch to the north.

B. Public Benefit

The proposed development will provide infill in an area surrounded by commercial uses. The architecture and surrounding landscape will provide an attractive location for a quality retail outlet on a highly visible site. The proposed development will pay its commensurate share of fees and taxes to support the existing infrastructure and operating expenses for the services required.

C. Project Compliance, Compatibility, and Impact

The development meets setbacks, access, parking, and landscape requirements of the existing C-2 zoning. The retail sales area of 13,690 s.f. requires 69 parking spaces. The 38 seats in the restaurant requires 13 spaces, totaling a gross parking requirement of 82 spaces. The proposal shows 91 spaces. The landscape requirements with the proposed tree counts and square footage's of landscaping are shown on the landscape plan.

The surrounding land uses are commercial, with the property to the east remaining undeveloped at this time

Infill Development: The proposed project meets the intent of the promoting the development of infill development, allowing for the efficient use of existing utilities and roadways.

Northwest Area: The Northwest Area Plan promotes the development of the portion of the City due to it's proximity to existing growth areas, accessibility, and existing commercial uses.

Patterson Road Guidelines: "commercial development is appropriate on the south side of Patterson Road from Highway 6&50 to 25-1/2 Road"; "Access points should be designed to serve more than one lot" (entry is designed to be shared with the adjacent lot to the east).

D. Development Schedule and Phasing

construction is scheduled to begin immediately following site plan approval and issuance of a building permit.



August 12, 1996

City of Grand Junction 250 North 5th Street Grand Junction, CO 81501

Re: 24 1/2 and F Road

Dear Sir:

This letter serves as The Sterling Co.'s authorization to allow SB Advisors, Inc., or their respective contractors, Ciavonne & Associates, or Thompson-Langford, to pursue site plan approvals on the southeast corner of 24 1/2 and F Roads. Attached is a legal description for the property.

If you have any questions regarding this authorization or the extent of this approval, please contact me at your earliest convenience.

Sincerely,

Richard Scariano The Sterling Co.

RS/fah

Final Drainage Report

Barnes and Noble Book Store

August 14, 1996

Prepared for:

Tim Sittema Sittema - Bullock 5445 DTC Parkway, Penthouse Four Englewood, CO 80111

Prepared by:

THOMPSON-LANGFORD ©ORPORATION 529 251/2 RD., SUITE B-210 Grand Junction, CO 81505 PH. 243-6067

Job No. 0293-001

Engineer's Certification

I hereby certify that this report was prepared by me or under my direct supervision for the Owner's hereof.

James E. Langford, PE & LS Reg. No. 14847

Introduction

I. GENERAL LOCATION AND DESCRIPTION:

A. Site and Major Basin Location:

The site of the future Barnes and Noble Book Store is located in Northwest 1/4 of the Northeast 1/4 of Section 9, Township 1 South, Range 1 West of the Ute Meridian. In more local terms, it is located immediately north of the Carmike theater at the intersection of 24 1/2 Road and Patterson Road.

B. Site and Major Basin Description:

The site, located at the southeast corner of the intersection of 24 1/2 Road and Patterson Road, is a parcel approximately 400 feet long by 200 feet wide. The property is hydraulically separated from Patterson Road by the Independent Ranchmen's Ditch; the portion of the ditch to the west running beneath Patterson Road and into the Mall area. The site is presently unoccupied and devoid of vegetation.

II. EXISTING DRAINAGE CONDITIONS:

A. Major Basin:

The site as it presently exists is nearly flat, sloping at slightly less than 0.5% to the west. Due to this lack of gradient, ponding does occur in the center of the site. Stormwater impacting the site either drains out the west end of the parcel onto 24 1/2 Road, or leaches into the ground.

B. Site:

Given the hydraulic isolation of the property, the major drainage basin and the site are essentially the same.

III PROPOSED DRAINAGE CONDITIONS:

A. Changes in Drainage Patterns:

Site stormwater drainage either ponds on-site or overflows into the curb and gutter and runs south along 24 1/4 Road to inlets near the intersection of 24 1/4 Road with State Highway 6 & 50. We are proposing to collect the site stormwater in a detention facility near the northwest corner of the site and regulate the flow to historic levels

directly into the buried portion of the Independent Ranchmen's Ditch.

B. Maintenance Issues:

The on-site collection and detention facilities will be the responsibility of the lot owner.

IV DESIGN CRITERIA AND APPROACH:

A. GENERAL CONSIDERATIONS:

Natural conditions such as the tight adobe soils, high ground water and the extremely flat slopes have historically made this site difficult to drain.

As mentioned earlier in this report, the site is very flat and does not drain well. When a storm does occur that is large enough to cause runoff, the site drains down 24 1/2 Road. With development of this site, drainage down 24 1/2 Road is no longer possible because of the inability of the storm sewer near Highway 6 & 50 to handle the increased flow.

The site is within the 100-year floodplain of the Independent Ranchmen's Ditch. We will be elevating the building to place it at least 1-foot above the identified maximum water surface elevation.

B. Hydrology:

Pre-development Runoff coefficients used in the Rational equation were based on the hydrologic soil group index for the soil type found within the project. For post-development conditions we used the range of coefficients for the various proposed surface covers found in Appendix "B" of the above referenced manual.

According to the Soil Conservation Service soil survey for the Grand Junction Area, the dominant soil type is the Billings soil group having a hydrologic soil group index of "C". The soils on the site are hard packed which would suggest that the site has been used for some commercial or light industrial purpose. This would have suggested classifying the area as a traffic area. To do so would have made the historic runoff too high, closing the gap between the historic and the developed condition thus limiting the required detention. It was felt that using runoff coefficients based on HSG "A" versus HSG "C" would better represent the runoff potential and yield a more realistic detention volume.

Times of Concentration were calculated using the procedures outlined in Appendix "E" of the Storm Water Management Manual.

Given the small size of the project, the site was considered as one basin and analyzed using the Rational Method as described in Section VI. Hydrology, City of Grand Junction Storm Water Management Manual. Stormwater runoff for the 2-year and 100-year events were quantified and routed through a detention facility located in the northwest corner of the site.

C. Hydraulics:

The flow capacity of concrete pans, curb and gutter or underground conduits was calculated using Manning's Equation and the requisite coefficients all found in Appendices "G" & "H".

The detention facility was designed to detain both the 2-year and 100-year events, discharging through a two stage outlet only at the historic rates. Discharge calculations are included as an appendix to this report assuring that during the 2-year event, only the historic 2-year flow is released from the facility, and during the 100-year event the combinations of the outlets will discharge only the historic 100 year flow.

The two stage outlet control structure was sized using the procedures outlined in Appendix "K".

IV Results and Conclusions

Runoff Results:

2-year historic runoff rate = 0.82 CFS

2-year developed runoff rate = 2.56 CFS

100-year historic runoff rate = 3.10 CFS

100-year developed runoff rate = 6.65 CFS

Detention Facility:

Storage volume for 2-year event = 2242.79 cu-ft.

Storage volume for 100-year event = 3513.87 cu-ft.

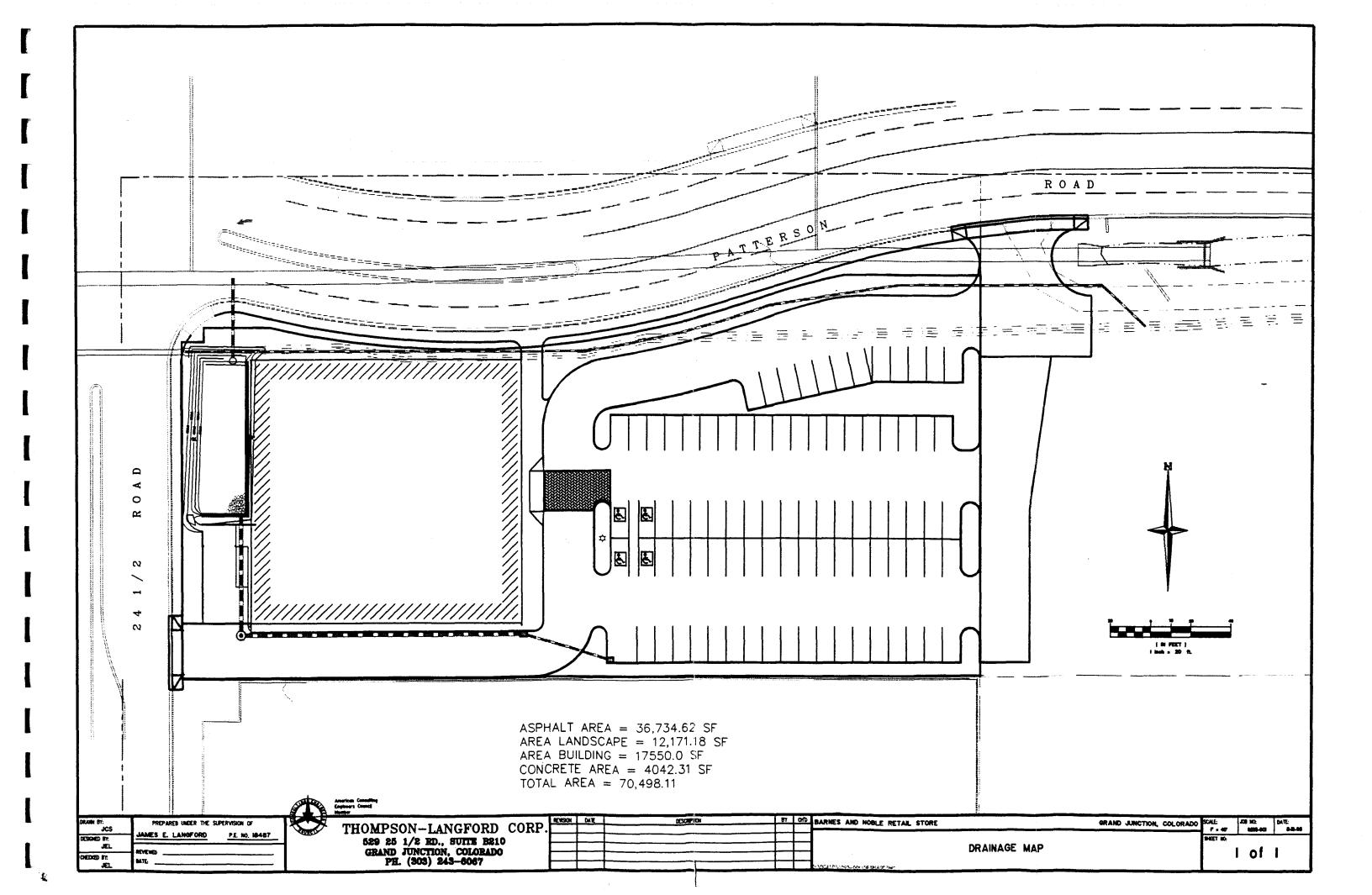
The outlet works will be constructed using a modified CDOT Type "C" inlet with a close mesh grate. An orifice will be constructed in the front face of the box at an invert elevation of 4553.00 which also coincides with the bottom of the pond. A weir will be constructed at the top, just below the close mesh grate. The orifice, having a diameter of 0.50 feet, has been sized to pass the 2-year historic flow when the water surface of the pond is up to the 2-year storage level of 4554.33. The second stage outlet ,or weir, which has been set at the 2-year water surface elevation, and given a width of 1.08 feet in length, will in combination with the orifice, pass the 100-year event when the water surface elevation is up to the 100-year storage level of 4554.9.

The top of the box is to be set at the 100-year water surface elevation and is to be covered with a standard CDOT close mesh steel grating. The steel grating will serve as an emergency overflow in the event of a storm in excess of the maximum design event. The outfall from the outlet works is to be 18-inch PVC pipe.

References

"STORMWATER MANAGEMENT MANUAL (SWMM)", City of Grand Junction, June 1994

"MESA COUNTY STORM DRAINAGE CRITERIA MANUAL", Mesa County, Colorado, Final Draft March 1992.



BARNES & NOBLE, 0293-001 TWO STAGE OUTFALL CALCULATION

Procedure as described in the City of Grand Junction's Storm Water Management Manual See Page N-5

NOTE:

- * Enter data from Drainage Study
- ** Vary this number until the desired result is obtained
- X Calculated by spreadsheet (no entry required)

Orifice Flow (2-year event)

- * Water Surf. El. 4554.33 Ft.

 * Orifice Invert 4553.00 Ft.

 ** Orifice Dia. (d) 0.50 Ft. **Vary orifice diameter until

 * Discharge (Qr) 0.82 CFS areas match

 * "Co" Coef. 0.60
- X Area = $(3.1416)d^2/4$ = 0.20 SF X = $Qr/0.82C(2gh)^0.5$ = 0.20 SF

Combined Wier Flow and Orifice Flow (100-year event)

* Water Surf. El. 4554.90 X Wier Invert El. 4554.33

The 100-year storage elevation is dictated by pond configuration. The elevation of the invert of the wier is set equal to the 2-year storage elevation. The wier width will be calculated such that the discharge when added to the orifice discharge equals the 100-year discharge.

- * Q100 discharge = 2.56 CFS
 - Q (orifice) = $0.82CoA(2gh)^0.5 = 1.01 CFS$

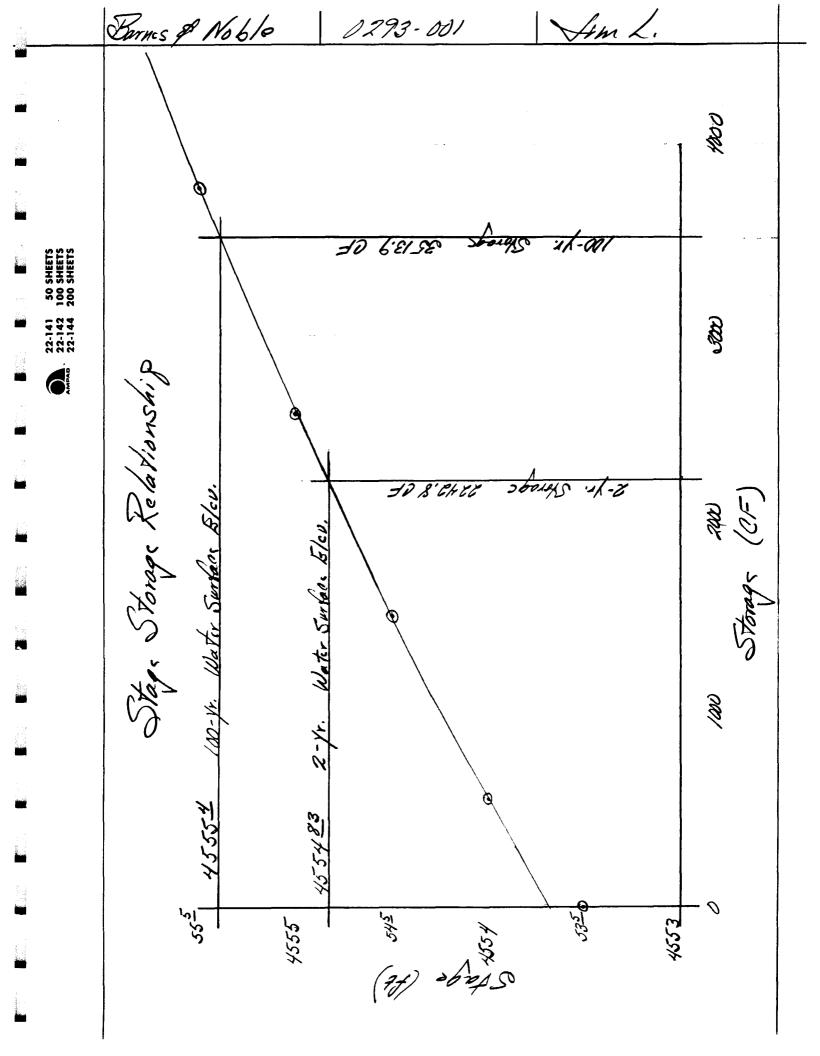
Wier Flow Equasion

Q= CwLH^1.5

- X Wier discharge = 1.55 CFS
 * "Cw" Coef. 3.33
 X Flow Depth (H) = 0.57 Ft.
 th Wier Length (L)
- ** Wier Length (L) 1.08 Ft. **Vary unit1 "Q" = Q100

Q= Wier Flow + Orifice Flow

2.56 CFS **If this calculated flow equals the historic 100-year flow then the wier length is correct.



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	in the state		

B&N Detention Volume

DETENTION VOLUME

For: Barnes and Noble

USING

METHOD OUTLINED ON PAGE N-4 SWMM

```
Td = Time of critical storm duration, minutes
                                                                                  0.81
      = Runoff coefficient (2-Year Event)
Cl00 = Runoff coefficient (100-Year Event)
                                                                                  0.83
A = Area in acres (developed condition)
                                                                                  1.62
Qr2 = Detention pond average release rate, cfs (Note that this will
                                                                                  0.82
    not likely be the historic rate Oh, nor even Omax)
Qr100 = Detention pond average release rate, cfs (Note that this will
                                                                                  3.10
    not likely be the historic rate Qh, nor even Qmax)
                                                                                  28.20
Tch2 = Time of concentration (historic), minutes (2-year event)
Tch100 = Time of concentration (historic), minutes (100-year event)
                                                                                 18.50
Tcd2 = Time of concentration (developed), minutes (2-year event)
                                                                                  5.00
Tcd100 = Time of concentration (developed), minutes (100-year event)
                                                                                  5.00
      = Intensity at Td, inches per hour (2-year event)
                                                                                  1.95
Id100 = Intensity at Td, inches per hour (100-year event)
                                                                                  4.95
Qd = Runoff rate at Td, cfs
K = Ratio of pre-and post-development Tc
V2 = Storage volume (2-year event) cu. ft.
V100 = Storage volume (100-year event) cu. ft.
Td2
       = (((633.4*Cd2*A)/(Qr2-(Qr2^2*Tcd2)/(81.2*Cd2*A)))^0.5)-15.6
               16.85 Min.
Td100
       = (((1832*Cd*A)/(Qr100-(Qr100^2*Tcd)/(213*Cd*A)))^0.5)-17.2
               11.77 Min.
```

B&N Detention Volume

Qd2 = Cd*A*Id2

= 2.56 cfs

Qd100 = Cd*A*Id100

6.65 cfs

K2 = Tch2/Tcd2

5.64

K100 = Tch100/Tcd100

= 3.70

 $V2 = 60[Qd2*Td2-Qr2*Td2-Qr2*Tcd2+K2*Qr2*Tcd2/2+Qr2^2*Tcd2/(2Qd2)]$

= 2,242.79 cu-ft.

V100 = 60[Qd100*Td100-Qr100*Td100-Qr100*Tcd100+K100*Qr100*Tcd100/2+Qr100^2*Tcd100/(

= 3,513.87 cu-ft.

RUNOFF VOLUME

For: Barnes and Noble

USING

RATIONAL METHOD Q=CxCfxIxA

BASIN	Q Volume	C Composite Coefficient	Cf Antecedent Precip. Fac.	I* Rainfall Intensity	A Basin Area
	cfs	n/a	n/a	in/hr	acres
Historic (2-Yr)	0.82	0.55	1	0.92	1.6184
Historic (100-Yr)	3.10	0.65	1	2.95	1.6184
Developed (2-Yr)	2.56	0.81	1	1.95	1.6184
Developed (100-Yr)	6.65	0.83	1	4.95	1.6184

^{*}Rainfall intensity was picked from Table A-1, the Intensity/Duration curves for the City of Grand Junction, based on Time of Concentration

TABLE - 2

For: Bar:	nes and Nob	L	s	N*	V*	Tt2	Tt100	Tc2	Tc100	2-Year i	100-Year i
	Descrip. of Flow	Length	Slope	Mannings	Vel.	Travel Time	Travel Time	Time Concen	of tration	Intensity Grd. Jctn.	Intensity Grd. Jctn.
		ft.	*	coef.	fps	min.	min.	min.	min.	Curves	Curves
"Full Site"	1										
Historic	overland*	300	0.50%	0.030				28.2	18.5	0.92	2.95
	Nat. Ch.***	170	0.50%	n/a	0.70	4.05	4.05				
	C&G**	0	0.00%	0.000	0.00	0.00	0.00				
"Full Site"	•										
Developed	Landscape*	0	1.00%	0.400				4.0	4.0	1.95	4.95
As	ph. Swale***	275	1.00%	n/a	1.75	2.62	2.62	** Used 5	minutes as	a minimum	
C	Conc. Swale**	170	0.50%	0.000	2.50	1.13	1.13				

Pipe Flow 80 0.50% 0.000 5.00 0.27

^{*} Overland "To" based on SCS formula pg. E-2 Storm Water Management Manual

^{**}Mannings Equa. was used to determine gutter and natural swale velocities.

Mannings n=0.016 was used for curb and gutter, and n=0.030 was used for natural swales.

^{***}Figure "E-3", Pg. E-9, Storm Water Management Manual was used for shallow flows.

TABLE - 1a

COMPOSITE RUNOFF COEFICIENTS

For: Barnes and Noble

USING

GRAND JUNCTION RECOMMENDED RUNOFF COEFICIENTS

	Hydro.	Slope <2%			BASIN Histori	c	BASIN Develop	ed
Description	Soils	Runoff	Sel.		Unit	Wt'd	Unit	Wt'd
Surface Area	Group	Coeff.'s	Coeff.		Area	Value	Area	Value
Pavement and Roofs	B*	0.93	0.93	2-Yr.			1.34	1.25
	В*	0.95	0.95	100-Yr.			1.34	1.27
Green landscaping	B*	0.14 to 0.22	0.22	2-Yr.			0.28	0.06
lawns and parks	В*	0.20 to 0.28	0.28	100-Yr.			0.28	0.08
Traffic Area	A	0.55 to 0.65	0.55	2-Yr.	1.62	0.89		
Soil and Gravel	A**	0.65 to 0.70	0.65	100-Yr.	1.62	1.05		
Total Basin Area:					1.62		1.62	
COMPOSITE "C" VALUE	(2-year))				0.55		0.81
COMPOSITE "C" VALUE	(100-yea	ar)				0.65		0.83

^{*} The natural soils are in HSG "C", but because we are filling the site with pit run material to get it above the 100-year floodplain, we have used HSG "B" .

^{**} The surface soils at the site are hard packed silts and clays devoid of any vegetation. The area's runoff potential is somewhere between bare ground and a traffic area. It was felt that the lowest classification of traffic area (HSG "A") was most appropriate.

LAND USE OR		SCS	HYDRO	LOGICS	OIL GRO	UP (SEE	APPENI	OIX "C" I	FOR DES	CRIPTIC	ONS)	
SURFACE CHARACTERISTICS	A			В		С			D			
	0-2%	2-6%	6%+	0-2%	2-6%	6%÷	0-2%	2-6%	6%+	0-2%	2-6%	6%+
UNDEVELOPED AREAS Bare ground	.1020	.1626	.2535	.1422	.2230	.30 • .38	.2028	.2836	.3644	.2432	.3038	.4048
	.1424	.2232	.3040	.2028	.2836	.37 • .45	.2634	.3543	.4048	.3038	.4048	.5058
Cultivated/Agricultural	.0818	.1323	.1626	.1119	.1523	.2129	.1422	.1927	.2634	.1826	.2331	.3139
	.1424	.1828	.2232	.1624	.2129	.2836	2028	.2533	.3442	.2432	.2937	.4149
Pasture	.1222	.2030	.3040	.1826	.2836	.3745	.2432	.3442	.4452	.3038	.4048	.5058
	.1525	.2535	.3747	.2331	.3442	.4553	:.3038	.4250	.5260	.3745	.5058	.6270
Meadow	.1020	.1626	.2535	.1422	.2230	.3038	.20 - 28	.2836	.3644	.24 - 32	.3038	.4048
	.1424	.2232	.3040	.2028	.2836	.3745	.2634	.3543	.4452	.3038	.4048	.5058
Forest	.0515	.0818	.1121	.0816	1119	.1422	.1018	.1321	.1624	.1220	.1624	.2028
	.0818	.1121	.1424	.1018	.1422	.1826	.1220	.1624	.2028	.1523	.2028	.2533
RESIDENTIAL AREAS	.4050	.4353	.4656	.4250	.4553	.5058	.4553	.4856	.5361	.4856	.5159	.5765
1/8 acre per unit	.4858	.5262	.5565	.5058	.5462	.5967	.5361	.5765	.6472	.5664	.6068	.6977
1/4 acre per unit	.2737	.3141	.3444	.29 - 37	.3442	.3846	.3240	.3644	.4149	.3543	39 - 47	.4553
	.3545	.3949	.4252	.3846	.4250	.4755	.4149	.4553	.5260	.4351	47 - 55	.5765
1/3 acre per unit	22 - 32	.2636	.2939	.2533	.2937	.3341	.2836	3240	.3745	.3139	.3543	.4250
	31 - 41	.3545	.3848	.3341	.3846	.4250	.3644	.4149	.4856	.3947	.4351	.5361
1/2 acre per unit	.1626	.2030	.2434	.1927	.2331	.2836	2230	.2735	.3240	.2634	.3038	.3745
	.2535	.2939	.3242	.2836	.3240	.3644	.3139	.3543	.4250	.3442	.3846	.4856
l acre per unit	.1424	.1929	.2232	.1725	.2129	.2634	.2028	.2533	.3139	.2432	.2937	.3543
	.2232	.2636	.2939	.2432 -	.2836	.3442	.2836	.3240	.4048	.3139	.3543	.4654
MISC. SURFACES Pavement and roofs	,93	.94	.95	.93	94	.95	.93	.94	.95	.93	.94	.95
	.95	.96	.97	.95	.96	.97	.95	.96	.97	.95	.96	.97
Traffic areas (soil and gravel)	.5565	.60 -).70	.6474	.6068	6472	.6775	.6472	.6775	.6977	.7280	.7583	.7785
	.6570	.7075	.7479	.6876	.7280	.7583	.72 - 80	.7583	.7785	.7987	.8290	.8492
Green landscaping (lawns, parks)	.1020	.1626	.2535	.1422	.2230	.3038	.2028	.2836	.3644	.2432	.3038	.4048
	.1424	.2232	.3040	.2028	2836	.3745	.2634	.3543	.4252	.3038	.4048	.5058
Non-green and gravel landscaping	.3040	.3646	.4555	.4555	4250	.5058	.4048	.4856	.5664	.4452	.5058	.6068
	.3444	.4252	.5060	.5060	4856	.5765	.46 - 54	.5563	.6472	2.5058	.6068	.7078
Cemeteries, playgrounds	.2930	.2636	.3545	.3545	.3240	.4048	.3038	.3844	.4654	.3442	.4048	.5058
	.2434	.3242	.4050	.4050	.3846	.4755	.3644	.4553	.5462	.4048	.5058	.6068

NOTES: 1.

RATIONAL METHOD RUNOFF COEFFICIENTS
(Modified from Table 4, UC-Davis, which appears to be a modification of work done by Rawls)

Values above and below pertain to the 2-year and 100-year storms, respectively.

The range of values provided allows for engineering judgement of site conditions such as basic shape, homogeneity of surface type, surface depression storage, and storm duration. In general, during shorter duration storms (Tc < 10 minutes), infiltration capacity is higher, allowing use of a "C" value in the low range. Conversely, for longer duration storms (Tc) 30 minutes), use a ""C value in the higher range.

For residential development at less than 1/8 acre per unit or greater than 1 acre per unit, and also for commercial and industrial areas, use values under MISC SURFACES to estimate "C" value ranges for use.

^{3.}

	TABLE "A-1" INTENSITY-DURATION-FREQUENCY (IDF) TABLE								
Time (min)	2-Year Intensity (in/hr)	100-Year Intensity (in/hr)	Time (min)	2-Year Intensity (in/hr)	100-Year Intensity (in/hr)				
5	1.95	(4.95)	33	0.83	2.15				
6	1.83	4.65	34	0.82	2.12				
7	1.74	4.40	35	0.81	2.09				
8	1,66	4.19	36	0.80	2.06				
9	1.59	3.99	37	0.79	2.03				
10	1.52	3.80	38	0.78	2.00				
11	1.46	3.66	>39	0.77	1.97				
12	1.41	3.54	40	0.76	1.94				
13	1.36	3.43	41	0.75	1.91				
14	1.32	3.33	42	0.74	1.88				
15	1.28	3.24	43	0.73	1.85				
16	1.24	3.15	44	0.72	1.82				
17	1.21	3.07	45	0.71	1.79				
18	1.17	2.99	46	0.70	1.76				
19	1.14	2.91	47	0.69	1.73				
20	1.11	2.84	48	0.68	1.70				
21	1.08	2.77	49	0.67	1.67				
22	1.05	2.70	50	0.66	1.64				
23	1.02	2.63	51	0.65	1.61				
24	1.00	2.57	52	0.64	1.59				
25	0.98	2.51	53	0.63	1.57				
26	0.96	2.46	54	0.62	1.55				
27	0.94	2.41	55	0.61	1.53				
28	0.92	2.36	56	0.60	1.51				
29 '	0.90	2.31	57	0.59	1.49				
30	0.88	2.27	58	0.58	1.47				
31	0.86	2.23	59	0.57	1.45				
32	0.84	2.19	60	0.56	1.43				
Source: Mesa	County 1991								

A-2 JUNE 1994

REPRODUCED FROM TABLE 2, LECTURE 2, DAY 2, ACOE 1990

SURFACE	N VALUE	SOURCE
ASPHALT/CONCRETE	0.05 - 0.15	Develope d Use O.UPB History
BARE PACKED SOIL FREE OF STONE	0.10	£
FALLOW - NO RESIDUE	0.008 - 0.012	1 AUB // /
CONVENTIONAL TILLAGE - NO RESIDUE	(0.06 - 0.12)	Use U.UIB MISYON
CONVENTIONAL TILLAGE - WITH RESIDUE	0.16-0.22	В
CHISEL PLOW - NO RESIDUE	0.06 - 0.12	В
CHISEL PLOW - WITH RESIDUE	0.10 - 0.16	В
FALL DISKING - WITH RESIDUE	0.30 - 0.50	В
NO TILL - NO RESIDUE	0.04 - 0.10	В
NO TILL (20-40 PERCENT RESIDUE COVER)	0.07 - 0.17	В
NO TILL (60-100 PERCENT RESIDUE COVER)	0.17 - 0.47	В
SPARSE RANGELAND WITH DEBRIS:		
O PERCENT COVER	0.09 - 0.34	В
20 PERCENT COVER	0.05 - 0.25	В
SPARSE VEGETATION	0.053 - 0.13	F
SHORT GRASS PRAIRIE	0.10 - 0.20	F
POOR GRAGS COVER ON MODERATELY	0.30	C
ROUGH BARE SURFACE		
LIGHT TURF	0.20	٨
AVERAGE GRASS COVER	0.4	С
DENSE TURF	0.17 - 0.80	A, C,E, F
DENSE GRASS	0.17 - 0.30	D
BERMUDA GRASS	0.30 - 0.48	D
DENSE SHRUBBERY AND FOREST LITTER	0.4	A

"N" values provided in this table pertain to both the SCS TR-55 "To" and FHWA 1984 HEC-12 "To" methods

A) CRAWFORD AND LINSLEY (1966).

B) ENGMAN (1986).

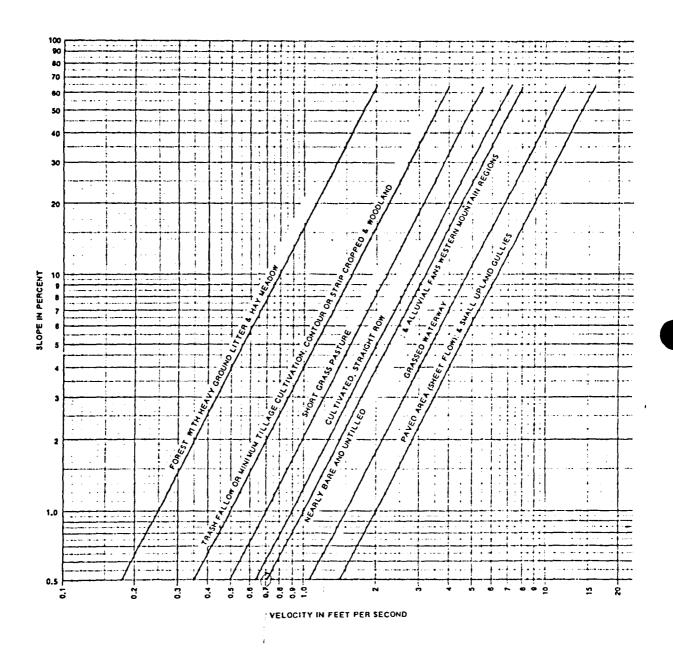
C) HATHAWAY (1945).

D) PALMER (1946).

E) RAGAN AND DURU (1972).

F) WOOLHISER (1975).

REPRODUCED FROM FIGURE 15.2, SCS 1972



DETERMINATION OF "Ts"

FIGURE "E-3"

E-9

United States Department of Agriculture

Soil Conservation Service

Engineering Division

Technical Release 55

June 1986



Urban Hydrology for Small Watersheds



Appendix A: Hydrologic soil groups

Soils are classified into hydrologic soil groups (HSG's) to indicate the minimum rate of infiltration obtained for bare soil after prolonged wetting. The HSG's, which are A, B, C, and D, are one element used in determining runoff curve numbers (see chapter 2). For the convenience of TR-55 users, exhibit A-1 lists the HSG classification of United States soils.

The infiltration rate is the rate at which water enters the soil at the soil surface. It is controlled by surface conditions. HSG also indicates the transmission rate—the rate at which the water moves within the soil. This rate is controlled by the soil profile. Approximate numerical ranges for transmission rates shown in the HSG definitions were first published by Musgrave (USDA 1955). The four groups are defined by SCS soil scientists as follows:

Group A soils have low runoff potential and high infiltration rates even when thoroughly wetted. They consist chiefly of deep, well to excessively drained sands or gravels and have a high rate of water transmission (greater than 0.30 in/hr).

Group B soils have moderate infiltration rates when thoroughly wetted and consist chiefly of moderately deep to deep, moderately well to well drained soils with moderately fine to moderately coarse textures. These soils have a moderate rate of water transmission (0.15-0.30 in/hr).

Group C soils have low infiltration rates when thoroughly wetted and consist chiefly of soils with a layer that impedes downward movement of water and soils with moderately fine to fine texture. These soils have a low rate of water transmission (0.05-0.15 in/hr).

Group D soils have high runoff potential. They have very low infiltration rates when thoroughly wetted and consist chiefly of clay soils with a high swelling potential, soils with a permanent high water table, soils with a claypan or clay layer at or near the surface, and shallow soils over nearly impervious material. These soils have a very low rate of water transmission (0-0.05 in/hr).

In exhibit A-1, some of the listed soils have an added modifier; for example, "Abrazo, gravelly." This refers to a gravelly phase of the Abrazo series that is found in SCS soil map legends.

Disturbed soil profiles

As a result of urbanization, the soil profile may be considerably altered and the listed group classification may no longer apply. In these circumstances, use the following to determine HSG according to the texture of the new surface soil, provided that significant compaction has not occurred (Brakensiek and Rawls 1983):

HSG Soil textures

- A Sand, loamy sand, or sandy loam
- B Silt loam or loam
- C Sandy clay loam
- D Clay loam, silty clay loam, sandy clay, silty clay, or elay

Drainage and group D soils -

Some soils in the list are in group D because of a high water table that creates a drainage problem. Once these soils are effectively drained, they are placed in a different group. For example, Ackerman, soil is classified as A/D. This indicates that the drained Ackerman soil is in group A and the undrained soil is in group D.

Exhibit A-1, continued: Hydrologic soil groups for United States soils

BELHONT	8 1	BERTRAM	8 1	BILLINGS.	e 1	BLACKNOLL	C I	BLUE LAKE	A
BELMORE	8 i	BERTRAND	8 [MODERATELY SLOW	1	BLACKDAR	8/01	BLUE STAR	8
BELPRE	c i		0/01	PERM	1	PLACKPIPE	c 1	BLUEBELL	c
BELSAC	B 1	BERVOLF	B	BILLYCREEK	C 1	BLACKPRINCE	8 1	BLUECHIEF	c
BELTED	0 1	BERYL	8 1	BILLYHAW	D 1	BLACKPOCK	8 1		D
BELTON	ci	BERZATIC	0 1	BILTPORE	A 1	BLACKSAN	8 1	BLUEDOME	C
BELTRAMI	8 1	BESEMAN	A/DI	BIMMEP	D	PLACKSPAR	D 1	BLUEFLAT	c
BELTSVILLE		BESHERM	c i	PINCO	0 1	BLACKSPOT	0 1	BLUEGROVE	c
BELUGA	0 1	BESNER	B 1	BINDLE	P	BLACKSTON	6 I	BLUEGULCH	8
BELUGA. DRAINED.		BESSEHER	c i		8	BLACKTHORN	8	BLUEHILL	c
SLOPING		BESSIE	ρi		8		0 1	BLUEHON	c
BELVOIR		BESTROM	c i		8		οi	BLUEJOINT	8
		BETHANY	è i			PLACKWELL	0 1		8
BELZAR		BETHEL	Bi		ם פ		D I		A
BENIOJI	âi		0 1		6		ōi		c
BEN LOHONO		BETHESDA	c i			PLAGD /	0 1		ō
BENCHLEY	-			BINS		BLAINE !		BLUESPRIN	c
BENCLARE	-	BETHLEHEM		BINTON		BLAIR	è i		Ď
BENCO		BETIS		BINTON RECLAIMED		6LAIRTON	ċ		Ā
BENDER	e 1		-				c i		D
BENDIRE	C		-	BIOYA	_	,	9 1		č
RENEAOLY	c i		c 1		- '	PLAKE	A I		٥/٥
BENEANH	0 1		-	BIRCHBAY	-	6LAKELAND			(
BENFIELD	C		B			PLAKENEY	C 1		
BENGAL	c l	BEVENT	A		-	BLAKEVELL	C 1		c
BENGE	8	BEVERIDGE	0 1	BIRDOM		PLALOCK		BLY	0
BENHAM	8 [BEVERLY	9 (EIADS	C/D	BLAMER	C I	BLYBURG	B
BENIN	D	BEVERLY. GRAVELLY	A 1	BIRDSALL	D	BLANCA	2	,	D
BENITO	D	864	C 1	BIRDSBORO	F	BLANCHARD	A 1	BOARDMAN	D
HIMALHER	o i	BEWLEYVILLE	0 1	BIADSLEY	D '	BLANCHE	B	BOARDTREE	Ç
BENKLIN		BEXAR	0	BIRDSVIE	A	BLANCHESTER	B/D I	BOASH	D
BENMAN	c i		o i	BIRKEECK	6	BLANCOT	8 1	BOAZ	c
BENNDALE	8 1		D i			. BLAND	C 1	1118808	c
BENNINGTON		8 188	c i		8	BLANDING	8	1 508 LLO	A
BENR IDGE	-	HIBLESPRINGS	ō			BLANEY	-	E08N808	c
	- •	BICE		BISBEE	_	BLANKET		8085	Ď
BENSLEY	0 1			BISCARO		BLANTON		BOBTAIL	č
BENSON						BLANTON.	a		B
BENTEEN		BICKETT				•		BOCA	B/D
BENYY	-	BICKLETON	8 1		-			BOCA+ DEPRESSIONAL	
BENZ	- •	BICKHORE	c I			BLANTON			
BEOR		BICONDOA		BISGANI. FLOODED		BLAPPERT		BOCA . TIDAL	D
BEOSKA	8		c i			BLAQUIERE	c i		Ð
BEOTIA	5 J	BIDDEFURD	0 t		- '	BLASDELL	A 1		D
BEOWAME	8 1	BIDDLEHAN	8 1	B1500D1	D I	BLASE	c 1		0
BEQUINN	8 1	BIDMAN	C	BISPING	e i	BLASINGAME	c 1		D
BERCUMB	8 1	BIDVELL		BISSELL	e	BLAYDEN	0 1	BODECKER	A
BERDA	8	BIEBER	0 1	BISSONNET	0	BLAZBIRD	0 1	BODELL	0
BEREA	C	BIEDELL	0 1	817	С.	BLAZON	0 1	BODEN	C
BERENICETON	8	PIEDSAW	c	BITTER	8	BLEAKWOOD	C 1	BODENBURG	8
BERGHOLI	c !	BIENVILLE	A 1	BITTER SPRING	8	BLEDSDE	C 1	BODINE	8
BERGLAND	0 1	BIG BLUE	0 1	BITTERROOT	C	BLEIBLERVILLE	0 1	BODORUMPE	c
9ERGOU15T	8 1	BIG HORN	8 J	BITTERWATER	8	6 LENCOE	0 1	60007	C
BERGS TROM	8 1	RIG TIMBÉR	0 1	BITTON	6	BLEND	0 1	BOEL	4
BERGSVIK	0 1	BIGARM	9 1	BIVANS	0	I BLENDON	8 1	BOEL. OVERWASH	C
BERINO		BIGBEE	A İ	N IXB Y	8	BLETHEN	e 1	60£LU\$	A
BERIT	0 1	BIGBEND	8 1	BIXLER	C	BLEVINS	0 !	BOERNE	8
BERKS	c i	BIGGROWN	c i	BJORK	c	BLEVINION	B 1	8065EL	c
BERKSHIRE	8 1	BIGELOW	8 1	BLACHLY	8	BLEVETT	D 1	BOESEL . PROTECTED	ð
BERLAKE	B 1	BIGETTY	8 1	BLACK BUTTE	8 1	BLICHTON	D I	BOETTCHER	C
BEALIN	c i		D I	BLACK CANYON	D	BLICKENSTAFF	a I	BOGAN	c
BERMESA		BIGFOOT	ci	BLACK CANYON.	c	86140	8 (POGART	5
BERHUDIAN		BIGFORK	ςi					BOGGS	c
DERNAL	0 1		n 1		0	BLINN	c i	BOGGY	c
BERNALDO		BIGHILL		BLACKA		86155		BOGRAP	8
BERNARD		BIGLAKE		BLACKSJAN		BLITZEN		BOGUE	0
BERNARDINO		BIGHEADOW		BLACKDRAW		BLOCKHOUSE		BOGUS	Ċ
BERNARDSTON	-			BLACKETT	-	BLOMFORD		BOHANNON	č
BERNHILL		BIGRIVER		BLACKFOOT		BLOOM		BOHENTAN	
BERNICE		BIGSHEEP		BLACKFOOT. DRAINED		BLOOMFIELD		BOHICKET	0
BERNING		BIGSPRING		BLACKHALL		L ELOOMING		BOHNA	8
BERNING							-	DUNNA Y	
		BIGWIN		BLACKHALL. WARM		BLOOMSDALE			0
BERRYLAND		BIGWINDER		BLACKHAMMER		BLOOR COLVELLY		BUHNSACK	8
BERRYHAN		AL ROLL B		BLACKHANK		BLOOR, GRAVELLY		BOISTFORT	8
BERSON		81700		BLACKHOOF		SUBSTRATUM		BOJAC	ð
BERTAG		811.60		PLACKHORSE		BLOUNT		8630	D
BERTELSON		BILGER		BLACFLEED		BLOWERS		BOLAN	В
BERTHOUD		BILLETT		BLACKLEG		BLUCHER		EOL AR	C
BERTIE		BILLINGS		BLACKLOCK		BLUE EARTH		BOLD	9
BERTO	0 [BLACKMAN		BLUE EARTH.		BOLENT	A
6ERTCLOTT!	8		1	BLACKHOUNT	8 1	SLOPING	1	80L E S	C

NOTES: TWO HYDROLOGIC SOIL GROUPS SUCH AS BYC INDICATES THE ORAINED/UNDRAINED SITUATION.
MODIFIERS SHOWN, E.G., BEDROCK SUBSTRATUM, REFER TO A SPECIFIC SOIL SERIES PHASE FOUND IN SOIL MAP LEGEND.

REPORT CHECKLIST AND OUTLINE

PRELIMINARY DRAINAGE REPORT

CHECKLIST	ОК	NA
Typed text		
8½ x 11" format		
Bound: Use bar or spiral binder or staple. Do not use a notebook.		
Title Page: Name of report and preparer, date of preparation and revision (if any)		
Exhibits: Maximum 11" high and 32" wide, bound in report and folded as required to 8½ x 11" size		
Maps attached to or contained in the report: Vicinity Map and Preliminary Major Basin Drainago Map		

OUTLINE

- I. GENERAL LOCATION AND DESCRIPTION
 - A. Site and Major Basin Location
 - 1. Streets in the vicinity
 - 2. Development in the vicinity
 - B. Site and Major Basin Description
 - 1. Acreage
 - 2. Ground cover types
 - 3. Hydrologic soil types
- II. EXISTING DRAINAGE CONDITIONS
 - A. Major Basin
 - 1. General topography, drainage patterns and features, canals, ditches, wetlands
 - 2. Previously determined 100-year floodplains
 - B. Site
 - 1. Historic drainage patterns
 - 2. Inflow characteristics from upstream
 - 3. Discharge characteristics to downstream sub-basins
- III. PROPOSED DRAINAGE CONDITIONS
 - A. Changes in Drainage Patterns
 - 1. Major basin
 - 2. Site
 - B. Maintenance Issues
 - 1. Access
 - 2. Ownership and responsibility
- IV. DESIGN CRITERIA & APPROACH
 - A. General Considerations
 - 1. Previous drainages studies performed for the area
 - 2. Master planning issues (large scale considerations)
 - 3. Constraints imposed by site and other proposed development
 - B. Hydrology
 - 1. Design storms and precipitation
 - 2. Runoff calculation method
 - 3. Detention/retention basin design method
 - 4. Parameter selection procedures
 - 5. Analysis and design procedures
 - 6. Justification of proposed methods not presented or referenced in SWMM
 - B. Hydraulics
 - 1. Hydraulic calculation methods
 - 2. Parameter selection procedures
 - 3. Analysis and dosign procedures
 - 4. Justification of proposed methods not presented or referenced in SWMM

COMMENTS

- 1. No calculations are required for the Preliminary Drainage Report.
- 2. It may not be necessary to cover all of the above topics, but the report should address all concerns applicable to the proposed project, even issues not identified above.

SOIL SURVEY

Grand Junction Area, Colorado



Series 1940, No. 19

Issued November, 1955

UNITED STATES DEPARTMENT OF AGRICULTURE

Soil Conservation Service

In cooperation with the

COLORADO AGRICULTURAL EXPERIMENT STATION

Billings silty clay loam, 0 to 2 percent slopes (Bc).—This soil, locally called adobe, is one of the most important and extensive in the Grand Valley. It covers nearly one-fifth of the Grand Junction Area. The areas occur on the broad flood plains and very gently sloping coalescing alluvial fans along streams. Many large areas are north of the Colorado River.

The soil is derived from deep alluvial deposits that came mainly from Mancos shale but in a few places from fine-grained sandstone materials. The deposits ordinarily range from 4 to 40 feet deep but in places exceed 40 feet. The deposits have been built up from thin sediments brought in by the streams that have formed the coalescing alluvial fans or have been dropped by the broad washes that have no drainage channel. The thickest deposit, near Grand Junction, was built up by Indian Wash.

The color and texture of the soil profile vary from place to place. The S- to 10-inch surface soil normally consists of gray, light-gray, light olive-gray, or light brownish-gray silty clay loam. This layer grades into material of similar color and texture that extends to depths of 3 or 4 feet. Below this depth the successive depositional layers show more variation. Although the dominant texture is silty clay loam, the profile may have a loam, clay loam, fine sandy loam, or a very fine sandy loam texture.

Where there are fairly uniform beds of Mancos shale and where the soil is not influenced by materials deposited by adjoining drainage courses, the profile varies only slightly within the upper 3 or 4 feet. In areas bordering drainage courses, however, the soil varies more in texture and color from the surface downward.

One small area about 1½ miles southeast of Loma consists of light grayish-brown or pale-brown heavy silty clay loam that shows only slight variation in texture to depths of 4 to 6 feet. The underlying soil material is more variable. Below depths of 6 to 10 feet the layers generally are somewhat thicker and have a higher percentage of coarse soil material.

Also included with this soil are several small areas totaling about 3 square miles that are dominantly pale yellow. These are located 2½ to 3½ miles northeast of Fruita, 5 miles north of Fruita, 2½ miles northeast of Loma, 3 to 5 miles north of Loma, 1½ miles northwest of Loma, and 4 miles northwest of Mack. In these areas the 8- or 10-inch surface soil is pale-yellow silty clay loam, and the subsoil is a relatively uniform pale-yellow silty clay loam to depths of 4 to 8 feet. The accumulated alluvial layers are difficult to distinguish, but in a few places transitional to Fruita soils there are small areas having a pale-brown to light-yellowish brown color. These transitional areas are included with Billings silty clay loam because they have a finer textured subsoil than is characteristic of the Ravola soils.

Although moderately fine textured, this Billings soil permits successful growth of deep-rooted crops such as alfalfa and tree fruits. Its permeability is normally not so favorable as that of the Mesa, Fruita, and Ravola soils. Its tilth and workability are fair, but it puddles so quickly when wet and bakes so hard when dry that good tilth can be maintained only by proper irrigation and special cultural practices. Runoff is slow and internal drainage is very slow.

Like all other soils in the area, this one has a low organic-matter content. Under natural conditions it contains a moderate concen-

tration of salts derived from the parent rock (Mancos shale). In places, however, it contains so much salt that good yields cannot be obtained. Some large areas are so strongly saline they cannot be used for crops. Generally, this soil is without visible lime, but it is calcareous. In many places small white flecks or indistinct light-colored streaks or seams indicate that lime, gypsum, or salts are present.

Use and management.—About 80 percent of this soil is cultivated. The chief irrigated crops are alfalfa, corn, dry beans, sugar beets, small grains, and tomatoes and other truck crops. Where the soil is

located so as to avoid frost damage, tree fruits are grown.

Most of the field crops are grown in the central and western parts of the valley, or from Grand Junction westward. The entire acreage in tree fruits—approximately 3 square miles—lies between Grand Junction and Palisade. Because the climate is more favorable near Palisade, the acreage in orchard fruits is greater there. A few small orchards are located northeast of Grand Junction in the direction of Clifton. The main fruit acreage is between Clifton and Palisade. Peach orchards predominate, but a considerable acreage is in pears, especially near Clifton. Yields depend on the age of the trees and other factors, including management, but the estimated potential vield is somewhat less on this soil than on Mesa soils. This takes into account the slower internal drainage of this soil and its susceptibility to salinity if overirrigated. Yields of other crops vary according to the length of time the land has been irrigated, internal drainage or subdrainage, salt content of the soil, management practices, and local climate.

The uncultivated areas of this soil are mostly inaccessible places adjoining the larger washes, which occur mainly in the western part of the area, and those places that cannot be cropped profitably because they have inadequate drainage and a harmful concentration of salts. The uncultivated land supports a sparse growth of greasewood, saltbush, shadscale, rabbitbrush, ryegrass, peppergrass, and saltgrass. From 70 to 90 acres are required to pasture one animal

during a season.

A number of places shown on the map by small marsh symbols are low and seepy. They could be ditched, but their acreage is likely too small to justify the expense. Left as they are, their salt content

makes them worthless for any use except pasture.

Sizeable acreages of this soil apparently were overirrigated in the past. Irrigation water applied at higher levels to the north seeps upward in this soil where it occurs in low areas toward the river. Even now, new saline areas are appearing, and existing areas are getting larger. The total acreage affected by salts has remained more or less the same for the last two decades, but affected areas will continue to change in size and shape because of seepage.

Most fields are ditched where necessary. Some uncultivated areas require both leveling and ditching. In places subdrainage is inadequate because irregularities in the underlying shale tend to create pockets and prevent underground water from flowing into the drainage ditches. Also, in some areas where the alluvial mantle is 30 to 40 feet thick, the ditches are not always deep enough to drain the soil. Some areas are seepy because there are no ditches running in an east-west direction to intercept lateral flow of ground water from the over-

irrigated, permeable, medium-textured, stratified soils on the upper parts of the fan to the north. After being leveled, uncultivated areas would have to be cropped for 3 years before their salt content would be reduced enough to permit good yields.

Farmers can increase the organic-matter content of this soil by applying manure liberally and by growing alfalfa or clovers at least part of the time. A combination field erop and livestock type of farming favors improvement of this soil. Many of the small imperfectly drained areas may be kept in pasture. Strawberry clover and sweetclover are well suited, and mixtures of pasture grasses grow well.

Billings silty clay loam, 2 to 5 percent slopes (BD).—This soil covers a relatively small acreage in the Grand Valley. The areas are widely scattered. Except for its stronger slope, the soil is almost the same as Billings silty clay loam, 0 to 2 percent slopes. In a few places, notably north of Loma, there are areas having a pale-yellow color rather than the gray typical of the Billings soils.

Use and management.—Only about 15 percent of this soil is cultivated. Many of the areas lie along large drainageways or washes where they are difficult to reach. Even a larger number have such an uneven surface that considerable leveling would have to be done before they could be cropped. The cost of leveling, together with the expense of controlling crosion and gullying, discourages farmers from using them.

Many of the uncultivated areas have moderate concentrations of salts, but they are not particularly difficult to reclaim because they border natural ditches or washes which afford free disposal of irrigation water. Furthermore, for the most part, they have a porous substratum.

About the same crops are grown on this soil as on Billings silty clay loam, 0 to 2 percent slopes. The average yields are approximately the same.

Billings silty clay, 0 to 2 percent slopes (Ba).—This soil, locally called heavy adobe, occurs well toward the Colorado River. It is on alluvial materials—4 to about 40 feet thick—that largely came from Mancos shale. Most of this soil lies east and southeast of Grand Junction and along the railroad between Grand Junction and Fruita.

The 8- or 10-inch surface soil consists of light brownish-gray, gray, or olive-gray silty clay. The layer is similar to the surface layer of Billings silty clay loam soils but it is harder and, in many places, darker. The subsoil consists of similarly colored layers of silty clay loam, silt loam, and silty clay. In places the soil is silty clay to depths exceeding 4 feet.

The entire profile is firm when moist and has a massive structure. The subsoil has many small irregularly shaped light-gray specks or indistinct mottles. Poorly defined light-colored streaks indicate the presence of lime, gypsum, or salts. The surface soil and subsoil are calcareous, the lime being well distributed. The fine texture of the soil greatly retards penetration of roots, moisture, and air.

Surface runoff is very slow to slow where the slope is less than 1 percent. Internal drainage is very slow because the subsoil is massive and very slowly permeable. Even with ample drainage ditches, the discharge of irrigation water is slow.

Tilth and workability are not good, because the soil has a fine texture and a low content of organic matter. Moreover, some fields contain areas 20 to 60 feet across that have excessive amounts of salts. Slick spots also occur. These salty areas and slick spots produce low or negligible yields of most crops and are extremely difficult to eliminate.

Use and management.—About 75 percent of this soil is cultivated. Most of the rest is affected by salts. Small grains, beans, sugar beets, and alfalfa are the chief crops. They yield less than on Billings silty clay leam. 0 to 2 percent slopes. Ordinarily, newly broken fields are cropped to oats or other small grains the first few seasons so that excess salts can be removed. Afterwards, if drainage is adequate, they may be planted to pinto beans, sugar beets, corn, or alfalfa. The very slow permeability of this soil makes it unsuitable for orchard crops. Also, it is located mainly in areas where the frost hazard is great. Probably the greater part of the irrigable acreage is used for sugar beets. Small grains, alfalfa, and pinto beans usually follow in the order named.

Billings silty clay, 2 to 5 percent slopes (B_B).—This soil is similar to Billings silty clay, 0 to 2 percent slopes. It differs mainly in having greater slopes and a slightly finer textured and darker gray surface soil. In places, below depths of 3 or 4 feet, the silty clay or clay material is light olive gray.

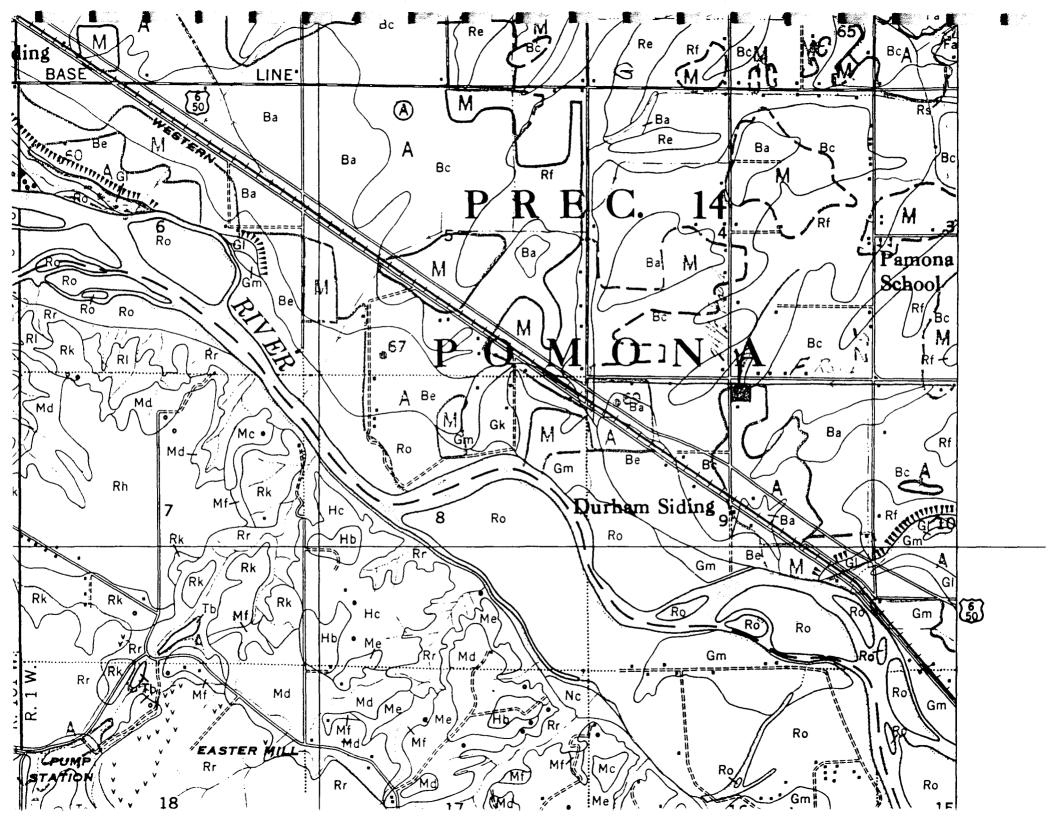
The tilth and workability are poor. Surface runoff is medium, and internal drainage is very slow. The soil is better suited to irrigation than most of the larger nearly level areas of Billings silty clay, 0 to 2 percent slopes, many of which are affected by salts. Approximately 12 acres of this soil is in peach orchards. All the rest is normally used for cultivated crops, principally corn, pinto beans, and alfalfa. This soil is suited to about the same crops as Billings silty clay, 0 to 2 percent slopes, but it generally produces better yields.

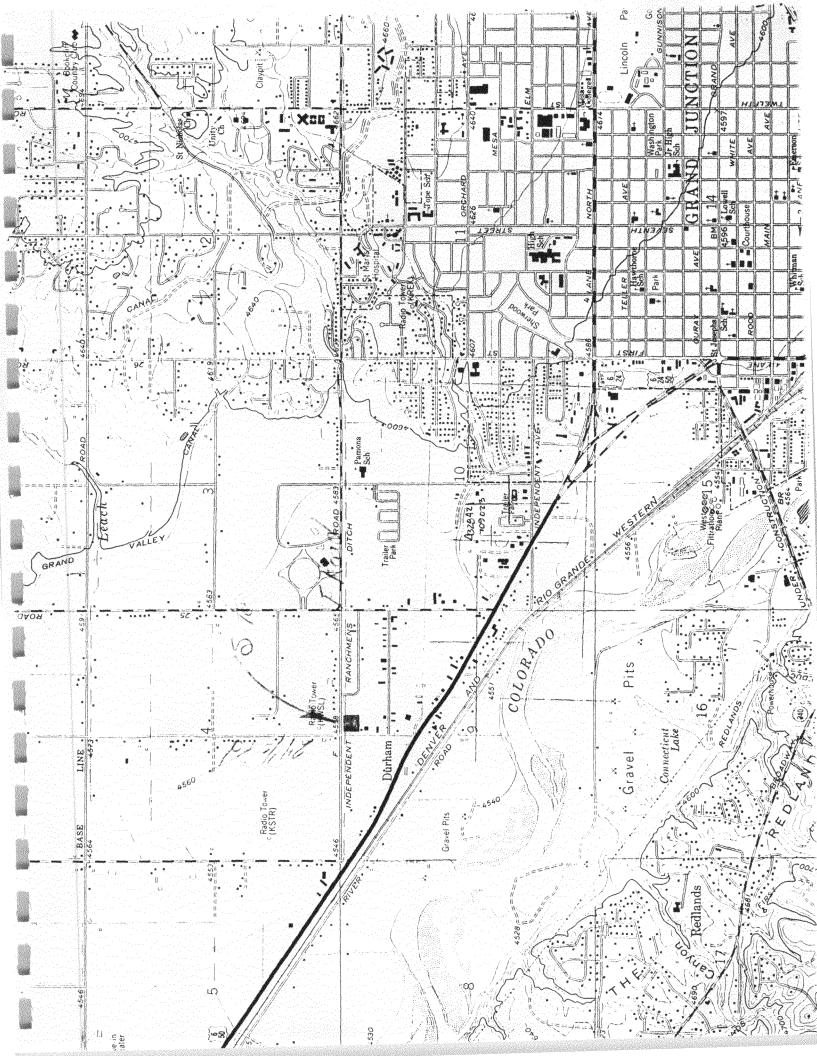
Billings silty clay, moderately deep over Green River soil material, 0 to 2 percent slopes (BE).—This soil occurs on the outer margin of coalescing alluvial fans where 1 to 4½ feet of fine-textured deposits derived from shale overlies Green River soil materials.

Except for a few strips only a few rods wide that adjoin low-lying areas of Green River soils, this soil has not been altered by high overflows from the Colorado River. It is not likely that the main part of the soil will be covered by floodwaters from the Colorado River, as it lies well above the level of normal overflow.

Use and management.—About \$5 percent of this soil is cultivated. The principal crops are alfalfa, corn, sugar beets, and pinto beans. A few peach orchards are on this soil near Clifton. Because the underlying strata are coarser, crops produce better on this soil than on most areas of the other Billings silty clay soils. Drainage and saline conditions have to be corrected before the soil will produce well.

Uncultivated acreages of this soil northwest of Grand Junction are saline, imperfectly drained, or both. Their tilth and workability are poor because they have a fine texture and a low content of organic matter.





BARNES AND NOBLE RETAIL STORE

Engineers Opinion of Cost

DATE:

8/20/96

NAME OF DEVELOPMENT:

BARNES AND NOBLE RETAIL STORE

LOCATION

NW/4 NE/4 SEC 9, T.1 S., R.1 W., U.P.M

PRINTED NAME OF PERSON PREPARING: JOHN C. SMITH

CONST	RUCTION COST ESTIMATE:			Unit	Total
1	Water system:	Units	Quant	Price	Price
1	8" Line Connect	EA	1	175.00	175
2	8" Fittings w/TB's	EA	5	180.00	900
3	6" Fittings w/TB's	EA	3	150.00	450
4	8" G.V.'s and box's	EA	2	500.00	1,000
5	6" G.V.'s and box's	EA	2	400.00	800
6	Fire Hydrant Assymblies	EA	2	1,200.00	2,400
7	8" PVC Waterline	LF	543	12.00	6,516
8	6" PVC Waterline	$_{ m LF}$	110	10.50	1,155
9	2 1/2" Fittings	EA	2	100.00	200
10	2 1/2" Serv. Line	$_{ m LF}$	26	9.50	247
			Sub-total	Potable Water:	13,843
				Unit	Total
:	Sewer system:	Units	Quant	Price	Price
1	8" Line Connect	EA	1	220.00	220
2	8" PVC Sewer Main	$_{ m LF}$	473	15.00	7,095
3	San. Sewer Manhole	EA	3	1,150.00	3,450
4	Drop Manhole Connection	EA	1	350.00	350
5	San. Sewer Serv. Tap	EA	1	50.00	50
6	6" Service Line	$_{ m LF}$	90	10.00	900
7	Service Cleanout	EA	1	650.00	650
			Sub-total !	Sanitary Sewer:	12,715
				Unit	Total
	Site grading and paving	Units	Quant	Price	Price
1	Unclassified Excavation	CY	230.00	1.50	345
2	Unclassified Embankment	CY	2100.00	9.00	18,900
3	Asphalt Removal	SY	85.00	3.00	255
4	Asphalt Patch	SY	85.00	30.00	2,550
5	Class-6 A.B.C.	CY	1015.00	16.50	16,748
6	3" Asphalt	SY	4736.00	5.50	26,048
7	6" Barrier Curb (1'Height)	$_{ m LF}$	895.00	11.00	9,845
8	2' Standard Curb/Gutter	LF	202.00	11.00	2,222
9	2.5' Drive Over Curb/Gutter	$_{ m LF}$	110.00	12.50	1,375
10	2.5' Door Walk	LF	162.00	10.00	1,620
11	5' Conc Walk	\mathtt{LF}	516.00	15.00	7,740
12	8' Conc Walk	$_{ m LF}$	118.00	24.00	2,832
13	1' Drive Over Curb	LF	350.00	11.50	4,025
14	3' Conc. V-Pan	$_{ m LF}$	280.00	16.00	4,480

FSF	400.00	11.50	4,600
FSF	350.00	17.50	6,125
SY	87.00	60.00	5,220
SY	50.00	55.00	2,750
EA	2.00	100.00	200
SY	72.00	35.00	2,520
EA	2.00	200.00	400
GAL	6.00	90.00	540
SY	28.00	55.00	1,540
SY	104.00	60.00	6,240
LS	1.00	4000.00	4,000
Sub-to	otal Site gradir	ng and paving:	133,120
		Unit	Total
Units	Quant	Price	Price
LS	1.00	800.00	800
LF	41	20.00	820
\mathtt{LF}	10	25.00	250
SY	20.00	50.00	1,000
uct EA	1.00	1,500.00	1,500
EA	1.00	1,500.00	1,500
	Sub-1	otal Drainage:	5,870
			Total
	~		Price
	_		500
			6,000
			1,500
	_	•	1,500
n Lt',			1,500
	Sub-t	otal irrigation:	11,000
Total S	ite Construc	tion Costs:	176,548
	FSF SY SY EA SY EA GAL SY SY LS Sub-t Units LF LF SY uct EA EA LF EA LF EA LF EA	FSF 350.00 SY 87.00 SY 50.00 EA 2.00 SY 72.00 EA 2.00 GAL 6.00 SY 28.00 SY 104.00 LS 1.00 Sub-total Site gradin Units Quant LS 1.00 LF 41 LF 10 SY 20.00 EA 1.00 EA 1.00 Sub-t Units Quant LF 41 LF 500.00 EA 1.00 Sub-t Sub-t LF 500.00 EA 1 Sub-t	FSF 350.00 17.50 SY 87.00 60.00 SY 50.00 55.00 EA 2.00 100.00 SY 72.00 35.00 EA 2.00 200.00 GAL 6.00 90.00 SY 28.00 55.00 SY 104.00 60.00 LS 1.00 4000.00 Sub-total Site grading and paving: Unit Units Quant Price LS 1.00 800.00 LF 41 20.00 SY 20.00 50.00 SY 20.00 50.00 SY 20.00 50.00 SY 20.00 50.00 EA 1.00 1,500.00 EA 1.00 1,500.00 Sub-total Drainage: Unit Units Quant Price EA 1.00 1,500.00 EA 1.00 25.00 Sub-total Drainage:

PERIBIT "BT



BARNES AND NOBLE RETAIL STORE Engineers Opinion of Cost

DATE: NAME OF DEVELOPMENT: LOCATION 9/10/96 BARNES AND NOBLE RETAIL STORE NW/4 NE/4, BEC 9, T.1 S., R.1 W., U.P.

PRINTED NAME OF PERSON PREP	ARING: JO	OKN C. SMITH		
CONSTRUCTION COST ESTIMATE:			Unit	Total
		·	Unit	ToLal
Site grading and paving	.Units	Quant	Price	Price
1 Unclassified Excavation	CY	65.00	2.00	1,30
2 Unclassified Embankment	CY	30.00	3.00	90
3 Class-6 A.B.C.	CY	48.00	16.50	792
4 5' Condrete Walk	LF	516.00	15.00	7,740
5 8" Concrete Driveway	SY	87.00	60.00	5,220
6 PSCO Relocates	LS	2	1,000.00	4,000
	Sub-ti	otal Site gradin	g and paving:	17,972
•	,		Unlt	Total
Drainage	Units	Quant	Price	Price
1 Connect to Outfall Ripe	EΆ	1.00	000.00	800
(with Grout Collar)		- · ·		
2 18" FVC Storm Sewer Pipe	LF	41.00	20.00	820
3 Rip-Rap	SY	20.00	50.00	1,000
4 Retention Fond	LS	1.00	5,000.00	5,000
5 Orifice Controlled Outlet	EA	1.00	1,500.00	1,500
Structure		Sub-t	otal Drainage:	9,120
	Total Off S	ite Construc	tion Costs:	27,092
	;			
5, + BUI	, Lie			0 011 0.
By:	BSZ	Manage	1	9-24-96
SIGNATURE O	F DEVELOPER	₹		DATE
I have reviewed the estimat on the plan layouts submitt take no exception to the ab	ed to date			
CITY ENGINE	ER	1		DATE

CITY ENGINEER DATE

COMMUNITY DEVELOPMENT DATE

EXHIBIT "B"

BARNES AND NOBLE RETAIL STORE Engineers Opinion of Cost

DATE: NAME OF DEVELOPMENT: 9/27/96

BARNES AND NOBLE RETAIL STORE

LOCATION

NW/4 NE/4, SEC 9, T.1 S., R.1 W., U.P.M

PRINTED NAME OF PERSON PREPARING:

JOHN C. SMITH

CONSTRUCTION COST ESTIMATE:

·			**	
			Unit	Total
ite grading and paving	Units	Quant		Price
Unclassified Excavation	CA	€5.00		130
Unclassified Embankment	CÄ	30.00		90
Class-6 A.B.C.	CA	48.00		792
5' Concrete Walk	LF	140.00	15.00	2,100
6' Concrete Walk	LF	384.00	17.00	6,528
8" Concrete Driveway	SY	95.00	60.00	5,700
PSCO Relocates	ĿS	1	4,000.00	4,000
	Sub-te	otal Site gradi	ng and paving:	19,340
ewer System	Units	Quant	Price	Price
8" Line Connect	EA	1.00	220.00	220
8" PVC Sewer Main	LF	458.00	15.00	6,87C
San Sewer Manhole	EA	2.00	1,150.00	2,300
Drop Manhole Connection	EA	1.00	350.00	350
San Sewer Serv Tap	EA `	1.00	50.00	50
6" PVC Sewer Bervice	T.F.	90.00	10.00	9.00
Service Cleanout	EA	1.00	650,00	650
		Sub-t	otal Drainage:	11,340
			Unit	Total
minage	Unita	Quant	Price	Price
Connect to Outfall Pipe (with Grout Collar)	£Ά	1.00	500.00	800
	LF	41.00	20.00	820
-	SY	20.00	50.00	1,000
•	LS	1.00	5,000.00	5,000
Orifice Controlled Outlet	EA	1.00	1,500.00	1,500
Structure		Sub-t	otal Drainage:	9,120
	Unclassified Excavation Unclassified Embankment Class-6 A.B.C. 5' Concrete Walk 6' Concrete Walk 8" Concrete Driveway PSCO Relocates **Wer System 8" Line Connect 8" PVC Sewer Main San Sewer Manhole Drop Manhole Connection San Sewer Serv Tap 6" PVC Sewer Bervice Service Cleanout **minage Connect to Outfall Pipe (with Grout Collar) 18" PVC Storm Sewer Pipe Rip-Rap Retention Pond	Unclassified Excavation Unclassified Embankment CY Class-6 A.B.C. CY 5' Concrete Walk Eff 6' Concrete Walk Eff 8" Concrete Driveway PSCO Relocates FWER System FWER System FWER System FUNITS B" Line Connect EA B" PVC Sewer Main FA Ban Sewer Manhole FA Ban Sewer Manhole FA San Sewer Serv Tap EA G" PVC Sewer Bervice FA Service Cleanout FA Connect to Outfall Pipe FA (with Grout Collar) T8" PVC Storm Sewer Pipe FA Retention Fond Corifice Controlled Outlet FA	Unclassified Excavation Unclassified Embankment CY 30.00 Class-6 A.B.C. CY 48.00 5' Concrete Walk LF 149.00 6' Concrete Walk LF 384.00 8" Concrete Driveway PSCO Relocates LS 1 Sub-total Site gradia EWER System Units Quant 8" Line Connect EA 1.00 8" PVC Sewer Main San Sewer Manhole Drop Manhole Connection San Sewer Serv Tap 6" PVC Sewer Service Service Cleanout EA 1.00 Sub-total Pipe (with Grout Collar) T8" PVC Storm Sewer Pipe Rip-Rap Retention Fond Orifice Controlled Outlet EA 1.00 Structure	Unclassified Excavation

Total Off Site Construction Costs:

39,800

SIGNATURE OF DEVELOPER

DATE

I have reviewed the estimated costs and time schedule shown above and, bas on the plan layouts submitted to date and the current costs of constructio take no exception to the above.

CITY ENGINEER	DATE
COMMUNITY DEVELOPMENT	DATE

October 04, 1996

To whom it may concern:

The site for the proposed Barnes & Nobel Bookstore, located at 2450 Patterson Rd. (tax schedule #2945-091-00-118), is zoned C-2 (heavy commercial). According to Section 4-3-4, Use/Zone Matrix, of the Zoning and Development Code, Retail Business is an allowed use in the C-2 zone.

If you have any further questions regarding this information, please do not hesitate to call the City of Grand Junction Community Development Department at (970)244-1430.

Sincerely,

Senta Costello Planning Technician

REVIEW COMMENTS

Page 1 of 2

FILE #SPR-96-190

TITLE HEADING: Barnes & Noble

LOCATION:

SE corner 24 ½ & Patterson Roads

PETITIONER:

Sittema-Bullock

PETITIONER'S ADDRESS/TELEPHONE:

5445 DTC Parkway, #4 Englewood, CO 80110

303-770-7275

PETITIONER'S REPRESENTATIVE:

Ciavonne & Associates

STAFF REPRESENTATIVE:

Michael Drollinger

NOTE: THE PETITIONER IS REQUIRED TO SUBMIT FOUR (4) COPIES OF WRITTEN RESPONSE AND REVISED DRAWINGS ADDRESSING ALL REVIEW COMMENTS.

CITY COMMUNITY DEVELOPMENT

Michael Drollinger

244-1439

- 1. Lighting Plan for parking lot is required as per Code.
- 2. Bicycle parking is required see Code; also provide bicycle rack detail.
- 3. Landscaping Plan is acceptable but will need to be resubmitted if site plan revisions are required.
- 4. "Stamped" concrete pedestrian decorative crosswalk detail not provided with Landscape Plan; please provide for review.
- 5. Please contact project planner (244-1439) regarding Comment #4 of the Grand Valley Irrigation Company BEFORE revising plans.

CITY DEVELOPMENT ENGINEER

9/4/96

Jody Kliska

244-1591

- 1. The city TEDS manual requires minimum on-site stacking distance at the driveway of 40'. It appears possible to reconfigure the parking near the driveway to meet this minimum stacking distance.
- 2. Please provide a copy of the common access agreement with the adjacent parcel.
- 3. Does the improvements agreement include the curb, gutter and sidewalk improvements on 24 ½ Road and on Patterson? Typically the improvements agreement covers the public improvements and drainage improvements.
- 4. Drainage report is acceptable.

CITY UTILITY ENGINEER

9/4/96

Trent Prall

244-1590

- 1. Please contact Jodi Romero of the City Customer Service Division at 244-1520 for information regarding sewer plant investment fees.
- 2. Please contact Dan Tonello with the Industrial Pretreatment section (244-1489) at the Persigo Sewer Treatment Plant for industrial waste review.
- 3. Public easement and sewer profile required for sewer extension.

SPR-96-190 / REVIEW COMMENTS / page 2 of 2

4. 6" sewer service line from Barnes and Noble building should terminate in a manhole at the 8" line.

CITY POLICE DEPARTMENT

9/3/96

Dave Stassen

244-3587

Is there a lighting plan. Further comments can hold until a lighting plan is provided.

CITY FIRE DEPARTMENT

9/3/96

Hank Masterson

244-1414

The Fire Department has no problems with this proposal.

A complete set of sealed building plans must be submitted to the Fire Department for our review. Also, a complete set of plans, specifications, and calculations for the fire sprinkler system and fire alarm system must be submitted. Allow 10 working days for completion of plan reviews.

MESA COUNTY BUILDING DEPARTMENT

8/28/96

Bob Lee

244-1656

Need to submit 2 sets of sealed plans and allow 10-14 days for plan review and permit issuance. City licensed contractors are required to perform work.

GRAND VALLEY IRRIGATION

9/3/96

Phil Bertrand

242-2762

- Need clear understanding of storm water into Independent Ranchman ditch (not sure it will be allowed).
- 2. A canal break and a unique storm on the Bookcliffs in the last 20 years has caused large amounts of run-off water to cross over this property.
- 3. Must show detailed design of Patterson Road access (this is not to interfere with canal access road).
- 4. NO permanent building/structures or plants within 30 feet of Independent Ranchman ditch or pipe.
- 5. Need clear documentation for crossing Independent Ranchman ditch.

UTE WATER

9/3/96

Gary Mathews

242-7491

- 1. Contact with Ute Water is needed to discuss the water line locations and back flow prevention for fire protection.
- 2. Water mains shall be c-900, class 150. Installation of pipe fittings, valves and services including testing and disinfection shall be in accordance with Ute Water standard specifications and drawings.
- 3. Construction plans required 48 hours before development begins.
- 4. Policies and fees in effect at the time of application will apply.

TO DATE, NO COMMENTS RECEIVED FROM:

Grand Junction Drainage District

August 20, 1996

Mr. Michael Drolinger Community Development Department City of Grand Junction 250 North 5th St. Grand Junction, CO. 81501

Dear Mr. Drolinger,

The following is a response to the review Agency Comments concerning the Barnes & Noble Site Plan Submittal, File #SPR-96-190.

Community Development Staff

- 1. Lighting Plan has been included. The isolux diagram as well as the electrical plan showing the locations has been included.
- 2. Bicycle parking has been shown on the Landscape Plan, included in this submittal, with a detail.
- 3. Landscaping has been adjusted to accommodate the entrance reconfiguration.
- 4. Detail for stamped concrete are shown on the site plan.
- 5. Grand Valley Irrigation has been contacted, and once the drainage system was explained, had no problems with the proposed system.

Community Development Engineer

- 1. Plans have been changed to reflect the stacking distance requirement.
- 2. Access agreement has been included with this submittal.
- 3. Improvement agreement has been included with this submittal.

Grand Junction City Utility Engineer

- 1. Thank you for direction concerning fees. City has been contacted concerning sewer investment fees.
- 2. Industrial pre-treatment has been addressed.
- 3. Easement is shown on the Site Plan. Sewer Profile has been included on Utilities composite.
- 4. Manhole has been added.

Grand Junction Police Department

1. Lighting Plan has been included.

Grand Valley Irrigation

- 1. Grand Valley Irrigation has been contacted, and once the drainage system was explained, had no problems with the proposed system.
- 3. Design of Patterson Road access an ditch crossing is shown.

Ute Water

1. Applicant has no problem with Ute requirements.

File #214-94

Sincerely,

Craig Roberts | Secretary/Treasurer Ciavonne & Associates, Inc.



September 24, 1996

Mr. Michael T. Drollinger City of Grand Junction 250 North 5th Street Grand Junction, CO 81501-2668

Re: Barnes & Noble Bookstore

Southeast Corner of 24 1/2 Road and F Road

Dear Michael:

I have enclosed two fully executed copies of the Development Improvements Agreement for the above referenced project. Thank you for sending me the form for this document.

I have attached, as Exhibit B, the cost estimate prepared by our civil engineer for the improvements anticipated by this Agreement. If the form and content of this document meets with your approval, I would appreciate your processing it for signature and returning one fully executed copy to me.

As a follow-up to a discussion we had last week, I would appreciate receiving from you a letter indicating that our proposed use of this site as a retail bookstore complies with the current zoning on this property. As I mentioned, our lender is requiring this "zoning compliance" letter.

Thank you again for all of your efforts on this project. I look forward to finalizing the planning process and beginning construction very shortly.

Sincerely,

SITBUL, LLC

A COLORADO LIMITED LIABILITY COMPANY

Timothy B. Sittema

Manager

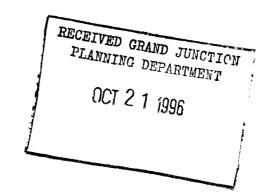
/dsm

Enclosures



October 18, 1996

Mr. Michael T. Drollinger City of Grand Junction 250 North 5th Street Grand Junction, CO 81501-2668



Re: Barnes & Noble Bookstore

Dear Michael:

I have enclosed three (3) executed copies of Exhibit B to the Development Improvements Agreement which we had discussed. It is my understanding that you will insert this revised Exhibit B into the documents that I have previously signed and forwarded to you. It is also my understanding that as this work is constructed by our General Contractor, he will be invoicing the City for this work and you will use these proceeds for payment to him. If this is not the case let me know.

I will forward a check in this amount next week. We look forward to getting going on our construction and sincerely appreciate all of your efforts on this project.

Sincerely,

SITBUL, LLC,

A COLORADO LIMITED LIABILITY COMPANY

Timothy B. Sittema Manager

/tas

October 29, 1996

Bob Lee Mesa County Building Department P.O. Box 20000 Grand Junction, CO 81502

Dear Bob,

After reviewing the <u>Restaurant Industrial Pretreatment Permit Application</u> submitted by Gordon Stanfield of Mac Gregor/Wathen Construction, for the Barnes & Noble Book Store to be built at 2450 Patterson Road, Grand Junction, it has been determined that this facility will not be required to install a grease interceptor. If you need more information, please call at 244-1489.

Sincerely,

Catherine Crabb Assistant Coordinator Industrial Pretreatment Program

cc: Trent Prall, Utility Engineer, City of Grand Junction Marcia Rabideaux, Community Development

To: Bobbie Paulson Cc: Kathy Portner

From: Michael Drollinger Subject: Barnes and Noble Date: 5/26/97 Time: 12:53PM

Bobbie:

If you get a C.O. on this while I'm gone please give it to Kathy for signature. I have inspected the site and the improvements have been installed satisfactorily.

Thanks.

mtd

Facsimile Cover Sheet



To: Mike Drollinger

Company: City of Grand Junction Planning Dept.

Phone: 970.244.1439 Fax: 970.244.1599

From: Michael Lewis

Company: Runyon Architects

Phone: 972.233.7705 Fax: 972.387.2553

Project: Grand Junction

RA+A No.: 9642

Date: 6.4.97

Pages incl. 2

cover:

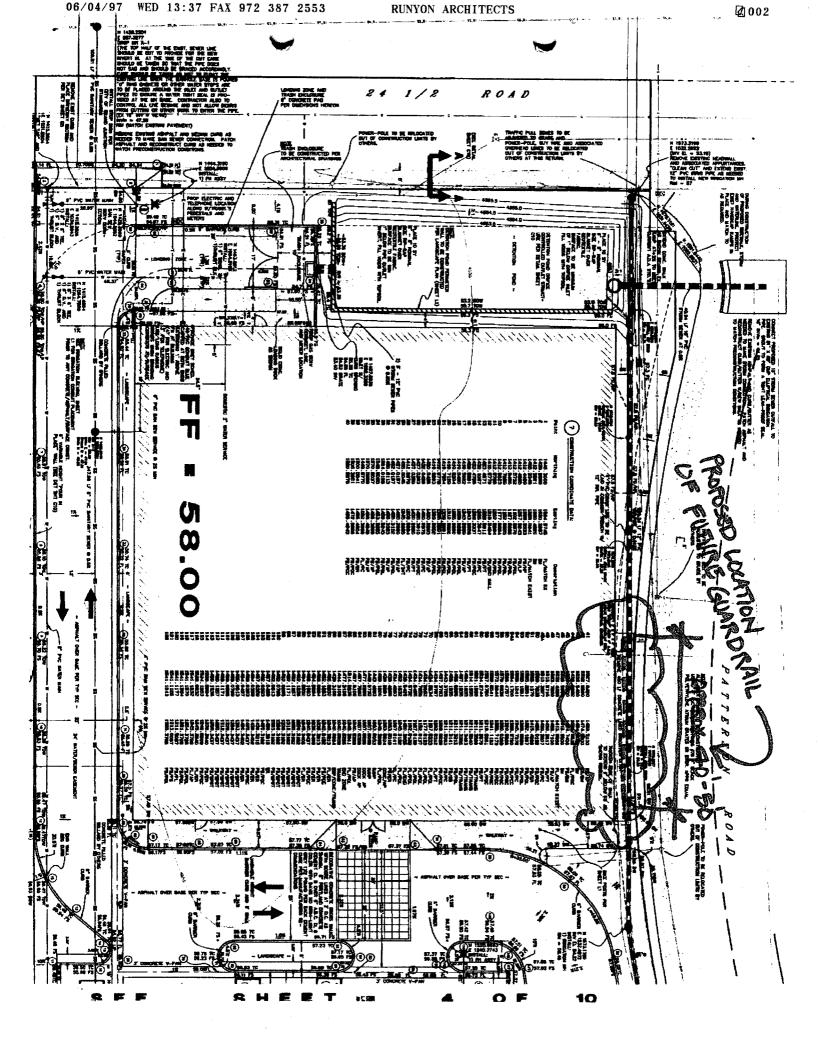
Comments:

Mike.

Per our discussion with you this morning, I have attached a site plan indicting the location for the proposed guardrail. The guardrail would be approx. 30" above finish grade and approx. 40' to 50' long. Please review the attached site plan and

Please contact me with questions and comments.

Respectfully, Michael Lewis



Facsimile Cover Sheet



To: Mike Drollinger

Company: City of Grand Junction Planning Dept.

Phone: 970.244.1439 Fax: 970.244.1599

From: Michael Lewis

Company: Runyon Architects

Phone: 972.233.7705 Fax: 972.387.2553

Project: Grand Junction

RA+A No.: 9642

Date: 7.7.97

Pages incl. 4

cover:

Comments:

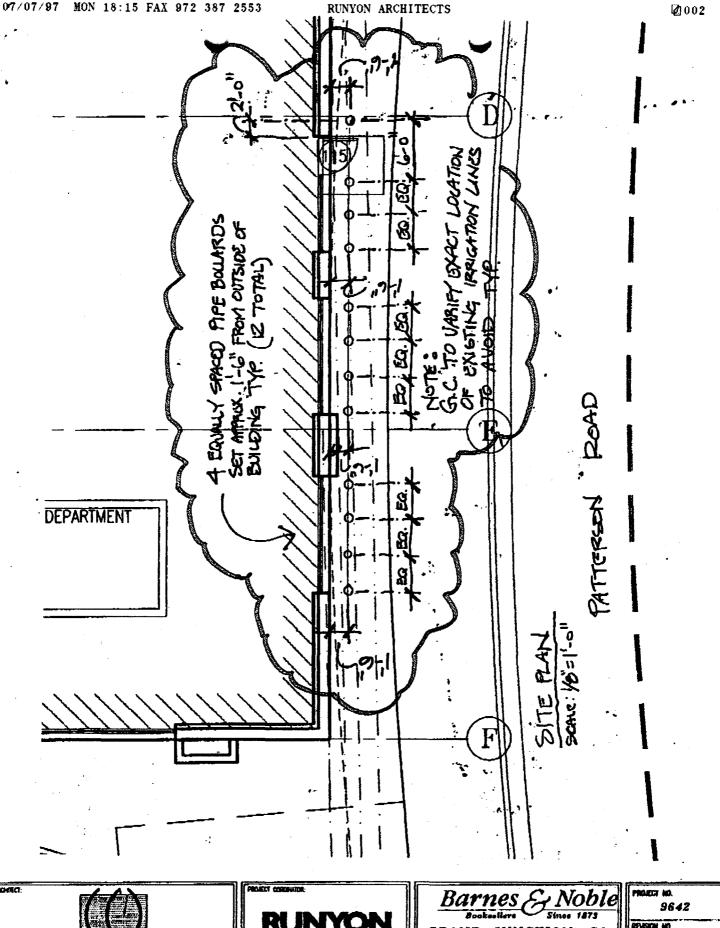
Mike.

Please review the attached sketches illustrating the pipe bollards proposed for the Grand Junction Barnes & Noble. If you have do not have any comments or revisions, we would like to issue this for construction as soon as possible.

Feel free to contact me with questions or comments.

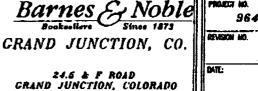
Respectfully, Michael Lewis

JUL 8 1997

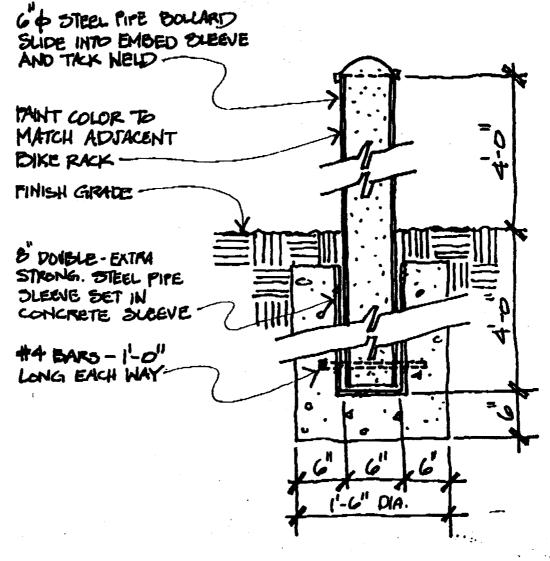








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PIPE BOLLARD.





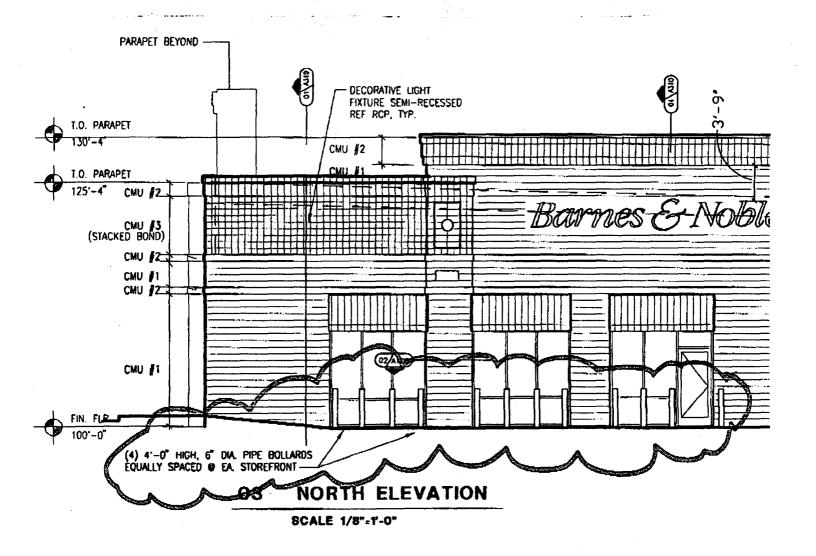
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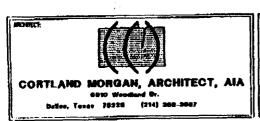
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GRAND JUNCTION, CO.

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CHANDLER SIGNS

3201 Manor Way Dallas, TX 75235-5909

FAX COVER SHEET

DATE:

March 14, 1997

TIME:

11:01 AM

TO:

Michael Drollinger

PHONE:

970-244-1439

City Planning

fax

970-244-1599

FROM:

Patty Pryor

PHONE:

214-902-2000

Chandler Signs

FAX:

214-902-2044

RE: BARNES & NOBLE BOOKSELLERS

Number of pages including cover sheet 1

MESSAGE:

Michael, I am sorry I missed you again. I got your voice mail this morning and found you will be out until Tuesday.

I Need to speak to you in regard to permitting information for

the Barnes & Noble Booksellers at 24-1/2 Road & "F" Road there in Grand Junction.

Please call me when you return.

Thanks

copy: Shannon Frost

CHANDLER SIGNS

3201 Manor Way Dallas, TX 75235-5909

FAX COVER SHEET

18

8130

DATE:

March 14, 1997

TIME: _11:04-4

TO:

Michael Drollinger

PHONE: 970-244-1439

City Planning

fax

970-244-1599

FROM:

Patty Pryor

PHONE:

214-902-2000

Chandler Signs

FAX:

214-902-2044

RE: BARNES & NOBLE BOOKSELLERS

Number of pages including cover sheet 1

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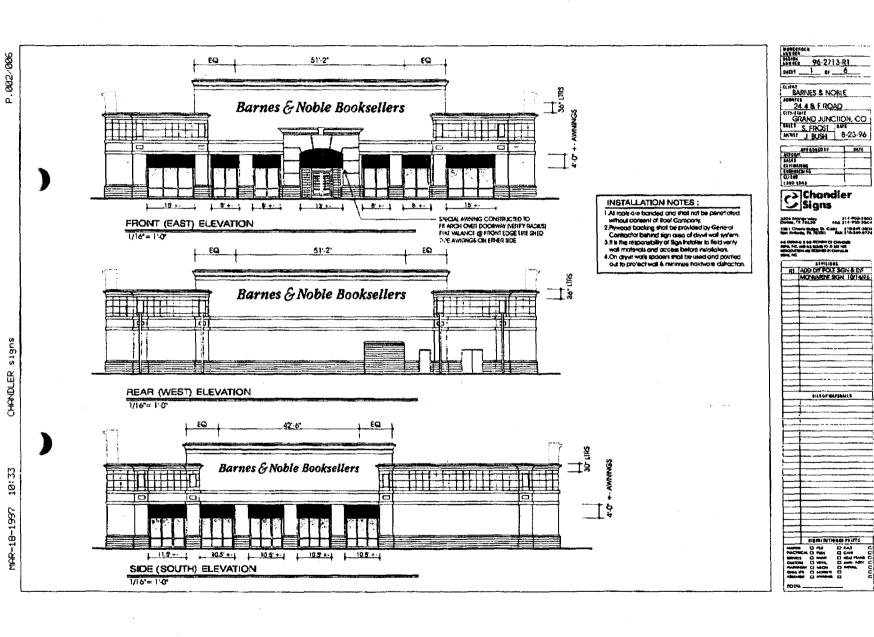
Thanks

copy: Shannon Frost

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CHANDLER

A ELEVATION - LETTER LAYOUT BN-SL-36 TWO (2) SIGNS REQUIRED

U.L. LASELS REQUIRED INSTALL IN ACCORDANCE WITH NATIONAL ELECTRICAL CODES

ELECTRICAL NOOX-UP TO BE PERFORMED BY CUSTOMER

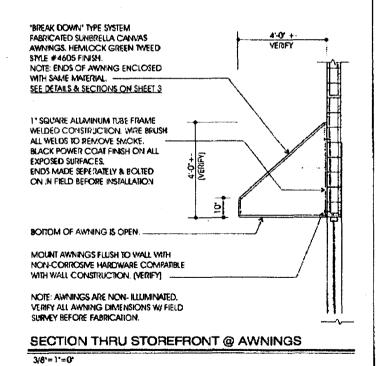
■ Barnes & Noble Booksellers

B ELEVATION - LETTER LAYOUT BN-SL-30

ONE (1) SIGN REQUIRED

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FACE LIT LETTER SECTION



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CERTIFICATE O. OCCUPANCY

BUILDING DEPARTMENT CITY OF GRAND JUNCTION (OR MESA DOUNTY)

PERMIT #58257	, ,	DATE 5-22-97
PERMISSION IS HEREBY GRANTED TO	Barnes & Noble Bookstore	TO OCCUPY THE
BUILDING SITUATED AT	2451 Patterson	
LOT BLOCK FILING	SUB1 IVISION	
TAX SCHEDULE NUMBER294	15-091-00-118	•
FOR THE FOLLOWING PURPOSE:	Bookstore with coffee shop	
	4.	
THIS CERTIFICATE ISSUED IN CONFORM.	ITY TO SECTIO 307, UNIFORM BUILDING	CODE
	INSPECTOR Horold	Oreal
	City Planning Way	n. l. d

SPR-1996-110

City of Grand Junction

Community Development Department
Planning ● Zoning ● Code Enforcement
250 North 5th Street
Grand Junction, CO 81501-2668

Phone: (970) 244-1430 FAX: (970) 244-1599

August 14, 1997

Deborah Alexus
The Canada Life Assurance Company
c/o Terrix Financial Corporation
1777 South Harrison Street #507
Denver CO 80210

Dear Ms. Alexus:

As per your request I am forwarding you this letter to confirm that the Barnes and Noble Bookstore located at 2451 Patterson Road is an allowed use in the C-2 (Heavy Commercial) zoning district, and that as of the issuance of the Certificate of Occupancy in June 1997 the store was in compliance with City zoning regulations.

I trust that this letter contains the information you requested. Please do not hesitate to contact me if you have any questions or require additional information.

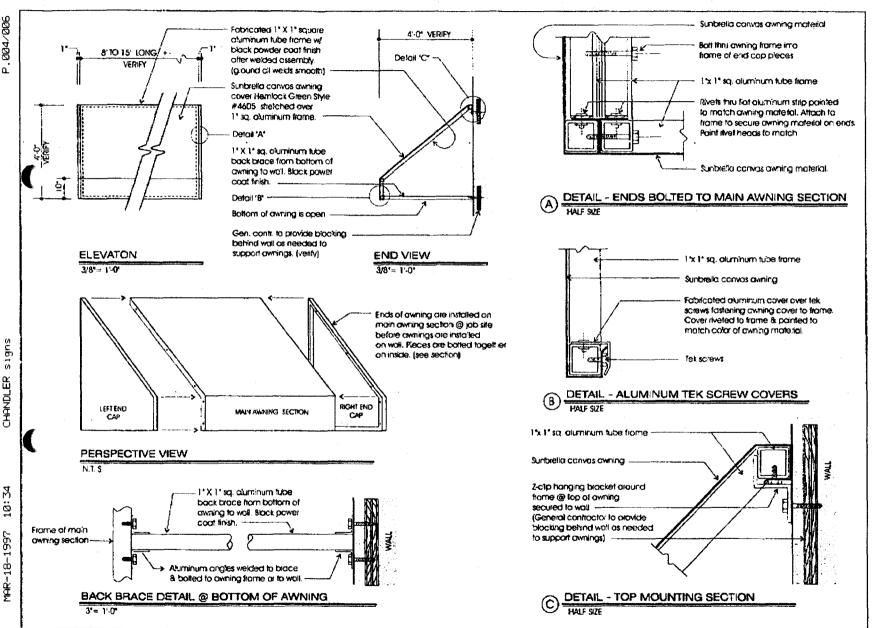
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Sincerely yours

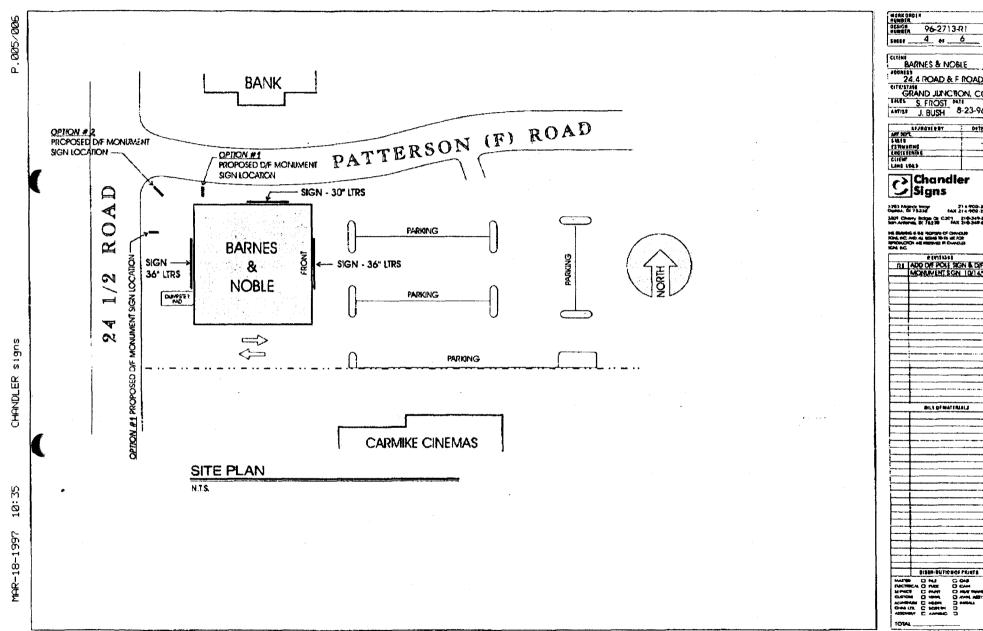
Michael T. Drollinger

Senior Planner

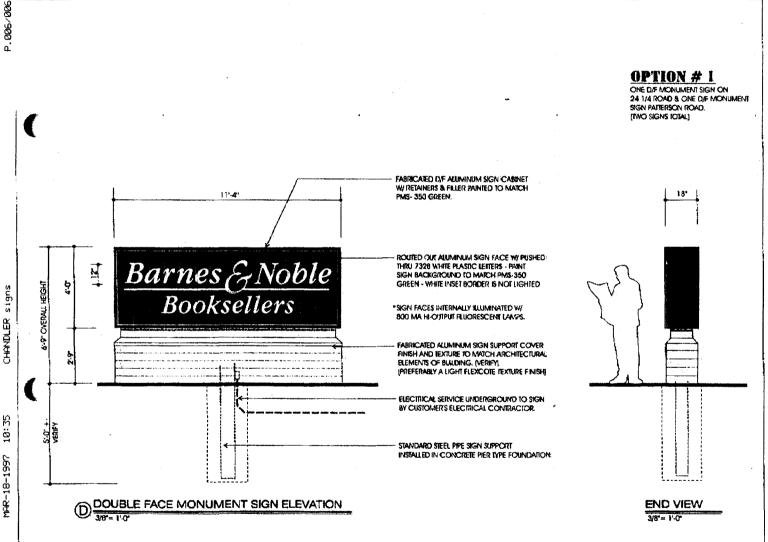
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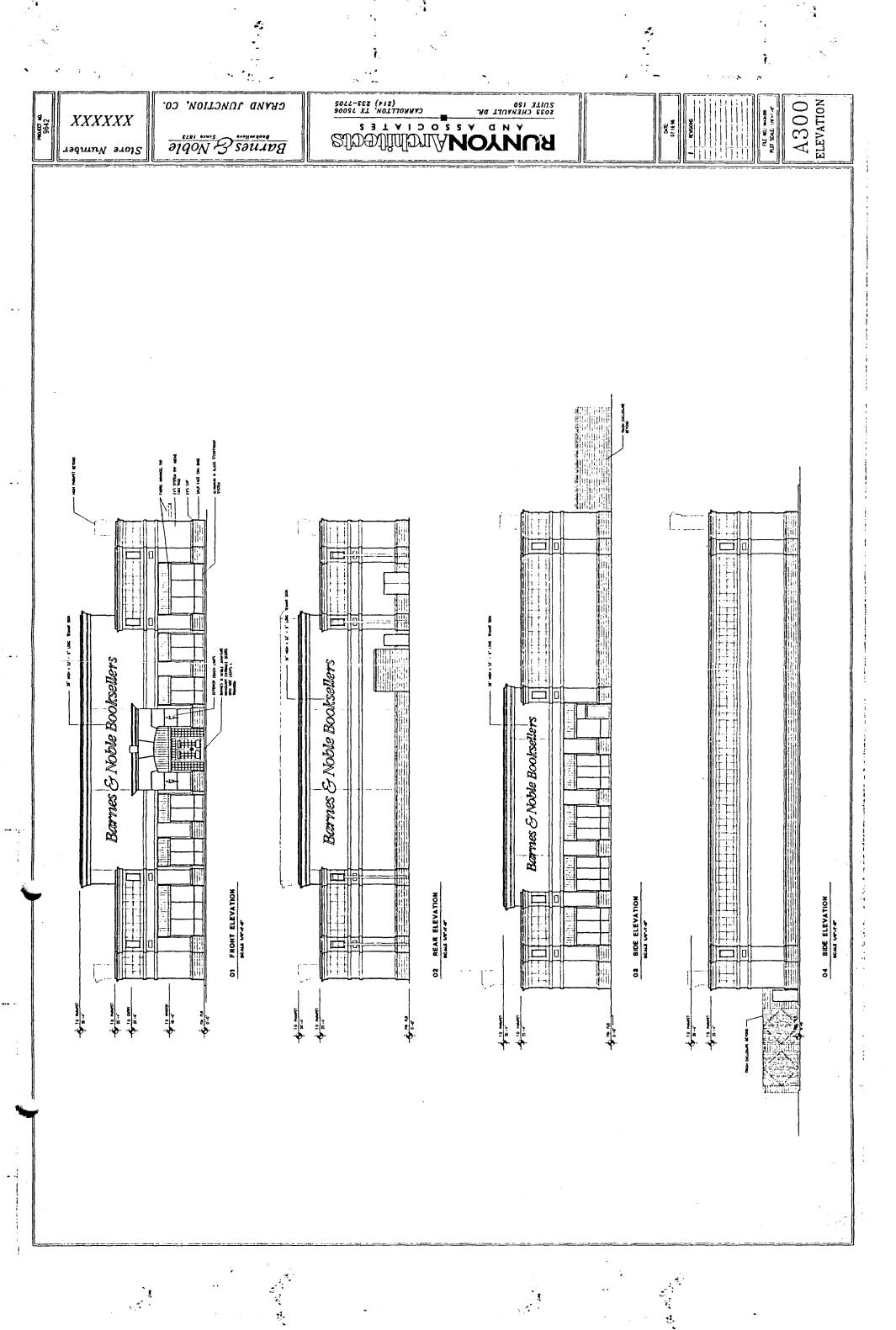
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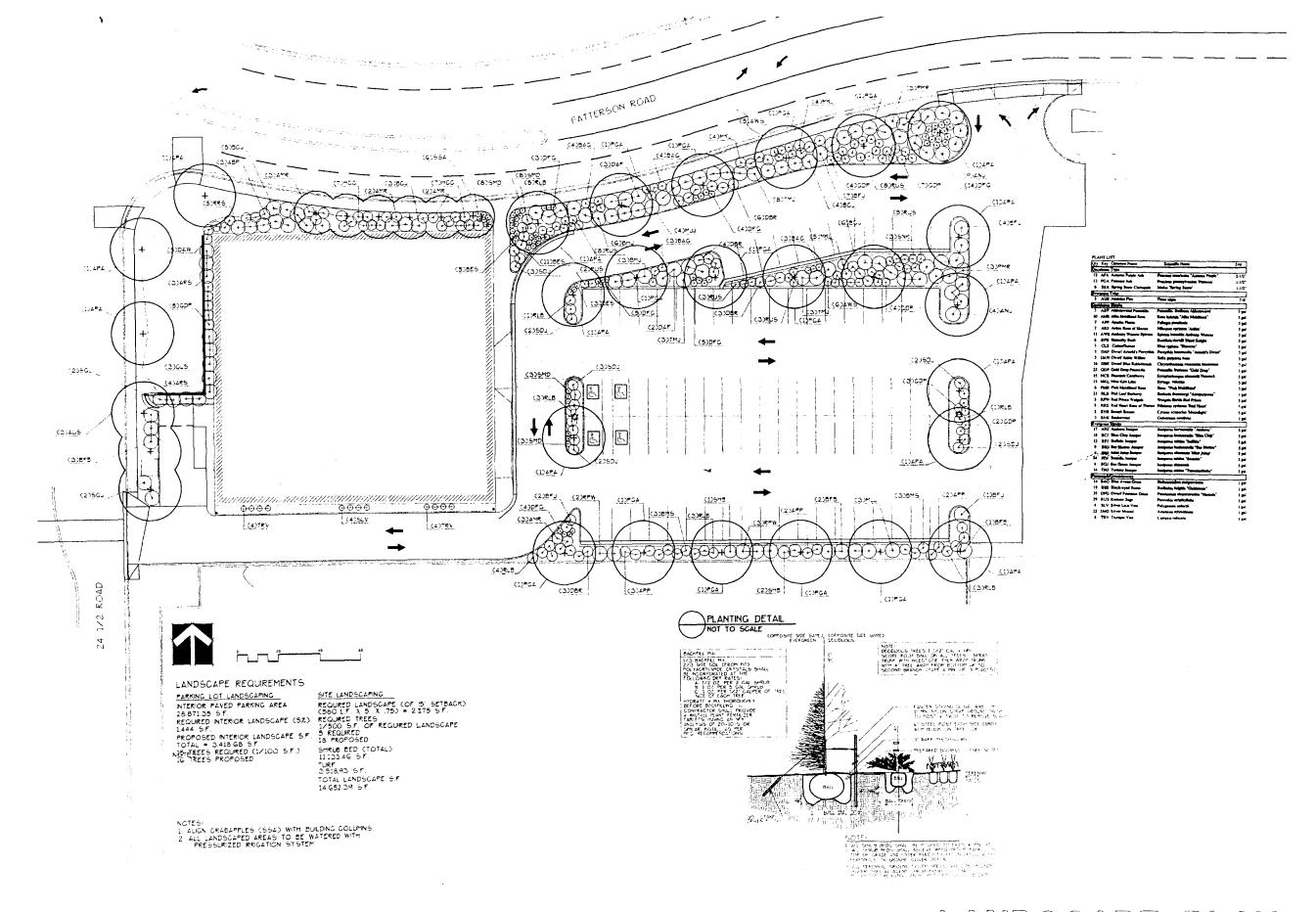
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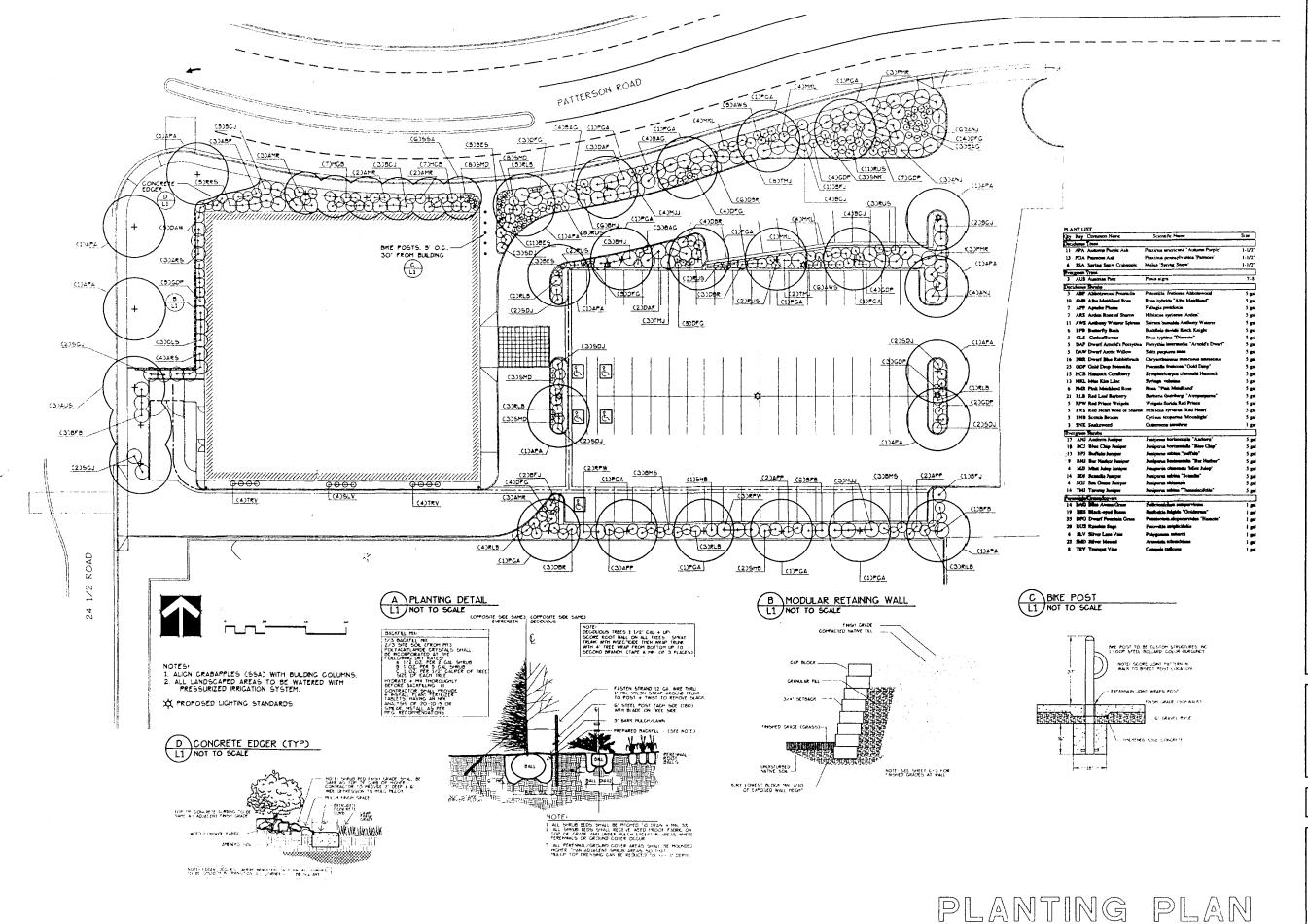
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CIAVONNE & ASSOCIATES, INC.

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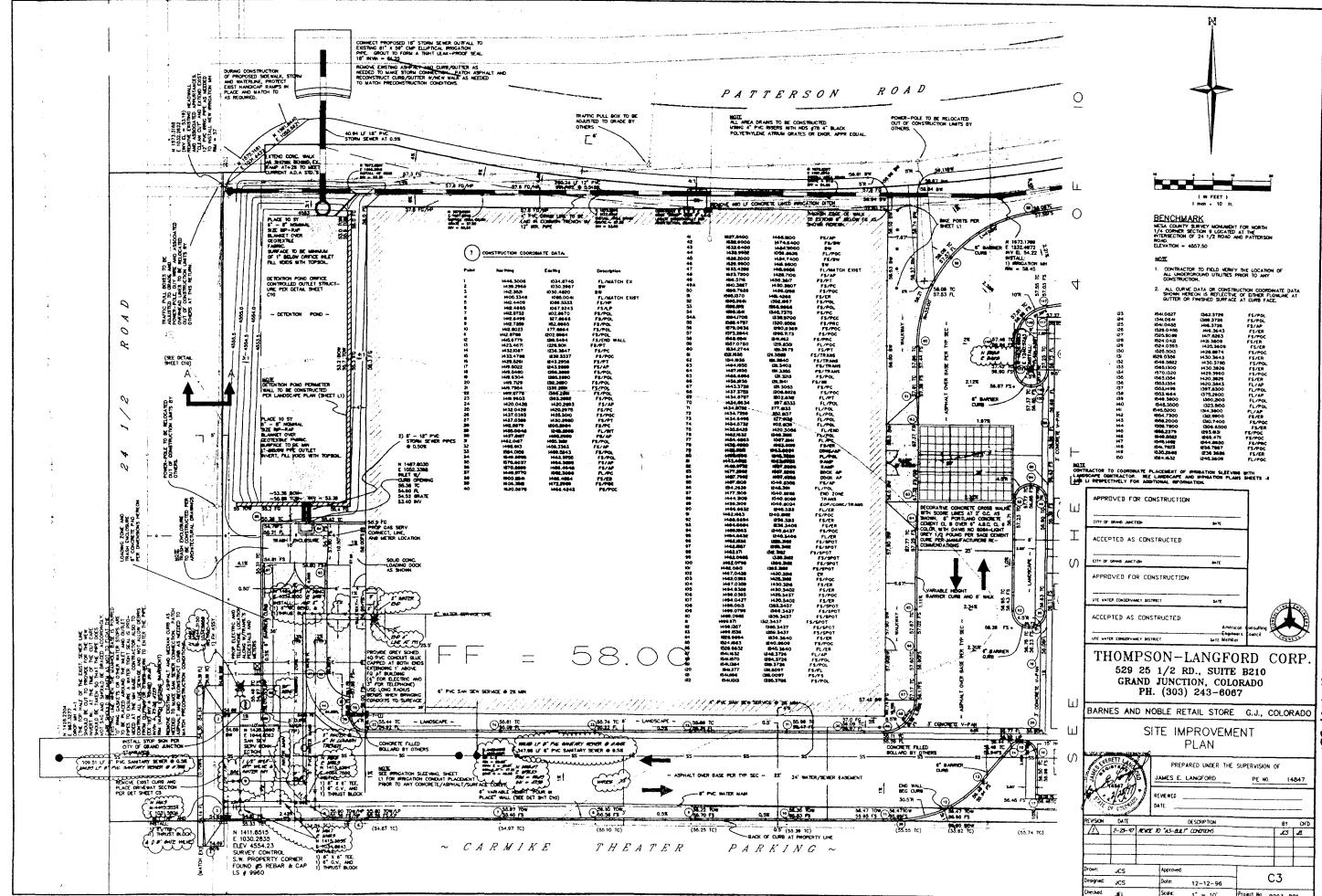
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LANDSCAPE AND PLANNING ARCHITECTS 844 CHAND AVE #1 GRAND JUNCTION, CO 970-241-0765 FAX 970-241-0765



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