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PRE-APPLICATION CONFERENCE							
Date: $3/26/96$ Conference Attendance: $10019$ Proposal: $3PR - Farm 1$ Location: $2/73$ <i>Piver R</i> Tax Parcel Number: $1697 - 3$ Review Fee: $5400 + $15$ (Fee is due at the time of submittal. A	5 Holfman Kiisten Emploments 264-0,0-061 ZC 264-0,0-061 ZC 164-0,0-061 ZC 164-0,0-061 ZC 164-0,0-061 ZC	Ashbeck ACX15 = 360 + 460					
Additional ROW required? Adjacent road improvements required Area identified as a need in the Maste Parks and Open Space fees required? Recording fees required? Half street improvement fees TCP re Revocable Permit required? State Highway Access Permit required On-site detention/retention or Drainage Applicable Plans, Policies and Guide	d? er Plan of Parks and Recreation? quired? <u>Per engineerin</u> ed? ge fee required? <u>Fer engin</u> lines	Estimated Amount: Estimated Amount: gEstimated Amount: Reening					
Located in identified floodplain? FIF	RM panel #						
Located in established Airport Zone? Avigation Easement required?	Clear Zone, Critical Zone, Area of	Influence?					
While all factors in a development pro items are brought to the petitioner's concern may be identified during the	oposal require careful thought, prep attention as needing special attenti review process.	aration and design, the following "checked" on or consideration. Other items of special					
Access Parking Drainage O Floodplain/Wetlands Mitigation O Other Related Files:	O Screening/Buffering X Landscaping O Availability of Utilities	O Land Use Compatibility O Traffic Generation O Geologic Hazards/Soils					
It is recommended that the applicant public hearing and preferably prior to	inform the neighboring property ow submittal to the City.	ners and tenants of the proposal prior to the					
PR	E-APPLICATION CONFE	RENCE					
WE RECOGNIZE that we, ourselves, and it is our responsibility to know w	, or our representative(s) must be pr hen and where those hearings are.	esent at all hearings relative to this proposal					
In the event that the petitioner is not re fee shall be charged to cover resched placed on the agenda. Any changes	presented, the proposed item will be duling expenses. Such fee must be to the approved plan will require a	dropped from the agenda, and an additional paid before the proposed item can again be re-review and approval by the Community					

WE UNDERSTAND that incomplete submittals will not be accepted and submittals with insufficient information, identified in the review process, which has not been addressed by the applicant, may be withdrawn from the agenda.

WE FURTHER UNDERSTAND that failure to meet any deadlines as identified by the Community Development Department for the review process may result in the project not being scheduled for hearing or being pulled from the

agenda.

Development Department prior to those changes being accepted.

m Signature(s) of Representative(s)

Signature(s) of Petitioner(s)

### GENERAL PROJECT REPORT October 28, 1996

This is an application for a Site Plan Review for a 23.6 acre parcel located at 2173 River Road. The zoning is I-1. We are proposing to use this property for sales of farm implements, farm supplies, horse trailers, etc. This is one of the allowed uses in the I-1 zone. We will build a 30 ft. by 40 ft. building from which we will operate.

The surrounding land uses are vacant land to the east, Persigo Wash to the west, I 70 on the southern boundary, and the railroad across the street to the north of River Road. The area is still very rural in nature and the use we are proposing is a perfect use to maintain the rural nature.

The site will be accessed from River Road and all traffic circulation will be on site on all weather gravel surfaces. Water is available from a Ute Water main in River Road. Sewer is available in River Road. Gas, electric and telephone are also available in River Road easements. We are proposing to install a fire hydrant on the River Road frontage to provide fire protection. Our impact on all of these utilities will be minimal as our proposed use is a low customer volume use. We will of course have no impact on schools or parks.

The soils and geology of the site have no impact. The site is typical desert with gradual to no slope. There is an existing pond which will be incorporated in our drainage improvements.

The hours of operation will be 8 to 5:30 +/- M-F some Saturdays, and we'll have seasonal fluctuations. Two to four people will be employed.

The signage for the business will consist of two 4 ft. by 8 Ft. plywood signs on the I 70 boundary installed in a wedge manner in order to be visible from both east and west bound I 70. There will be a small 20 in. by 40 in. sign on the gate to identify the entrance, and the Scott Murdock Trailer Sales logo on the building.

The schedule for development is to build the onsite improvements and building immediately upon obtaining a building permit, including the fire hydrant.

J1.34 pr.

DRAINAGE STUDY MURDOCK'S TRAILER SALES 2173 River Road Grand Junction, CO October, 1996

THIS DRAINAGE REPORT WAS PREPARED BY WESTERN ENGINEERS, INC. BASED ON PROPOSED SITE CONDITIONS AS SHOWN ON SITE PLAN FOR THE MURDOCK TRAILER SALES FACILITY BY WESTERN ENGINEERS, DRAWING 3913-1248-2, DATED NOVEMBER 1, 1996. IN ACCORDANCE WITH COMMENTS RECEIVED FROM THE CITY OF GRAND JUNCTION PLANNING DEPARTMENT, THE SITE PLAN FOR THE MURDOCK TRAILER SALES FACILITY WAS REVISED TO INCLUDE PAVED PARKING AND DRIVE AREAS AS SHOWN ON WESTERN ENGINEERS DRAWING 3913-1248-2, DATED DECEMBER 6, 1996. BUT, THIS DRAINAGE REPORT WAS NOT REVISED AND IS NOT APPLICABLE TO THE REVISED SITE PLAN.

Prepared by:

WESTERN ENGINEERS, INC. 2150 Highway 6 AND 50 Grand Junction, CO 81505

Work Order Number 3913

#### DRAINAGE STUDY MURDOCK'S TRAILER SALES

October, 1996

GENERAL:

The proposed trailer sales lot and associated building site is located South of River Road between Persigo Wash and Interstate 70 in Grand Junction, Colorado. The analyses presented in this report are for a parcel of about 8.6 acres identified as Lot 1 of the minor subdivision. The site has historically been under agricultural cultivation but is currently vacant land. The site exhibits slight drainage to the southwest. A stock watering pond exists in the southeast portion of the lot which is intended to be utilized as a detention pond for post-development runoff. No off-site runoff is collected by this property. Site soils are categorized by the National Resource Conservation Service as Billings Clay with a Hydrologic Soil Group classification of "C". All site runoff from Lot 1 will flow through the detention pond and, from there, westerly in an existing channel along the southern boundary of Lots 1 and 2 to an existing 18 inch diameter corrugated metal pipe used to drain runoff from Interstate 70 into Persigo Wash. In order to reach this culvert, runoff flows must drain into the drain ditch within the Interstate Right of Way near the culvert location.

DRAINAGE EVALUATION METHOD:

Hydrologic analyses for this site were performed by the Modified Rational Method. Hydrologic parameters and storm intensities were determined using the recommended values and procedures presented in the January, 1996 edition of the City of Grand Junction's <u>Stormwater Management Manual</u> (SWMM). The predevelopment conditions were treated as a single basin. The post-development Lot 1 areas were divided into two basins. Basin number 2 includes exclusively the areas to be developed and which will drain into the detention pond. Basin number 1 consists of the areas which will not be affected by site development and from which runoff drainage circumvents the detention pond. Drainage basin delineations are shown on figure 1. Since the detention pond currently exists, the procedure for determining the routing effects of the pond on runoff rates was determined using the characteristics of the existing pond.

#### **CONCENTRATION TIMES:**

The upper 300 feet of both the pre-development drainage basin and the postdevelopment basin number 1 were considered to exhibit sheet flow. Concentration times were calculated by the three following equations presented in the SWMM:

SCS TR-55 Method

$$T_0 = 0.42 (NL)^{0.8} / P^{0.5} S^{0.4}$$

FHWA Method

$$T_{0} = 0.93 (NL)^{0.6} / I^{0.4} S^{0.3}$$

FAA Method

 $T_0 = 1.8(1.1-C)L^{0.5}/S^{0.33}$ 

Tables 1, 2 and 3 show comparisons of the results of these three methods. The results of the FHWA method were used because they seemed to represent approximately the average of the values produced by the three methods and because the method is suited to the Rational Method for runoff evaluation.

The lower portion of both the pre-development drainage basin and the postdevelopment basin number 2 were considered to exhibit shallow concentrated flow. The SCS, TR-55 method was used to determine concentration times attributed to this type of flow. The "Nearly bare and untilled and alluvial fans western mountain regions" curve was used. The contribution of shallow concentrated flow to the concentration times are shown on tables 1 and 2.

The longest runoff path for the post-development Basin number 2 area

consists almost exclusively of channel flow. The estimated hydraulic characteristics for this channel are shown on Table 4. The concentration times for post-development Basin number 2 were calculated using the channel velocities determined by the Manning Formula at the respective flows. This calculation is shown on Table 3.

#### RATIONAL RUNOFF COEFFICIENTS:

Using the Hydrologic Soil Group of "C", the runoff coefficients were determined in accordance with the recommendations presented in the SWMM. Both the pre-development drainage basin and the post-development basin number 1 consist entirely of bare ground. The post-development basin number 2 includes a combination of gravel surfaced drive and parking areas along with the building roof. The weighted runoff coefficients for this basin were determined as follows:

STORM RECUR INTERVAL (Y	RENCE EARS)	SURFACE TYPE	AREA (ACRES)	RUNOFF COEFFICIENT	WEIGHTING VALUE
2 2		Gravel Roof	0.506	0.68 0.93	0.344 0.026
	Totals		0.534		0.370
	Weighte	1 Runoff Coef	ficient		0.693
100		Gravel	0.506	0.76	0.385
100	Totals	Roof	<u>0.028</u> 0.534	0.95	$\frac{0.027}{0.412}$
	Weighted	1 Runoff Coef	ficient		0.772

Following is a summary of the runoff coefficients used in the analyses:

DEVELOPMENT	BASIN	STORM RECURRENCE	RUNOFF
STATUS	IDENTIFICATION	INTERVAL (YEARS)	COEFFICIENT
Pre-Development	N/A	2	0.24
Pre-Development	N/A	100	0.30
Post-Development	1	2	0.24
Post-Development	1	100	0.30
Post-Development	2	2	0.69
Post-Development	2	100	0.77

#### DETENTION FACILITIES:

The storage rating for the existing pond is depicted on figure 2. Because the existing pond is large relative to the size of the area to be developed, detention discharge control can be adequately provided by the dirt channel to be established. This outlet ditch runs to the west along the southern boundaries of Lots 1 and 2, drains into the Interstate 70 drain ditch and discharges into an 18 inch diameter culvert which flows into Persigo wash, as shown on figure 1. The ditch will be regraded as part of the development work. This ditch provides adequate detention discharge control because it is quite flat. The ditch was treated as a release weir for the purposes of determining the appropriate Modified Rational average release rate to maximum release ratio (.55).

#### CONCLUSIONS:

Because the post-development conditions include a drainage basin which bypasses the detention pond, it is necessary to evaluate the site discharges for two conditions to determine which is the critical. The first condition includes the storm intensity which produces the greatest discharge from the detention pond. The second condition involves use of the storm intensity which produces the greatest discharge from the basin which bypasses the detention pond (Basin 2). Modified Rational runoff routing calculations are summarized on Table 6. Following is a summary of both pre and post-development site discharges for the 2 and 100 year storms:

DEVELOPMENT STATUS	STORM RECURRENCE INTERVAL (YEARS)	BASIN IDENTIFICATION	STORM INTENSITY (INCH/HR)	SITE DISCHARGE (CU FT/SEC)
Pre-Development	2	N/A	0.51	1.06
Pre-Development	100	N/A	1.67	4.32
Post-Development ( Basin 2 Control	2	1	0.09	0.17
Post-Development ( Basin 2 Control	2	2	0.09	0.004
Post-Development ( Basin 2 Control	2	combined	0.09	0.174
Post-Development ( Basin 2 Control	100	1	0.89	2.16
Post-Development ( Basin 2 Control	100	2	0.89	0.11
Post-Development ( Basin 2 Control	100	combined	0.89	2.27
Post-Development ( Basin 1 Control	2	1	0.31	0.60
Post-Development ( Basin 1 Control	2	2	0.31	0.003
Post-Development ( Basin 1 Control	2	combined	0.31	0.603
Post-Development ( Basin 1 Control	100	1	1.57	3.82
Post-Development ( Basin 1 Control	100	2	1.57	0.09
Post-Development ( Basin 1 Control	100	combined	1.57	3.91

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It is seen that the time of concentration for the post-development Basin 1 controls the critical combined rainfall intensity and that, under all conditions, the post-development flows are less than those for the pre-development conditions for comparable storm intensities.

THIS DRAINAGE REPORT WAS PREPARED BY WESTERN ENGINEERS, INC. BASED ON PROPOSED SITE CONDITIONS AS SHOWN ON SITE PLAN FOR THE MURDOCK TRAILER SALES FACILITY BY WESTERN ENGINEERS, DRAWING 3913-1248-2, DATED NOVEMBER 1, 1996. IN ACCORDANCE WITH COMMENTS RECEIVED FROM THE CITY OF GRAND JUNCTION PLANNING DEPARTMENT, THE SITE PLAN FOR THE MURDOCK TRAILER SALES FACILITY WAS REVISED TO INCLUDE PAVED PARKING AND DRIVE AREAS AS SHOWN ON WESTERN ENGINEERS DRAWING 3913-1248-2, DATED DECEMBER 6, 1996. BUT, THIS DRAINAGE REPORT WAS NOT REVISED AND IS NOT APPLICABLE TO THE REVISED SITE PLAN.

RS, INC. P.E. ruce'D. 12-6-96





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FIGURE 2

#### PRE-DEVELOPMENT CONDITIONS OVERLAND FLOW COMPONENT

OVERLAND FLOW RESISTANCE FACTOR, N =	0.07
FLOW PLANE LENGTH, L (FEET) =	_ 300
2 YEAR PRECIPITATION DEPTH, P(2) (INCH) =	0.42
100 YEAR PRECIPITATION DEPTH, P(100) (INCH) =	0 0018
AFERAGE DEVICE OF FLOW FLARE, D = 9 VEAD DAINEANT INTENETTY 1(9) (INCH/HD) =	0.0040
100 YEAR RAINFALL INTENSITY, 1(100), (INCH/HR) =	1.33
2 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT. C(2) =	0.24
100 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT, C(100) =	0.3

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OVERLAND FLOW TRAVEL TIME

PRECIPITATION RECURRENCE INTERVAL (YEARS)	SCS TR-55 Method	FHWA, HEC-12 METHOD	FAA METHOD
2	62.65	44.14	34.16
100	34.31	25.58	31.78

#### SHALLOW CONCENTRATED FLOW COMPONENT

FLOW LENGTH (FEET) =	950
GROUND SLOPE =	0.0044
VELOCITY, SCS TR-55, (FT/SEC) =	0.7
SHALLOW CONCENTRATED FLOW TRAVEL TIME (MINUTES) =	22.6

#### CHANNEL/PIPE FLOW COMPONENT

CHANNEL/PIPE	LENGTH (FEET)	=	0
CHANNEL/PIPE	SLOPE =		0

PRECIPITATION RECURRENCE INTERVAL (YEARS)	DISCHARGE FLOW BATE (CU FT/SEC)	DISCHARGE FLOW VELOCITY (FT/SEC)	CHANNEL/PIPE TRAVEL TIME (MINUTES)
2	. 0	0.00	0.00
100	0	0.00	0.00

#### TOTAL TIME OF CONCENTRATION

PRECIPITATION RECURRENCE INTERVAL (YEARS)	OVERLAND FLOW TIME (MINUTES)	SHALLOW CONCENTRATED FLOW TIME (MINUTES)	CHANNEL AND PIPE FLOW TIME (MINUTES)	TOTAL TIME OF CONCENTRATION (MINUTES)
2	44.14	22.6	0	66.74
100	25.58	22.6	0	48.18

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#### POST-DEVELOPMENT CONDITIONS BASIN 1 OVERLAND FLOW COMPONENT

OVERLAND FLOW RESISTANCE FACTOR. N =	0.07
FLOW PLANE LENGTH, L (FEET) =	300
2 YEAR PRECIPITATION DEPTH, P(2) (INCH) =	0.42
100 YEAR PRECIPITATION DEPTH, P(100) (INCH) =	1.4
AVERAGE SLOPE OF FLOW PLANE, S =	0.0048
2 YEAR RAINFALL INTENSITY, 1(2), (INCH/HR) =	0.54
100 YEAR BAINFALL INTENSITY, 1(100), (INCH/HR) =	2.15
2 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT, C(2) =	0.24
100 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT, C(100) =	0.3

	OVERLAND FLOW TRAVEL	. TIME	
PRECIPITATION RECURRENCE INTERVAL (YEARS)	SCS TR-55 Method	FHWA, HEC-12 METHOD	FAA METHOD
2	62.65	36.68	34.16
100	34.31	21.11	31.78

#### SHALLOW CONCENTRATED FLOW COMPONENT

FLOW LENGTH (FEET) =	950
GROUND SLOPE =	0.0048
VELOCITY, SCS TR-55, (FT/SEC) =	0.7
SHALLOW CONCENTRATED FLOW TRAVEL TIME (MINUTES) =	22.6

#### CHANNEL/PIPE FLOW COMPONENT

CHANNEL/PIPE LENG	ITH (FEET) =	0
CHANNEL/PIPE SLOP	YE =	0

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PRECIPITATION RECURRENCE INTERVAL (YEARS)	DISCHARGE FLOW RATE (CU FT/SEC)	DISCHARGE FLOV VELOCITY (FT/SEC)	CHANNEL/PIPE TRAVEL TIME (MINUTES)
2	0	0.00	0.00
100	0	0.00	0.00

#### TOTAL TIME OF CONCENTRATION

PRECIPITATION RECURRENCE INTERVAL (YEARS)	OVERLAND FLOW TIME (MINUTES)	SHALLOW CONCENTRATED FLOW TIME (MINUTES)	CHANNEL AND PIPE FLOW TIME (MINUTES)	TOTAL TIME OF CONCENTRATION (MINUTES)
2	44.14	22.6	0	66.74
100	25.58	22.6	0	48.18

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#### POST-DEVELOPMENT CONDITIONS BASIN 2 OVERLAND FLOW COMPONENT

OVERLAND FLOW RESISTANCE FACTOR, N =	0.00
FLOW PLANE LENGTH, L (FEET) =	0.00
2 YEAR PRECIPITATION DEPTH, P(2) (INCH) =	0.00
100 YEAR PRECIPITATION DEPTH, P(100) (INCH) =	0.00
AVERAGE SLOPE OF FLOW PLANE, S =	0.00
2 YEAR RAINFALL INTENSITY, 1(2), (INCH/HR) =	0.00
100 YEAR RAINFALL INTENSITY, 1(100), (INCH/HR) =	0.00
2 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT, C(2) =	0.00
100 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT, C(100) =	0.00

PRECIPITATION RECURRENCE INTERVAL (YEARS)	SCS TR-55 Method	FHWA, HEC-12 METHOD	FAA METHOD
2	0.00	0.00	0.00
100	0.00	0.00	0.00

#### SHALLOW CONCENTRATED FLOW COMPONENT

FLOW LENGTH (FEET) = GROUND SLOPE =		0
VELOCITY, SCS TR-55, (FT/SEC) SHALLOW CONCENTRATED FLOW TRAVI	= EL TIME (MINUTES) =	Ŏ

CHANNEL/PIPE FLOW COMPONENT

CHANNEL/PIPE	LENGTH (FEET)	2	450
CHANNEL/PIPE	SLOPE =		0.0044

FROM DETENTION POND DISCHARGE V-CHANNEL FLOW RATING DATA:

PRECIPITATION RECURRENCE INTERVAL (YEARS)	DISCHARGE FLOW RATE (CU FT/SEC)	DISCHARGE FLOW VELOCITY (FT/SEC)	CHANNEL/PIPE TRAVEL TIME (MINUTES)
2	0.11	0.07	107.14
100	0.64	0.102	73.53

#### TOTAL TIME OF CONCENTRATION

PRECIPITATION RECURRENCE INTERVAL (YEARS)	OVERLAND FLOW TIME (MINUTES)	SHALLOW CONCENTRATED FLOW TIME (MINUTES)	CHANNEL AND PIPE FLOW TIME (MINUTES)	TOTAL TIME OF CONCENTRATION (MINUTES)
2	0	0	107.14	107.14
100	0	0	73.53	73.53

V-CHANNEL FLOW CAPACITY:

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RIGHT SIDE SLOPE =	0.125
LEFT SIDE SLOPE =	0.125
CHANNEL SLOPE =	0.0044
MANNING'S ROUGHNESS =	0.5

050000		WETTED PERIMETER	FLOW AREA	FLOW VELOCITY	FLOW CAPACITY
DEPIH	(LEEI)	(FEEI)	(SQ FEEI)	(F1/SEC)	(CU FI/SEC)
	0.1	1.612	0.080	0.027	0.002
	0.2	3.225	0.320	0.042	0.014
	0.3	4.837	0.720	0.056	0.040
	0.4	6.450	1.280	0.067	0.086
	0.5	8.062	2.000	0.078	0.156
	0.6	9.675	2.880	0.088	0.254
	0.7	11.287	3.920	0.098	0.383
	0.8	12.900	5.120	0.107	0.547
	0.9	14.512	6.480	0.115	0.748
	1	16.125	8.000	0.124	0.991
	1.1	17.737	9.680	0.132	1.278
	1.2	19.349	11.520	0.140	1.612
	1.3	20.962	13.520	0.148	1.995
	1.4	22.574	15.680	0.155	2.431
	1.5	24.187	18.000	0.162	2.922
	1.6	25.799	20.480	0.169	3.471

V-CHANNEL FLOW CAPACITY:

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RIGHT SIDE SLOPE =	0.125
LEFT SIDE SLOPE =	0.125
CHANNEL SLOPE =	0.0027
MANNING'S ROUGHNESS =	0.5

DEPTH	(FEET)	WETTED PERIMETER (FEET)	FLOW AREA (SQ FEET)	FLOW VELOCITY (FT/SEC)	FLOW CAPACITY (CU FT/SEC)
	0.1	1.612	0.080	0.021	0.002
	0.2	3.225	0.320	0.033	0.011
	0.3	4.837	0.720	0.043	0.031
	0.4	6.450	1.280	0.053	0.067
	0.5	8.062	2.000	0.061	0.122
	0.6	9.675	2,880	0.069	0.199
	0.7	11.287	3.920	0.077	0.300
	0.8	12.900	5.120	0.084	0.428
	0.9	14.512	6.480	0.090	0.586
	1	16.125	8.000	0.097	0.776
	1.1	17.737	9.680	0.103	1.001
	1.2	19.349	11.520	0.110	1.262
	1.3	20.962	13.520	0.116	1.563
	1.4	22.574	15.680	0.121	1.904
	1.5	24.187	18.000	0.127	2.289
	1.6	25.799	20.480	0.133	2.719

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#### PRE-DEVELOPMENT:

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2 YEAR RAINFALL INTENSITY, $I(2)$ , (INCH/HR) = 100 YEAR RAINFALL INTENSITY, $I(100)$ , (INCH/HR) = 2 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT, $C(2) =$ 100 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT, $C(100) =$ 2 YEAR TIME OF CONCENTRATION, $T(C, 2)$ , (MINUTES) = 100 YEAR TIME OF CONCENTRATION, $T(C, 100)$ , (MINUTES) = BASIN AREA, A (ACRES) =	0.31 1.57 0.24 0.3 66.7 48.2 8.63
POST-DEVELOPMENT, BASIN 1:	
2 YEAR RAINFALL INTENSITY, I(2); (INCH/HR) = 100 YEAR RAINFALL INTENSITY, I(100), (INCH/HR) = 2 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT, C(2) = 100 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT, C(100) = 2 YEAR TIME OF CONCENTRATION, T(C,2), (MINUTES) = 100 YEAR TIME OF CONCENTRATION, T(C,100), (MINUTES) = BASIN AREA, A (ACRES) =	0.31 1.57 0.24 0.3 66.7 48.2 8.1
POST-DEVELOPMENT, BASIN 2:	
2 YEAR RAINFALL INTENSITY, I(2), (INCH/HR) = 100 YEAR RAINFALL INTENSITY, I(100), (INCH/HR) = 2 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT, C(2) = 100 YEAR WEIGHTED RATIONAL RUNOFF COEFFICIENT, C(100) = 2 YEAR TIME OF CONCENTRATION, T(C,2), (MINUTES) = 100 YEAR TIME OF CONCENTRATION, T(C,100), (MINUTES) = FOR PERLY DAND DISCHARCE.	0.12 1.13 0.69 0.77 115 74
2 YEAR AVERAGE POND DISCHARGE, Q(R,2), (CU FT/SEC) = 100 YEAR AVERAGE POND DISCHARGE, Q(R,100), (CU FT/SEC) =	0.0022 0.061
FUR PLAN DASIN I DISCHARGE: 2 YEAR AVERAGE POND DISCHARGE, Q(R,2), (CU FT/SEC) = 100 YEAR AVERAGE POND DISCHARGE, Q(R,100), (CU FT/SEC) = BASIN AREA, A (ACRES) = RATIO OF PEAK OUTFLOW TO PEAK INFLOW TIME, K =	0.00165 0.05 0.534 2

MODIFIED RATIONAL DETENTION CHARACTERISTICS, BASIN 2

PRECIPITATION RECURRENCE INTERVAL (YEARS)	POST-DEVELOPMENT CONCENTRATION TIME GOVERNING BASIN	CRITICAL STORM DURATION (MINUTES)	CRITICAL STORM INTENSITY (INCH/HOUR)	RUNOFF RATE INTO DETENTION POND (CFS)	REQUIRED DETENTION VOLUME (CUBIC FEET)
2	2	274.53	0.09	0.03	516.51
100	2	99.66	0.89	0.36	1836.00
2	1	66.7	0.31	0.11	453.00
100	1	48.2	1.57	0.64	1726.52

SUMMARY 1	TABLE
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PRECIPITATION RECURRENCE INTERVAL (YEARS)	PRE-DEVELOPMENT FLOW RATE (CU FT/SEC)	POST-DEVELOPMENT CONCENTRATION TIME GOVERNING BASIN	POST-DEVELOPMENT STORM INTENSITY (INCH/HOUR)	POST-DEVELOPMENT BASIN 1 DISCHARGE (CU FT/SEC)	POST-DEVELOPMENT BASIN 2, DETENTION DISCHARGE {CU FT/SEC}	POST-DEVELOPMENT COMBINED DISCHARGE (CU FT/SEC)
2	0.64	2	0.09	0.17	0.004	0.174
100	4.06	2	0.89	2.16	0.11	2.27
2	0.64	1	0.31	0.60	0.003	0.603
100	4.06	1	1.57	3.82	0.09	3.91

It is seen that the time of concentration for the post-development Basin 1 controls the critical combined rainfall intensity and that, under all conditions, the post-development flows are less than those for the pre-development conditions for comparable storm intensities.



# **REVIEW COMMENTS**

Page 1 of 2

FILE #SPR-96-238

TITLE HEADING: Murdock Trailer Sales

**LOCATION:** 2173 River Road

**PETITIONER:** Scott Murdock

**PETITIONER'S ADDRESS/TELEPHONE:** 

3550 SCR 5 Loveland, CO 80537 970-663-1100

PETITIONER'S REPRESENTATIVE:

Lewis Hoffman

**STAFF REPRESENTATIVE:** Kristen Ashbeck

# **NOTE: THE PETITIONER IS REQUIRED TO SUBMIT FOUR (4) COPIES OF WRITTEN RESPONSE AND REVISED DRAWINGS ADDRESSING ALL REVIEW COMMENTS.**

CITY COMMUNITY DEVELOPMENT	11/15/96	
Kristen Ashbeck	244-1437	

- 1. Parking requirement cannot be determined as it is based on total display area. Revise plan to delineate area(s) for equipment display.
- 2. As per Development Engineer comments, all parking and circulation areas must be paved. Is the display area by I-70 to be accessed by customers? If so, the area and the access drive to it must also be paved.
- 3. Landscaping in the public right-of-way between the property line and edge of pavement is required.
- 4. A table listing quantities, species and planting size of all proposed landscaping is required.
- 5. Irrigation of landscaped areas is required. Add a note to the plan that all landscaped areas will have an underground, pressurized irrigation system.
- 6. Based on the street frontages of Lot 1 only, the landscape requirement is 7,062 sf + 5% of the parking area (once determined). At a minimum, landscaping should be provided near the building and a row of trees planted along the I-70 frontage.
- 7. An Improvements Agreement and Guarantee is required for installation of the fire hydrant. See enclosed form.

CITY DEVELOPMENT ENGINEER	11/14/96
Jody Kliska	244-1591

- 1. The drainage report and plan is acceptable.
- 2. Section 5-1-4 of Zoning and Development Code requires all required parking and vehicular traffic areas to be paved. This means from the street connection to the required parking and maneuvering area. Once you determine the amount of required parking, you may want to re-orient your site to minimize the amount of paved area required.

Nin driveway - Jody Hauk

# SPR-96-238 / REVIEW COMMENTS / page 2 of 2

CITY UTILITY ENGINEER	11/15/96
Trent Prall	244-1590
Disease contact Indi Domento of the City Cystomer Se	mine Division at 244 1520 for information recording

Please contact Jodi Romero of the City Customer Service Division at 244-1520 for information regarding sewer plant investment fees.

CITY FIRE DEPARTMENT	11/14/96
Hank Masterson	244-1414

1. Place the fire hydrant at the entrance to your business along River Road.

2. A building permit clearance form is required from the Fire Department. A copy of this form must be submitted to the Building Department before you receive your building permit.

CITY ATTORNEY	11/12/96
Dan Wilson	244-1505

Gravel roads?

MESA COUNTY BUILDING DEPARTMENT	11/5/96		
Bob Lee	244-1656		
$\mathbf{N} = 1$	$T_{\rm res} = T_{\rm res} = 1.000$		

Need to allow 10-15 days for plan review and permit issuance. To expedite, submit building plans as soon as possible. City licensed general contractor is required to perform work.

PUBLIC SERVICE COMPANY	11/6/96	
Tom Boughton	244-2675	
GAS & ELECTRIC: no objections.		
GRAND JUNCTION DRAINAGE DISTRICT	11/11/96	
John Ballagh	242-4343	
See Grand Junction Drainage District response to Minor Subd	livision, #MS-96-239.	
UTE WATER	11/6/96	
Gary Mathews	242-7491	

1. No objections.

2. Construction plans required 48 hours before development begins.

3. Policies and fees in effect at the time of application will apply.

# **RESPONSES TO REVIEW COMMENTS**

# FILE #SPR-96-238 Murdock Trailer Sales

2173 River Road

December 6, 1996

# **CITY COMMUNITY DEVELOPMENT**

We have made the site plan revisions pursuant to our meeting of 11-20-96.

- 1. Revised plan to show 4 parking spaces and delineation of display area.
- 2. Revised plan to show paved areas.
- 3-6. Revised plan to show landscaping as agreed upon.
- 7. We are bidding the work and will provide the agreement prior to site plan final approval.

## **CITY DEVELOPMENT ENGINEER**

2. Revised plan per comment.

## **CITY FIRE DEPARTMENT**

1. The revised plan has moved the building closer the hydrant so the hydrant can remain where it is to accommodate future plans for this property.

Other agency review comments were either statements of facts or were responded to in our revisions to the site blan.

## Temporary Co.O. good for 1 week from date of issuance

#### CERTIFICATE OF OCCUPANCY

BUILDING DEPARTMENT CITY OF GRAND JUNCTION (OR MESA COUNTY)

PERMIT #58767	DATE	3-20-97
PERMISSION IS HEREBY GRANTED TO Ford_Const		TO OCCUPY THE
BUILDING SITUATED AT 2173 River Rd.	· · · · · · · · · · · · · · · · · · ·	
LOT BLOCK FILING SUBDIVISION		
TAX SCHEDULE NUMBER2697-364-00-061	·	
FOR THE FOLLOWING PURPOSE: farm Implement Sales Buil	ding	

THIS CERTIFICATE ISSUED IN CONFORMITY TO SECTION 307, UNIFORM BUILDING CODE

INSPECTOR City Planning

#### CERTIFICATE OF OCCUPANCY

#### BUILDING DEPARTMENT CITY OF GRAND JUNCTION (OR MESA COUNTY)

<i>PERMIT</i> #58767	DATE_	3-24-97
PERMISSION IS HEREBY GRANTED TO Ford Const		TO OCCUPY THE
BUILDING SITUATED AT 2173 River Rd.		
LOT BLOCK FILING SUBDIVISION		
TAX SCHEDULE NUMBER2697-364-00-061		
FOR THE FOLLOWING PURPOSE: farm imp. sales Buildin	ng	

THIS CERTIFICATE ISSUED IN CONFORMITY TO SECTION 307, UNIFORM BUILDING CODE

INSPECTOR m City Planning

## **City of Grand Junction**

Community Development Department Planhing • Izoning • Code Enforcement 250 North 5th Street Grand Junction, CO 81501-2668 Phone: (970) 244-1430 FAX: (970) 244-1599



Scott Murdock Murdock Trailer Sales 3550 SCR 5 Loveland, Colorado 80537

RE: SPR-96-238 Murdock Trailer Sales -2173 River Road

Dear Mr. Murdock,

As of March 27, 1997, the Temporary Certificate of Occupancy of the new building constructed at the site mentioned above has expired. At the time it expired, you indicated that the remainder of the required site improvements, including site paving and landscaping, would be installed within a few weeks. It has been three weeks since that time and I have not heard from you as to the completion of the improvements. If you desire to continue to occupy the building, execution of a Development Improvements Agreement and Guarantee (see enclosed form) is required for the remainder of the improvements which included site paving and landscaping.

Please do not hesitate to contact me at 970/244-1437 if you have questions regarding this information.

Sincerely,

Kristen Ashbeck Planner

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NOTICE: According to Colorado law you <u>must</u> commence any legal action based upon any defect in this survey within three years after you first discover such defect. In no event, may any action based upon any defect in this survey be commenced more than len years from the date of the certification shown hereon. NOTES: 1) The word "Certify" is understood to be an expression of Professional opinion by the Land Surveyor which is based on his best knowledge, information and belief. As such it constitutes neither a guarantee or warranty. 2) wastern Engineers from making additions, deletions, revisions, edits or reproductions of documents or digital files which have been prepared by and/or stamped and signed by Western Engineers, Inc.









CLANKE & CO GRAND JUNCTION